



AGENDA
CITY OF GLENWOOD SPRINGS
Planning and Zoning Commission
Regular Meeting
OCTOBER 27, 2020
Council Chambers, First Floor
101 W. 8TH STREET
6:00 PM

1 Meeting Attendance Instructions

A. **Please click the link below to join the webinar:**

<https://us02web.zoom.us/j/83077029662>

Or iPhone one-tap :

US: +13462487799,,83077029662# or +16699006833,,83077029662#

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Webinar ID: 830 7702 9662

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2 Roll Call

3 Receipt of the Minutes

A. Minutes September 22, 2020

B. Minutes September 29, 2020

4 Comments from citizens appearing for items not on the agenda

5 New Items

- A. 27-20 Consideration of a Major Site Architectural Plan Review for the expansion of the Iron Mountain Hot Springs including a new entry building, mineral pools, an enclosed pool. The application includes consideration of an Alternative Equivalence Compliance for an architectural design that meets the intent of 070.040.090-Nonresidential Design Standards.
- B. 51-20 Consideration of a Special Use Permit for a retail marijuana establishment in the M2-Mixed Use Central Core zoning district in Glenwood Meadows, pursuant to Municipal Code section 070.030.030(e)(11).
- C. 54-20 Consideration of a Location and Extent Review application for the construction of a new 11,000 square foot vehicle storage and office building and a 2,100 square foot communication building with 60 foot tall communication tower at the Colorado Department of Transportation maintenance facility.
- D. 63-20 Consideration of a Code Amendment to Section 070.030.030 Use Specific Standards of the *Glenwood Springs Municipal Code* for marijuana related businesses and cultivation facilities

6 Commissioner Comments

7 Director Comments

8 Adjournment



MINUTES
City of Glenwood Springs
Planning and Zoning Commission
Regular Meeting
September 22, 2020
Zoom
6:00PM

1. Roll Call

Present at roll call were Commissioners: George Shaver, Marco Dehm, Kathryn Grosscup, Carolyn Cipperly, Sumner Schachter, Ben West

Absent at roll call were Commissioners: Ingrid Wussow, Amber Wissing, Dean Kinkel

Also present were: Jenn Ooton, Asst. City Manager, Terri Partch, City Engineer, Gretchen Ricehill, Asst. Economic/Community Development Director, Trent Hyatt, Senior Planner, Emery Ellingson, Planner I, Anna S. Itenberg, City Attorney's Office, Alissa Owsley, IT Director

2. Seating of Alternate Commissioner

Motion was made by Commissioner Schachter to seat Alternate Commissioner Ben West for the meeting. Motion was seconded by Commissioner Grosscup.

Motion passed unanimously.

3. Receipt of Minutes- Receipt of August 25, 2020 minutes.

Commissioner Shaver made a motion to accept the minutes. Commissioner Grosscup seconded the motion.

Motion passed unanimously.

4. Public Comments

Comments from citizens appearing for items not on the agenda. No citizens were present.

5. Continued Items

- a. *Planning File 43-20 Consideration of variances from the required side and rear yard setbacks and to exceed the Required Rear Yard Coverage limit for the construction of an accessory structure which includes a 321 square foot detached garage and 168 square foot storage space.. (PIN #2185-094-015-013)*

Commissioner Schachter made a statement to attest viewing the previous Planning & Zoning Commission Meeting of August 25th, 2020 in preparation for discussion of Planning File 43-20 at the September 22, 2020 meeting.

Planner I, Emery Ellingson presented on the variance application for an accessory structure at 820 Bennett Avenue. Ellingson summarized the proposed project including both variances. The following variances were requested:

Action Item 1: Consideration of variances from required side and rear yard setbacks. Applicant seeks to building structure set back 3.0' from the rear lot line and 2.0' from the side lot line.

Action Item 2: Consideration of variance from Required Rear Yard Coverage. Code limits Required Rear Yard Coverage to 40% of Required Rear Yard.

Staff recommended denial of the application based on the findings found on page 11 & 12 of the staff report.

Commissioner Schachter asked about drainage and building separation. Staff explained that the applicant indicated that drainage from the house would drain directly onto the west garage wall. Staff explained that 5' was standard fire separation distance between structures. Schachter also asked about the rear setback for ADUs which staff confirmed is a 5' setback.

Commissioner Grosscup asked about the Required Rear Yard Coverage. Staff explained that the requirement ensures some open space on the rear yard of a lot and that the 40% for rear lot was a smaller amount than total lot coverage.

Commissioner Shaver asked about code requirements for building separation. Staff confirmed that the standard fire separation distance was 5.0'. Shaver stated if a condition approval could require that the wall on the side property line be constructed to the standard of less than 5.0' building separation. Shaver asked if the garage wall on the side property line had any fenestrations. Staff confirmed that the Commission could set conditions for approval and that the garage wall did not have any fenestrations.

Commissioner Schachter asked if a reduction in storage space would eliminate the need for Action Item 2. Staff explained that a reduction of 92.93 square feet in the Required Rear Yard would eliminate the need for a variance.

Commissioner Cipperly asked for confirmation on the building separation. Staff confirmed the separation distance was 5.0' between the proposed garage and existing house.

Applicant, Kurt Carruth, 820 Bennett Avenue and Julie Carruth, 820 Bennett Avenue, explained the changes to the design, including reducing size of garage and increasing the rear yard setback.

Applicant also indicated willingness to work with Public Works on the foundation construction and connecting to the alley. Applicant also indicated that the principal structure is set back a little farther than other houses in the neighborhood and that they were willing to construct a 1-hour fire wall on the north side of the garage.

Commissioner Dehm asked staff if there were any more public comments. Staff confirmed that there were no additional public comments since the letters read into the record at the August 25th, 2020 meeting.

Commissioner Grosscup made a motion to approve Variance Request – Action Item 1. Commissioner Grosscup cited parking and revision in designs as reasons for approval. Commissioner Dehm asked Commissioner Grosscup to explain how the application met each of the individual variance criteria. Grosscup explained how the application met the individual variance criteria as reasons for approval.

Commissioner Schachter seconded the motion.

Commissioner Schachter stated the Commission may wish to consider adding conditions to the motion. Commissioner Shaver expressed support for adding a condition that the north wall meet Fire Code requirements for buildings with less than 5.0' separation. Commissioner Schachter also stated the Commission may wish to consider conditions to include requiring Public Works approval for the foundation as well as an indemnification agreement.

Commissioner Cipperly questioned if the applicant met variance criteria items 5 & 7 which related to hardship and if the variance requested was the minimum variance needed.

Commissioner Schachter asked if what the applicant's options would be pending variance denial. Assistant Director Gretchen Ricehill and City Attorney Anna Itenberg explained that the application could be denied, approved, or continued. Staff provided further explanation on the different scenarios. Commissioner Grosscup asked what would happen if Action Item 1 were approved and Action Item 2 denied. Staff explained that the applicant's design would be evaluated based on the whichever variances were approved or denied.

Commissioner Grosscup made a motion to amend the original motion. Grosscup made a motion to add the following conditions: that the north side of the garage be constructed to meet Fire Code requirements for buildings with less than 5.0' separation, that the foundation be constructed to meet all Engineering and Public Works requirements, and that the owner enter an indemnification agreement with the City for any damages incurred from work in the alley.

Commissioner Schachter seconded the motion.

Community Development Director Jenn Ooton asked to confirm that the condition regarding the north garage wall was to require construction to meet the Fire Code requirements for buildings with less than 5.0' separation. Commissioner Grosscup confirmed this.

Commissioner Dehm asked the applicant if their opinion on the conditions. The applicant stated that they could work with the conditions presented. Commissioner Dehm called for a vote on Action Item 1.

Commissioners Schachter, Grosscup, West, Cipperly, and Dehm voted to approve.

Commissioner Shaver voted to deny.

Motion to approve Action Item 1 passed 5-1.

Commissioner Cipperly made a motion to deny the variance request of Action Item 2 with the findings on page 11 and 12 of the staff report.

Commissioner Shaver seconded the motion.

Commissioner Schachter stated that the findings for this variance indicate the applicant has several options to eliminate this variance request.

Commissioner Dehm voiced support for this variance, citing density and the fact the applicant had already revised the design.

Commissioner Dehm called for a vote on Action Item 2.

Commissioners Cipperly, Shaver, Grosscup, and Schachter voted to approve the motion in favor of denial.

Commissioners Dehm and West voted to deny the motion in favor of denial.

The motion passed 4-2 in favor of denying the variance request of Action Item 2.

6. New Items

- a. Planning File 15-20 Consideration of a request for a Preliminary Plat to amend the Oasis Creek Tract A Subdivision to allow for the construction of duplexes on lots previously platted for single-family homes*

Commissioner Grosscup recused herself from this item and left the meeting.

Senior Planner Trent Hyatt provided the Commission a summary of the request and process to amend a Preliminary Plat. Hyatt stated that Staff recommended approval with conditions and findings on page 7 of the staff report.

Commissioner Cipperly asked about the concerns from the Oasis Creek HOA and the Oasis Creek architectural control committee (ACC). Hyatt explained that the City does not administer HOA or other neighborhood covenants and that this project would require City approval as well as approval from the associated HOA and ACC.

Commissioner Dehm asked to confirm if these properties could be duplexes based on their existing zoning. Hyatt explained that because the original subdivision was for single family homes, that staff felt it was necessary to amend the original subdivision approval.

Commissioner Schachter asked to confirm what Planning & Zoning Commission approval would signify in relation to the additional neighborhood requirements. Hyatt stated that Planning & Zoning Commission would indicate City approval. Schachter also asked how, assuming City approval of the request, how a building permit would be assessed in terms of compliance with HOA and ACC requirements. Hyatt indicated staff would inquire to the status of the neighborhood approval prior to moving forward with building permit approval.

Commissioner Shaver asked if Tract 7 would remain open space in perpetuity. Hyatt explained that designating Tract 7 as open space was a condition recommended by staff. Hyatt also stated that if this condition were approved, that any future changes in use to Lot 7 would require an amendment to the subdivision.

Commissioner Schachter asked about language found in the annexation agreement which stated that Municipal Code applied to the properties within the PUD unless specifically exempted in the PUD agreement. Schachter asked how this applied to this request with the understanding that the original PUD did not prohibit multi-family housing. Hyatt explained that this language would apply to items required by Municipal Code that are not mentioned in the PUD requirements. Schachter also asked for staff to confirm that a building permit would not be issued until the project met all neighborhood (HOA and ACC) requirements. City Attorney Anna Itenberg stated that the building permit issuance would not require proof of compliance with private approvals such as the Oasis Creek HOA or ACC. Hyatt explained that it was the City's responsibility to ensure this request meets Municipal Code requirements and that the enforcement of standards of private groups such as the HOA or ACC was beyond the responsibility of the City.

Doug Pratte, Land Studio, 365 River Bend Way, Glenwood Springs, provided Commission with a short presentation on the proposed project which included a site plan. Pratte indicated that it was the desire of the applicant to preserve Tract A7 as open space.

Fellow applicant, Scott Grosscup, Balcomb & Green, stated that Doug Pratte and staff had summarized the application well and did not have additional comment.

Fellow applicant, Mary Steinbrecher, 611 Harvard Drive, stated support for the proposal.

Commissioner Schachter asked for confirmation between tract and lots. Applicant Doug Pratte explained that the Oasis Creek HOA covenants designate that lots are for single family homes and tracts are for multifamily homes. Pratte also stated that the intent of this project is to build duplexes on the tracts. Schachter also asked how the neighborhood ACC and City design requirements would govern the project. Pratte explained that the project would need to comply with the neighborhood ACC as well as all City design requirements. Schachter also asked who maintained the gravel path on Tract A7. Pratte explained that Tract A would have an HOA which would maintain that path.

Commissioner Shaver asked the applicant what the next step would be for the project, pending Planning & Zoning approval. Applicant Doug Pratte indicated that they would proceed to City Council with the project with compliance to any conditions. Shaver asked if the applicant had been in communication with the HOA and ACC as part of the process. Pratte stated that the

applicants had been in contact with the ACC. Pratte also indicated that there was not an existing HOA for the Oasis Creek neighborhood. Commissioner Shaver also asked about combining driveways for the duplexes and the number of parking spaces provided per unit. Pratte explained that each unit would have 2 indoor spaces and 2 outdoor spaces. Commissioner Shaver's Zoom connection cut out.

Commissioner Cipperly asked if on-street parking was allowed per the Oasis Creek covenants. Commissioner Schachter stated it was not allowed per the covenants. Commissioner Cipperly also asked about a pedestrian connection to the nearby park. Applicant Doug Pratte stated that the connection to the park would be preserved as part of the project. Commissioner Cipperly also asked who enforced the parking requirements in Oasis Creek. Hyatt stated that the parking was in the public right-of-way and that there was no signage regarding parking.

Commissioner Shaver asked about an empty lot on the site plan. Applicant Scott Grosscup and Applicant Doug Pratte confirmed that the lot in question was a city owned lot.

Commissioner Dehm read a comment from the Oasis Creek ACC. The comment expressed concern that a decision by Planning & Zoning Commission should be delayed until the local ACC approves a variance which would allow this development. Commissioner Dehm asked the applicant how they have approached this issue.

Applicant Scott Grosscup stated that the applicants had been working on the issue and that it was a private matter between the applicant and the local ACC.

Commissioner Dehm asked for staff to confirm the role of Planning & Zoning's decision in the matter. Hyatt explained that the application was submitted to the City and reviewed according to the City processes, which does not require the applicant to receive ACC prior approval for the project.

Commissioner Schachter asked about the conditions of approval, specifically condition 11A of the staff report. Commissioner Schachter expressed concern that the City could approve the project and that development could begin without approval of the ACC. Applicant Scott Grosscup stated that the scenario described by Schachter was a possibility.

Commissioner Dehm expressed concern that Planning & Zoning Commission would approve this request before the private local ACC made its own decision.

Commissioner Dehm explained that a decision on the item would not occur at this meeting and provided details on providing public comment on the Planning Item. Dehm opened the item to public comment.

Mark Lea, 506 Castle Court expressed concerns regarding the covenants. Mark Lea stated that no covenants had been enforced during their time (29 years) in the neighborhood and that the covenants were essentially non-existent. Lea stated that the proposed project was a reasonable proposal and voiced support for the request.

Greg Grier, stated that development of the property was a legitimate use of the property and stated that it was important that the neighborhood was included in the decision, both with the Planning & Zoning Commission as well as the local ACC.

Jason Wilson, resident on North Traver Trail, expressed support for the project stating that the site plan was a good use of the space.

Sue & Bob Ludtke, 406 Yale Circle expressed support for the project. Ludtke stated that the proposed project would fill a need for single floor single story homes in the area.

Mike Fattor, called in and was not heard on Zoom.

Terry Kates, a neighborhood resident expressed concerns over density, height requirements, landscaping, parking, and the lack of covenant enforcement.

Mike Fattor, provided Planning & Zoning Commission with information about staffing of the ACC. Fattor also asked if the applicant could provide more information about the difficulty of polling the neighborhood.

Jeff Peterson, a member of the local ACC stated that he did not represent the homeowners of the neighborhood. Pederson indicated that they were actively working with the applicant to address specific concerns. Pederson did state concerns that approval of the application by Planning & Zoning would override the ACC and requested Planning & Zoning Commission to wait to decide until the ACC was able to discuss the issue further.

Commissioner Dehm closed the public comment.

Applicant Doug Pratte informed the Commission that the applicants intended to continue working with the neighborhood and demonstrate that they meet all applicable City standards as the project moves forward.

Commissioner Schachter asked the applicant to confirm the process for the neighborhood to change the covenants as well as the zoning. Applicant Scott Grosscup indicated that the zoning was created by the City as part of the PUD process and that zoning language was incorporated into the neighborhood covenants.

Commissioner Shaver asked for confirmation on the language of the covenants regarding multi-family housing and the parcel in question. Applicant Scott Grosscup provided further explanation and background on the history of the parcel. Trent Hyatt also provided further information on the City's process to review this proposal.

Commissioner Schachter made a motion to continue the hearing on the item for the next scheduled Planning & Zoning meeting on September 29th.

Commissioner Shaver seconded the motion.

Motion passed unanimously.

Commissioner Dehm called for a five-minute recess.

b. Planning File 44-20 Consideration of a request for a Special Use Permit for a retail marijuana establishment in the M1 Mixed-Use Corridor District

Senior Planner Trent Hyatt provided the Commission a summary of the request for the Special Use Permit. Hyatt recommended approval with conditions of Action Item 1 with findings and conditions on page 9 of the staff report.

Commissioner Schachter asked what the applicant intended to do with the existing pole sign and canopy from the previous use. Hyatt informed the Commission that the pole sign was likely non-conforming and that the canopy would exist as an accessory structure.

Commissioner Shaver asked if the existing fuel storage tanks would need to be removed. Trent Hyatt indicated there may be a state requirement to remove tanks. Shaver also asked about proposed parking and access from Mel Ray. Hyatt explained there would be requirements for parking and access both from CDOT and the City and explained there would need to be delineations on the property for pedestrian safety as well. Hyatt also explained there were some limitations with circulation and access given the current configuration of the site.

Commissioner Dehm asked if any plan for future changes had been submitted and if the applicant had plans to purchase the property. Hyatt confirmed that a plan for future changes had not been submitted.

Applicant Peter Fornell explained that the intent was to apply and acquire a Special Use Permit and that future changes would adhere to any City requirements.

Trent Hyatt informed the Commission that any substantial future changes to the property would require an amendment to the Special Use Permit.

Commissioner Dehm opened the hearing to public comment.

No members of the public were present to comment.

Commissioner Dehm closed the hearing to public comment.

Commissioner Schachter made a motion to continue the hearing until the next scheduled Planning & Zoning meeting on September 29th. Commissioner Cipperly seconded the motion.

Motion passed unanimously.

7. Commissioner Comments

Commissioner Cipperly stated the Oasis Creek project seemed like an appropriate location. Commissioner Shaver requested more information, such as legal information, on the Oasis Creek application. Commissioner West stated the agreement with Commissioner Cipperly's statements. Commissioner Dehm asked about setting up future workshops and discussions.

8. Director Comments

Director Jenn Ooton informed Commission members of upcoming items.

9. Adjournment

Meeting was adjourned at 9:22 PM.



MINUTES
City of Glenwood Springs
Planning and Zoning Commission
Regular Meeting
September 29, 2020
Zoom
6:00PM

1. Roll Call

Present at roll call were Commissioners: George Shaver, Marco Dehm, Carolyn Cipperly, Sumner Schachter, Ben West

Absent at roll call were Commissioners: Ingrid Wussow, Amber Wissing, Dean Kinkel, Kathryn Grosscup

Also present were: Jenn Ooton, Asst. City Manager, Terri Partch, City Engineer, Gretchen Ricehill, Asst. Economic/Community Development Director, Trent Hyatt, Senior Planner, Anna S. Itenberg, City Attorney's Office, Alissa Owsley, IT Director, Steve Boyd, Chief Operating Officer

2. Public Comments

Comments from citizens appearing for items not on the agenda. No citizens were present.

3. Seating of Alternate Commissioner

Motion was made by Commissioner Schachter to seat Alternate Commissioner Ben West for the meeting. Motion was seconded by Commissioner Shaver.

Motion passed unanimously.

4. Continued Items

- a. *Planning File 15-20 Consideration of a request for a Preliminary Plat to amend the Oasis Creek Tract A Subdivision to allow for the construction of duplexes on lots previously platted for single-family homes*

Senior Planner Trent Hyatt provided the Commission a summary of proposal, the previous meeting, and additional comments that had been received by staff. Hyatt clarified that an addition had been made to the staff report to indicate that the final decision would go to City Council.

Hyatt explained that staff had received a number of public comments, the following which were read by staff into record:

Dale Reed, 542 Traver Trail, stated opposition to the proposal citing concerns of density, parking and traffic.

Buzz Fairbanks stated support for the project and stated the units would be a good fit for the neighborhood and appropriate increase in density.

Elizabeth Hinkel from Castle Court, in a letter received September 24th expressed support for the project stating the project seemed to be an appropriate density for the neighborhood.

Oasis Creek Architectural Control Committee (ACC) in a statement received September 25th, stated that they looked forward to continuing to work with the applicant to ensure compliance with all requirements of the Oasis Creek Architectural Control Committee.

Janette Henderson, 820 N. Traver Trail, expressed support for the project, stating the project would be an improvement to the neighborhood and that the proposal seems to meet current City requirements.

Jared Lombardo, 820 N. Traver Trail, in a statement received September 28th expressed support for approval of the project. Jared stated that the project seemed to meet current City requirements and that it would be an improvement to the neighborhood. Lombard mentioned some concerns about off-street parking.

Sharon Anderson, in a statement received September 27th expressed support for approval of the proposal. Anderson stated that the construction of duplexes would be a good addition to the neighborhood and another housing option for the community.

Alex De La Garza, in a statement received September 27th expressed support for approval, citing that the proposed project was appropriate density and heights for the neighborhood.

Bob & Sandy Ketchum in a statement received September 28th, expressed support for approval stating that the proposal would provide a needed housing option for the community.

Commissioner Schachter asked staff to confirm that this approval would bind applicant that there would be no more than 12 units and if the units would be required to be single level.

Hyatt confirmed that this proposal would limit the future development to 12 units. Hyatt explained limiting the project to a single level design would require additional condition language.

Commissioner Schachter asked if the current owner had asked for a larger development in 2016, if the staff recommendation would have been similar.

Hyatt explained that such a hypothetical development would have had to meet all applicable zoning standards at that time.

Schachter also asked about the current pedestrian path and if the City had any authority or responsibility for the maintenance and upkeep of the pedestrian path. Hyatt explained the path was on private property and that maintenance and upkeep of the path was the responsibility of the owner.

Schachter also asked why there were some differences in the applicant's application in 2016 as compared to 2020. Hyatt explained that the previous applicant in 2016 had approached the Oasis Creek ACC prior to approaching the City and that in 2020, that this applicant approached the City first. Hyatt also confirmed that City code does not designate specific uses for tracts or lots.

Commissioner Shaver asked about the differences between the City definition of tracts and lots. Hyatt explained that Code does not designate specific uses for tracts or lots.

Applicant Doug Pratte stated general agreement to the staff recommendations and conditions listed in the staff report. Pratte asked about condition 6 and 11(b) and the language on density in those conditions.

Commissioner Schachter asked if the applicant would be willing to commit to a single level design for the duplexes. Applicant Doug Pratte stated that this was the intent of the proposal to construct single level units and that some units would have walk-out basements due to grade change on the site.

Commissioner Schachter made a motion to approve the item with the findings and conditions listed on page 7 of the staff report with the following amendments:

- Item 6 of the conditions on page 7 state that use of lots and tracts shall be limited to a maximum of 1 duplex on each lot.
- Item 11(b) state a maximum of one duplex per lot.

Commissioner Shaver seconded the motion.

Commissioner Dehm asked Schachter for some confirmation on the amendment to the conditions.

Applicant Grosscup recommended including the language of maximum density per lot.

Motion was made by Commissioner Schachter to amend original motion to the following:

- *Item 6 of the conditions on page 7 state that use of lots and tracts shall be limited to a maximum density of 1 duplex on each lot.*
- *Item 11(b) state a maximum density of one duplex per lot.*

Amendment to original motion seconded by Commissioner Shaver.

Motion passed unanimously.

- b. Planning File 44-20 Consideration of a request for a Special Use Permit for a retail marijuana establishment in the M1 Mixed-Use Corridor District*

Senior Planner Trent Hyatt provided the Commission with a summary of the continued application and informed Commission that no additional public comment had been received for this application. Commissioner Dehm closed the public comment.

Commissioner Schachter made a motion to approve with findings and conditions found on page 8 of the staff report. Schachter added the condition that any future development on the site would require all signage to conform to City Code and that the existing pole sign to be considered a non-conforming sign.

Commissioner Cipperly seconded the motion.

Commissioner Dehm asked staff to confirm if the amendment as proposed by Commissioner Schachter was viable. Hyatt confirmed that the amendment was viable.

Commissioner Dehm called for the question.

Motion passed unanimously.

5. Commissioner Comments

Commissioner Shaver stated the Oasis Creek project meet a need in the community and was a good example of infill development. Commissioner Shaver also asked if future meetings would also be recorded. Commissioner Schachter stated support for future recorded meetings. Commissioner Cipperly expressed questions and concerns about the process of subdivision review and how HOAs are involved in that process. Commissioner West stated the Oasis Creek project met a need in the community and was a good example of infill development.

6. Director Comments

Director Jenn Oton informed Commission members of upcoming items.

7. Adjournment

Meeting was adjourned at 6:55 PM.



Planning and Zoning Commission Report

Date: October 27, 2020

To: Planning and Zoning Commission

From: Gretchen Ricehill, Assistant Director Community

Development Dept.

Subject: #27-20- Major Site/Architectural Plan Review and Alternative Equivalence Compliance Request

REQUEST	Consideration of a request to expand the existing Iron Mountain Hot Springs to include 13 new mineral pools; an enclosed freshwater pool in a 4,700 square foot building; a new 12,000 square foot two story entrance building; and riparian restoration. The application includes an Alternative Equivalence Compliance request for a building design that meets the intent but not the letter of Municipal Code requirements.
APPLICANT	Doug Pratte, Land Studio, INC
OWNER	Iron Mountain Hot Springs, LLC
ZONE	RE-Resort
LOCATION:	Address 281 Centennial Street PIN: 2185-092-00-031
SURROUNDING LAND USES	North: Interstate 70 South: Commercial/Office; CDOT maintenance facility; city park East: Interstate Highway 70; Commercial; Hotel; Tram West: Colorado River; Railroad; Industrial
LOT SIZE	10.25 acres

ACTION ITEMS

Action 1:

Consideration of a Major Site/Architectural Plan review for the expansion of Iron Mountain Hot Springs including a new 12,000 square foot entrance building, enclosed pool, and new mineral pools.

Staff recommends **approval with findings on page 10 and conditions on page 10 and 11** of this report.

Action 2:

Consideration of an Alternative Equivalence Compliance request for a building design that meets the intent but not the specific standards of 070.040.090(e)-*Nonresidential and Mixed-Use Building Design Standards*.

Staff recommends **approval with findings found on page 11** of this report.

INTRODUCTION

The Iron Mountain Hot Springs (IMHS) was originally approved in 2014 and consisted of a 2-story entrance, bathhouse & office building, maintenance and snack buildings, mineral pools and a 90-space parking lot. The development occupies approximately the north half of the 10.25-acre parcel. (Note: as shown on the aerial below, much of the property's acreage includes the Colorado River.)



The proposed project involves expanding the existing Iron Mountain Hot Springs to improve a 146,940 square foot vacant area to the south of the existing development. Plans include-

- a 12,000 square foot, two story entrance building with offices, locker rooms, restaurant and retail uses;
- A freshwater pool in a 4,700 square foot building;
- 10 new mineral pools located south of the existing facility;
- 4 private cabanas each with small mineral pools;
- Improved parking area; and

The proposal also calls for adding three new mineral pools within the existing facility and performing significant riverbank restoration adjacent to the development site.

Application materials which are attached for review include a project narrative, architectural and civil plans, landscaping plan, and related reports.

REVIEW PROCESS

The proposed project expands the existing, approved development which according to Municipal Code section 070.060.030(i)(4), requires a new application that is reviewed in accordance with the full procedure applicable to the original application. Because the original application was a “major development”, this proposed amendment will require both Planning and Zoning Commission and City Council review. The Commission will be a recommending body to Council.

REVIEW & ANALYSIS

In reviewing the site/architectural application, the Planning & Zoning Commission shall consider how the project conforms to the following approval criteria –

1. Consistency with the general purpose and intent of the Code
2. Consistency with dimensional, design, development, and all other standards of the Code;
3. Consistency with previously approved land use approval;
4. Consistency with the Comprehensive Plan and other adopted city policies and plans.

The applicant’s narrative provides a detailed summary of the project and analysis of Municipal Code requirements. Below is a summary of the more important project components as they relate to the Code, approval criteria, and applicable prior approvals.

Sensitive Area Protection (070.040.020)

The project calls for constructing new mineral pools within 35 feet of the highwater line. Paragraph 24 of Section 070.040.020 (d)-*Development in Waterways and Areas of Special Flood Hazard* (commonly known as riparian zone regulations) was amended by City Council September 17, 2020 to increase and redefine setback requirements and to further define the amount and type of activity allowed/not allowed within the “riparian zone”. Improvements, including the development of hot springs pools, which do not result in a material increase in water quality degradation, stream bank erosion, or reduction in the function and quality of a riparian area or wetland is exempt from permitting and engineering studies required elsewhere in the Code.

This project also includes riparian restoration along the shoreline adjacent to the proposed expansion. The applicant provided a Shoreline Enhancement Plan (attached) which shows planting of 50 native shrubs and 60 perennials and grasses. The plan did not include a planting key. Prior to submitting Construction Plans, staff recommends a condition requiring a plan revision to include a key for all trees, shrubs, and perennials/grasses. See additional discussion below- Landscaping & Common Open Space.

Common Open Space & Landscaping

Ten percent of the lot area is required to be common open space. Areas counted as open space include formal plantings and gardens; squares, courtyards, plazas; and required buffer and landscape areas. Similarly, in the RE-resort zoning district ten percent of the lot area is required to consist of landscaping. In this case, the site or lot area is just that portion of the property proposed for new development, or 146,940 square feet. This includes the new, expanded parking and the building and pool areas and is depicted on sheet 2 of the revised Landscaping Plan (dated 8-31-2020).

The project proposes 25,821 square feet of landscaping, and nearly 45,000 square feet of common open space. The latter area includes the ten new mineral pools as they can be classified as open space.

Proposed plantings include the code compliant number of trees, shrubs, and perennials. Note: these plantings are in addition to those shown on the shoreline restoration plan.

Off-street Parking

Number of Spaces Required

Outdoor recreation uses are required to provide 1 parking space per 400 sf of building area plus 1 parking space/ 10,000 sf of site area. For purposes of calculating parking “building area” the Code excludes non-habitable spaces such as utility areas, hallways, restrooms. For simplicity, for this application, staff used the entire building square footages.

Use	Square Footage	Required Parking
Current Entrance/office building:	7,000 sf	18
Proposed Entrance building:	12,000 sf	30
Proposed enclosed pool building:	4,700 sf	12
Existing site area:	77,025 sf (approx)	8
Proposed site area:	55,561 sf	<u>6</u>
TOTAL REQUIRED PARKING		74 spaces

The parking plan shows a total of 233 spaces which far exceeds the minimum requirement. The Code sets a maximum parking amount of 125% above the required parking. However, it is noted that this parking lot is shared by other related uses, namely the Tram, and future plans call for the construction of a hotel on an adjoining parcel. The sufficiency and supply of parking will be reevaluated at the time the owner submits a development application for the hotel use. This reevaluation may require submission of a parking demand study.

Parking Lot Improvements

In 2014, the original hot springs development consisted of 90 parking spaces. The approved parking plan allowed IMHS to use a gravel surface: deferring paving until 2019. Earlier this year, IMHS was granted an extension to 2021. However, in granting the extension it was noted that IMHS had expanded the number of parking spaces beyond what was originally approved so, the extension was conditioned on IMHS submitting a formal parking expansion plan for review by December 31, 2020. This application suffices for that required plan.

As indicted above, the parking plan submitted with this application shows a total of 233 parking spaces in six rows. Center parking rows are separated by wide planting strips and terminus planting islands. One of the rows (developed in 2014) has been improved with landscaping, the other was



developed later and is partially improved, as shown in the aerial at left.

In 2014 IMHS received a variance from parking lot landscaping requirements, allowing the use of a linear landscape island between parking rows rather than individual islands between each eight parking spaces. This center linear island also serves as a stormwater drainage basin for the parking lot.

Plans call for the eastern-most island to be expanded southward, and for both islands to continue functioning as drainage basins for the parking lot.

As the Commission may recall, earlier this year, the Code was

recently amended to allow bioswales/linear planting islands as an alternative to constructing a landscape island every 8 parking spaces. Therefore, additional variances for the design, construction and expansion of the bioswale/linear planting island is not required.

The proposed landscaping within the parking lot amounts to nearly 27 percent of the lot area which exceeds the minimum 20 percent Code requirement. The plan also meets the Code required number of trees (30 trees required; 31 provided). Finally, the Code allows no more than 50 percent of the landscape strip to be non-living ground cover. Detailed parking lot landscaping plans were not included in this submission therefore staff has included a condition of approval that the applicant submit plans showing compliance with this Code provision.

Access & Circulation

The applicant submitted a traffic study dated April and July 2019 which included an analysis of the mixed use (Caverns Village) project; a 100-room hotel with 10 private mineral pools for hotel guest use only; and three additional pools at the existing IMHS site. The study included a series of measures to mitigate the development's impact on existing roadway infrastructure. Included were new sidewalks; improvements to Devereux Road, Centennial Street, and US 6 intersections; and removing certain sight obstructions at US 6 and Devereux. Making roadway and pedestrian improvements were a condition of the mixed use (Caverns Village) approval and the state access permit and will need to be completed before the city issues a certificate of occupancy for that building. The Caverns Village development was also approved with the condition that the owner

work with the city on designing a sidewalk or trail along the western edge of Two Rivers Park, and committing to finance 50 percent or up to \$300,000 of the construction. The City's Engineering Department, CDOT and Caverns Village owner continue to work through the design details of all pedestrian and roadway improvements associated with the approved Caverns Village development.

Because this proposed development does not include the hotel, Dan Cokley the project engineer, provided an email to City Engineer Terri Partch stating that the ten mineral pools, constructed in advance of the hotel, will not change the typical trip generation rate of the Hotel Land Use as defined in the ITE Trip Generation Manual and as applied in the 2019 traffic study. In fact, he stated that the ten pools will generate fewer design hour trips than the future build out of the hotel. Although he also noted that the pools are planned to be part of the hotel property, staff believes that unlikely due to their location in relation to future hotel site. That said, an update of the traffic study will be required when the hotel development application is submitted. No additional pedestrian improvements are required as part of this development. Per the Engineering Standards, Devereux Road only requires a sidewalk on one side. As noted above, a sidewalk will be constructed as part of the previous Caverns Village approval.

Building Design

The new entrance building's design and materials complement existing IMHS buildings. Materials include a stone base, wood siding and decorative wall shingles, and a metal roof in an earth-tone palette. The tallest portion of the building is about 46 feet from grade to the highest roof ridge. The maximum height of gabled roofs is measured from grade to the midpoint of the ridge therefore, the building meets the maximum 60-foot height requirement of the RE-resort zoning district.



Proposed Entrance Building

The new enclosed, 4,700 square foot pool building, pictured below, is a single-story building with an earthen sloping roof. The primary wall material is stone with wood accents, and all but the east elevation includes large windows, most of which can be opened for indoor/outdoor use. The non-commercial design standards require at least three architectural elements each 30 feet of building length. Additionally, the code requires projections/recessions, dormers, and other changes in roof height every 50 feet of length to reduce the visual impact of roof mass. The applicant submitted an Alternative Equivalence Compliance request to address these deviations from the code for the east elevation. See discussion and analysis below.



Proposed Enclosed Pool

ALTERNATIVE EQUIVALENCE COMPLIANCE

To encourage create and unique design, the Code allows consideration of alternative designs that meet the overall intent but not the strict letter of the design standards through what is referred to as “alternative equivalence compliance”. This is not a general waiver of regulations. Rather, this process authorizes a site-specific plan that is equal to or better than the strict application of the standard.

At issue is the proposed east elevation of enclosed pool building and the building’s east and west rooflines.

The Nonresidential and Mixed-Use Building Design Standards (Section 070.040.090(e)) requires breaking up building elevations such that no wall section exceeds 30 feet without providing certain architectural elements including recessions/projections, window/doors, a change in texture or masonry patter, change in building height, and awnings or canopies. Additionally, the design standards require articulation when the roofline exceeds 50 feet in length.

The general intent of the design standards is to encourage creative design that, among other things-

- Enhances the visual and physical qualities of Glenwood Springs;
- Considers how the building relates to the site, neighboring properties, and the community;
- Provides strong pedestrian orientation by visually connecting the building’s interior with the public exterior;
- Uses high quality, long lasting materials with proven ability to withstand Glenwood Springs’ environment;
- Incorporates a palette of colors that adds visual interest to the development parcel and the is compatible with the surrounding natural environment.

And to discourage-

- Buildings that are out of scale or have little physical or visual connection to the street, neighboring properties, or the community;
- Building elements, materials, and colors that are out of character with the community;
- Large expanses of blank, flat wall surfaces.

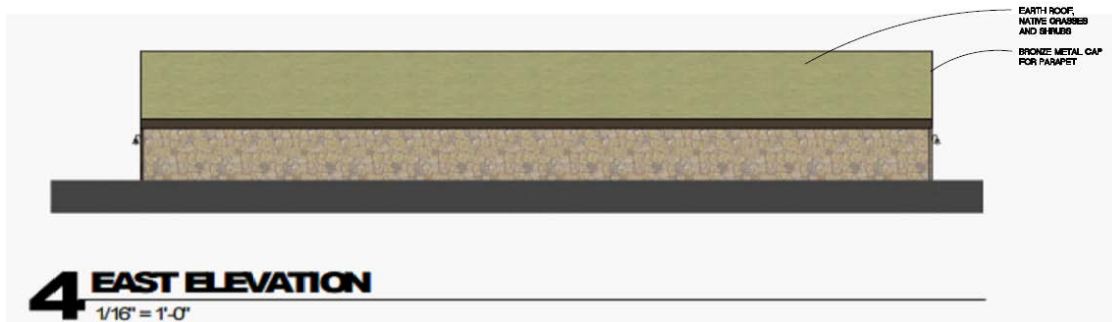
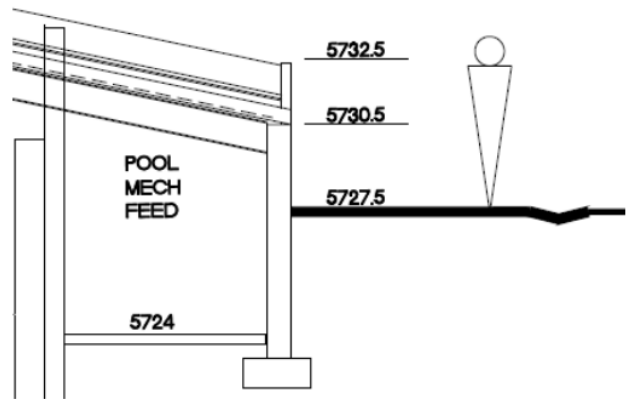
When considering design alternatives, the applicant needs to show that the proposed design –

1. Achieves the intent of the subject standard to the same or better degree than the subject standard;
2. Advances the goals and policies of the Code to the same or better degree than the subject standard;
3. Results in benefits to the community that are equivalent to or exceed benefits associated with the subject standard; and
4. Imposes no greater impacts on adjacent properties than would occur through compliance with the specific requirements of this Code.

Triumph, the applicant's architect provided a narrative which addressed the benefits of the alternative design (refer to attached Alternative Compliance file).

East Elevation

The east elevation (facing the parking area) is 94'8" inches in length and does not incorporate the Code-required three architectural elements. The applicant's architect noted that the enclosed pool building is designed to work with the property's natural grade which slopes to the west. Tall, glazed windows/doors on the west and south elevations will open allowing for indoor/outdoor use. A mechanical room will be located along the shorter east elevation. From the sidewalk, paralleling the parking area, this east elevation will only be five to 7.5 feet in height. The wall material will be stone so will appear as a retaining wall to pedestrians. Visual impacts of this wall will be mitigated or softened by the earthen, landscaped roof (depicted below – see also example photograph) that gently slopes upward (2:12 pitch).



Roof Design

The enclosed pool building will feature an earthen roof, covered with 12 to 18 inches of soil, and

planted with native grasses. As far as staff is aware, this will be the first such roof in the city. The applicant's architect addressed the unique benefits of this design over the strict Code requirements. Important points include:

- The earthen, landscaped roof which slopes to the east and the parking lot, meets the Code intent of reducing the visual impact of roof mass. The native grasses and vegetation on the roof will be visible as guests arrive and provide a softer appearance than a flat, metal roof;



For reference only:

- The earthen roof will not result in glare as would a standard metal roof. The roof landscaping will blend rather than take away from the view of the Colorado River.

COMPREHENSIVE PLAN COMPLIANCE

The Comprehensive Plan is the community's vision for how it would like to develop into the future and provides a framework for decision making.

The area between the Colorado River, Interstate 70 and Devereux Road is identified on the future land use map as appropriate for commercial development. *"This land use designation provides a wide range of general retail goods and services for both regional and local markets, in attached and freestanding structures....."*

This development proposal which expands an existing tourism or resort use is supported in the Comprehensive Plan with the following goals:

- ***Retain, Enhance and Expand the Long-standing Tourist Market***
- ***Maintain Glenwood Springs as the regional tourism, retail, commercial and governmental center of Garfield County.***
- ***Promote long-term, sustainable, diverse economic development.***
- ***Maintain Glenwood Springs as the regional tourism, retail, commercial and governmental center of Garfield County.***
- ***Direct development to locations and building forms that are cost-effective to serve.***
- ***Preserve natural resources***

And, the shoreline restoration plan is supported with the goal: ***Preserve natural resources – Preserve and enhance the river corridor environment***

REVIEWING AGENCY COMMENTS

Staff distributed the application to City departments and outside agencies for review and comment. We received comment from the Engineering and Public Works Departments which are attached for review. The Planning Department also submitted comments that are attached.

ACTION ITEMS AND STAFF RECOMMENDATIONS

This project requires that the Planning and Zoning Commission consider 2 separate actions.

Section 070.060.030(g)(1)d outlines the review and decision procedures for applications being considered by the Planning and Zoning Commission and/or City Council. The Commission shall approve, approve with conditions, or deny the application based on the applicable approval criteria listed in the application-specific procedures. The Commission may also continue the hearing to with a request for specific additional information.

Action 1:

Consideration of a Major Site/Architectural Plan review for the expansion of Iron Mountain Hot Springs including a new 12,000 square foot entrance building, enclosed pool, and new mineral pools.

Staff recommends **approval with findings on page 10 and conditions on page 10 and 11** of this report.

Recommended Findings

The proposed expansion is -

1. Consist with the general purpose and intent of the Code.
2. Consist with dimensional, design, development, and all other standards of the Code.
3. Consist with previously approved land use approval.
4. Consist with the Comprehensive Plan and other adopted city policies and plans.

Recommended Conditions

1. The applicant shall comply with all verbal representations made in the public hearings, as outlined in all application materials, and/or as revised per conditions herein and approved by the Community Development Director.
2. The applicant shall obtain and abide by all necessary access permits required for the project from the Colorado Department of Transportation.
3. Prior to submitting a building and/or grading permit -
 - a. The applicant shall submit a Construction Plans review application per Municipal Code Section 070.060.050(c). Plans shall address applicable conditions of approval and reviewing agency comments; and shall comply with the Engineering Standards and other applicable City standards and policies.
 - b. The applicant shall enter into a Development Agreement that memorializes the development approvals and conditions associated with the project.
 - c. The applicant shall execute a Cost Reimbursement agreement which reimburses any city attorney services (beyond two hours) and expenses related to this project.
 - d. Revise landscape plans to include parking lot landscape strips/bioswales. Plans shall demonstrate compliance with Code requirements including a 50 percent limit on non-living ground covers.

- e. Revise landscape plans and shoreline restoration plans to add planting key for all plant species/types.
4. Any modifications of the approved plans or condition of approval shall require review and approval of the Community Development Director for compliance with applicable Municipal Code requirements. Modifications may require a new application or amendment of the approved plans per Section 070.060.030(i)(4).

Action2:

Consideration of an Alternative Equivalence Compliance request for a building design that meets the intent but not the specific standards of 070.040.090(e)-*Nonresidential and Mixed-Use Building Design Standards*.

Staff recommends **approval with findings** found on page 11 of this report.

Findings

The alternative design:

1. Achieves the intent of the subject standard to the same or better degree than the subject standard;
2. Advances the goals and policies of the Code to the same or better degree than the subject standard;
3. Results in benefits to the community that are equivalent to or exceed benefits associated with the subject standard; and
4. Imposes no greater impacts on adjacent properties than would occur through compliance with the specific requirements of this Code.

**Engineering
Department**

Date: September 3, 2020

**Ryan Johnson
Asst. City
Engineer**

To: Community Development

Re: Iron Mountain Hot Springs – Major Site Plan Submittal

Please see the following comments from Engineering for the Major Site Plan Submittal. These comments are based on the information provided. Additional comments may be made at the time the development plan and/or the construction documents are submitted.

1. The civil plans indicate connection to storm sewer facilities that are part of the Lift Station Scope of work. Is the lift station construction anticipated before the construction of these facilities?
2. Show the 100-year floodplain line on the civil plans.
3. Better define (hatch) the proposed impervious paving areas adjacent to the pools and buildings. There looks to be a substantial amount of impervious area that it is not clear where it is draining to. Impervious areas will need to have WQ treatment prior to discharge to the Colorado river.
4. Provide the river restoration plans anticipated for this scope of work.
 - a. How do these restoration plans correspond in regard to grading to the proposed site and pools?
The grading plans should be coordinated between scopes of work.
5. The site has some significant changes to what was approved in the recent traffic report and approved CDOT access permit. A revised traffic analysis should be completed to evaluate if there are any increase in traffic demands. A revised access permit with CDOT may be necessary.
6. The proposed parking seems inadequate for the proposed improvements. There are times that the existing spaces are full in current conditions. It would be difficult to assume that the addition of 108 additional spaces would be sufficient for the addition of potentially 100 rooms and all of the proposed improvements at the pool that could draw more capacity.
 - a. The site does not appear to meet ADA parking requirements. If the hotel is not considered, 10 ADA spaces are required. If the hotel is considered, 11 ADA spaces are required.
7. Will this work require 404 permitting? The ordinary high-water elevation is not indicated on the sketch plan. If so, provide the acquired 404 permit submittals to the engineering department.

Items to be addressed prior to Grading/Building Permit issuance:

1. It appears that the combined development improvements will be over one (1) acre and will require a Construction Stormwater Discharges permit from CDPHE.
2. This proposed scope of work will require its own drainage compliance letter/report and address the site drainage areas that are being affected by the proposed work south of the master drainage report basin N2. This drainage report should demonstrate compliance with the current Master drainage report from the site developed by Boundaries Unlimited.
3. A grading permit will be required for the proposed improvements
4. If a revised CDOT Access Permit is required, it should be approved prior to any work taking place on site.

Information only/Requirements Prior to Certificate of Occupancy:

1. The City will require a maintenance plan and agreement with the development for the detention and debris system/s, ***if not already in place***. If the system/s are not being maintained properly and having a negative effect on downstream facilities, the City will need access to perform the maintenance and back charge the facility owner.
2. Site improvements will require a certification letter from the design engineer that verifies that all improvements were constructed per the approved plan.
3. Record (as-built) drawings will be required to be submitted to the City along with electronic drawings (CAD).

From: Matthew Langhorst <matthew.langhorst@cogs.us>

Sent: Monday, August 31, 2020 3:27 PM

To: Trent Hyatt <trent.hyatt@cogs.us>; Mike Hoffman <mike.hoffman@cogs.us>; Doug Hazzard <doug.hazzard@cogs.us>; Michael Gardner <michael.gardner@cogs.us>; Gretchen E Ricehill <gretchen.ricehill@cogs.us>

Subject: RE: August 31 Department Review Meeting

- Iron Mountain Hot Springs
 - What is the estimated domestic water consumption for the project in Gallons Per Day?
 - Developer should fill out the Power Load form attached so the City can determine electrical needs, to include the new lift station.
 - Please add Boundaries Unlimited sewer easement to the new Lift Station location for reference.
 - The existing water quality pond will be removed with the new lift station design, where is the WQ pond being replaced at and what is the new WQ system look like with additional WQ needs due to the proposed site?
 - All new sewer services are required to have a sewer backflow preventor installed on private property and maintained by the property owner.
 - Utility Contacts: Public Works: Matthew Langhorst. Sewer/Water: Mike Hoffman. The engineer may want to check the BH contact.
 - Coordinate sewer and water taps with Mike Hoffman, not Matt Langhorst as noted.
 - Please coordinate with Mike Gardner for FO line installation to the new buildings, CBN.
 - Any fire sprinkler system will need to meet the NFPA 24 design requirements.
 - Here are items that we will need to see on the plans before we can approve:
 - Since the new tap and service will be used for combined potable/fire, the service will need to be designed, installed, and tested to meet NFPA 24 requirements. Prior to construction, an engineered detail of the service line must be submitted to the City for review and approval. At a minimum, the detail must show the following:
 - 1) A plan and profile view of the piping alignments extending from the tap location to the building interior penetration.
 - 2) Materials for construction.
 - 3) Thrust restraint locations and specifications.
 - 4) A comment indicating that the design conforms to all NFPA 24 requirements .
 - 5) A note indicating that the service line must be pressure- and flow-tested in accordance with NFPA 24.
 - 6) The stamp and signature of the Professional Engineer (PE) certifying the design.
 - 7) The applicant is responsible for providing the City with a signed copy of the completed "Contractor's Material and Testing Certificate for Underground Piping" form (attached) prior to issuance of a Certificate of Occupancy. This form certifies that the service line was designed, installed, and tested in accordance with all NFPA 24 requirements.

Attachments:

- Load Information
- Contractor's Material and Testing Certificate for Underground Piping

**CITY OF GLENWOOD SPRINGS
REQUEST FOR LOAD INFORMATION**

Owner's Name: _____ Date: _____

Address: _____

Person Contact: _____

Telephone #: _____

Building Classification:

☐ Residential ☐ Industrial

☐ Commercial ☐ Other _____

Building Features: Square Footage: _____

Type Use (Office, Retail Store, etc.): _____

Electrical Data: Voltage Request:

☐ 120/240, 1 ϕ , 3-w ☐ 277/480, 3 ϕ , 4-w

☐ 120/208, 3 ϕ , 4-w ☐ Other _____

Secondary Main Breaker Size:

☐ 100-amp ☐ 400 amp

☐ 200 amp ☐ Other _____

Load Profile: Total Connected KW: _____

Estimated Maximum Demand KW: _____

Transformer Request Size KVA: _____

Equipment List:

Heat: ☐ Gas ☐ Electric ☐ Other _____

Air Conditioning: ☐ Yes ☐ No

Largest Motor Load, HP _____ Qty _____

Largest Resistance Load, KW _____ Qty _____

Utility Service Information

Electrical Summary:

Transformer Size: _____ KVA

Service Voltage: _____

Meter Classification:

Voltage Configuration: _____ Amp Classification: _____

Instrument Rated (CTS): _____ Meter Type: _____

Fault Current: _____

Contractor's Material and Test Certificate for Underground Piping			
PROCEDURE Upon completion of work, inspection and tests shall be made by the contractor's representative and witnessed by an owner's representative. All defects shall be corrected and system left in service before contractor's personnel finally leave the job. A certificate shall be filled out and signed by both representatives. Copies shall be prepared for approving authorities, owners, and contractor. It is understood the owner's representative's signature in no way prejudices any claim against contractor for faulty material, poor workmanship, or failure to comply with approving authority's requirements or local ordinances.			
Property name		Date	
Property address			
Plans	Accepted by approving authorities (names)		
	Address		
	Installation conforms to accepted plans ☐ Yes ☐ No		
	Equipment used is approved ☐ Yes ☐ No		
Instructions	If no, state deviations		
	Has person in charge of fire equipment been instructed as to location of control valves and care and maintenance of this new equipment? ☐ Yes ☐ No If no, explain		
Location	Supplies buildings		
	Have copies of appropriate instructions and care and maintenance charts been left on premises? ☐ Yes ☐ No If no, explain		
Underground pipes and joints	Pipe types and class		Type joint
	Pipe conforms to _____ standard		☐ Yes ☐ No
	Fittings conform to _____ standard		☐ Yes ☐ No
	If no, explain		
Test description	Joints needing anchorage clamped, strapped, or blocked in accordance with _____ standard ☐ Yes ☐ No		
	If no, explain		
Flushing tests	Flushing: Flow the required rate until water is clear as indicated by no collection of foreign material in burlap bags at outlets such as hydrants and blow-offs. Flush at one of the flow rates as specified in 10.10.2.1.3. Hydrostatic: All piping and attached appurtenances subjected to system working pressure shall be hydrostatically tested at 200 psi (13.8 bar) or 50 psi (3.5 bar) in excess of the system working pressure, whichever is greater, and shall maintain that pressure ± 5 psi (0.35 bar) for 2 hours. Hydrostatic Testing Allowance: Where additional water is added to the system to maintain the test pressures required by 10.10.2.2.1, the amount of water shall be measured and shall not exceed the limits of the following equation (for metric equation, see 10.10.2.2.6): $L = \frac{SD\sqrt{P}}{148,000}$ L = testing allowance (makeup water), in gallons per hour S = length of pipe tested, in feet D = nominal diameter of the pipe, in inches P = average test pressure during the hydrostatic test, in pounds per square inch (gauge)		
	New underground piping flushed according to _____ standard by (company) ☐ Yes ☐ No If no, explain		
Flushing tests	How flushing flow was obtained		Through what type opening
	☐ Public water ☐ Tank or reservoir ☐ Fire pump		☐ Hydrant butt ☐ Open pipe
	Lead-ins flushed according to _____ standard by (company) ☐ Yes ☐ No		
	If no, explain		
Flushing tests	How flushing flow was obtained		Through what type opening
	☐ Public water ☐ Tank or reservoir ☐ Fire pump		☐ Y connection to flange and spigot ☐ Open pipe

FIGURE 10.10.1 Sample of Contractor's Material and Test Certificate for Underground Piping.

Hydrostatic test	All new underground piping hydrostatically tested at _____ psi for _____ hours		Joints covered ☐ Yes ☐ No	
Leakage test	Total amount of leakage measured _____ gallons _____ hours			
	Allowable leakage _____ gallons _____ hours			
Forward flow test of backflow preventer	Forward flow test performed in accordance with 10.10.2.5.2: ☐ Yes ☐ No			
Hydrants	Number installed	Type and make	All operate satisfactorily ☐ Yes ☐ No	
Control valves	Water control valves left wide open If no, state reason		☐ Yes ☐ No	
	Hose threads of fire department connections and hydrants interchangeable with those of fire department answering alarm		☐ Yes ☐ No	
Remarks	Date left in service			
Signatures	Name of installing contractor			
	Tests witnessed by			
	For property owner (signed)	Title	Date	
	For installing contractor (signed)	Title	Date	
Additional explanation and notes				
© 2012 National Fire Protection Association NFPA 24 (p. 2 of 2)				

FIGURE 10.10.1 Continued

Iron Mountain Hot Springs Expansion

Glenwood Springs, Colorado



Major Site/Architectural Plan Review

Parcel ID# 218509200031

Parcel 3 (Tracts 1, 2, 3, & 4)

Iron Mountain Hot Springs Parcels

Subdivision Exemption Plat for Lot Line Adjustment

Updated August 7, 2020 for:
Iron Mountain Hot Springs LLC
281 Centennial Street
Glenwood Springs, CO 81601

Prepared by:
The Land Studio, Inc.
365 River Bend Way
Glenwood Springs, CO 81601
Phone: (970) 927-3690
landstudio2@comcast.net

Iron Mountain Hot Springs Expansion

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Land Use Application Form**LAND USE APPLICATION**

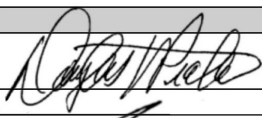
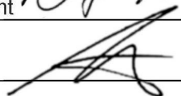
City of Glenwood Springs
Economic and Community Development Dept.
101 West 8th Street
Glenwood Springs, CO 81601
970-384-6411

Ordinance Amendments		Development Permits	
☐	Rezoning	☐	Site/Architectural Plan Review
☐	Rezoning to Planned Unit Development (PUD)	☐	Administrative
☐	Annexation	☐	Minor
☐	Condominiumization	☒	Major
☐	Annexation	☐	Master Plan
☐	Street Vacation	☐	Construction Plans
		☐	Location & Extent
Subdivisions		☐	Right of Way Encroachment License
☐	Minor Subdivision	☐	Floodplain Development Permit
☐	Preliminary Plat	☐	Special Use Permit
☐	Final Plat	Flexibility and Relief Procedures	
☐	Vacation of Right-of-Way	☒	Variance
		☐	Administrative Adjustment
Other Land Use Applications		☐	Appeal
☐	Outdoor Seating	☐	

Applicant/Property Owner Information:			
The Land Studio, Inc. / Doug Pratte		Iron Mountain Hot Springs, LLC / Steve Beckley	
Applicant		Owner	
Planner / Representative			
Relationship to Owner		Owner	
365 River Bend Way, Glenwood Springs, 81601		281 Centennial Street, Glenwood Springs, CO	
Applicant Address		Owner Address	
970-927-3690	landstudio2@comcast.net	970-618-7127	sbeckley@glenwoodcaverns.com
Phone	Email Address	Phone	Email Address

Property Information:	
281 Centennial St, Glenwood Springs, 81601	218509200031
Address	Parcel Number
Iron Mountain Hot Springs Parcels Subdivision Exemption Plat	Parcel 3 (Tract 1, 2, 3, and 4)
Subdivision	Lot and Block Number
Resort (RE) District	Vacant and Existing IMHS & Parking
Zone District	Existing Land Use

General Project Description:
Expansion of Iron Mountain Hot Springs including new entry building, retail, locker rooms, offices, outdoor pools, enclosed pool, food service, cabanas, river bank restoration, landscape, and reconfigured parking.

Signatures:		
The owner or applicant must be present at the hearing. All public hearings must be properly noticed according to G.S.M.C. Section 070.010.030. Signatures of all owners of the property must appear before the application is accepted. Partnerships or corporations may have the authorized general partner or corporate officer sign the application. (Attach additional incorporation documents if necessary.) I declare under the penalty of perjury that the above information is true and correct to the best of my knowledge.	Applicant 	7-28-2020
	Owner 	7-28-2020
		Date

Project Description

The Application

Iron Mountain Hot Springs, LLC is submitting a request for Major Site/Architectural Plan Review and a Variance request to the City of Glenwood Springs for expansion of Iron Mountain Hot Springs. The Applicant wishes to expand the existing spa facility to include a new entry building with retail space and food service, including additional men's and women's locker rooms with sauna facilities. The expanded facility will have access control and be a separate experience from the existing pools. The expansion will provide a limited number of entries to keep the environment tranquil and restorative. The Variance request is to provide alternative compliance for the landscape islands required in the IMHS parking lot.

The new expanded area includes ten outdoor mineral pools, and an enclosed heated freshwater pool with an attached hot tub. There will also be a set of private cabanas that have small mineral pools within. Three new mineral pools will also be added to the existing Iron Mountain Hot Springs area. The expansion project will include a comprehensive riverbank restoration plan along the entire Iron Mountain Hot Springs property, which will be incorporated into the new landscape features for the expanded facility. Lastly the parking area will be reconfigured to improve traffic flow, capacity and accessibility and the lot will be paved with asphalt. The Iron Mountain Hot Springs Master Plan, attached as an Exhibit, illustrates all of these improvements in context with existing facilities and the approve Caverns Village Project.

The Development

With close-proximity to the existing Iron Mountain Hot Springs bathhouse, the new development will be an expansion to those services allowing for better points of entrance and egress from the facility. The plan proposes to add a 12,000 SF two level building directly south of existing Iron Mountain Hot Springs structures. A second building will be an enclosed pool structure with 4,700 SF under roof. The hot springs pool count in the expansion is increasing by ten new spas tucked in along the river. Four self-contained Cabanas will provide a private experience for special occasions.

The Applicant believes the proposed plan represents a meaningful improvement to the currently constructed buildings, allowing for entrance to the site through the new structure and exiting through the current path. The new pool and spas will allow for an enhanced guest experience while visiting the resort.

Parking will be shared in the new proposed asphalt paved lot east of the proposed expansion. This parking lot will be shared between the existing facility, the proposed hot springs expansion, and a future hotel.

Overview of Building Design

The new buildings are designed and located to work with the natural grade of the site. Entrance to the resort will now be through the main level, check in to existing spas OR check in and downstairs to the new addition. On this level will be a restaurant/seating, and associated retail. There is an upper floor that will house offices and storage. At the lower level will be walk out for both men's and women's locker rooms and a large mechanical space for the new uses. The Entrance building is planned to be constructed as a wood-framed structure. The lower level will consist of a slab on grade foundation. The second and third floors will be wood framed with 1 1/2" of lite weight concrete for code separation and quiet construction.

Exterior materials will be specified to complement the existing palette of the bath house. A stone base is proposed varying in height for building base protection and accents around the structure. Siding material will 1x8 natural channel cedar siding with 2x trims. Shaketown shingles will be installed on the second floor exterior behind heavy timber trusses. The fascia will be 2"x12" with metal gutters and downspouts painted to match. Roofing will be standing seam metal with color to match existing buildings. Aluminum clad windows/doors will create a color contrast.

The new pool building will be approximately 4,700 SF with a single use of a new pool. The large format doors to the south and west will open to allow for indoor/outdoor use. A continuous mechanical room will run along the east side to feed the pool and new spas. The roof is designed with earth cover, this will be 12-18" of soil cover for native grasses. The pool building will be a steel structure to carry the roof load and large spans for the intended use. Expansive glazing for solar gain and guest experience will be a main part of the exterior make up.

The Site

Pathways and seating areas will be a combination of colored/stamped concrete and flagstone. The proposed landscape will complement the existing landscape materials and transition to riverbank restoration and riparian habitat upgrades below the ten new pools to the Colorado River down the slope.

The Landscape Plans and Civil Engineering Plans attached as an Exhibit illustrate the approved parking and access extents as shown on the "Recently Constructed Site Features" plan sheet prepared by SGM and dated 06/30/2015. The previously approved and constructed parking area contains 94 parking spaces including four accessible spaces.

The expanded Iron Mountain Hot Springs proposes 139 additional parking spaces including three accessible parking spaces with two of these sized to accommodate vans. In total Iron Mountain Hot Springs will provide 233 parking spaces with seven accessible parking spaces, which includes two accessible van spaces. The Applicant agrees to monitor parking so that when the parking lot is full no additional vehicles will be allowed at the facility, and no additional guests will be allowed to enter. SGM has prepared a Traffic Impact Study that is attached as an Exhibit that also addresses traffic and parking.

Ownership

Special Warranty Deeds and a Title Commitment are included as an Exhibit to this Application as proof of ownership.

Public Notice

Notice will be mailed to property owners within 300 feet and the mineral owners and lessees on the property.

CITY OF GLENWOOD SPRINGS
101 W. 8TH STREET
GLENWOOD SPRINGS, CO 81601

COLO RIVER WATER CON DIST
PO BOX 1120
GLENWOOD SPRINGS, CO 81602

TRPP AT 300, LLC
PO BOX 2021
GLENWOOD SPRINGS, CO 81602

1303 GRAND, LLC
PO BOX 1576
GLENWOOD SPRINGS, CO 81602

TWO RIVERS PP OFFICE CONDO ASSOC
PO BOX 1576

GLENWOOD SPRINGS, CO 81602

274 MOUNTAIN SHADOWS DR
GLENWOOD SPRINGS, CO 81601

D&S FAMILY HOLDINGS LLC & HALVERSON
TRUST
PO BOX 3780
EAGLE, CO 81631

SRE COLORADO 4 RF LLC
4401 COLWICK ROAD
CHARLOTTE, NC 28211

CDOT
4201 E. ARKANSAS AVE
DENVER, CO 80222

CERTIFICATION OF NOTICE PURSUANT TO C.R.S. 24-65.5-103

I hereby affirm that an examination of the records in the office of the County Clerk and Recorder was made in accordance with C.R.S. 24-65.5-103 et seq. and

(Check applicable box and fill in the information)



After such examination on Nov. 8, 2019, which is not less than thirty (30) days before the date scheduled for the initial public hearing on the application for the development known as _____, notice was sent, by certified mail, return receipt requested, or by a nationally recognized overnight carrier, to the below-named mineral right owner(s) as listed in the records of the County Clerk and Recorder, containing the time and place of the initial public hearing, and contact information of the Community Development Department, the nature of the hearing, the location and legal description of the property that is the subject of the hearing, and the name of the applicant(s). Notice was also sent, by certified mail, return receipt requested, or by a nationally recognized overnight carrier, to the City of Glenwood Springs, containing the name and address of the mineral right owners to whom the foregoing notice was sent. A copy of the notices sent is attached hereto.

Listing of Mineral Right Owner(s)

Address

None



Such records do not identify any mineral right owners; or



The mineral right owner(s) have provided a signed waiver of the right to notice in writing, a copy of which is attached hereto.

Name:

Mogli Cooper

Applicant Signature

Surface Owner's Signature

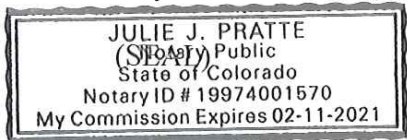
Iron Mountain Hot Springs LLC, by Mogli Cooper

Print Name

Print Name

Subscribed and sworn to before me, a notary public in and for the County of
Garfield, State of Colorado, this 8th day of
November 2019, A.D.

Witness my hand and official seal



Julie J. Pratte
Notary Public

My commission expires: 2 - 11 - 2021

**FAILURE TO PROVIDE THIS CERTIFICATION, INDICATING COMPLIANCE
WITH C.R.S. 24-65.5-103 ET SEQ., IS LIKELY TO RESULT IN A CONTINUANCE OF
THE HEARING.**

Code Compliance

070.020.130: Zone District, Resort (RE) District

Dimensional Standards:

Lot area (sq ft)	none
Landscaped area	10%
Setbacks, Front, min	5 ft
Front, max	60 ft
Side, min	5 ft
Rear, min	10 ft
Building height	60 ft.

The proposed Iron Mountain Hot Springs project complies with the above setbacks, which have been illustrated on the Site Plan attached as an Exhibit to this Application.

The maximum building height is 46'-0" at the highest ridge, west side of second floor to new grade. This complies with the allowed maximum building height of 60 ft.

Minimum Landscaped Area is 12,994 sq. ft. Refer to the Landscape Plan attached as an Exhibit regarding conformance to this standard.

070.030.020: Table of Allowed Uses

The proposed entry building, retail space, locker rooms, offices, outdoor pools, indoor pool, food service, and cabanas are permitted uses by right in the RE zone district.

070.040.020(d)(24): Development in Waterways and Areas of Special Flood Hazard.

Article 070.040.020(d)(24) currently states that no improvement, building, structure, excavation, dumping or backfill shall be placed, built, undertaken or approved within a thirty-foot setback area horizontally from the high water mark of any river or live stream, except for bridges, paths for non-motorized use, irrigation structures or flood control and erosion protection devices.

The Applicant is requesting a variance from the requirement of Municipal Code Section 070.040.020(d)(24) Development in Waterways and Areas of Special Flood Hazard to allow mineral pools in the 30' set back from high water line at Iron Mountain Hot Springs. See Variance Request in the Table of Contents for the Applicant's demonstration of compliance with the Variance Criteria contained in the Section 070.060.070(a)(3)(e.)(2) of the Municipal Code.

This Application does contain new mineral pools and paths that encroach on the proposed thirty foot setback from the ordinary high-water mark. To address water quality, stream bank erosion, and riparian area quality, the Applicant has obtained a U.S.

Army Corps of Engineers Clean Water Act Section 404 Permit that proposes shoreline enhancements at Iron Mountain Hot Springs to include grading, bank stabilization, cobble removal, and riparian vegetation enhancements in order to create riverbank protection, improve public safety, and enhance the riparian zone. The Army Corps of Engineers Clean Water Act Section 404 Application and Permit is attached as an Exhibit to this document.

070.040.030 (e): Site Development and Improvement Standards

- (1) Drainage Plan.
A Major Drainage Report for Iron Mountain Hot Springs has been prepared by Boundaries Unlimited Inc. and is included as an Exhibit to this Application.
- (3) Lot and Block Design.
The project has been designed in conformance with applicable zone district regulations, refer to the Site Plan attached as an Exhibit.
- (5) Water Distribution.
A water distribution plan has been included in the Civil Engineering Plans included as an Exhibit to this Application.
- (6) Wastewater Collection
A wastewater collection system plan has been included in the Civil Engineering Plans included as an Exhibit to this Application.
- (7) Underground Utilities
All Utilities are located on the Civil Engineering Plans included as an Exhibit to this Application.
- (5) Fire and Emergency Services Impact Fee
The Applicant agrees to the City calculations for emergency service impact fees.

070.040.040: Common Open Space

Minimum Total Common Open Space required is 10% of 146,940 sq. ft., or 14,694 sq. ft.
Common Open Space provided is 44,927 sq. ft.

Buildings = 10,634 sq. ft.

Parking = 74,385 sq. ft.

Total Non-Open Space = 85,019 sq. ft.

Total Open Space provided = 44,927 sq. ft. or 65%

070.040.050 (c): Landscaping, Screening, and Fencing

- (1) Minimum Landscape Site Area required is 10% of 146,940 sq. ft., or 14,694 sq. ft. Landscape Site Area provided, not including parking landscape is 25,821 sq. ft.
- (2) Type and amount of Plant Material.

Required perennials 1 per 50 sq. ft. of required landscaped area

Required shrubs 1 per 100 sq. ft. of required landscaped area

Required trees 1 per 400 sq. ft. of required landscape area in addition to those required for parking areas.

The minimum number of trees required is 36.

The minimum number of shrubs required is 142.

The minimum number of perennials required is 284.

The Applicant has obtained a U.S. Army Corps of Engineers Clean Water Act Section 404 Permit that proposes shoreline enhancements at Iron Mountain Hot Springs to include grading, bank stabilization, cobble removal, and riparian vegetation enhancements in order to create riverbank protection, improve public safety, and enhance the riparian zone. The Army Corps of Engineers Clean Water Act Section 404 Application and Permit is attached as an Exhibit to this document.

070.040.060 (h) 6: Parking Area Landscaping

Parking area landscaping requirements for this development equal 20 percent of the total surface parking area (70,984 sq. ft) for a total of 14,197 sq. ft. of required landscaping. The proposed parking area landscaping covers 19,151 sq.ft.

Parking Area landscaping requirements also identify that trees shall be planted at a rate of 1 tree per 8 parking spaces and shall be located throughout the parking lot such that at maturity, the parking lot will be shaded, to the greatest extent practicable. Additional parking area screening shall be provided pursuant to 070.040.060(h)(6)(c)(3), Screening Parking Areas and Drive Aisle. To meet this requirement the Applicant proposes to utilize 10'-15' wide parking islands between the parking bays in the 233 car parking lot. These large islands have been planted with a total of 31 trees, which is one tree per 7.5 parking spaces. These islands have also been planted with shrubs for additional screening. These trees and shrubs will be maintained in good health or replaced as necessary.

Refer to the Landscape Plan attached as an Exhibit to this Application for the locations of the existing plant materials and the species, sizes, and locations of the proposed plant materials.

070.040.060 (c): Off-Street Parking and Loading

Iron Mountain Hot Springs will have 233 existing and proposed parking spaces including seven accessible parking spaces with two of these sized to accommodate vans. The Applicant agrees to monitor parking so that when the parking lot is full no additional vehicles will be allowed at the facility, and no additional guests will be allowed to enter. SGM has prepared a Traffic Impact Study that is attached as an Exhibit that also addresses traffic and parking.

070.040.060 (f): Bicycle Parking

The number of bicycle parking spaces required is 12. The Applicant has provided 18 bicycle parking spaces, which exceeds the requirement of this subsection.

070.040.070: Access and Circulation

SGM has prepared the Iron Mountain Hot Springs Traffic Study, which is attached as an Exhibit. This study is prepared as a CDOT Level Three Traffic Impact Study and provides estimates for peak hour traffic generation for the Iron Mountain Hot Springs (IMHS) property development, proposed to be constructed at the north end of Centennial Street in Glenwood Springs, Colorado. The purpose of the traffic analysis is to document the existing traffic conditions in the vicinity of the site, analyze the trip generation and trip distribution of the proposed development, estimate traffic volumes to the 20-year planning horizon, and analyze the intersections within the study area for operational impacts to the highway or city road system. Access to the site will continue to be provided from Centennial Street and the majority of traffic will use Devereux Road and US 6 for primary access.

070.040.090: Nonresidential and Mixed-Use Site and Building Design

As mentioned earlier in the Project Description, the Applicant wishes to expand the existing spa facility to include a new entry building with retail space and food service, including additional men's and women's locker rooms with sauna facilities. The expanded facility will have access control and be a separate experience from the existing pools. The expansion will provide a limited number of entries to keep the environment tranquil and restorative.

The buildings are designed and located to work with the natural grade of the site. Entrance to the resort will now be through the main level, check in to existing spas OR check in and downstairs to the new addition. On this level will be a restaurant/seating, and associated retail. There is an upper floor that will house offices and storage. At the lower level will be walk out for both men's and women's locker rooms and a large

mechanical space for the new uses. The Entrance building is planned to be constructed as a wood-framed structure. The lower level will consist of a slab on grade foundation. The second and third floors will be wood framed with 1 1/2" of lite weight concrete for code separation and quiet construction.

Exterior materials will be specified to complement the existing palette of the bath house. A stone base is proposed varying in height for building base protection and accents around the structure. Siding material will 1"x8" natural channel cedar siding with 2"x trims. Shaketown shingles will be installed on the second floor exterior behind heavy timber trusses. The fascia will be 2"x12" with metal gutters and downspouts painted to match. Roofing will be standing seam metal with color to match existing buildings. Aluminum clad windows/doors will create a color contrast.

The new pool building will be approximately 4,700 SF with a single use of a new pool. The large format doors to the south and west will open to allow for indoor/outdoor use. A continuous mechanical room will run along the east side to feed the pool and new spas. The roof is designed with earth cover, this will be 12"-18" of soil cover for native grasses. The pool building will be a steel structure to carry the roof load and large spans for the intended use. Expansive glazing for solar gain and guest experience will be a main part of the exterior make up.

For details related to the above narrative refer to the Architectural Plans attached as an Exhibit to this Application.

070.040.100 (f): Exterior Lighting, General Lighting Requirements

The proposed project is located within Lighting District 2. The following standards will apply:

Light Trespass Levels must be not more than 0.5 at any point along the property line and 0.2 at a point 10 across property line.

Mounting height for parking area lighting shall not exceed 20 feet, with a minimum of 0.3 foot candles to 3 foot candles at grade.

Lighting of walkways and common areas shall be illuminated to a minimum of .2 foot candles to a maximum of 3 foot candles.

All Fixtures shall be full cut-off

A Lighting Plan that meets the above standards will be developed in the Architectural Plan Set at the time of building permit application.

070.060.030: Neighborhood Meeting

A Neighborhood Meeting was held on July 24, 2020 at 1:00 p.m. to present project details and receive feedback from the neighborhood group. The Applicant presented a site plan and building renderings showing the proposed expansion. Neighborhood attendees asked questions about timing for the project, the existing pump station and its expansion, riverbank restoration and parking configuration. The Applicant did remind the neighbors they would be receiving public notice as public hearings are scheduled with the City of Glenwood Springs.

An Exhibit showing the list of those notified by mail, sent on July 9, 2020, those in attendance at the meeting and a copy of the Exhibits used at the meeting has been included with this application.

Variance Request

The following is a Variance Request to the City of Glenwood Springs. The Applicant is requesting a variance from the requirement of Municipal Code Section 070.040.020(d)(24) Development in Waterways and Areas of Special Flood Hazard Variance to allow mineral pools in the 30' set back from high water line at Iron Mountain Hot Springs.

VARIANCE CRITERIA

In reviewing a Variance Application, the Planning and Zoning Commission must determine that all of the following exist:

1. The subject property has an exceptional shape, topography, building configuration or other exceptional site condition that is not a general condition throughout the zone district;

The site is the location of a historic hot springs property that has fallen into disrepair along the riverbank where the proposed expansion will take place. The property has little vegetation at the river's edge, the bank is a mix of discarded aggregate materials, stone and rip-rap from when the previous owners abandoned the site. The riverbank along the property line is in stark contrast to the adjacent properties landscaped, retained and stable riverbank.

2. The strict application of the Code standards for which a variance is sought would produce undue hardship;

Without repair, re-vegetation, and incorporation of the riverbank into the expansion of spa area, the property would not be suitable for resort development.

3. The Applicant did not create the hardship by his/her own actions;

The site was previously abandoned by others and has degraded to a level not suitable to develop without significant riverbank restoration.

4. The variance requested does not harm the public and does not impair the intent or purposes of this Code, goals, and policies, including the specific regulation for which the variance is sought;

The requested variance will only improve conditions along the river section of property. Having natural spa pools inside the set-back in no way harms the public or impairs the intent of code.

5. The variance request demonstrates exceptional hardship not related to purposes of convenience or financial burden;

Without the variance the available suitable area for development is below the level at which the property can be developed..

6. The variance request will not violate building or fire code requirements; and

Allowing natural spa pools inside the river set-back has no effect on the structures in the proposed property and will have no impact on building or fire codes.

7. The variance is the minimum variance that will afford relief of the subject standards of the Code.

Spa pools inside the river set-back does not adversely affect the flow, direction or level of the adjacent river and is the absolute minimum change condition that will allow the property to be developed.

Exhibits

- A. Title Policy and Warranty Deeds
- B. Iron Mountain Hot Springs Master Plan
- C. CTL Thompson Geologic and Geotechnical Investigation
- D. Updated SGM Traffic Impact Study
- E. Army Corps of Engineers 404 Permit
- F. Civil Engineering Plan Set
- G. Architectural Plan Set
- H. Landscape Plan
- J. Neighborhood Meeting Documents
- K. Major Drainage Report for Iron Mountain Hot Springs



September 4th. 2020

Re: Iron Mountain Hot Springs “Pool building” roof design, Glenwood Springs, CO

Overview of Building Design

The new pool building is designed and located to work with the natural grade of the site, with approximately 4,700 SF under roof. The large format doors to the south and west will open to allow for indoor / outdoor use. A mechanical room will run along the east side to feed the pool and new spas.

The roof is designed with earth cover, this will be 12-18” of soil cover for native grasses.

The pool building will be a steel structure to carry the roof load and large spans for the intended use. Expansive glazing for solar gain and guest experience will be a main part of the exterior make up.

Exterior materials will be specified to match the existing palette of Bath house. Stone base, varying in height for building base protection and accents around the structure. 1x8 natural channel cedar siding with 2x trims finishes the upper portions.

Aluminum clad windows / doors to create that color contrast.

- Roofing of the existing bath house and other structures is a standing seam metal roof. The new entrance building will be of same material. As this building will be in proportion with Bath house, metal roof will continue the existing design theme.

With the positioning and elevation of the new pool building, this would create a solid barrier with glare. At a 5’ spring point at entry sidewalk (and a 24” parapet to support earth), the plantings will present an environmentally friendly solution for this single pitch roof.

- To advance the goals of development code:

(4) *Roofs.*

- a. To reduce the visual impact of roof mass, no roofline along any building elevation shall exceed fifty (50) feet in length without a visual variation that incorporates:

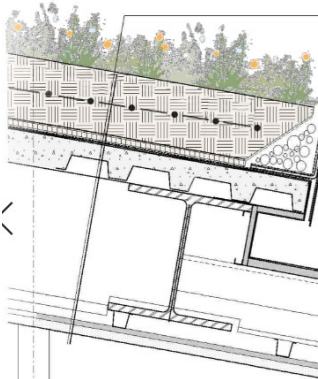
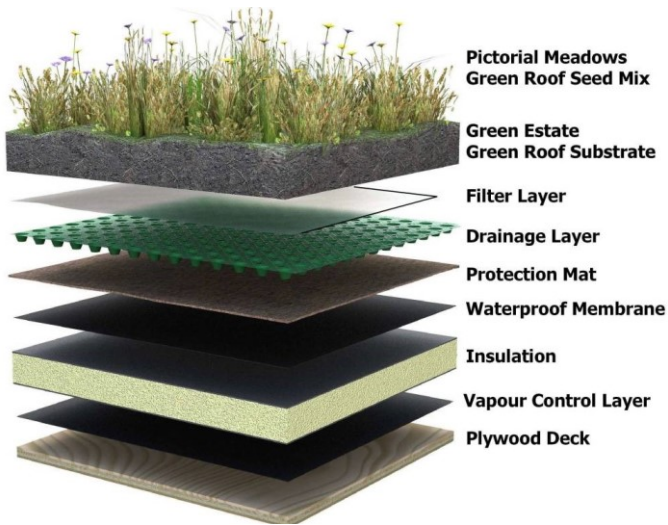
The softer earth covering will be in-line with the mission of the town, reducing the visual impact. By allowing for native grasses and vegetation to be visible as guest arrive for their experience, this progresses the resorts mission,

Situated alongside the beautiful Colorado River in Glenwood Springs and surrounded by Rocky Mountain views that soothe and inspire, our 16 geothermal soaking pools offer guests a pure soaking experience—the way nature intended. Our freshwater family pool is cooler and ideal for all ages. An attached elevated jetted spa provides yet another opportunity to relax. Thoughtful amenities are built in throughout: towel tree sculptures at each soaking pool, changing rooms stocked with complimentary shower products and a restaurant that serves food and drinks adds to the ambiance. Welcome to Iron Mountain Hot Springs. We invite you to relax, restore and rejuvenate.



For reference only:

- By creating this earth roof, the community and guest will further experience Rocky Mountain native plantings, resulting in a soft surface. IF staying with conventional roofing materials on this plane, it would not “soothe” as intended.
- By allowing this earth finish, there will be reduction in glare to the surrounding properties. This application will result in the appearance of less structure and not take away from the Colorado River.
- Irrigation: This will need to have permanent irrigation to keep the landscaping green and thriving. Supply lines will be buried for aesthetic reasons, then Multi Stream Rotator irrigation heads will distribute the water in a fashion as to not overspray and create flooding.



Plant Type	Plant Height	Minimum Soil Depth
Annuals	Any	3"
Turf Grass	NA	4" (3" in warm climates)
Perennials	<8"	4" (3" in warm climates)
Perennials	8"-16"	6"-8"
Perennials/ Ornamental Grasses/ Shrubs	16"-24"	12"-18"

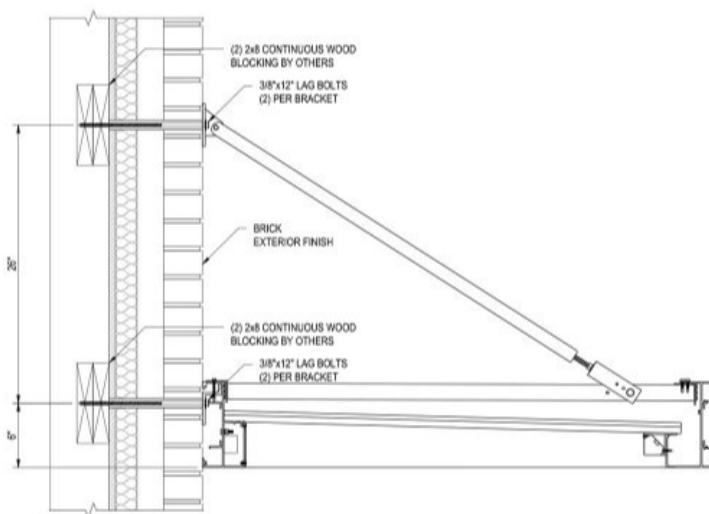




September 4th. 2020

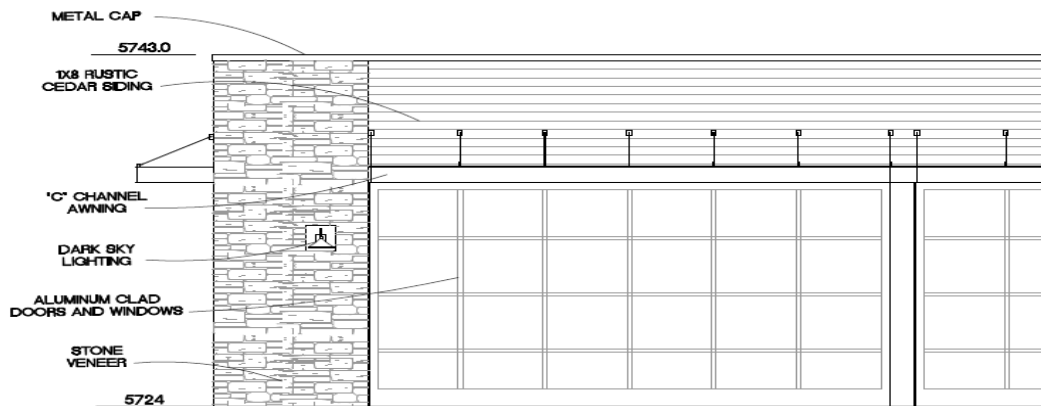
Re: Iron Mountain Hot Springs “Pool building” elevations, Glenwood Springs, CO
Overview of Building Design

After studying the elevations further for the proposed Pool Building,
We have added “C” channel awnings at the operable doors.



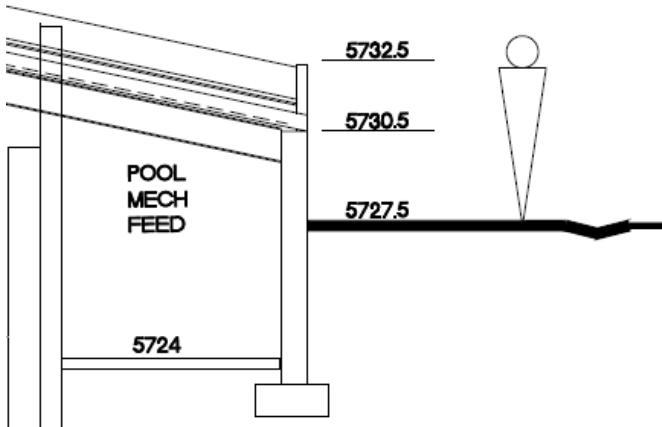
With these adds, we now achieve the “3” elements of approvable elevations.

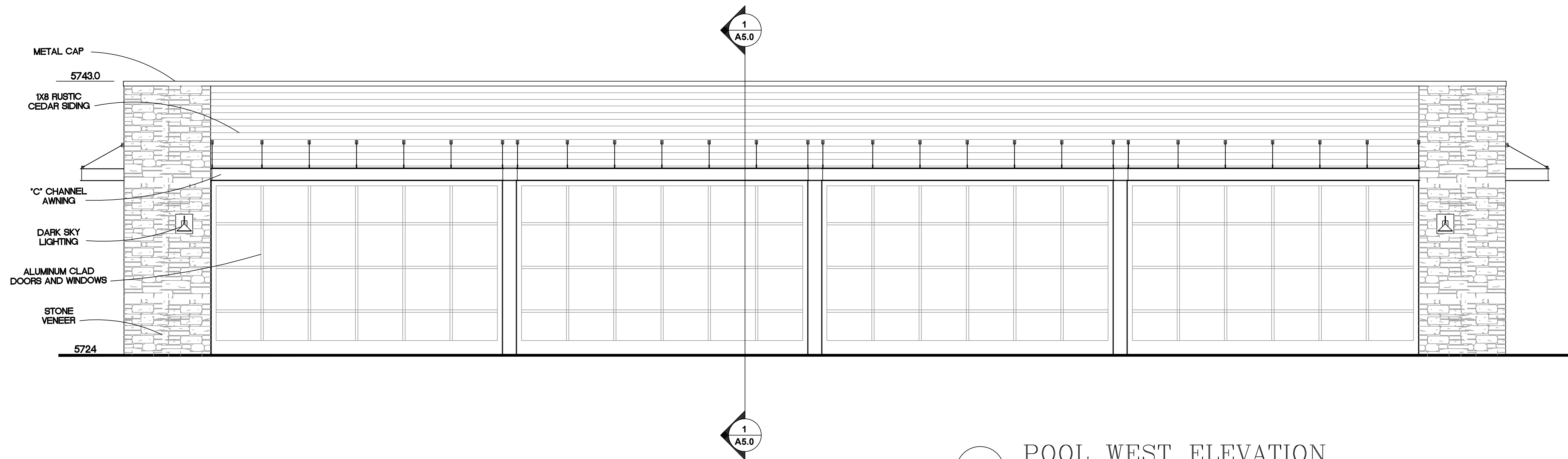
1. Combination of materials :
 - Natural stone to match existing structures / Horizontal wood siding / metal window frames
2. Over 30% of glazing, both windows and operable doors.
3. “C” channel awnings with a 48” projection from façade face.



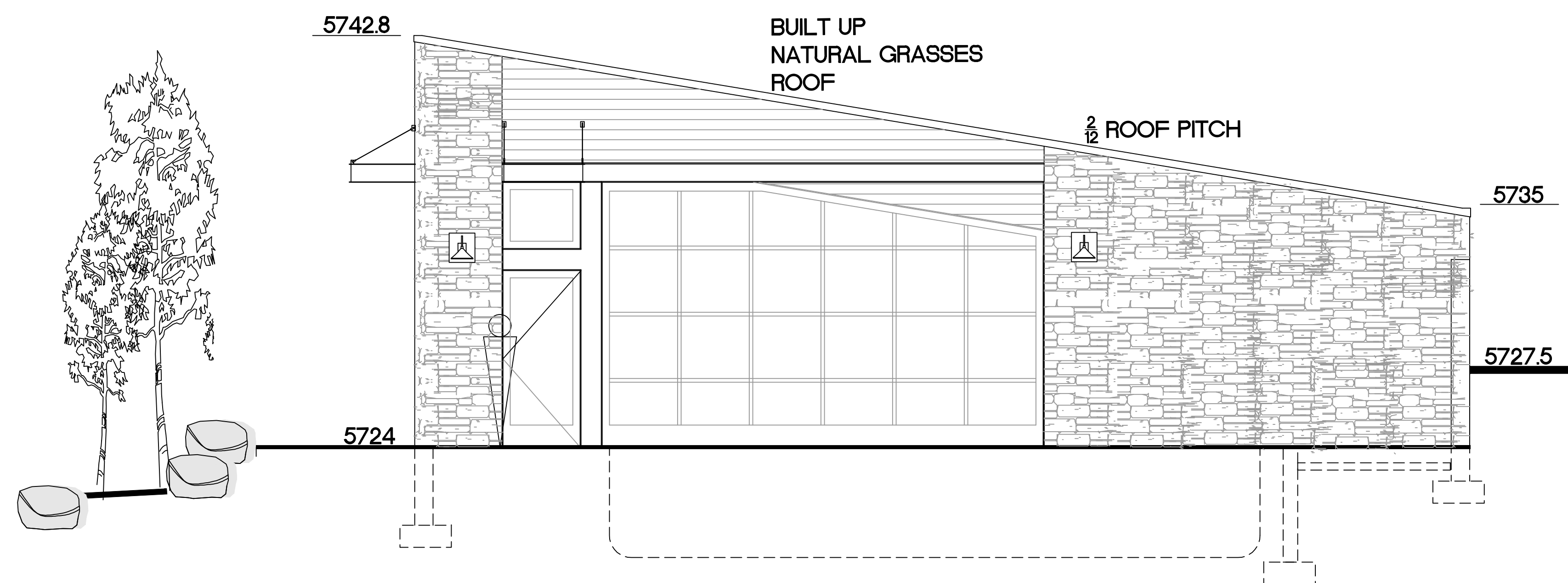


The east wall of the structure, facing parking lot, is to appear as if it is a retaining wall, faced with matching stone. 60" tall with landscaping (earth roof) above, climbing at a 2:12 pitch.

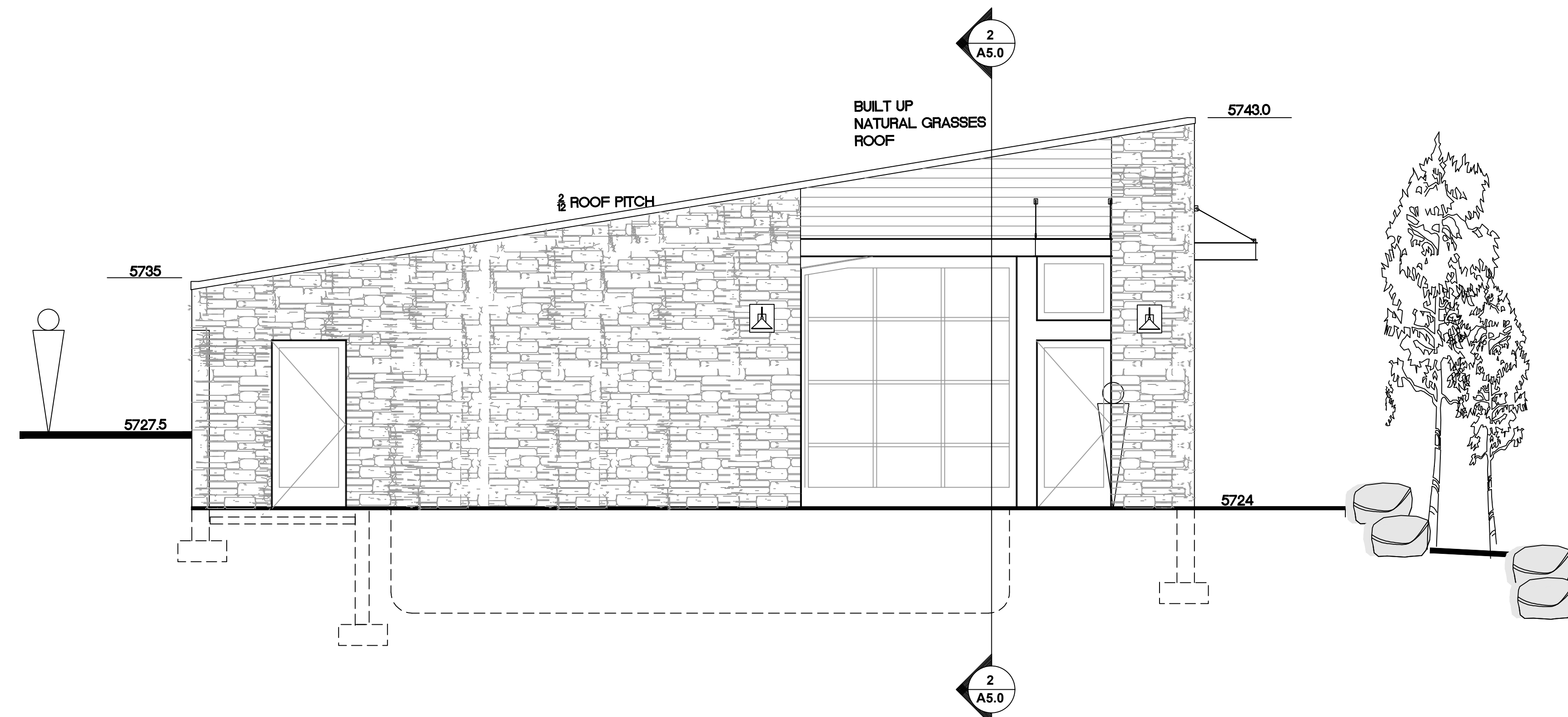




1 POOL WEST ELEVATION
1/4" = 1'-0"



3 POOL SOUTH ELEVATION
1/4" = 1'-0"



2 POOL NORTH ELEVATION
1/4" = 1'-0"

POOL BUILDING

Revisions:

Sheet title
POOL
BUILDING
ELEVATIONS

Stamp

Date: 9-04-20

Project No.

Drawn by: MEF

Checked by: MEF

© 2020 TRIUMPH

Sheet

AB4.0

**Commonwealth Title Company
of Garfield County, Inc.**

127 E. 5th Street
Rifle, CO 81650
(970) 625-3300 Phone
(970) 625-3305 Fax

Date: August 14, 2014

To: Iron Mountain Hot Springs, LLC, a Colorado limited liability company
51000 Two Rivers Plaza Road
Glenwood Springs, CO 81601

**Thank you for your order.
Enclosed please find the following
in connection with our File No. 1307020-3:**

- £ Commitment
 - ☒ Title Policy
 - £ Endorsement
 - £ Tax Certificate
 - £ Other
-

POLICY OF TITLE INSURANCE

SCHEDULE A

Amount of Insurance: **\$5,000,000.00**

Policy No. **8130606-91758288**

Premium **\$8670.00**

File No. **1307020-3**

Date of Policy: **August 1, 2014 at 5:00 PM**

1. Name of Insured

Iron Mountain Hot Springs, LLC, a Colorado limited liability company

2. The Estate or interest in the land described herein and which is covered by this policy is Fee Simple and is at the date of Policy vested in:

Iron Mountain Hot Springs, LLC, a Colorado limited liability company

3. The land referred to in this policy is described in the said instrument, is situated in the County of GARFIELD, State of CO, and is identified as follows:

See Attached Exhibit "A"

Countersigned:

Authorized Officer or Agent

NM 1 PA 10
ALTA Owner's Policy (6/17/06)
Form 1190-2 Schedule A

EXHIBIT "A"

Parcel No. 1:

Lot 2
Block 1
North Glenwood Center
In the City of Glenwood Springs

Parcel No. 2:

Lot 1
Block 2
North Glenwood Center
In the City of Glenwood Springs

Parcel No. 3:

Tract 1) That part of Parcel No. 35-A of the State Department of Highways, Division of Highways, State of Colorado, Project No. I 70-1(8)113 Sec. 2, in the NW1/4NW1/4, Section 9, and Lot 8 Section 4, Township 6 South, Range 89 West of the 6th P.M., described as follows:

Beginning at a point on the South line of Section 4, Township 6 South, Range 89 West, from which point the SW corner of Section 4 bears N. 88°21' W. 727.5 feet;
thence S. 88°21' E., along the South line of Section 4, 41.4 feet;
thence along the arc of a curve to the right, having a radius of 5829.6 feet, 174.4 feet (the chord of this arc bears N. 36°02'30" W. 174.4 feet);
thence S. 23°38'30" E. 152.6 feet, more or less, to the point of beginning.

Tract 2) That part of said Parcel No. 35-A described as follows:

Beginning at a point on the North line of Section 9, Township 6 South, Range 89 West, from which point the NW corner of Section 9 bears N. 88°21' W. 727.5 feet;
thence S. 23°38'30" E., along the Southerly right-of-way line of the Denver & Rio Grande Western Railroad, 561.9 feet to a concrete monument, being that monument described in Book 340 at Page 565, of the Garfield County records;
thence N. 39°06'30" E. 186.2 feet;
thence along the arc of a curve to the right, having a radius of 5829.6 feet, 476.6 feet (the chord of this arc bears N. 39°14' W. 476.4 feet) to the North line of Section 9;
thence N. 88°21' W., along the North line of Section 9, 41.4 feet, more or less, to the point of beginning.

EXCEPTING all that portion of said Parcels 1) and 2) above described as follows, to-wit:

Beginning at the concrete monument referred to in the description of Tract 2) above, being that monument as described in Book 340 at Page 565, of the Garfield County records;
thence N. 39°06'30" E. 186.21 feet;
thence along the arc of a curve to the right having a radius of 5829.6 feet, 137.64 feet (the chord of this arc bears N. 40°53'57" W. 137.64 feet);
thence S. 42°31'33" W. 48.80 feet;
thence S. 85°00' W. 84.50 feet;
thence S. 23°38'30" E. 224.00 feet, more or less, to the point of beginning.

Tract 3) A tract of land in the SW1/4SW1/4 Section 4 and the NW1/4NW1/4 Section 9, Township 6 South, Range 89 West of the 6th P.M., described as follows:

Beginning at a point whence the common corner to Sections 4, 5, 8 and 9, Township 6 South, Range 89 West of the 6th P.M., bears N. 88°21' W. 727.5 feet;
thence S. 23°38'30" E. 561.9 feet;
thence S. 39°06'30" W. 217.5 feet;
thence N. 20°55'30" W. 239.3 feet;
thence S. 46°38'30" W. 435.4 feet;
thence N. 05°33'10" W. 1340.4 feet;
thence S. 41°17'30" E. 579.5 feet;
thence S. 23°38'30" E. 152.6 feet, more or less, to the point of beginning.

Parcel No. 4:

A tract or parcel of land No. E-35 of the State Department of Highways, State of Colorado, Project No. I 70-1(8)113 Sec. 2, in Lot 8 of Section 4, Township 6 South, Range 89 West of the 6th Principal Meridian, in Garfield County, Colorado, said tract or parcel being more particularly described as follows:

Beginning at a point from which the SW corner of Section 4, Township 6 South, Range 89 West of the 6th P.M., bears S. 79°52'30" W., a distance of 676.5 feet;

1. Thence N. 29°47'30" W., a distance of 336.3 feet;
2. Thence N. 72°07' W., a distance of 130.8 feet;
3. Thence S. 41°17'30"E., a distance of 441.9 feet, more or less to the point of beginning.

SCHEDULE B

This Policy does not insure against loss or damage by reason of the following:

1. Rights or claims of parties in possession not shown by the Public records.
2. Easements, or claims of easements, not shown by the public records.
3. Discrepancies, conflicts in boundary lines, shortage in area, encroachments, and any facts, which a correct survey and inspection of the premises would disclose, and which are not shown by the public records.
4. Any lien, or right to a lien, for services, labor or material heretofore or hereafter furnished, imposed by law and not shown by the public records.
5. Unpatented mining claims, reservations or exceptions in patents or in acts authorizing the issuance thereof.
6. Taxes and assessments for the year 2014, not yet due or payable.
7. Any and all water rights, claims, or title to water, whether or not the matters excepted are shown by the public record.
8. Easements and rights of way for all ditches, pipelines, laterals and canals.
9. Any question, dispute or adverse claims as to any loss or gain as a result of any change in the river bed location by other than natural causes, or alteration through accretion, reliction, erosion or avulsion of the center thread, bank, channel or flow of waters in Colorado lying within subject land; and any questions as to the location of such center thread, bed, bank or channel as a legal description monument or marker for purposes of describing or locating subject lands.
10. Right of the proprietor of a vein or lode to extract and remove his ore therefrom, should the same be found to penetrate or intersect the premises hereby granted and a right of way for ditches or canals as constructed by the authority of the United States, as reserved in United States Patent recorded February 10, 1892 in Book 12 at Page 106.
11. Right of the proprietor of a vein or lode to extract and remove his ore therefrom, should the same be found to penetrate or intersect the premises hereby granted and a right of way for ditches or canals as constructed by the authority of the United States, as reserved in United States Patent recorded April 7, 1892 in Book 12 at Page 131.
12. Permanent Easement granted to the Department of Highways, State of Colorado as described in instrument recorded October 6, 1967 in Book 388 at Page 291.
13. Each and every right or rights of access to and from any part of the right-of-way for I-70 as described in instrument recorded October 6, 1967 in Book 388 at Page 291 and in Book 427 at Page 91.
14. Reservation of the Hobo Spring Pump and Pipeline and an easement for a road right-of-way 10 feet in width and the right to enter the land to produce, pump, direct and convey away any and all fluids and substances from Hobo Spring as described in instrument recorded February 15, 1977 in Book 493 at Page 460.
15. Easements, rights of way and all matters shown on the Final Plat of North Glenwood Center recorded February 13, 1976 at Reception No. 271331.
16. Terms, conditions and all matters set forth in Agreement recorded February 20, 1981 in Book 565 at Page 943.

(Continued)

Exceptions Number 1,2,3 AND 4 are hereby omitted.

SCHEDULE B
(Continued)

17. Terms, conditions and all matters set forth in City of Glenwood Springs Ordinances as follows:

Ordinance Number	Recording Date	Book	Page
17 Series of 1975	March 12, 1982	<u>594</u>	<u>500</u>
8 Series of 1975	March 12, 1982	<u>594</u>	<u>502</u>
40 Series of 1981	March 12, 1982	<u>594</u>	<u>519</u>

18. Easement for a water line as described in instrument recorded October 23, 1991 in Book 816 at Page 113.
19. Terms, conditions and all matters set forth and described in Settlement Agreement by and between Pitkin Iron Corporation and the Glenwood Hot Springs Lodge and Pool, Inc. recorded October 1, 2007 at Reception No. 734175.
20. Rights of way, apparent easement for overhead utilities, right of way for pedestrian fishing easement, apparent right of way for concrete culvert and all matters shown on Improvement Survey prepared by Gamba and Associates, Inc., dated August 15, 2013, Project No. 13829.
21. Deed of Trust from Iron Mountain Hot Springs, LLC, a Colorado limited liability company to the Public Trustee of Garfield County for the use of Pitkin Iron Corporation, a Colorado corporation, showing an original amount of \$4,500,000.00, dated July 31, 2014 and recorded August 1, 2014 as Reception No.852108.

Recorded at _____ o'clock _____ M. _____
Reception No. _____ Recorder.

SPECIAL WARRANTY DEED

THIS DEED, made on July 31, 2014,

between **Delaney Family Investments, LLLP as to an undivided 1/2 interst in Parcel 1; Pitkin Iron Corporation, a Colorado Corporation as to Parcels 2, 3 and as to an undivided 1/2 interest in Parcel 1**

of the County of Garfield, and State of Colorado, grantor,

and **Pow, Inc., a Colorado Corporation as to an undivided 50% interest as tenants in common with Turtlepoop LLC, a Colorado limited liability company as to an undivided 50% interest**

whose legal address is : 51000 Two Rivers Plaza Road, Glenwood Springs, CO 81601

of the County of Garfield, and State of Colorado, grantee:

WITNESS, that the grantor, for and in consideration of the sum of ten dollars and other good and valuable consideration, the receipt and sufficiency of which is hereby acknowledged, has granted, bargained, sold and conveyed, and by these presents does grant, bargain, sell, convey and confirm unto the grantee, its successors and assigns forever, all the real property, together with improvements, if any, situate, lying and being in the County of Garfield and State of Colorado described as follows:

See Attached Exhibit "A"

also known by street and number as:

TOGETHER with all and singular the hereditaments and appurtenances thereunto belonging, or in anywise appertaining, and the reversion and reversions, remainder and remainders, rents, issues and profits thereof, and all the estate, right, title, interest, claim and demand whatsoever of the grantor, either in law or equity, of, in and to the above bargained premises, with the hereditaments and appurtenances;

TO HAVE AND TO HOLD the said premises above bargained and described, with the appurtenances, unto the grantee, its successors and assigns forever. The grantor, for itself, its successors and assigns does covenant and agree that it shall and will WARRANT AND FOREVER DEFEND the above bargained premises in the quiet and peaceable possession of the grantee, its successors and assigns, against all and every person or persons claiming the whole or any part thereof, by, through or under the grantor.

IN WITNESS WHEREOF, the grantor has executed this deed on the date set forth above.

Delaney Family Investments, LLLP as to an undivided 1/2 interst in Parcel 1

By: Diane Delaney
Diane Delaney, General Partner

Pitkin Iron Corporation, a Colorado Corporation as to Parcels 2, 3 and as to an undivided 1/2 interest in Parcel 1

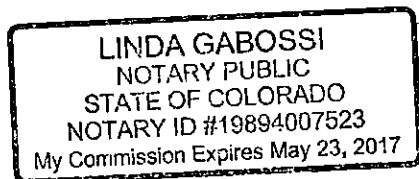
By: Diane D. Delaney
Diane D. Delaney, Vice-President

STATE OF COLORADO)
) ss.
COUNTY OF GARFIELD)

The foregoing instrument was acknowledged before me on July 31, 2014, by Diane Delaney as General Partner of Delaney Family Investments, LLLP as to an undivided 1/2 interst in Parcel 1; Diane D. Delaney as Vice-President of Pitkin Iron Corporation, a Colorado Corporation as to Parcels 2, 3 and as to an undivided 1/2 interest in Parcel 1.

WITNESS my hand and official seal

My commission expires:



Linda Gabossi

Notary Public

Commonwealth File No. 1307020-2
Return to:
Iron Mountain Hot Springs LLC
51000 Two Rivers Plaza Road
Glenwood Springs, CO 81601

Doc 500.00
2

File No. 1307020-2

EXHIBIT "A"

Parcel No. 1:

Lot 2
Block 1
North Glenwood Center
In the City of Glenwood Springs

Parcel No. 2:

Lot 1
Block 2
North Glenwood Center
In the City of Glenwood Springs

Parcel No. 3:

Tract 1) That part of Parcel No. 35-A of the State Department of Highways, Division of Highways, State of Colorado, Project No. I 70-1(8)113 Sec. 2, in the NW1/4NW1/4, Section 9, and Lot 8 Section 4, Township 6 South, Range 89 West of the 6th P.M., described as follows:

Beginning at a point on the South line of Section 4, Township 6 South, Range 89 West, from which point the SW corner of Section 4 bears N. 88°21' W. 727.5 feet;
thence S. 88°21' E., along the South line of Section 4, 41.4 feet;
thence along the arc of a curve to the right, having a radius of 5829.6 feet, 174.4 feet (the chord of this arc bears N. 36°02'30" W. 174.4 feet);
thence S. 23°38'30" E. 152.6 feet, more or less, to the point of beginning.

Tract 2) That part of said Parcel No. 35-A described as follows:

Beginning at a point on the North line of Section 9, Township 6 South, Range 89 West, from which point the NW corner of Section 9 bears N. 88°21' W. 727.5 feet;
thence S. 23°38'30" E., along the Southerly right-of-way line of the Denver & Rio Grande Western Railroad, 561.9 feet to a concrete monument, being that monument described in Book 340 at Page 565, of the Garfield County records;
thence N. 39°06'30" E. 186.2 feet;
thence along the arc of a curve to the right, having a radius of 5829.6 feet, 476.6 feet (the chord of this arc bears N. 39°14' W. 476.4 feet) to the North line of Section 9;
thence N. 88°21' W., along the North line of Section 9, 41.4 feet, more or less, to the point of beginning.

EXCEPTING all that portion of said Parcels 1) and 2) above described as follows, to-wit:

Beginning at the concrete monument referred to in the description of Tract 2) above, being that monument as described in Book 340 at Page 565, of the Garfield County records;
thence N. 39°06'30" E. 186.21 feet;
thence along the arc of a curve to the right having a radius of 5829.6 feet, 137.64 feet (the chord of this arc bears N. 40°53'57" W. 137.64 feet);
thence S. 42°31'33" W. 48.80 feet;
thence S. 85°00' W. 84.50 feet;
thence S. 23°38'30" E. 224.00 feet, more or less, to the point of beginning.

Tract 3) A tract of land in the SW1/4SW1/4 Section 4 and the NW1/4NW1/4 Section 9, Township 6 South, Range 89 West of the 6th P.M., described as follows:

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thence S. 46°38'30" W. 435.4 feet;
thence N. 05°33'10" W. 1340.4 feet;
thence S. 41°17'30" E. 579.5 feet;
thence S. 23°38'30" E. 152.6 feet, more or less, to the point of beginning.

SPECIAL WARRANTY DEED

THIS DEED, made on July 31, 2014,

between **Pow, Inc., a Colorado Corporation as to an undivided 50% interest and Turtlepoop LLC, a Colorado limited liability company as to an undivided 50% interest**

of the County of Garfield, and State of Colorado, grantor(s), and

Iron Mountain Hot Springs LLC, a Colorado limited liability company

whose legal address is : 51000 Two Rivers Plaza Road, Glenwood Springs, CO 81601

of the County of Garfield, and State of Colorado, grantees:

WITNESS, that the grantor(s), for and in consideration of the sum of ten dollars and other good and valuable consideration, the receipt and sufficiency of which is hereby acknowledged, have granted, bargained, sold and conveyed, and by these presents do grant, bargain, sell, convey and confirm unto the grantees, their heirs and assigns forever, all the real property, together with improvements, if any, situate, lying and being in the County of Garfield and State of Colorado described as follows:

See Attached Exhibit "A"

also known by street and number as:

TOGETHER with all and singular the hereditaments and appurtenances thereunto belonging, or in anywise appertaining, and the reversion and reversions, remainder and remainders, rents, issues and profits thereof, and all the estate, right, title, interest, claim and demand whatsoever of the grantor(s), either in law or equity, of, in and to the above bargained premises, with the hereditaments and appurtenances;

TO HAVE AND TO HOLD the said premises above bargained and described, with the appurtenances, unto the grantees, their heirs and assigns forever. The grantor(s), for themselves, their heirs, and personal representatives, successors and assigns do covenant and agree that they shall and will WARRANT AND FOREVER DEFEND the above bargained premises in the quiet and peaceable possession of the grantees, their heirs and assigns, against all and every person or persons claiming the whole or any part thereof, by, through or under the grantor(s).

IN WITNESS WHEREOF, the grantor(s) have executed this deed on the date set forth above.

Pow, Inc., a Colorado Corporation

Turtlepoop LLC, a Colorado limited liability company

By: Steve Beckley, President

By: Mogli Cooper, Member/Manager

STATE OF COLORADO)
) ss.
COUNTY OF GARFIELD)

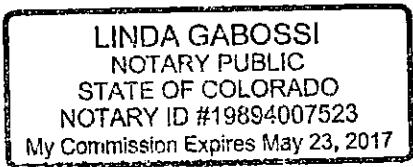
The foregoing instrument was acknowledged before me on July 31, 2014, by Steve Beckley as President of Pow, Inc., a Colorado Corporation and Mogli Cooper as Member/Manager of Turtlepoop LLC, a Colorado limited liability company.

WITNESS my hand and official seal

Linda Gabossi
Notary Public

My commission expires:

Commonwealth File No. 1307020-3
Return to:
Iron Mountain Hot Springs LLC
51000 Two Rivers Plaza Road
Glenwood Springs, CO 81601



File No. 1307020-2

EXHIBIT "A"

Parcel No. 1:

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Block 1
North Glenwood Center
In the City of Glenwood Springs

Parcel No. 2:

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In the City of Glenwood Springs

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Tract 1) That part of Parcel No. 35-A of the State Department of Highways, Division of Highways, State of Colorado, Project No. 1 70-1(8)113 Sec. 2, in the NW1/4NW1/4, Section 9, and Lot 8 Section 4, Township 6 South, Range 89 West of the 6th P.M., described as follows:

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thence S. 41°17'30" E. 579.5 feet;
thence S. 23°38'30" E. 152.6 feet, more or less, to the point of beginning.

Iron Mountain Hot Springs Master Plan

Glenwood Springs, Colorado

Updated July 28, 2020 by The Land Studio, Inc.
for City of Glenwood Springs Pre-Application Conference



0 60 120 180 300

proposed river bank restoration

existing pool

existing cafe
food service

existing
mechanical

existing
locker rooms
retail
offices

Existing
Iron Mountain
Hot Springs

**New Entry
Building**

locker rooms
food service
retail
mechanical

**Iron Mountain
Hot Springs
Expansion**

**New
Enclosed
Pool**

cabanas

new lift station

proposed river bank restoration

future path connection to the LoVa trail

INTERSTATE 70

**Reconfigured
Iron Mountain Hot Springs Parking**

**Approved
Caverns
Village**

CDOT offices
(19) apartments

Approved Caverns Village
parking

Approved Caverns Village
parking

CDOT
maintenance
facility

Future Hotel

entry
monument

Glenwood Caverns Adventure Park
additional parking

GEOLOGIC AND
GEOTECHNICAL INVESTIGATION
IRON MOUNTAIN HOT SPRINGS
GLENWOOD SPRINGS, COLORADO

Prepared For:

IRON MOUNTAIN HOT SPRINGS, LLC
51000 Two River Plaza Road
Glenwood Springs, CO 81601

Attention: Ms. Mogli Cooper

Project No. GS05875-145

June 29, 2014

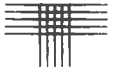


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SCOPE

This report presents the results of our geologic and geotechnical investigation for the proposed Iron Mountain Hot Springs development to be built in Glenwood Springs, Colorado. The purpose of our investigation was to evaluate site geology, geologic hazards and to provide geotechnical design and construction criteria for the proposed buildings, water features and associated paved parking area and drives. This investigation was conducted in general accordance with our Proposal No. GS 14-0191, dated July 16, 2014.

This report includes descriptions of the site geology and potential geologic hazards and subsoil and groundwater conditions encountered in our previous test pits excavated at the site. We are providing recommended foundation systems and pavement sections and design and construction criteria for details influenced by the subsoils. This report was prepared using data from our 1995 field and laboratory investigations at the site and our experience.

SITE CONDITIONS

The site is a triangle shaped parcel of approximately 7.5 acres. The Colorado River borders the site to the west. Interstate 70 borders the site to the north and east. Colorado Department of Transportation property is adjacent to the east. A vicinity map is shown on Figure 1 of this report. Numerous single family residential buildings, sheds and a hot springs lodge have previously been razed at the site. Man-made fill has been stockpiled at the southeast and southwest parts of the site. Access drives and parking areas are on the southwest and central parts of the site. An artesian geothermal well is located in the south central part of the site.



Ground surfaces are flat to gently rolling and slope down to the west. Vegetation consists of deciduous trees and areas of brush, grasses and weeds along the Colorado River. Existing site conditions are shown on Figure 2 of this report.

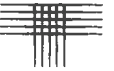
PROPOSED CONSTRUCTION

Plans for water features were developed by Fieldscape of Avon, Colorado and were dated July 7, 2014. Building plans were by The Drawingboard of Glenwood Springs and dated July 7, 2014. Locations of the water features and buildings are shown on Figure 3 of this report.

Water features will include an approximately 40 feet by 60 feet main pool with a hot tub and three separate 15 foot diameter pools. An in-ground slide will be accessed by a path from the main pool and drop down to the bottom pond. The slide will go below a new pedestrian bridge connecting heated stone walkways. The bridge will provide access to the northern area of the development.

The main building will be to the east, and above the water features and west of a drop-off circle. A maintenance building and concession building will be north and west of the main building, respectively. The main building will be approximately 30 feet by 60 feet, wood frame with slab-on-grade floors. The building will have a partial basement for storage and will be one-story above ground. Foundation loads are anticipated to be light. The maintenance and concessions buildings will be lightly loaded one-story, wood frame buildings with slab-on-grade floors.

The following elevations are shown on plans we reviewed. Building elevations are finished floor, pool elevations are bottom of pool.

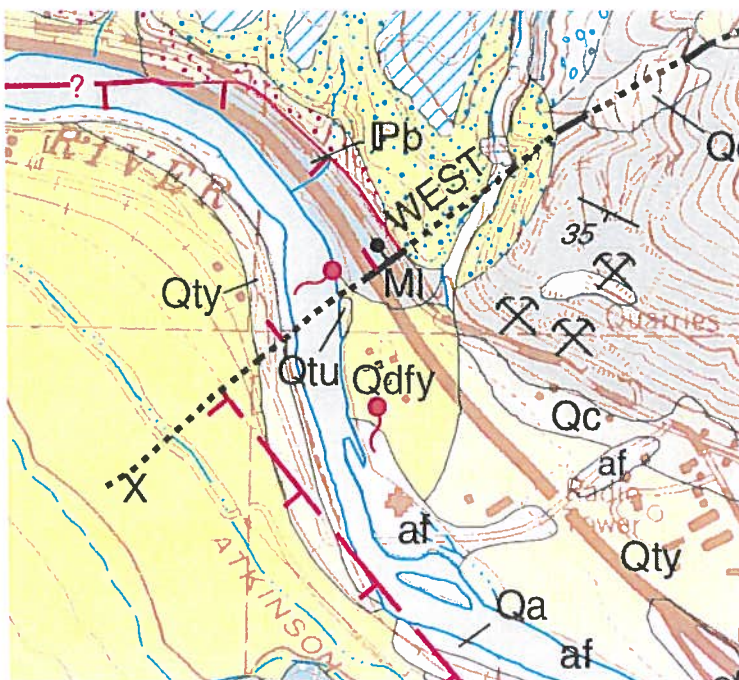
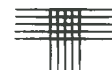


Main Building	5735 feet
Mechanical Building	5738 feet
Concession Building	5728 feet
Main Pool	5728 feet
Smaller Polls	5718 to 5728 feet

SITE GEOLOGY AND GEOLOGIC HAZARDS

Site geology and geologic hazards on the Iron Mountain Hot Springs parcel were evaluated by David A. Glater, P.E., C.P.G., using a review of available literature, aerial photographs and data from our exploratory pits and a site visit on July 20, 2014. This section of the report is intended to address Article 070.100 from the Glenwood Springs City Code entitled "***Development in Areas of Geologic Hazards.***" Literature references are cited at the end of this section.

Mapping by the Colorado Geological Survey (2008, Reference 1) indicates the strata beneath the parcel consist of a thin layer of Younger Debris Flow Fan deposits (map symbol Qdfy) over cobbles and boulders from the Younger Terrace Alluvium of Quaternary age (map symbol Qty). Some of the soils are cemented with calcium travertine related to hot springs activity on and above the site. We believe the debris fan and alluvium deposit continues for many feet to bedrock at an unknown depth. An excerpt from Reference 1 is presented below.



Excerpt from CGS 2008 Geologic Map of Glenwood Springs Quadrangle. The Iron Mountain site is in the approximate center of the map proximate to the red hot spring symbol south of the West Glenwood Fault.

The Iron Mountain Hot Springs parcel is situated along the Colorado River just west of Glenwood Springs. The hot springs activity is related to the West Glenwood Fault, which is considered tectonically inactive. Mineral-rich waters created the travertine deposits.

Geologic hazards typical in Colorado are described in Reference 2. We found no evidence that avalanches, landslides, rockfalls, mudflows, subsidence features, expansive soil and bedrock affect the site. Earthquake hazard is no higher than the regional exposure, and that is considered low risk. The site contains prehistoric debris flow fan deposits. Due to proximity to the Colorado River, we believe the debris flow fan deposits are relatively thin and have likely been re-worked and flooded many times in the thousands of years prior to development of Glenwood Springs, thus the hazard from hydrocompactive soils is deemed low. The hazard from future debris flow is considered nil due to urban development that has changed natural drainage patterns. The site contains soils that may be considered highly corrosive to steel and concrete. Special precautions to prevent



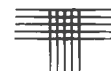
premature degradation of structures composed of these materials will have to be taken. The slopes along the river are moderately steep and could be subject to some erosion during high flows. Development is planned above 100-year flood boundaries, which we believe mitigates the hazard from flooding and erosion.

The soil and bedrock units are not expected to respond unusually to seismic activity. Liquifaction potential is considered nil. We believe the parcel can be considered to be Site Class C. Only minor damage to relatively new, properly designed and built structures would be expected.

In summary, we find no geologic hazards that preclude development of this parcel for the planned uses.

Geology Section References

1. "Geologic Map of the Glenwood Springs Quadrangle, Garfield County, Colorado" compiled by Kirkham, et al, Colorado Geological Survey Map Series 38, scale 1:24,000, 2008
2. "Guidelines and Criteria for Identification and Land-Use Controls of Geologic Hazard and Mineral Resource Areas" by W.P. Rogers, et. al, Special Publication 6, Colorado Geologic Survey, 1974
3. Aerial Photography by Google Earth dated September 22, 2011
4. Geologic Hazards of the Glenwood Springs Metropolitan Area, Garfield County Colorado, Colorado Geological Survey Open File Report 78-10, Plate 10, 1978



SUBSURFACE CONDITIONS

We performed field and laboratory investigations at the site in 1995. Sub-surface conditions were investigated by excavating twelve (12) test pits. We have included results of our previous field and laboratory investigations in Appendix A. Our representative was on site during excavating operations to log the soils found in our test pits and obtain samples. Test pits were excavated with a trackhoe. Test pit locations, graphic logs of the soils found in our test pits and laboratory test data are presented and summarized on the Table in Appendix A.

Our test pits penetrated up to 2 feet of organic clays underlain by medium dense to very dense, clayey to silty gravels with cobbles. A hard travertine cemented layer was found below the gravels at 3 to 11 feet deep in our test pits TP-1 through TP-4 and TP-7 through TP-12. Very soft to medium dense, sandy to gravely clays and/or man-made fill was above the gravels in our test pits TP-5 and TP-7 through TP-9. Artesian groundwater was encountered below and percolating up through the travertine cemented layer in our test pits TP-1 through TP-4, TP-8 and TP-12. Non artesian groundwater was found in our test pits TP-5 and TP-6.

OVERLOT GRADING

The site will be overlot graded to achieve rough grades for buildings, water features and landscaping. It appears grading on the south half of the site will consist of shallow cuts and thin fills of 5 feet or less. The thickest fills and deepest cuts appear to be required for the slide construction from the bridge to the lower pond. Removal of the overburden soils above the travertine cemented layer could result in heaving and cracking of the layer due to removal of surcharge pressure above and hydrostatic pressure below. The travertine cemented layer must not be disturbed or loss of control of the geothermal resource may result. We recom-



mend limiting excavations at the site to provide a minimum 2 feet deep cover above the travertine cemented layer.

We believe that the clayey to silty gravels are Type C and the clays are Type B as described in the October, 1989 Occupation Safety and Health Administration (OSHA) Standards published by the Department of Labor governing excavations. The publication indicates a maximum temporary slope of 1.5 to 1 horizontal to vertical) for the Type C soils and 1 to 1 for Type B soils above the water table. Excavations will tend to collapse to 4 to 1 or flatter below the water table.

Artesian groundwater was found in our test pits as shallow as 3 feet deep. We are concerned that groundwater may enter excavations on this site during construction. As a precaution, excavation floors should be sloped to direct groundwater to a positive gravity outfall or to sumps where water can be removed by pumping, if needed.

We recommend that foundations bear on the natural gravels or densely compacted granular structural fill (see "Foundation" section). Foundation recommendations will need to be considered during overlot grading. Areas to receive fill should be grubbed and stripped of all man-made fill, organic soils or other deleterious materials. Existing construction and utilities should be completely removed from below and within 5 feet of areas to be built on. The resulting stripped surface should be moisture treated and proof rolled with a heavy pneumatic tired vehicle prior to fill placement. Soils that deflect significantly should be stabilized or removed.

Fill can consist of the on-site clayey to silty gravels or similar free of debris, organics, rock larger than 4 inches in diameter or other deleterious materials. The on-site clays and existing fill, free of organics, vegetation, debris or other deleterious materials can be mixed with the on-site gravels or import materials to achieve a fill material with 100 percent finer than 4 inches and a minimum 5 percent silt



and clay size particles (passing the No. 200 sieve). Specifications should prohibit the use of expansive clay soils below buildings. If import soils are required for fill material, a sample should be evaluated by our office prior to importing to the site. Fill should be moisture conditioned to within 2 percent of optimum moisture content and placed in 10 inch maximum loose lifts and compacted. Structural fill placed below building footprints and adjacent flatwork should be compacted to at least 98 percent of the maximum dry density determined by the standard Proctor density test (ASTM D 698). Fill placed in areas not to be built on can be placed as common fill and be compacted to at least 95 percent of ASTM D 698. The performance of the fill is critical to structure performance. A representative of CTL/Thompson, Inc. should observe fill placement technique and check fill density and moisture during placement.

FOUNDATION

Structures can be founded with footings bearing on the natural gravels or densely compacted, granular structural fill. A positive alternative to footings would be a mat foundation. A mat foundation would act as the building foundation and floor. Mat foundations result in less differential settlement than footings and slabs-on-grade. Resistance to uplift can be achieved with a mass of concrete in the foundation. Installation of micropiles to resist uplift is not recommended because of potential adverse affects of penetrating the travertine cap and subsequent release of groundwater under pressure. The maximum bearing pressure recommended below should result in total settlements on the order of 1 inch. Differential settlement of about one-half of the actual total settlement is anticipated along 10 feet of continuous footing and between footing pads. Future settlement of a mat foundation would be about one-half that anticipated for footings. Because of the granular nature of the soils, settlement due to dead loads will occur during construction and settlements due to live loads upon first application of the live



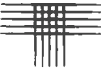
load. Footings or mat foundations can be designed and constructed using the following criteria:

Footings

1. Footings should bear on the undisturbed natural gravels, granular structural fill placed on the natural gravels or on an at least 3.0 feet thick pad of densely compacted granular structural fill where fill is placed on the natural clays. Soils loosened during the excavation or forming process should be compacted. Footings bearing on the natural gravels can be designed for a maximum soil bearing pressure of 5,000 psf. Footings bearing on structural fill can be designed for a maximum bearing pressure of 4,000 psf on the structural fill and not to exceed 1,000 psf on the natural clays (see Figure 8, Appendix A);
2. Foundation walls for continuous footings should be reinforced top and bottom to span undisclosed loose soils pockets. Reinforcement should be designed by a qualified structural engineer;
3. Minimum footing sizes are desirable. We suggest a minimum width of 16 inches for continuous footings and at least 2 feet by 2 feet for isolated column pads. Larger sizes will be required based on the structural loads;
4. The soils below exterior footings should be protected from freezing. We suggest a frost protection depth of 36 inches. The owner should verify the frost depth requirement with the local area building department;
5. Backfill along foundation walls and in utility trenches should be compacted to between 90 and 95 percent of ASTM D 698.

Mat Foundations

Reinforced concrete mat foundations can be constructed on the undisturbed, natural gravel soils. Mats should be designed for a maximum allowable soil pressure of 2,000 psf if constructed on the undisturbed, natural sand and gravel soil.



Modulus of subgrade reaction (K_s) is normally used for mat foundation design. The modulus of subgrade reaction is dependent upon the compressibility of the foundation soils and the size (or effective loaded area) of the foundation. If the entire mat foundation is uniformly loaded, then a K_s value of 325 pci should be used for the gravel soil.

To resist lateral loads, a coefficient of friction of 0.45 can be used for concrete in contact with the gravel or structural fill. Lateral loads can be resolved by evaluating passive resistance using an equivalent fluid density of 375 pcf for the sand and gravel, provided the backfill is compacted and is not removed. A moist unit weight of 130 pcf can be assumed for backfill soils. These values have not been factored; appropriate factors of safety should be applied in design. The soils beneath the mat should be protected from freezing. We recommend a minimum 4 inch thick layer of clean gravel below the mat to break capillary rise.

FLOOR SLABS

Floors will bear on natural gravels and/or densely compacted fill built with similar granular soils (see OVERLOT GRADING). Fill required below floors should be gravels with a maximum particle diameter of 4 inches and a minimum 5 percent silt and clay size particles (passing the No. 200 sieve). A Colorado Department of Transportation Class 5 or Class 6 aggregate base course would also be suitable fill. Structural fill should be moisture conditioned to within 2 percent of optimum moisture content, placed in 10 inch maximum loose lifts and compacted to at least 98 percent of the maximum dry density determined by a standard Proctor density test (ASTM D 698). Slab-on-grade floors can be designed and constructed using the following criteria:

1. Floor slabs should bear on a minimum 4-inch thick layer of clean gravel (washed 3/4-inch rock) to break capillary rise of water placed atop the natural, silty gravels or fill placed as discussed above;



2. Slabs-on-grade should be separated from exterior walls and interior bearing members. Vertical movement of the slabs should not be restricted;
3. Plumbing and utilities which pass through the slabs should be isolated from the slabs;
4. Exterior flatwork and sidewalks should be separated from the structure. These slabs should be reinforced to function as independent units;
5. Frequent control joints or reinforcement should be provided in all slabs to reduce problems associated with shrinkage. The American Concrete Institute (ACI) recommends joint spacing of 15 feet in both directions.

POOLS

A main pool, hot tubs and water slide with lower pond will be built. It appears the main pool excavation will be in natural gravels and the groundwater will be between 5 to 10 feet below the existing ground surface depending on time of year and runoff. We anticipate that the slide will be approximately 4 to 6 feet deep. We anticipate that the pools and slide will be concrete lined. We believe pool excavations will be made and the excavations will then be coated with several layers of gunite to produce the slide and pools liners. Decking adjacent to the pools and slide will be slabs-on-grade. We recommend a 12 inch thick layer of densely compacted, granular structural fill be placed below the pool deck. Fill should be placed as recommend in the OVERLOT GRADING section.

If a near surface water table develops, buoyancy forces will heave and crack the pool and stream liners. To reduce the potential for development of buoyancy forces, we recommend provision of a gravel sub-drain beneath the pool and slide complex. A detail for construction of the drainage layer is shown on Figure 5.



BELOW GRADE CONSTRUCTION

Building foundation walls which extend below-grade should be designed for lateral earth pressures where backfill is not present to about the same extent on both sides of the wall. Many factors affect the values of the design lateral earth pressure. These factors include, but are not limited to, the type, compaction, slope and drainage of the backfill, and the rigidity of the wall against rotation and deflection. For a very rigid foundation wall where negligible or very little deflection will occur, an "at-rest" lateral earth pressure should be used in design. For walls which can deflect or rotate 0.5 to 1 percent of wall height (depending upon the backfill types), lower "active" lateral earth pressures are appropriate. Our experience indicates that building foundation walls deflect or rotate slightly under normal design loads, and that this deflection results in satisfactory wall performance. Thus, the earth pressures on the walls will likely be between the "active" and "at-rest" conditions.

If the on-site soils, with maximum particle size of 4 inches and free of deleterious material, are used as backfill, we recommend design of below-grade walls using an equivalent fluid density of at least 50 pcf for this site. This equivalent density does not include allowances for sloping backfill, surcharges or hydrostatic pressures. Backfill placed adjacent to foundation wall exteriors should be moisture treated and compacted to between 90 and 95 percent of standard Proctor maximum dry density (ASTM D 698).

We understand that below grade building construction will be limited to a partial basement below the main building. We have recommended a layer of washed gravel to break the capillary rise of water below the floor slabs. We suggest that the below slab layer of gravel be connected to a drain placed along the perimeter of the building to allow removal of water collected in the washed gravel



layer. The drain should consist of slotted PVC pipe encased in "free draining" gravel and lead to a positive gravity outfall. A typical drain detail is presented on Figure 4.

ACCESS ROAD CONSTRUCTION

Utility Construction

We believe excavations for utility installation in the soils at the site can be performed with conventional heavy-duty or large backhoes. Permanent groundwater levels are controlled by, and directly related to, water levels in the Colorado River. If groundwater is encountered during construction in shallow excavations, dewatering can likely be accomplished by sloping excavations to occasional sumps where water can be removed by pumping. Piping would need to be designed to resist buoyancy force.

Utility trenches should be sloped or shored to meet local, State and federal safety regulations. The clay soil will classify as a Type B soil and the gravels will classify as a Type C soil based on OSHA standards. OSHA recommended temporary excavation slopes are provided in the OVERLOT GRADING section. Excavation slopes specified by OSHA are dependent upon soil types and ground water conditions encountered. Contractors should identify the soils encountered in the excavation and refer to OSHA standards to determine appropriate slopes.

Water and sewer lines are usually constructed beneath paved roads. Compaction of trench backfill can have a significant effect on the life and serviceability of pavements. Trench backfill should be placed in thin, loose lifts, and moisture conditioned to within 2 percent of optimum content. Trench backfill should be compacted to at least 95 percent of maximum dry density (ASTM D 698). The placement and compaction of fill and backfill should be observed and



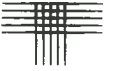
tested by our firm during construction. Backfill soils maximum diameter should be limited to 4 inches to avoid nesting of larger diameter rock in the trench. If imported trench backfill is required, we recommend a CDOT Class 6 aggregate base course.

PAVEMENT CONSTRUCTION

Areas to be paved should be stripped to the natural soils. The subgrade surface below pavements should be moisture treated to within 2 percent of optimum moisture content and compacted to at least 95 percent of the standard Proctor maximum dry density (ASTM D 698) before placement of base course and/or pavement. The entire area should be proof rolled with a heavy pneumatic tire vehicle (i.e. a loaded 10 wheel dump truck) before fill, base course or pavement is placed. Soils which deform excessively should be removed and replaced with structural fill or otherwise stabilized.

For pavement areas for automobile traffic and automobile parking, we suggest 5.0 inches of full depth asphalt or 3.0 inches of asphalt over 6.0 inches of aggregate base course. Areas subject to truck or bus traffic should be paved with 6 inches of portland cement concrete. The performance history of a combination asphalt and aggregate base course section has shown higher maintenance costs and decreased serviceability compared to either full depth asphalt or concrete pavement. The above pavement sections are based on our anticipation of relatively light traffic loads. If requested, we could perform a subgrade investigation and pavement design. We would need anticipated vehicle traffic information for this investigation.

The performance of pavement sections depends on the quality of the materials used. Aggregate base course should conform to the Colorado Department of Transportation specifications for Class 6 aggregate and should have a minimum



R-value of 78. The base course should be moisture conditioned to near optimum moisture content and compacted to at least 95 percent of modified Proctor maximum dry density (ASTM D 1557). Asphalt should have a total resistance (Rt) of at least 94 and should be compacted to at least 95 percent of the maximum laboratory density. Portland cement concrete should contain at least 5.5 sacks of cement per cubic yard, 5 to 7 percent entrained air and have a minimum modulus of Rupture of 650 psi. We recommend observation and testing of material placement by a representative of our firm during construction.

Garbage, drywall, sod and concrete trucks are often times over the legal weight limit. The driving and parking of these types of overweight trucks can cause pavement rutting in areas where the underlying subgrade is wet. We recommend these types of trucks be required to be beneath the legal load limit of 18,000 pounds per axle. Where possible, drivers should be instructed to avoid driving and/or parking near the curb lines. These precautions should reduce the risk of additional pavement damage during construction.

Pavement Materials

Material properties and construction criteria for the pavement alternatives are provided below. These criteria were developed from analysis of the field and laboratory data and our experience. If the materials cannot meet these recommendations, then the pavement design should be reevaluated based upon available materials.

Asphalt Concrete (AC) or Hot Mix Asphalt (HMA)

1. HMA should be composed of a mixture of aggregate, filler, hydrated lime and asphalt cement. Some mixes may require polymer modified asphalt cement, or make use of up to 20 percent reclaimed asphalt pavement (RAP). A job mix design is recommended and periodic checks on the job site should be made to verify compliance with specifications.



2. HMA should be relatively impermeable to moisture and should be designed with crushed aggregates that have a minimum of 80 percent of the aggregate retained on the No. 4 sieve with two mechanically fractured faces.
3. Gradations that approach the maximum density line (within 5 percent between the No. 4 and 50 sieve) should be avoided. A gradation with a nominal maximum size of 1 or 2 inches developed on the fine side of the maximum density line should be used.
4. Total void content, voids in the mineral aggregate (VMA) and voids filled should be considered in the selection of the optimum asphalt cement content. The optimum asphalt content should be selected at a total air void content of approximately 4 percent. The mixture should have a minimum VMA of 14 percent and between 65 percent and 80 percent of voids filled.
5. Asphalt cement should meet the requirements of the Superpave Performance Graded (PG) Binders. We recommend using PG 58-28 or PG 58-22 asphalt cement for this project.
6. Hydrated lime should be added at the rate of 1 percent by dry weight of the aggregate and should be included in the amount passing the No. 200 sieve. Hydrated lime for aggregate pretreatment should conform to the requirements of ASTM C 207, Type N.
7. Paving should only be performed when subgrade temperatures are above 40°F and air temperature is at least 40°F and rising.
8. HMA should not be placed at a temperature lower than 245°F for mixes containing PG 58-28 or PG 58-22 asphalt, and 290°F for mixes containing polymer modified asphalt. The breakdown compaction should be completed before the mixture temperature drops 20°F.
9. The maximum compacted lift should be 3.0 inches and joints should be staggered. No joints should be placed within wheel paths.
10. HMA should be compacted to between 92 and 96 percent of Maximum Theoretical Density. The surface shall be sealed with a finish roller prior to the mix cooling to 185°F.
11. Placement and compaction of HMA should be observed and tested by a representative of our firm. Placement should not commence until the subgrade is properly prepared (or stabilized), tested and proof-rolled. Proof rolling should be performed with the heaviest machine



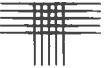
available at the time. The proof roller should be selected from machines providing both mass and high contact pressure.

Aggregate Base Course (ABC)

1. A Class 5 or 6 Colorado Department of Transportation (CDOT) specified aggregate base course should be used. A recycled concrete alternative which meets the Class 5 or 6 designation is also acceptable.
2. Aggregate base course should have a minimum Hveem stabilometer value of 77. Aggregate base course or recycled concrete material must be moisture stable. The change in R-value from 300 psi to 100 psi exudation pressure should be 12 points or less.
3. Aggregate base course or recycled concrete should be laid in thin lifts not to exceed 8 inches; moisture treated to within 2 percent of optimum moisture content and compacted to at least 95 percent of maximum modified Proctor dry density (ASTM D 1557, AASHTO T 180).
4. Placement and compaction of aggregate base course or recycled concrete should be observed and tested by a representative of our firm. Placement should not commence until the underlying subgrade is properly prepared and tested.

Moisture Treated Subgrade

1. Subgrade should be stripped of organic matter, scarified, moisture treated and compacted.
2. Subgrade soils should be moisture conditioned to between 2 percent below and 2 percent above optimum moisture content and compacted to at least 95 percent of standard Proctor maximum dry density (ASTM D 698, AASHTO T 99).
3. Final grading of the subgrade should be carefully controlled so the design cross-slope is maintained and low spots in the subgrade that could trap water are eliminated.
4. Once final subgrade elevation has been reached and the subgrade compacted and tested, the area should be proof-rolled with the heaviest machine available at the time. The proof roller should be



selected from machines providing both mass and high contact pressure.

5. The proof-roll should be performed while moisture contents of the subgrade are still within the recommended limits. Drying of the subgrade prior to proof-roll or paving should be avoided. Areas of soft or wet subgrade should be remedied.

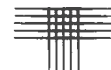
CONSTRUCTION DETAILS

The design of a pavement system is as much a function of the quality of the paving materials and construction as the support characteristics of the subgrade. The construction materials are assumed to possess sufficient quality as reflected by the strength coefficients used in the flexible pavement design calculations. These strength coefficients were developed through research and experience to simulate expected material of good quality, as explained herein. During construction careful attention should be paid to the following details:

- Placement and compaction of trench backfill.
- Compaction at curb lines and around manholes and water valves.
- Excavation of completed pavements for utility construction and repair.
- Design slopes of the adjacent ground and pavement to rapidly remove water from the pavement surface.

MAINTENANCE

Routine maintenance, such as sealing and repair of cracks, is necessary to achieve the long-term life of a pavement system. We recommend a preventive maintenance program be developed and followed for all pavement systems to assure the design life can be realized. Choosing to defer maintenance usually re-



sults in accelerated deterioration leading to higher future maintenance costs, and/or repair. A recommended maintenance program is outlined in Appendix B.

CONCRETE

Soils in the vicinity of mineral springs in the area are known to have high concentrations of water soluble sulfates. We believe that concrete exposed to the natural soils or groundwater will be exposed to severe sulfate exposure. We recommend Type V or equivalent sulfate resistant cement to be used in all concrete exposed to the natural soils or groundwater at this site. We recommend the use of de-icing salts on concrete pavement or exterior flatwork be minimized during the first year after construction.

SURFACE DRAINAGE

Performance of concrete flatwork and foundations is influenced by the moisture conditions in the subsoils. Risk of wetting of the subsoils can be reduced by carefully planned and maintained surface drainage. We recommend the following precautions be observed during construction and maintained at all times after construction is completed:

1. Wetting or drying of open excavations should be avoided;
2. Surface grades should provide for the rapid runoff of water off of pavements and away from buildings. Ponding of water adjacent to buildings or on pavements should be eliminated;
3. The ground surface surrounding the exterior of buildings should be sloped to drain away from the buildings in all directions. We recommend a minimum slope of at least 6 inches in the first 10 feet for landscaped areas. Exterior slabs adjacent to buildings should also slope as much as allowable away from the buildings;



4. Backfill of foundation walls should be moistened and compacted as outlined under "Overlot Grading". Deep backfill zones will settle over time, even with proper compaction. Settlement of 0.5 to 2 percent of the fill height can occur. Support of flatwork, entrances or other structures in backfill zones should consider the potential settlement. The use of structural sections supported on foundation walls should be considered in sensitive areas;
5. Roof downspouts and drains should discharge well beyond the limits of all backfill. Splash blocks and downspout extensions should be provided at all drainage locations;
6. Plastic membranes should not be used to cover the ground surface immediately surrounding the building. These membranes tend to trap moisture and prevent normal evaporation from occurring. Geotextile fabrics can be used to limit weed growth and allow for evaporation.

ADDITIONAL CONSULTATION, CONSTRUCTION OBSERVATIONS AND MONITORING

This report has been prepared for the exclusive use of Iron Mountain Hot Springs, LLC and the design team for the purpose of providing geotechnical design and construction criteria for the proposed project. The information, conclusions, and recommendations presented herein are based upon consideration of many factors including, but not limited to, the type of structures proposed, the geologic setting, and the subsurface conditions encountered. The conclusions and recommendations contained in the report are not valid for use by others. Standards of practice evolve in geotechnical engineering. The recommendations provided are appropriate for about three years. If the proposed project is not constructed within about three years, we should be contacted to determine if we should update this report.

CTL | Thompson, Inc. should provide construction observation services to allow us the opportunity to verify whether soil conditions are consistent with those found during this investigation. If others perform these observations, they must



accept responsibility to judge whether the recommendations in this report remain appropriate.

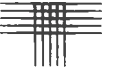
GEOTECHNICAL RISK

The concept of risk is an important aspect with any geotechnical evaluation, primarily because the methods used to develop geotechnical recommendations do not comprise an exact science. We never have complete knowledge of subsurface conditions. Our analysis must be tempered with engineering judgment and experience. Therefore, the recommendations presented in any geotechnical evaluation should not be considered risk-free. We cannot provide a guarantee that the interaction between the soils and a proposed structure will be as desired or intended. Our recommendations represent our judgment of those measures that are necessary to increase the chances that the structures will perform satisfactorily. It is critical that all recommendations in this report are followed during construction. Home owners must assume responsibility for maintaining the structure and use appropriate practices regarding drainage and landscaping.

LIMITATIONS

Our test pits were located to provide a reasonably accurate picture of subsurface conditions. Variations in the subsurface conditions not indicated by the pits will occur. A representative of our firm should be called to test structural fill placement and to observe the completed foundation excavation to confirm that the exposed soils are suitable for support of the footings as designed.

This investigation was conducted in a manner consistent with that level of care and skill ordinarily exercised by geotechnical engineers currently practicing under similar conditions in the locality of this project. No warranty, express or implied, is



made. If we can be of further service in discussing the contents of this report, please call.

CTL | THOMPSON, INC.

John Mechling, P.E.
Branch Manager

JM:cd

cc: Via email mogli@planbrealestate.com



SCALE: 1" = 2,000'



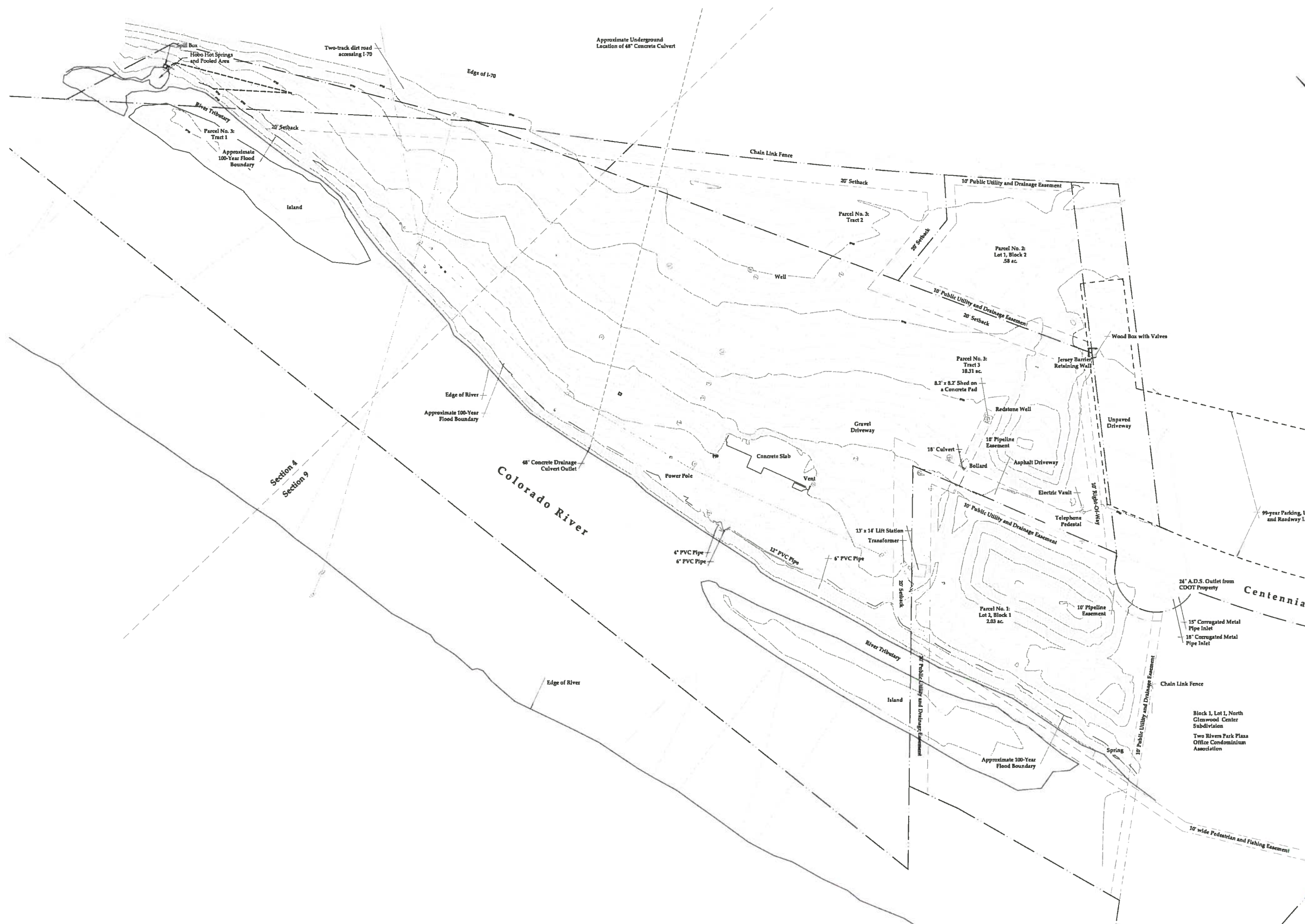
Vicinity Map

IRON MOUNTAIN HOT SPRINGS, LLC
IRON MOUNTAIN HOT SPRINGS
Project No. GS05875-145

Fig. 1



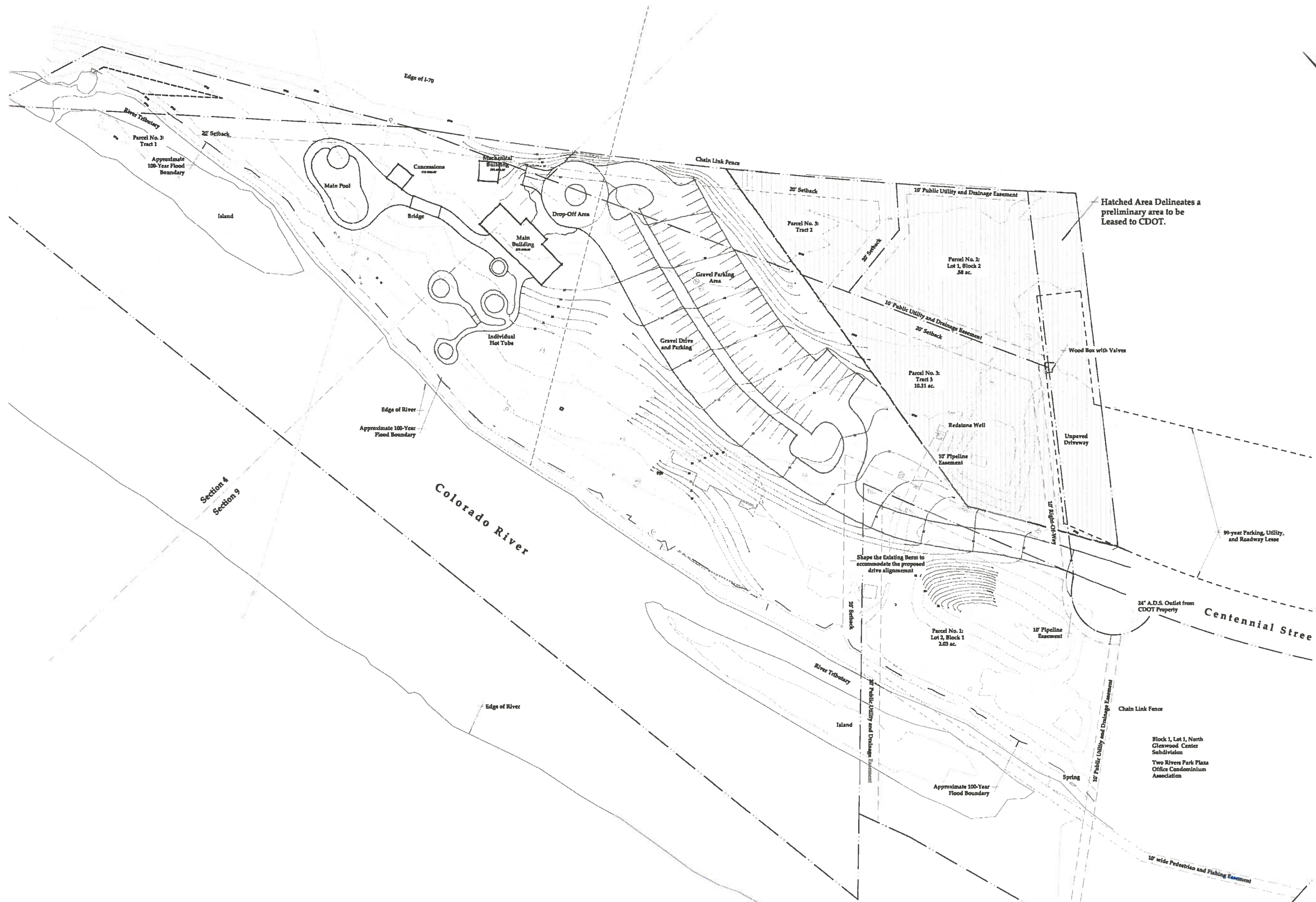
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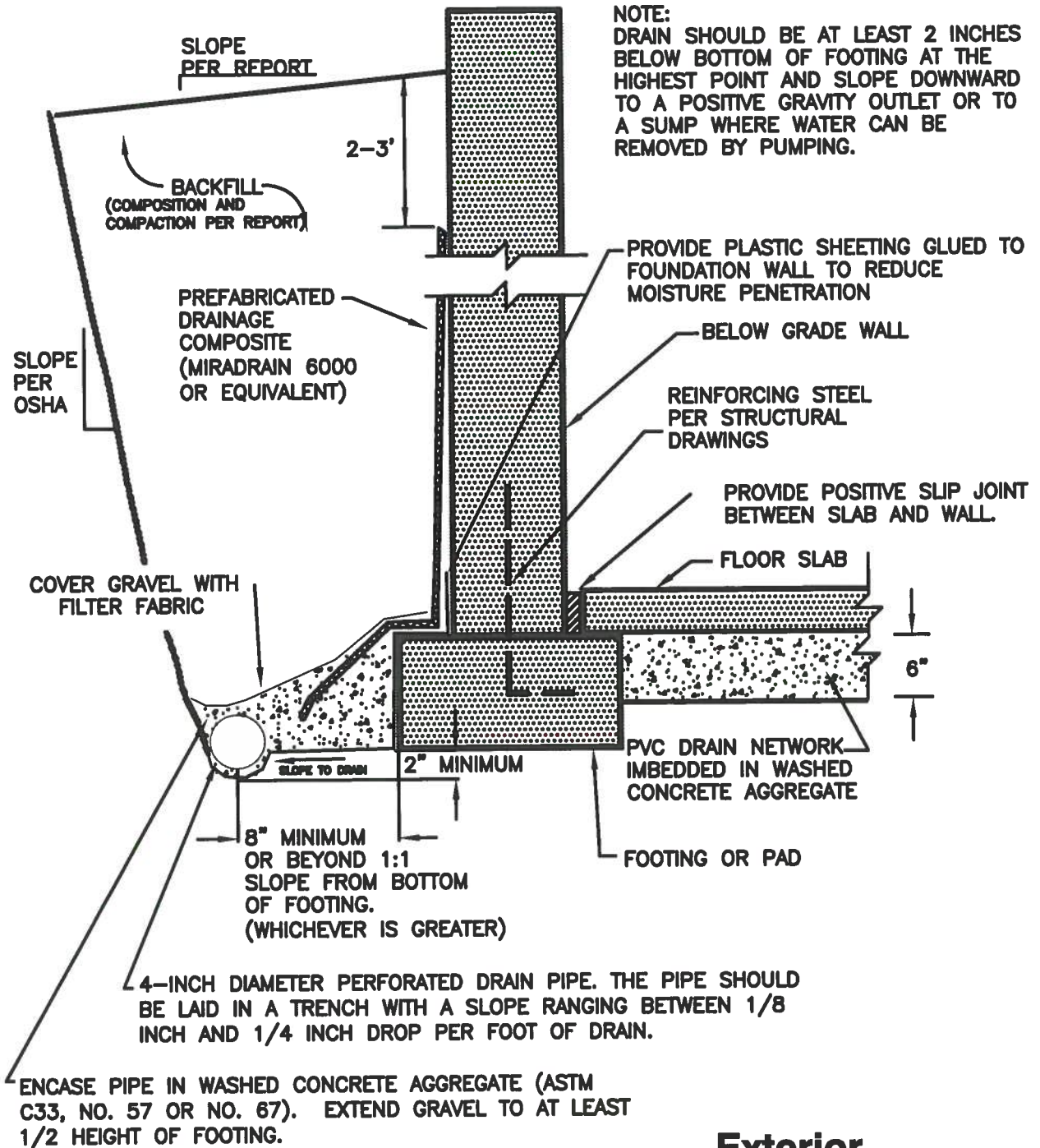
**Existing
Site
Conditions**



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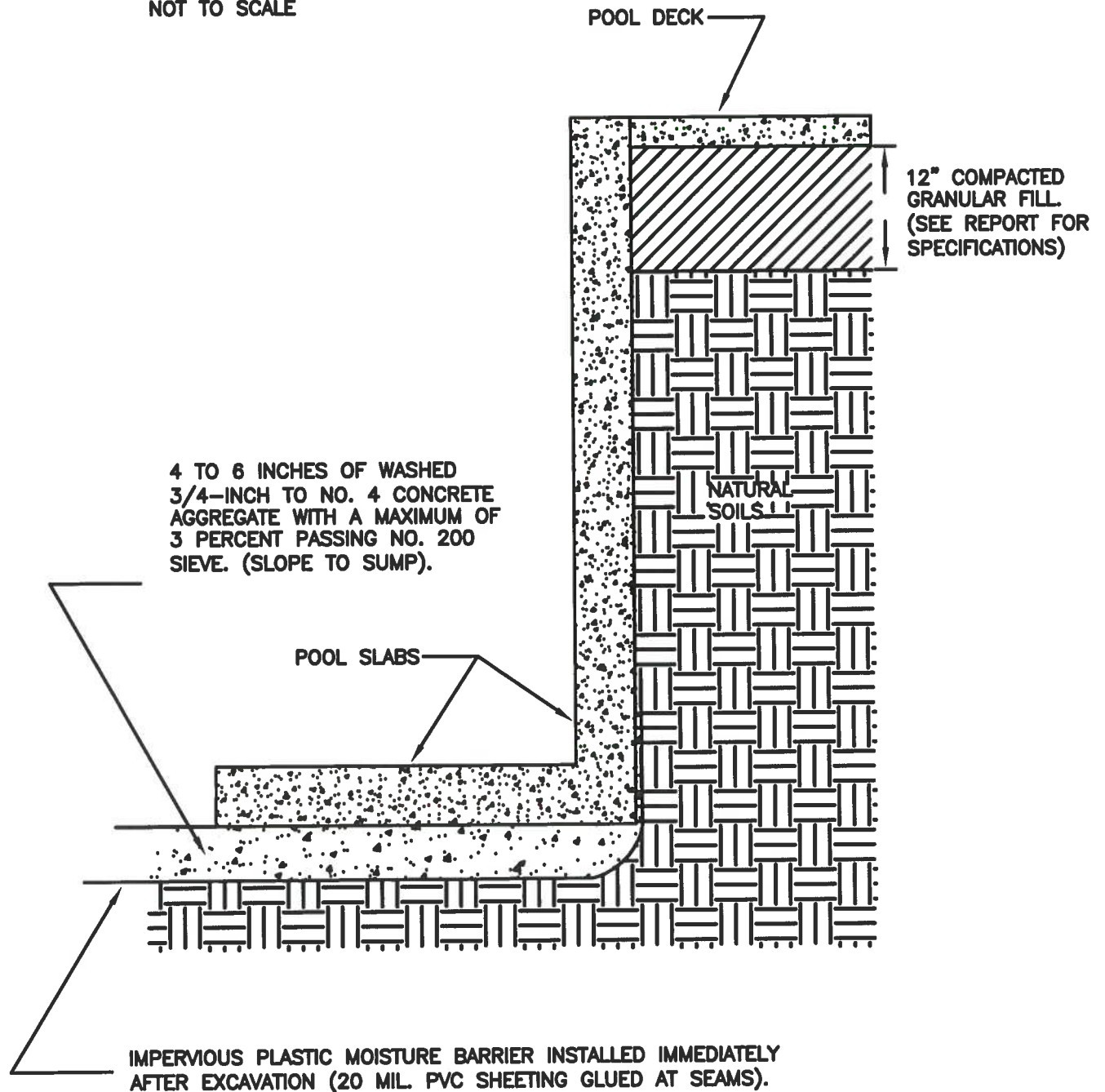
Proposed Site Plan



Exterior Foundation Wall Drain



NOT TO SCALE



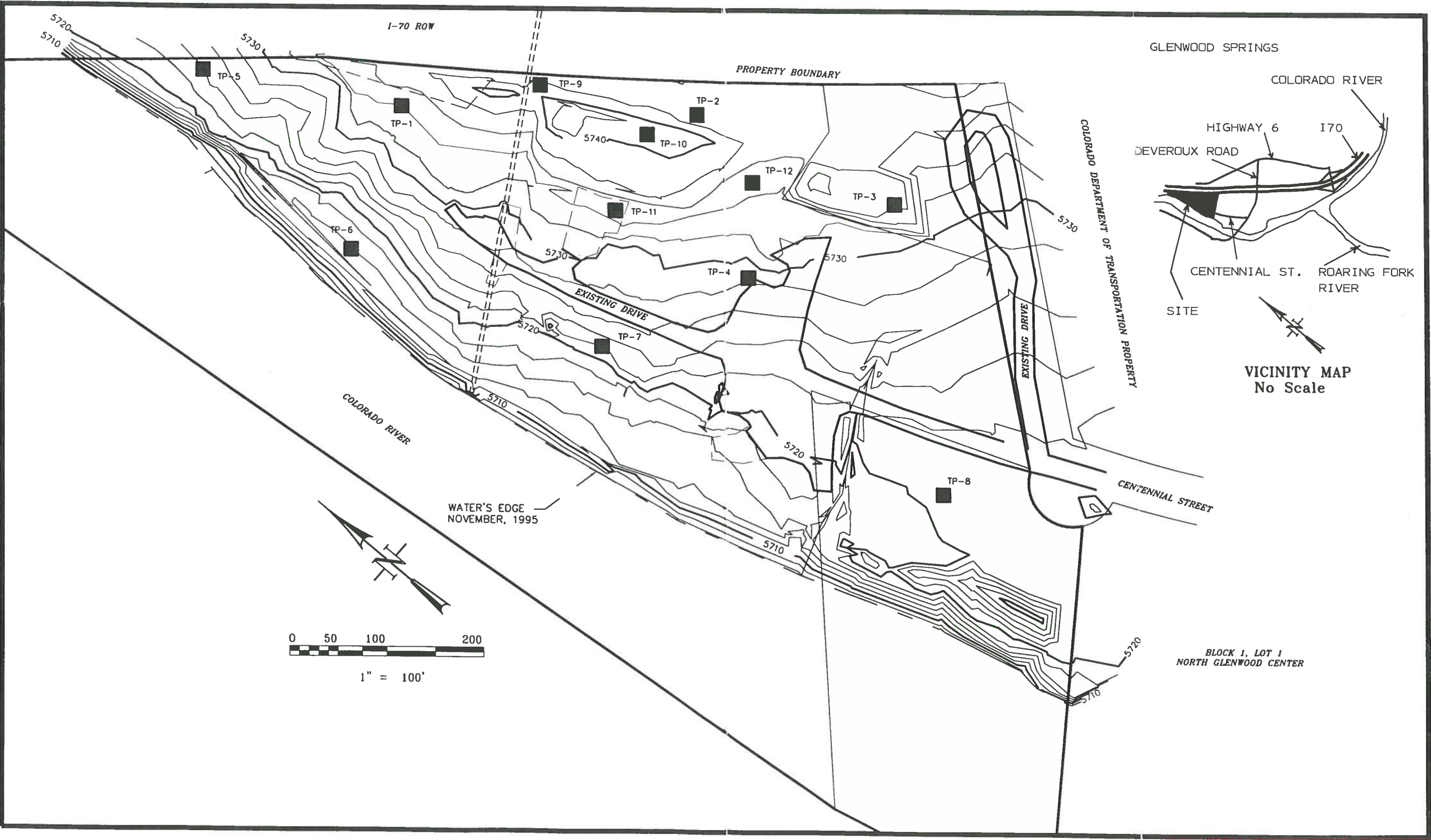
Recommended Pool Drain Detail

Fig. 5



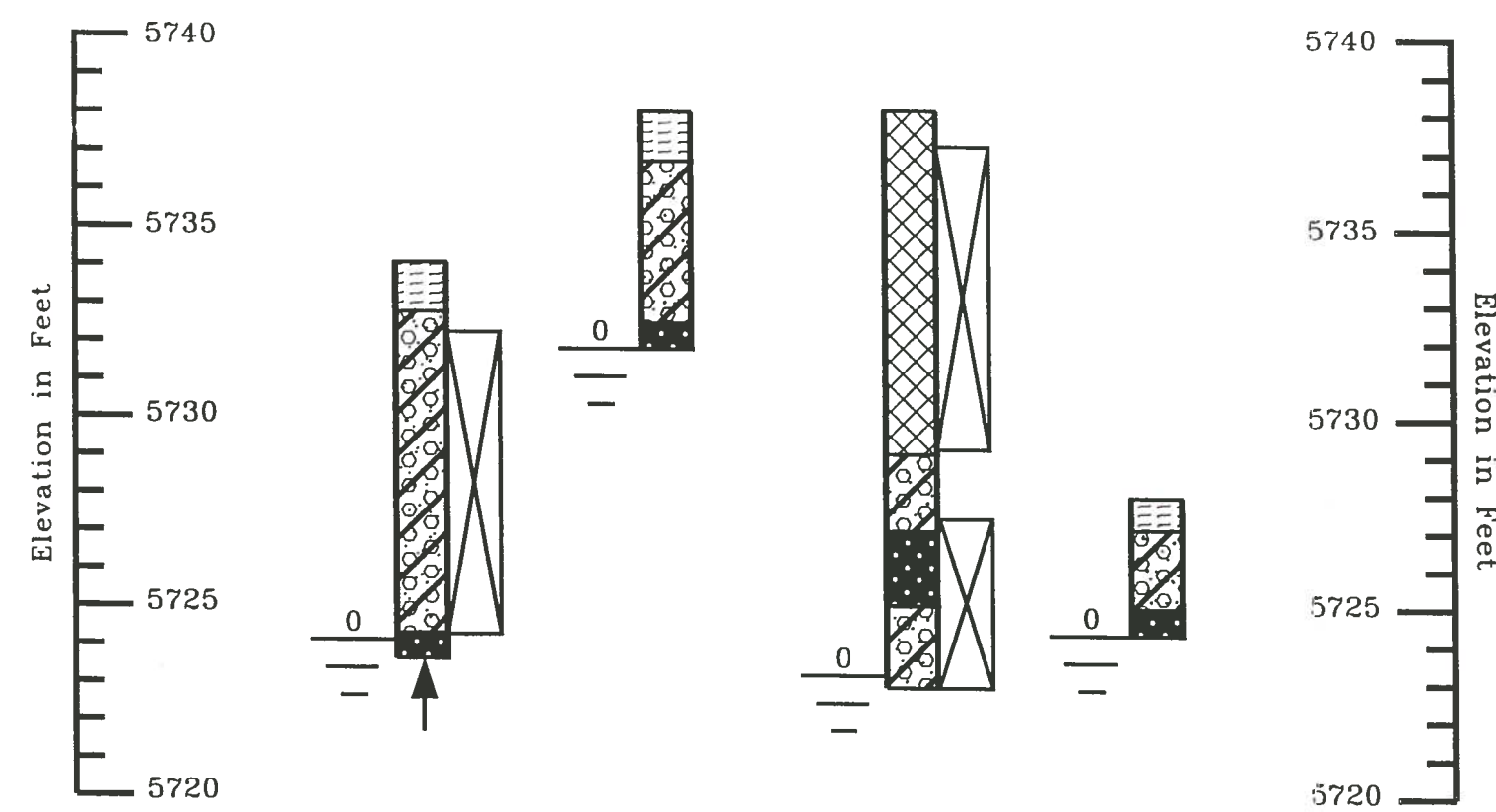
APPENDIX A
FIGURES FROM GS-1727

PROPOSED NORTH GLENWOOD WATER PARK
GLENWOOD SPRINGS, COLORADO



LOCATIONS OF TEST PITS

TP-1	TP-2	TP-3	TP-4
EL=5734	EL=5738	EL=5738	EL=5728



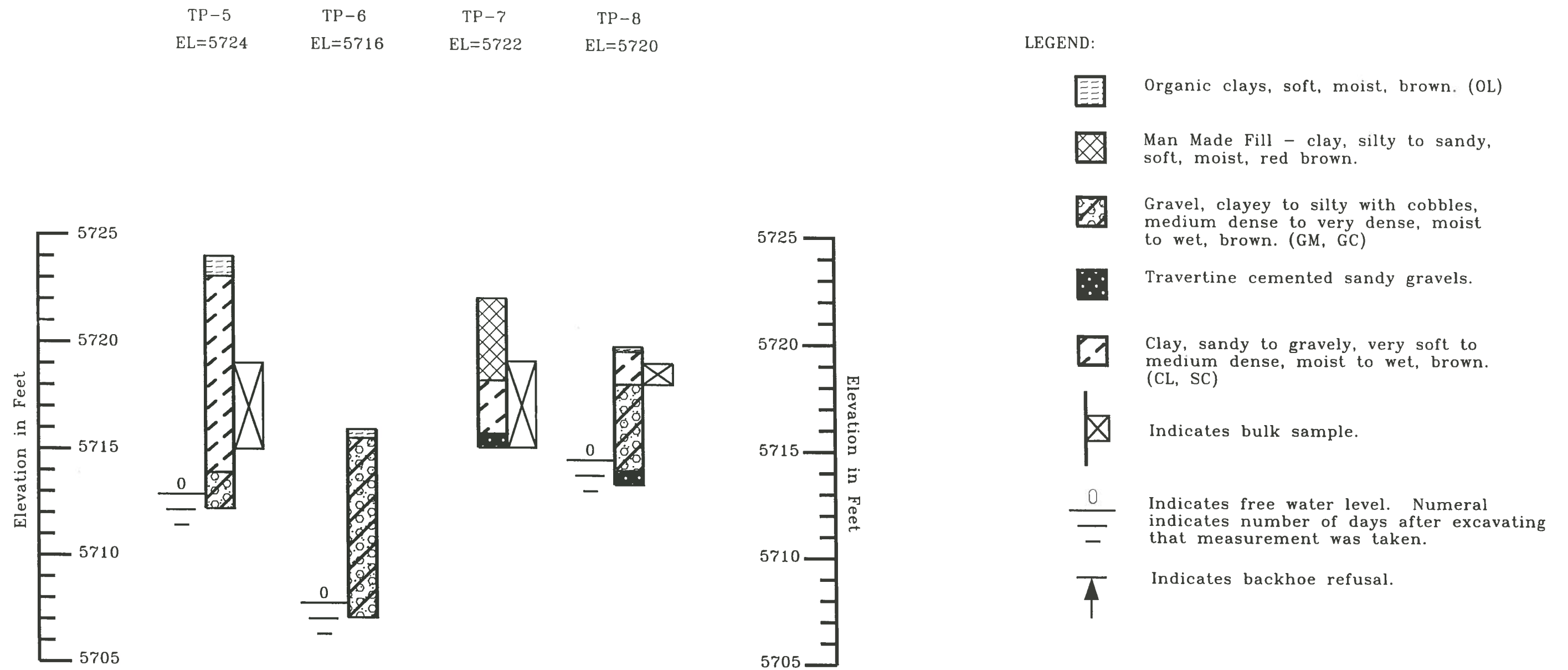
LEGEND:

- Organic clays, soft, moist, brown. (OL)
- Man Made Fill - clay, silty to sandy, soft, moist, red brown.
- Gravel, clayey to silty with cobbles, medium dense to very dense, moist to wet, brown. (GM, GC)
- Travertine cemented sandy gravels.
- Clay, sandy to gravelly, very soft to medium dense, moist to wet, brown. (CL, SC)
- Indicates bulk sample.
- Indicates free water level. Numeral indicates number of days after excavating that measurement was taken.
- Indicates backhoe refusal.

NOTES:

1. Test pits were excavated on December 14, 1995 with a trackhoe.
2. Elevations were obtained from topographic mapping provided by Jerome Gamba & Associates and are approximate.
3. These test pits are subject to the explanations, limitations and conclusions as contained in this report.

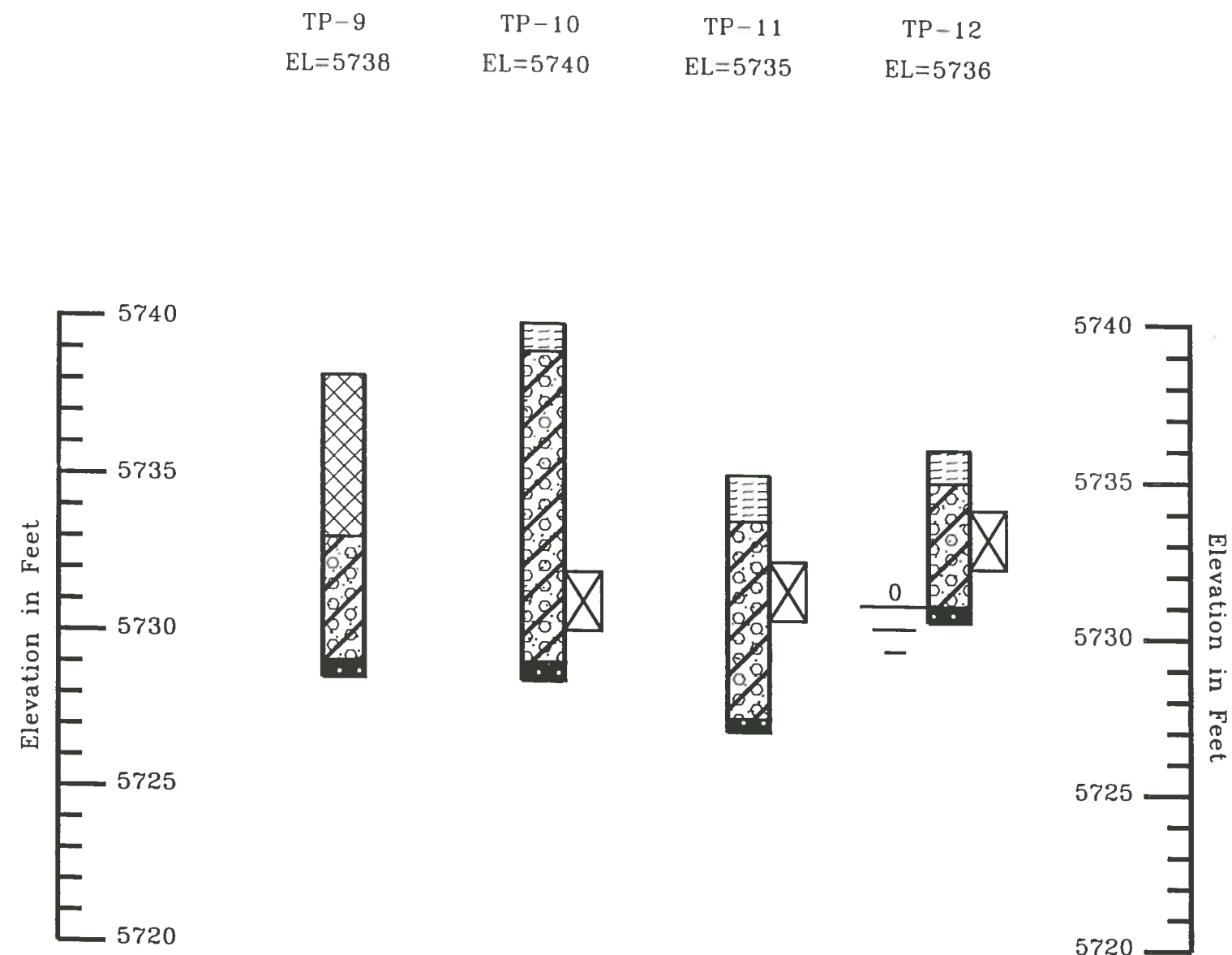
SUMMARY LOGS OF TEST PITS



NOTES:

1. Test pits were excavated on December 14, 1995 with a trackhoe.
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3. These test pits are subject to the explanations, limitations and conclusions as contained in this report.

SUMMARY LOGS OF TEST PITS



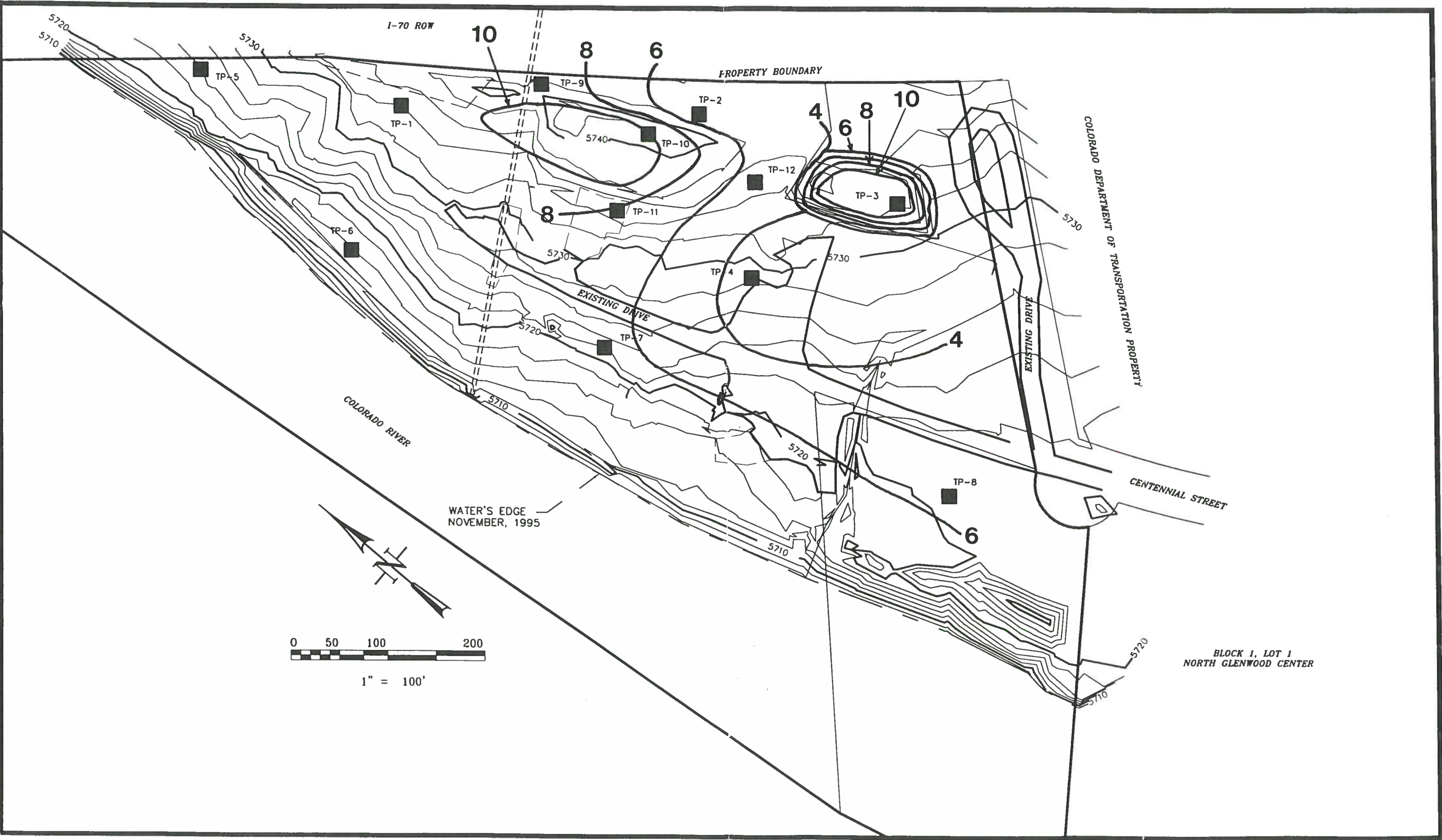
LEGEND:

- Organic clays, soft, moist, brown. (OL)
- Man Made Fill - clay, silty to sandy, soft, moist, red brown.
- Gravel, clayey to silty with cobbles, medium dense to very dense, moist to wet, brown. (GM, GC)
- Travertine cemented sandy gravels.
- Clay, sandy to gravelly, very soft to medium dense, moist to wet, brown. (CL, SC)
- Indicates bulk sample.
- Indicates free water level. Numeral indicates number of days after excavating that measurement was taken.
- Indicates backhoe refusal.

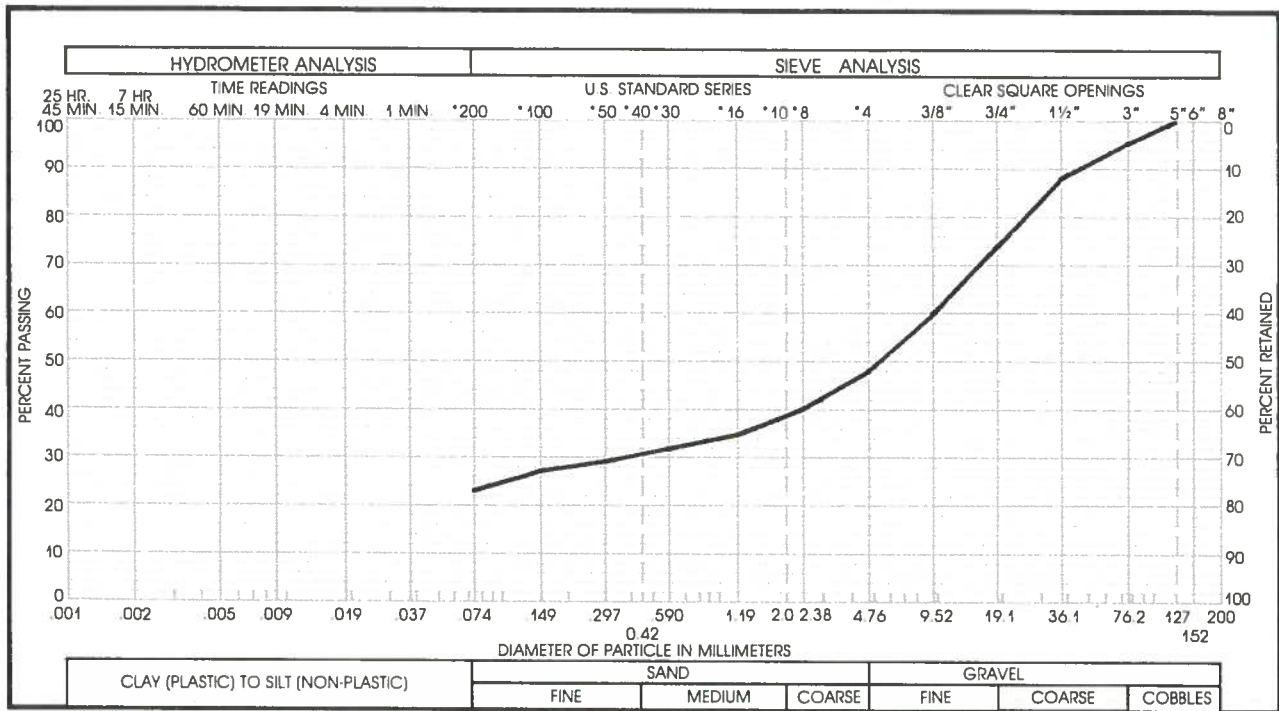
NOTES:

1. Test pits were excavated on January 3, 1996 with a trackhoe.
2. Elevations were obtained from topographic mapping provided by Jerome Gamba & Associates and are approximate.
3. These test pits are subject to the explanations, limitations and conclusions as contained in this report.

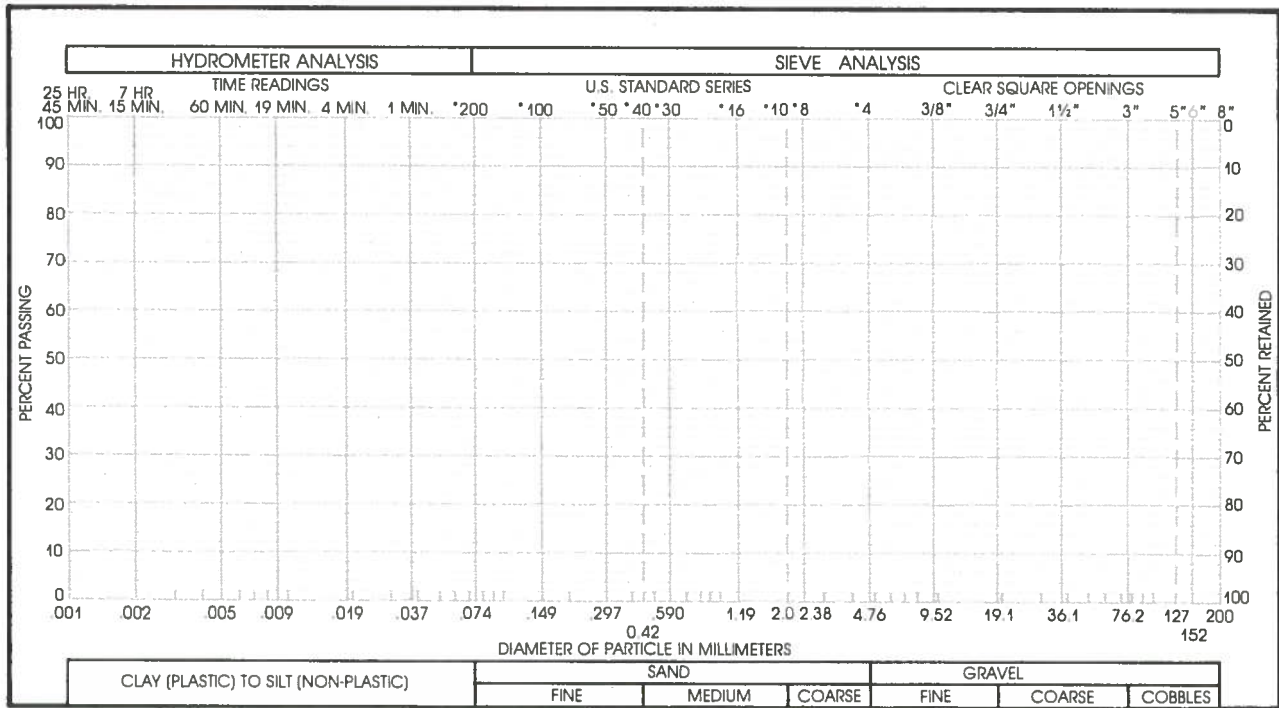
PROPOSED NORTH GLENWOOD WATER PARK
GLENWOOD SPRINGS, COLORADO



APPROXIMATE DEPTH TO TRAVERTINE CEMENTED LAYER CONTOUR MAP

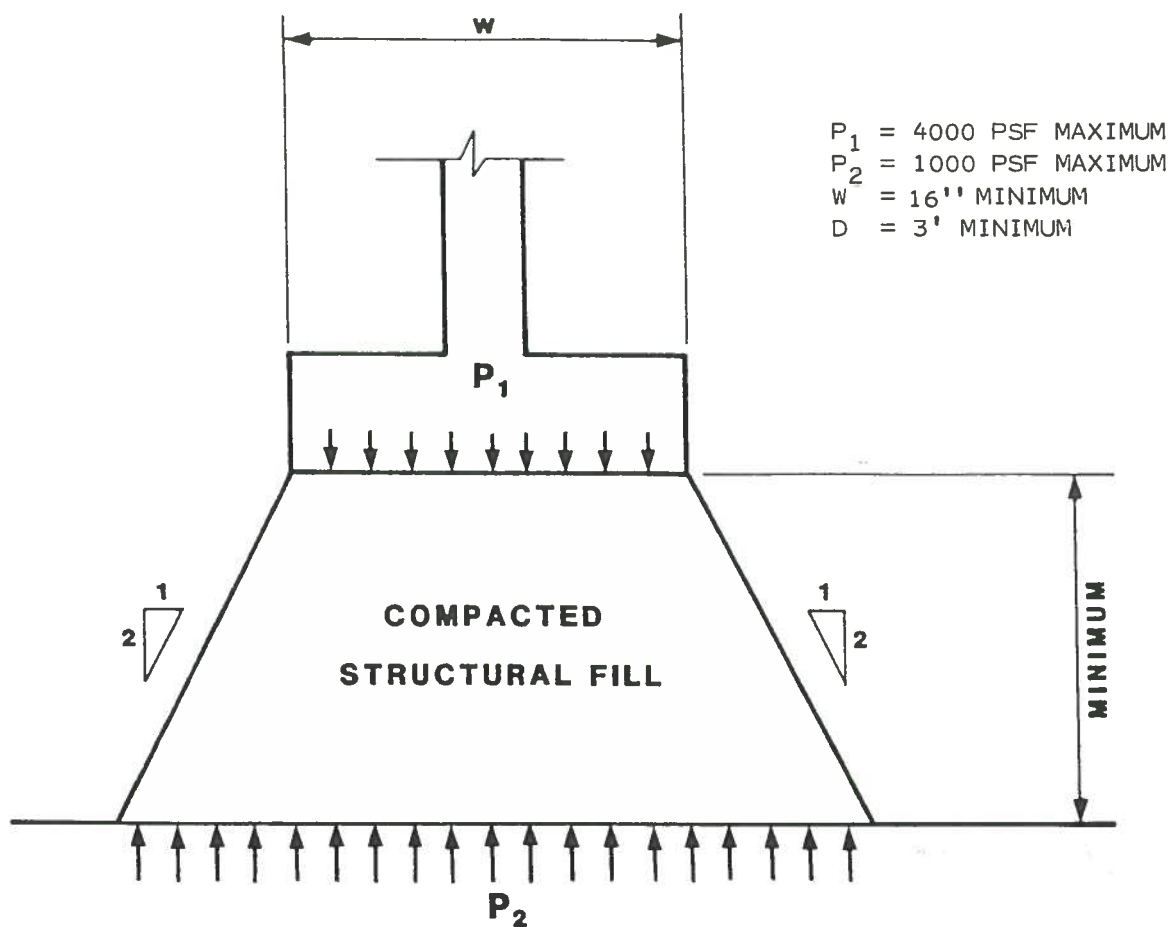


Sample of GRAVEL, SILTY (GM) GRAVEL 52 % SAND 25 %
From TP-1 AT 2 TO 10 FEET SILT & CLAY 23 % LIQUID LIMIT 23 %
PLASTICITY INDEX 8 %



Sample of _____ GRAVEL _____ % SAND _____ %
From _____ SILT & CLAY _____ % LIQUID LIMIT _____ %
PLASTICITY INDEX _____ %

Gradation Test Results



P_1 = ALLOWABLE PRESSURE ON COMPACTED FILL:

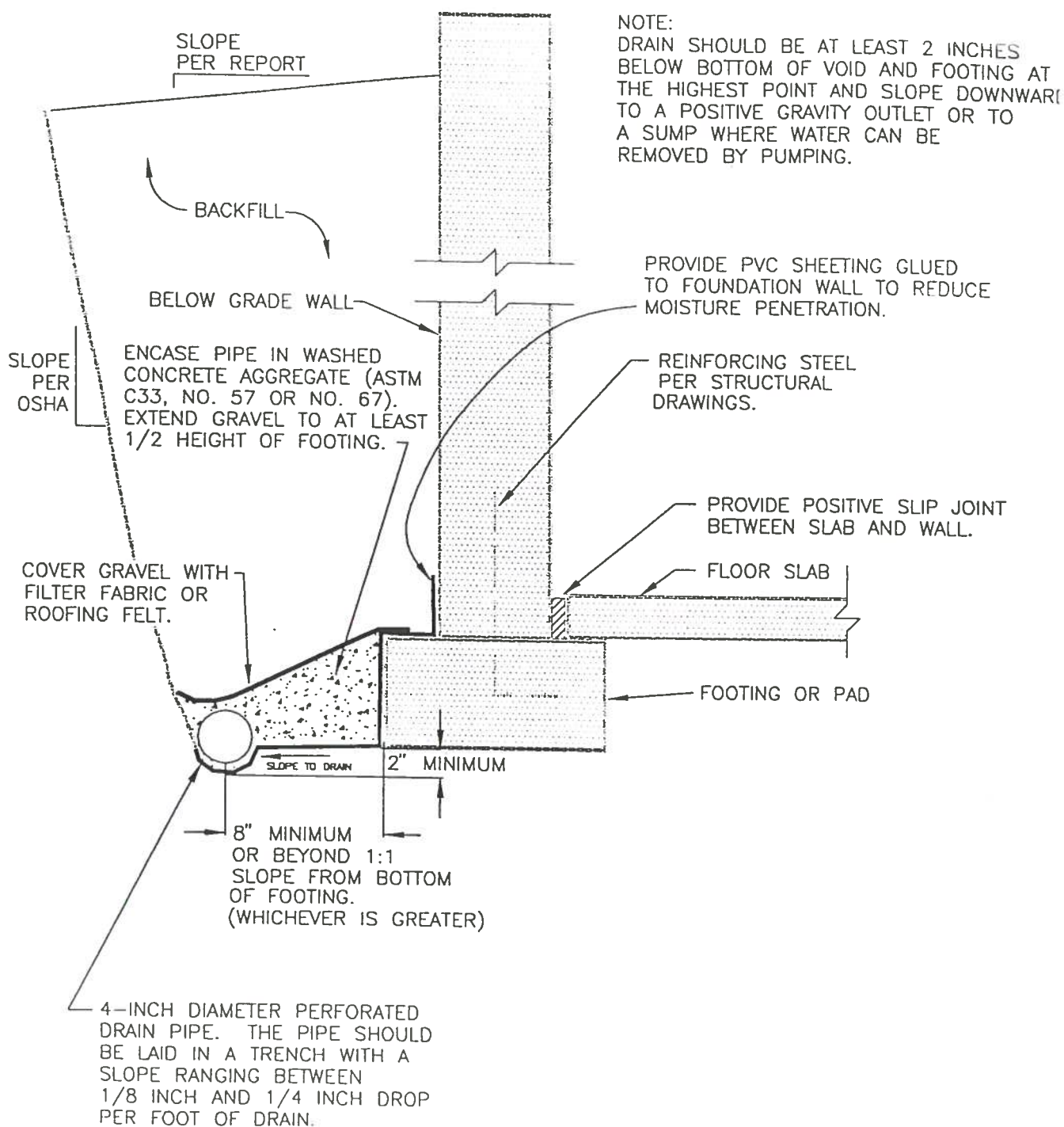
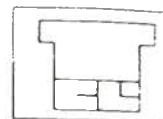
P_2 = ALLOWABLE PRESSURE ON NATURAL SOILS:

$$P_2 = \frac{P_1(w)}{(w+d)}$$

w = WIDTH OF FOOTING

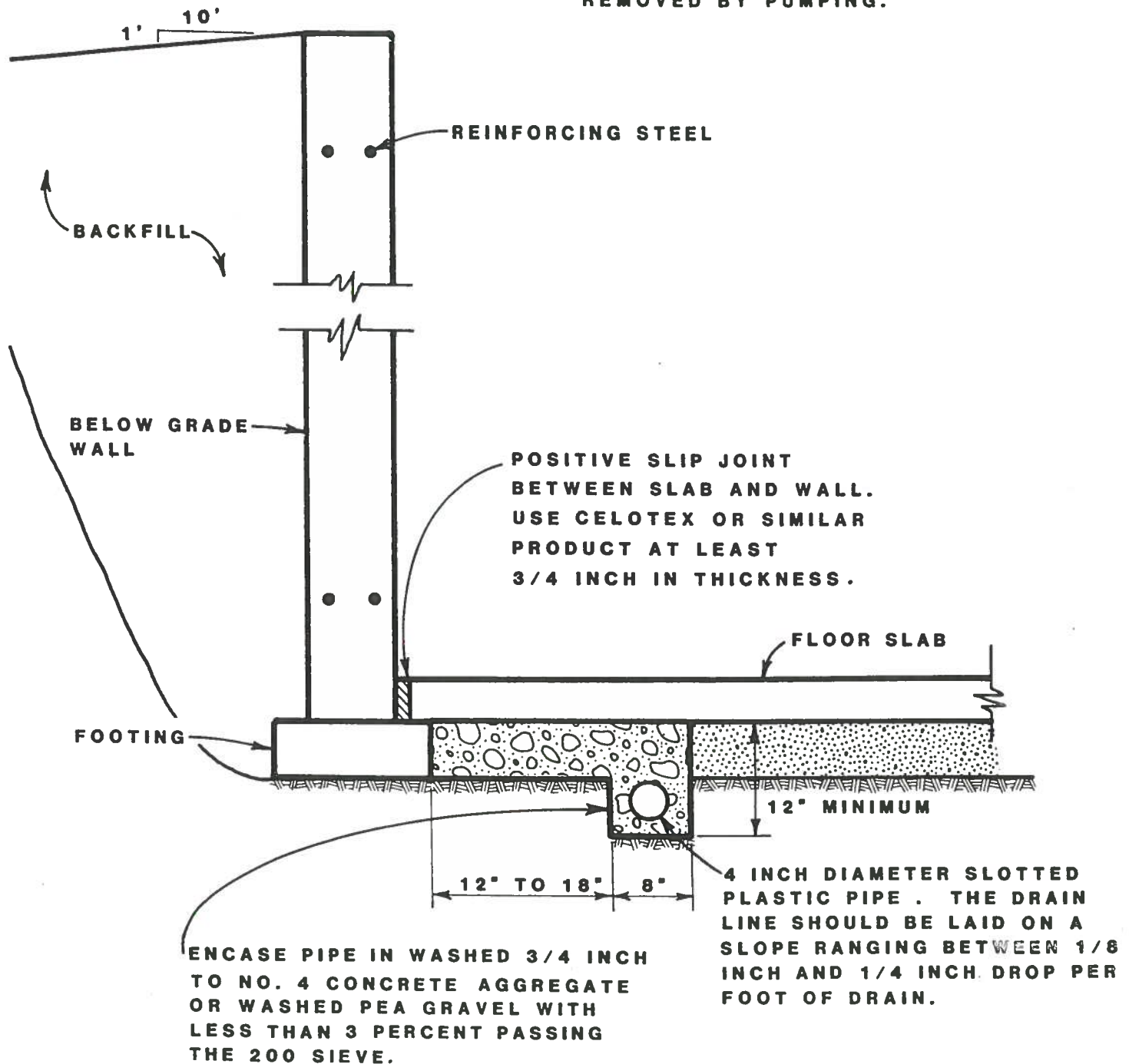
d = DEPTH OF FILL BENEATH THE FOOTING

DISTRIBUTION OF PRESSURE FOR FOOTINGS ON FILL

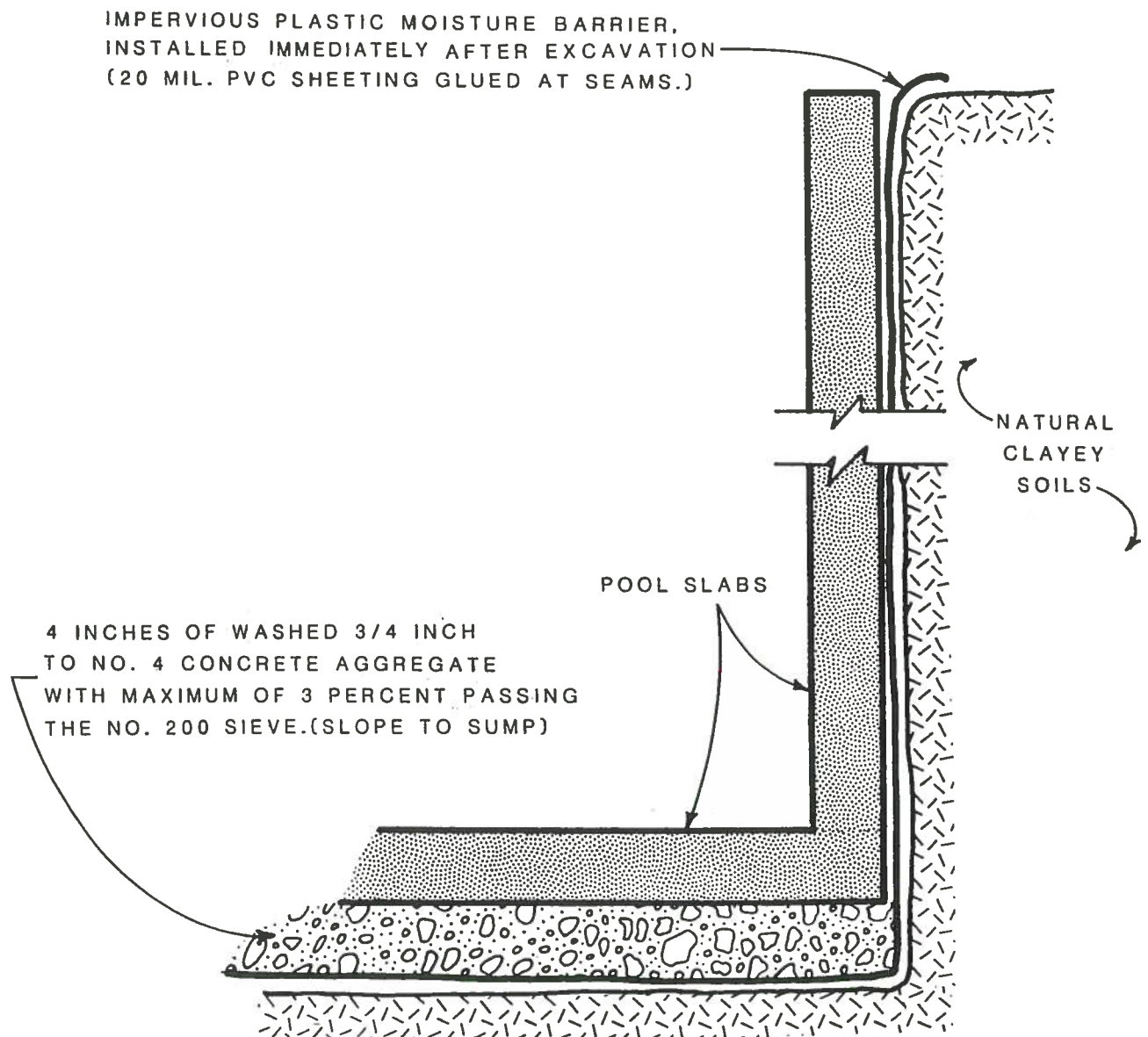


EXTERIOR FOUNDATION WALL DRAIN

NOTE: DRAIN SHOULD BE 12 INCHES BELOW BOTTOM OF SLAB AT THE HIGHEST POINT AND SLOPE DOWNWARD TO A POSITIVE GRAVITY OUTLET OR TO A SUMP WHERE WATER CAN BE REMOVED BY PUMPING.



TYPICAL DRAIN DETAILS

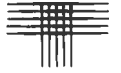


RECOMMENDED POOL DRAINAGE DETAIL

TABLE I

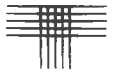
SUMMARY OF LABORATORY TEST RESULTS

[illegible]



APPENDIX B

GUIDELINE MAINTENANCE RECOMMENDATIONS



MAINTENANCE RECOMMENDATIONS FOR FLEXIBLE PAVEMENTS

A primary cause for deterioration of pavements is oxidative aging resulting in brittle pavements. Tire loads from traffic are necessary to "work" or knead the asphalt concrete to keep it flexible and rejuvenated. Preventive maintenance treatments will typically preserve the original or existing pavement by providing a protective seal or rejuvenating the asphalt binder to extend pavement life.

1. Annual Preventive Maintenance
 - a. Visual pavement evaluations should be performed each spring or fall.
 - b. Reports documenting the progress of distress should be kept current to provide information on effective times to apply preventive maintenance treatments.
 - c. Crack sealing should be performed annually as new cracks appear.
2. 3 to 5 Year Preventive Maintenance
 - a. The owner should budget for a preventive treatment at approximate intervals of 3 to 5 years to reduce oxidative embrittlement problems.
 - b. Typical preventive maintenance treatments include chip seals, fog seals, slurry seals and crack sealing.
3. 5 to 10 Year Corrective Maintenance
 - a. Corrective maintenance may be necessary, as dictated by the pavement condition, to correct rutting, cracking and structurally failed areas.
 - b. Corrective maintenance may include full-depth patching, milling and overlays.
 - c. In order for the pavement to provide a 20-year service life, at least one major corrective overlay should be expected.



MAINTENANCE RECOMMENDATIONS FOR RIGID PAVEMENTS

High traffic volumes create pavement rutting and smooth, polished surfaces. Preventive maintenance treatments will typically preserve the original or existing pavement by providing a protective seal and improving skid resistance through a new wearing course.

1. Annual Preventive Maintenance
 - a. Visual pavement evaluations should be performed each spring or fall.
 - b. Reports documenting the progress of distress should be kept current to provide information of effective times to apply preventive maintenance.
 - c. Crack sealing should be performed annually as new cracks appear.
2. 4 to 8 Year Preventive Maintenance
 - a. The owner should budget for a preventive treatment at approximate intervals of 4 to 8 years to reduce joint deterioration.
 - b. Typical preventive maintenance for rigid pavements include patching, crack sealing and joint cleaning and sealing.
 - c. Where joint sealants are missing or distressed, resealing is mandatory.
3. 15 to 20 Year Corrective Maintenance
 - a. Corrective maintenance for rigid pavements includes patching and slab replacement to correct subgrade failures, edge damage and material failure.
 - b. Asphalt concrete overlays may be required at 15 to 20 year intervals to improve the structural capacity of the pavement.

Email from Dan Cokley to Terri Partch dated July 7, 2020 Regarding IMHS Traffic Study

From: Dan Cokley <DanC@sgm-inc.com>
Date: July 7, 2020 at 8:50:52 AM MDT
To: Terri Partch <terri.partch@cogs.us>
Cc: Steve Beckley <sbeckley@glenwoodcaverns.com>
Subject: IMHS TIS - Pages from Level III_TIS_IronMtnP2-Final_070219

Hi Terri

I understand you recently met with Steve and had concerns about additional pools at IMHS not being accounted for in the approved TIS. I have attached pages of the TIS which describe the project, highlighted the Hotel description and noted on the Site Plan the location of the additional pools. Pools and recreational facilities are included in the ITE description of typical Hotel activities. The additional pools are planned for hotel guests. Cut/pasted from the TIS narrative below...

There are 10 fresh water pools located along the hotel frontage, these pools will be fenced and for hotel guests only.

I believe these pools will not change the typical trip generation rate of the Hotel Land Use as defined by the ITE Trip Generation Manual (10th Ed) and applied in the TIS.

I also understand that the pools are now planned to be constructed prior to the Hotel. The ten pools will generate fewer design hour trips than the future buildout of the Hotel. When the Hotel is constructed, these pools are planned to become part of the Hotel property as described in the TIS.

The current Access Permit volumes account for these planned development activities.

Please contact me to discuss as needed, and let me know if you need additional formal correspondence to address this issue.

Thanks,

Dan Cokley, PE, PTOE
Principal

TRAFFIC IMPACT STUDY

IRON MOUNTAIN HOT SPRINGS - PHASE 2



July 2019

Prepared by



DAN COKLEY, PE
LICENSE NO. 29799
7/2/2019



2.0 Project Description

The proposed site development shown in Figure 2 is proposed south of the IMHS and will include uses that complement the adjacent recreational uses at IMHS and the Glenwood Caverns Adventure Park (GCAP) by providing a hotel for recreational visitors, additional hot spring pools and an office / apartment building. The developer is negotiating with the State to obtain the parking lot and office structures (Parcel 1) currently owned by the State and occupied by CDOT and CSP.

The proposed development will consist of the following:

- Three (3) additional hot springs pools at IMHS
- Combined use Office (8900 sf) and employee housing apartments (20 units)
- Hotel (100 rooms)
- Procurement of CDOT/CSP office and parking (186 spaces)

The procurement of Parcel 1 will include the existing CDOT and CSP office spaces. These will be used as future operating space for IMHS and GCAP. The new office building located in Parcel 3 will house the relocated CDOT and CSP office space. There will be additional office space within the building, a total of 8900 sf of new office space will be available within the project.

The 20-unit employee housing apartment units will be available for City of Glenwood Springs residents.

The hotel will not include a restaurant, as the current IMHS and GCAP facilities both include restaurants. The hotel is located about 0.1 mile from the IMHS and approximately 0.4 mile from the GCAP tram. There are 10 fresh water pools located along the hotel frontage, these pools will be fenced and for hotel guests only. The hotel will also provide shuttle service.

The additional parking (186 spaces) will be used for GCAP overflow parking and will offset parking lost near the GCAP tram building currently leased from Land Rover (94 spaces). The CDOT/CSP spaces are about ¼ mile walking distance from the tram base and will be served by newly constructed pedestrian connection on the south side of Devereux Rd (along Two Rivers Park) between the overflow parking and the tram.

Walking distance is an important concept in the fields of transportation and public health. A ¼ mile is often used as an acceptable walking distance in U.S. research studies, that distance equates to about a 5-minute walk. Destination attractions significantly increase the potential for walking to a venue, and an increased acceptable walking distance. It is common for attractions such as stadiums and amusement parks to have a significant percentage of parking beyond a ¼ mile radius. The "CDOT" parking is located within a reasonable walking distance to be considered as on-site parking.

An illustrative concept plan is provided in Figure 2 on the following page.



Figure 2 - Proposed Concept Site Development Plan

TRAFFIC IMPACT STUDY

IRON MOUNTAIN HOT SPRINGS - PHASE 2



April 2019

Prepared by



DAN COKLEY, PE
LICENSE NO. 29799
4/8/2019



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1.0 Introduction

This study is prepared as a CDOT Level Three Traffic Impact Study and provides estimates for peak hour traffic generation for the Iron Mountain Hot Springs (IMHS) property Phase 2 development, proposed to be constructed at the north end of Centennial Street in Glenwood Springs, Colorado. The purpose of this traffic analysis is to document the existing traffic conditions in the vicinity of the site, analyze the trip generation and trip distribution of the proposed development, estimate traffic volumes to the 20-year planning horizon, and analyze the intersections within the study area for operational impacts to the highway or city road system. Access to the site will continue to be provided from Centennial Street and the majority of traffic will use Devereux Road and US 6 for primary access.

This study will assess the Level of Service (LOS) and existing auxiliary lanes along US 6 at the Devereux Road intersection to determine if they meet CDOT requirements under future conditions and discuss any other mitigation measures that may reduce the site's impact on the adjacent infrastructure.

IMHS opened in 2015, the remaining undeveloped site (with exception of CDOT/CSP) is shown shaded in red and the study intersections (Centennial Drive & Devereux Road, Devereux Road & US 6, and Traver Trail & US 6) are noted by the star symbol in the Figure 1 below.



Figure 1 -Project Area

2.0 Project Description

The proposed site development shown in Figure 2 is proposed south of the IMHS and will include uses that complement the adjacent recreational uses at IMHS and the Glenwood Caverns Adventure Park (GCAP) by providing a hotel for recreational visitors, additional hot spring pools and an office / apartment building. The developer is negotiating with the State to obtain the parking lot and office structures (Parcel 1) currently owned by the State and occupied by CDOT and CSP.

The proposed development will consist of the following:

- Three (3) additional hot springs pools at IMHS
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The procurement of Parcel 1 will include the existing CDOT and CSP office spaces. These will be used as future operating space for IMHS and GCAP. The new office building located in Parcel 3 will house the relocated CDOT and CSP office space. There will be additional office space within the building, a total of 8900 sf of new office space will be available within the project.

The 20-unit employee housing apartment units will be available for City of Glenwood Springs residents.

The hotel will not include a restaurant, as the current IMHS and GCAP facilities both include restaurants. The hotel is located about 0.1 mile from the IMHS and approximately 0.4 mile from the GCAP tram. There are 10 fresh water pools located along the hotel frontage, these pools will be fenced and for hotel guests only. The hotel will also provide shuttle service.

The additional parking (186 spaces) will be used for GCAP overflow parking and will offset parking lost near the GCAP tram building currently leased from Land Rover (94 spaces). The CDOT/CSP spaces are about ¼ mile walking distance from the tram base and will be served by newly constructed pedestrian connection on the south side of Devereux Rd (along Two Rivers Park) between the overflow parking and the tram.

Walking distance is an important concept in the fields of transportation and public health. A ¼ mile is often used as an acceptable walking distance in U.S. research studies, that distance equates to about a 5-minute walk. Destination attractions significantly increase the potential for walking to a venue, and an increased acceptable walking distance. It is common for attractions such as stadiums and amusement parks to have a significant percentage of parking beyond a ¼ mile radius. The “CDOT” parking is located within a reasonable walking distance to be considered as on-site parking.

An illustrative concept plan is provided in Figure 2 on the following page.



Figure 2 - Proposed Concept Site Development Plan

3.0 Methodology and Assumptions

This traffic analysis has been prepared in accordance with section 2.3(5) of the State Highway Access Code (Code) and the original assumptions have been vetted with the CDOT access manager and engineer for Region 3. The assumptions and methodology will provide a conservative analysis for the purposes of assessing traffic impacts resulting from buildout of the proposed development. Discussions with and concurrence from CDOT regarding methodology are documented in the Appendix.

The unsignalized intersections in the project area were analyzed using the Highway Capacity Manual (HCM) 2010 methodology.

Intersection analysis was performed using the Synchro/SimTraffic 10 analysis package to estimate the capacity of the unsignalized intersections in the study area. The measures of effectiveness that are compared for this study include Level of Service (LOS), delay and estimated 95th percentile queue lengths.

All Synchro/SimTraffic output is contained in the Appendix for reference.

Peak hour factors, heavy vehicle percentages, and other inputs were developed based on existing traffic counts taken in June and September 2018.

To provide a conservative analysis of site traffic impacts, no reductions of site traffic were assumed for multi-modal trips made by users or residents of the development. Users and residents may decide to make pedestrian, bicycle, shuttle, transit, or other trips rather than using their personal car, which should lessen the overall impact of the site when compared to the results of this analysis. The developer has previously committed to the shuttle service between IMHS and GCAP when demand exists.

The assumptions related to Design Hourly Volume (DHV) and background traffic; seasonal, 20-year growth, and parking demands are discussed in the Background Traffic section of this study.

4.0 Existing Traffic Conditions

Local Circulation Network

The Iron Mountain Hot Springs is currently located north of the Centennial Office Building and the CDOT/CSP regional headquarters on an extension of Centennial Street. No turn lanes exist along Devereux Road at the Centennial intersection. The initial IMHS development included the construction of pedestrian facilities consisting of a 6'-8' sidewalk along the west side of Centennial Street extending into the site.

US Highway 6 is the primary arterial that serves the north Glenwood Springs to West Glenwood Springs corridor. The segment of US Highway 6 (006K) within the study area is classified as an NR-B highway by the State Highway Access Category Assignment Schedule (2 CCR 601-1A, 9/13/13) and extends from Traver Trail on the west to the intersection of 6th and Laurel (SH 82) on the east. Westbound US 6 is posted at 25-mph from 6th and Laurel, increasing to 40 mph about 750-feet west of the Devereux intersection, while eastbound US 6 is posted at 35-mph about 800 feet west of the intersection, and then 25-mph at Traver Trail.

The intersection slopes down from west to east at an approximate 3% - 4% grade. Left and right turn deceleration lanes are provided on US 6 at Devereux and a short right turn pocket (60') is provided on the northbound Devereux Road approach controlled by a stop sign. Pedestrian facilities exist on all legs of the intersection, but no crossings are striped in either roadway. Large, painted medians separating the turn lanes at the intersection exist to provide delineation of the turning vehicular movements. Ramps are provided for the crossing of Devereux, but these are not compliant with the current PROWAG guidelines (ADA requirements) for design. The skew of the approach creates the need for drivers to pull ahead of the stop bar (existing) and into the pedestrian crossing when turning onto US 6.

West of Devereux Road, Traver Trail (220' west) is the only access within 1000' feet along US 6. Several intersections are located within 1000' of Devereux Road to the east including Linden Street (225'), W 1st Street south and Best Western driveway north (420'), Silver Spruce driveways south (580' and 740'), Best Western driveway north (790' and 840'), vacant Toyota dealership south (920'), and Hotel Access north (980').

CDOT's "Traffic Data" website provided the tables below. The 2017 Average Daily Traffic (ADT) on US 6, west of the intersection with SH 82/Laurel Street was approximately 10,000 vehicles per day (vpd) on an average weekday, including 6.4% truck traffic.

Table 1 - 2017 AADT at CDOT Station #10039

Route	Ref Point	End Ref Point	Start Point Description	Annual Average Daily Traffic	AADT Year	AADT Single Trucks	AADT Comb. Trucks	Percent Trucks	20 Year Factor	Design Hour Vol (% of AADT)
006K	0	0.338	ON SH 6, WEST 6TH ST W/O SH 82, GLENWOOD	10,000	2017	520	120	6.4%	1.04	8

Devereux Road is a two-lane collector roadway with two 12' travel lanes and a one-foot paved shoulder. The posted speed along Devereux Road is 25 mph. Devereux Road begins as the south leg of the US Highway 6 intersection and extends to the intersection with West Midland Avenue near I-70 interchange 114. The road generally serves recreational, office, industrial and service uses.

Centennial Drive is a two-lane local roadway with travel width that varies at its intersection with Devereux Road on the south, but generally is 24 feet to the north with two travel. The posted speed is 25 mph. Centennial Drive ends at the IMHS entrance and parking lot. Centennial Drive currently serves the Centennial office building, CDOT/CSP and IMHS. There is a three-way intersection with CDOT/CSP that was signed in June 2018 with temporary Stop signs for the northbound and southbound movements, while the westbound movement was uncontrolled.

Existing Traffic Volumes (2018)

The intersections in the immediate vicinity of the project area have had AM and PM peak hour turning movement counts performed within the past year. Those counts are summarized below;

Existing Traffic Count Data

- 6/13/18 Weekday (Wed) (Traver / Devereux / US 6) by All Traffic Data
- 9/29/18 Weekend (Sat) (Centennial; Devereux / Centennial; Devereux / US 6) by SGM
- 2/7/19 Weekday (Thu) (Devereux / Centennial; Devereux / US 6) by SGM

This study will use the weekday traffic data collected on 6/13/18 and the weekend traffic data collected on 9/29/18 for the existing traffic condition scenario.

The existing and proposed recreational / entertainment traffic generators result in peak traffic generation during the weekday PM and weekend mid-day peak hours, therefore the study will analyze the weekday PM and weekend mid-day peak hour scenarios.

Existing 2018 traffic volumes are provided in the Appendix for the study intersections.

Existing (2018) Intersection Capacity Analysis

Weekday PM and weekend mid-day level of service estimates were prepared in accordance with the Highway Capacity Manual (Transportation Research Board, 2010). For unsignalized intersections, the Highway Capacity Manual defines level of service and delay in terms of seconds of stopped delay per vehicle, which is based on the number of acceptable gaps in the conflicting traffic stream. In general, the traffic movements analyzed are those controlled by stop signs or yield signs, and the left turn movements from the uncontrolled major street. The following table represents the level of service criteria for unsignalized intersections:

Table 2 - Level of Service (LOS) Criteria

Unsignalized Intersections

Level of Service	Delay (seconds)
A (Highly Desirable)	< 10.0
B (Desirable)	10.1 to 15
C (Acceptable)	15.1 to 25
D (Acceptable in Urban Areas)	25.1 to 35
E (Unacceptable)	35.1 to 50
F (Unacceptable)	> 50.0

Source: Highway Capacity Manual, 2010

The “overall” intersection level of service at unsignalized intersections corresponds with the highest delay experienced on a minor street approach. In general, CDOT and the City of Glenwood Springs consider overall intersection operations of LOS “D” or better acceptable during the peak hours.

Using the existing 2018 traffic volumes, the capacity analysis was run in Synchro to determine level of service and delay for the study intersections. The following table shows the overall results of the capacity analysis for the impacted intersections in the study area. Upon review of the traffic data, it was determined the Traver Trail and US 6 intersection operates at acceptable LOS at existing conditions. Again, the LOS and delays reported for the unsignalized intersections correspond with the poorest performing side street approach movement (controlled by stop sign). The uncontrolled movements at these intersections all operate at LOS “A” or better.

Table 3 - Existing 2018 Intersection Level of Service Summary

Intersection	Weekday PM		Weekend	
	LOS	DELAY ¹ (s)	LOS	DELAY (s)
Devereux & US 6	C	16.7	C	16.9
Traver Trail & US 6	B	13.1	-	-
Centennial & Devereux	A	8.2	A	9.4

¹ – Delay expressed as average delay per vehicle in seconds/vehicle

As the table shows, the unsignalized intersections operate acceptably under existing conditions.

Existing (2018) Intersection Queuing Analysis

A second measure of effectiveness that was evaluated in this study was queue length. Queue length was evaluated to provide insight into operations and capacity of existing turn lanes, specifically at the Devereux Road & US 6 intersection. Queuing will be monitored for three approaches at two study intersections to determine peak hour queuing lengths. The three approaches and available storage lengths are:

- US 6 Westbound Left Turn approach to Devereux Rd (200 ft length)
- Devereux Rd Northbound Right Turn approach to US 6 (60 ft length)
- Centennial Southbound approach to Devereux Rd (no lane)

The following table shows the baseline analysis 95th percentile queue lengths estimated by the SimTraffic model. Five simulation runs for 1 hour were conducted to estimate the queue lengths in the table below.

Table 4 - Existing (2018) 95th Percentile Queue Summary

Approach	Weekday PM (ft)	Weekend (ft)	Available Length (ft)
US 6 WB Left Turn	43	55	200
Devereux Rd NB Right Turn	28	14	60
Centennial SB approach	52	47	-

The 95th percentile queue length represents the maximum queue length that occurs during 95% of the hour, so there may be times when it is exceeded for a short period of time during peak hours. The summary above shows that all movements operate at an acceptable level based upon existing storage length.

5.0 20-Year Background Traffic

The existing traffic volumes were used as a basis to develop the 20-year background traffic volumes. The 2018 volumes were adjusted for annual seasonal and 20-year growth factors.

Seasonal factors were applied to the existing traffic counts to determine a Design Hourly Volume, which is generally defined as the 30th highest hourly volume in the design year. The definition infers that if a facility is to adequately serve throughout its life, its physical capacity will only be exceeded for about 30 hours out of the total 8,760 hours in the “design” year. The choice of the 30th highest hourly volume is a long-held concept which stems from

research published in A Policy on Geometric Design of Rural Highways from the American Association of State Highway Officials (AASHTO).

A seasonal factor for east-west oriented US 6 through traffic was determined from CDOT's OTIS website, using daily volumes collected at CDOT count station 000214 located just south of Glenwood Springs on SH 82. A monthly summary of that station is shown in Table 5.

Table 5 - US 6 Seasonal Factor Data

 OTIS Online Transportation Information System Highway Data Traffic Data Data Catalog Reports Map View Help												
N SH 82, GLEN AVE S/O BLAKE AVE, GLENWOOD SPRINGS (Station Id: 000214)												
Daily Monthly Summaries Annual												
Export to Excel												
Yr	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
2019	22,045	22,387										
2018	23,365	23,237	24,444	24,974	25,824	26,106	25,420	25,235	24,793	23,781	22,025	21,717
2017	22,576	23,590	24,797	25,270	25,389	27,223	27,128	23,295	21,072	21,144	22,774	23,736

Table 5 shows that June is generally the highest monthly average volume, therefore a seasonal factor was not applied to the June weekday traffic volumes. Weekend volumes are in the range of 75% of weekday volumes, and those volumes are not driven by consistent commuter traffic. A seasonal factor was not applied to the September weekend traffic counts for US 6.

Seasonal factors for the IMHS and GCAP facilities were calculated using the DHV concept defined above. Attendance records were used to determine the 30th highest daily attendance for each site, those 30th highest attendance days were compared to the attendance on the traffic count days and a seasonal factor calculated. This calculation assumes one peak hour period daily for the facility, weekday PM and weekend midday. See Tables 6 & 7.

Table 6 - 30th Highest Attendance Data

Attendance Data - IMHS / GCAP			Attendance			Count Day	
			IMHS	GCAP	Combined	IMHS	GCAP
2018	WEEKEND/HOLIDAY	30th HIGHEST at 1 PH/Day	1237	1277	2514	1342	998
	WEEKDAY	30th HIGHEST	772	1032	1804	503	752
20 yr Growth	WEEKEND/HOLIDAY	30th HIGHEST at 1 PH/Day	1468	1867	3335		
	WEEKDAY	30th HIGHEST	916	1509	2425		

Table 7 - Seasonal Factor Data

Seasonal Factors				Count Day
	IMHS	GCAP	Combined	
WEEKEND/HOLIDAY	92%	128%	107%	9/29/2018
WEEKDAY	153%	137%	144%	6/13/2018
WEEKEND/HOLIDAY	109%	187%	143%	9/29/2018
WEEKDAY	182%	201%	193%	6/13/2018

The seasonal factors were applied to the existing and 20-year IMHS and GCAP traffic volumes.

The 20-year factor used for US 6 based on CDOT data at count station 000391 was 1.04 (Table 5), or an overall growth of 4% over 20 years applied to the US 6 through traffic at the Devereux Road and Traver Trail intersections.

The 20-year growth factor for IMHS/GCAP was calculated based upon an assumed capacity of 300,000 for each facility as provided by the Owner. The calculation of the 20-year growth factor is shown in Table 8, based upon 2018 annual attendance at each location as compared to the future capacity.

Table 8 - IMHS/GCAP Growth Factor Data

20-year GROWTH FACTOR		GCAP	IMHS	TOTAL
2018 ANNUAL ATTENDANCE		205,208	252,787	457,995
FUTURE CAPACITY		300,000	300,000	600,000
		146%	119%	131%

Another aspect of the proposed development involves the redistribution of existing parking for the GCAP site. As noted previously, 94 parking spaces located immediately northeast of the tram are currently leased from Land Rover. The Land Rover spaces would no longer be used and would be replaced by 186 spaces that currently exist to the west between Centennial Drive and the existing CDOT/CSP office facility, shown within Parcel 1 of Figure 2. Available parking by location is summarized in Table 9.

Table 9 - Parking Summary

On-Site Parking			
Location	Existing	Proposed	Change
GCAP	345	251	-94
"CDOT"		186	186
IMHS	144	171	27
Hotel		100	100
Housing		40	40
Office		32	32
Total	489	780	291

There is a total of 345 existing parking spaces available for use near the GCAP tram. This study uses a reasonable assumption based on the observation of daily operations that existing parking capacity is exceeded at approximately 1200 visitors, equating to 3.5 visitors per vehicle. Using the 30th highest weekday & weekend attendance coupled with the available re-distributed parking results in the following calculation of parking needs and trips diverted from GCAP to the CDOT parking lot at IMHS. Those results are shown in Table 10.

Table 10 - Parking Diversion

Location	Pkg Need (veh)		Pkg Diversion (veh)		Design Year
	Weekday	Weekend	Weekday	Weekend	
"CDOT"	297	367	46	116	2018
	434	537	183	286	20-year

The seasonal and growth factored parking needs and diverted trips are included in the 20-year background traffic

20-year Background Traffic Volumes

20-year background traffic volumes (seasonal and growth factored) are provided in the Appendix for the study intersections.

20-year Background Intersection Capacity Analysis

Using the 20-year background traffic volumes, the capacity analysis was run in Synchro to determine level of service and delay for the study intersections. The following table shows the overall results of the capacity analysis for all three intersections in the study area. Again, the LOS and delays reported for the unsignalized intersections correspond with the poorest performing side street approach movement (controlled by stop sign). The uncontrolled movements at these intersections all operate at LOS "B" or better.

Table 11 - 20-Year Background Intersection Level of Service Summary

Intersection	Weekday PM		Weekend	
	LOS	DELAY ₁ (s)	LOS	DELAY (s)
Devereux & US 6	C	19.7	C	21.2
Traver Trail & US 6	B	13.4	-	-
Centennial & Devereux	A	8.4	A	9.5

As the table shows, the unsignalized intersections operate acceptably under 20-year Background traffic conditions. The weekend Devereux northbound left turn movement delay is just below meeting the LOS D threshold of 25 seconds.

20-year Background Intersection Queuing Analysis

The following table shows the baseline analysis 95th percentile queue lengths estimated by the SimTraffic model. Five simulation runs for 1 hour were conducted to estimate the queue lengths in the table below.

Table 12 - 20-year Background 95th Percentile Queue Summary

Approach	Weekday PM (ft)	Weekend (ft)	Available Length (ft)
US 6 WB Left Turn	48	66	200
Devereux Rd NB Right Turn	40	14	60
Centennial SB approach	51	45	-

The 95th percentile queue length represents the maximum queue length that occurs during 95% of the hour, so there may be times when it is exceeded for a short period of time during peak hours. The summary above shows that all movements operate at an acceptable level based upon existing storage length.

6.0 20-year Total Traffic

Project Trip Generation

ITE's Trip Generation (10th Edition) was used to provide trip generation rates for the proposed development uses. The reference does not provide trip generation rates for a "Hot Springs" pool facility. However, 2018 weekend traffic counts (9/29/18) at the IMHS entrance on Centennial Drive were used to establish a per pool rate for that day. That rate is adjusted based upon attendance to provide a 30th design hourly rate for weekday and weekend. The GCAP overflow (OF) parking rates are based upon the discussion in section 5.0. The employee housing (apartment), office and hotel rates are based upon standard ITE rates.

Table 13 - Estimated Weekday Traffic Generation Summary

Land Use	Number of Units	ITE Code	Weekday Traffic	Weekday Design Hour Traffic			
				AM IN	AM OUT	PM IN	PM OUT
IMHS pool	3	n/a	0	0	0	5	3
Employee Housing	20	220	110	2	8	9	5
Hotel	100	310	702	27	18	25	24
Office	9	710	104	9	1	2	9
GCAP OF Parking	186	n/a	366	0	0	14	14
			1,282	38	27	55	55

Table 14 - Estimated Weekend Traffic Generation Summary

Land Use	Number of Units	ITE Code	Saturday Traffic	Saturday Design Hour Traffic	
				IN	OUT
IMHS pool	3	n/a		8	5
Employee Housing	20	220		7	7
Hotel	100	310		40	32
Office	8	710		2	2
GCAP OF Parking	186	n/a	372	18	10
				75	56

IMHS Pool - based upon 2018 Sat. Count (factored)

ITE Code 220 - Multi-Family Housing (Low-Rise) (20 apartments)

ITE Code 310 - Hotel (100 rooms)

ITE Code 710 - General Office (8900 sf)

GCAP Overflow Parking (occurs at 1200 visitors)

Trip Distribution and Assignment

The distribution and assignment of site generated trips follows the distributions established in the original traffic study for Iron Mountain Hot Springs, "Level 2 Traffic Assessment - Iron Mountain Hot Springs" (SGM, December 2014), which generally followed "Iron Mountain PUD - Initial Traffic Evaluation" (Turnkey Consulting, April 2013).

For this analysis, 73% of site traffic is oriented to the east on US 6, 15% is oriented to the west on US 6, 10% is oriented to the west on Devereux Road, and 2% is oriented to and from Traver Trail.

Project Generated Traffic Volumes

The developed project traffic volumes are provided in the Appendix for the study intersections.

20-year Total Traffic Volumes

The 20-year traffic volumes are the sum of the 20-year background traffic volumes plus the proposed site-generated traffic volumes.

The percent increase in traffic at the study intersections, based on the peak hour background plus site-generated traffic versus 2018 volumes are summarized in the table below.

Table 15 - Percentage Increase in Traffic

	Weekday PM		Weekend	
	Overall	Project Leg	Overall	Project Leg
Centennial / Devereux	174%	211%	173%	266%
Devereux / US 6	120%	152%	130%	177%
Traver Trail / US 6	107%	108%	-	-

20-year traffic volumes are provided in the Appendix for the study intersections.

20-year Total Intersection Capacity Analysis

Table 16 - 20-year Intersection Level of Service Summary

Intersection	Weekday PM		Weekend	
	LOS	DELAY ¹ (s)	LOS	DELAY (s)
Devereux & US 6	C	24.3	D	27.9
Traver Trail & US 6	B	13.6	-	-
Centennial & Devereux	A	9.3	B	10.2

1 – Delay expressed as average delay per vehicle in seconds/vehicle

Using the 20-year total traffic volumes, the capacity analysis was run in Synchro to determine level of service and delay for the study intersections. The following table shows the overall results of the capacity analysis for the impacted intersections in the study area. The LOS and delays reported for the unsignalized intersections correspond with the poorest performing side street approach movement (controlled by stop sign). The uncontrolled movements at these intersections all operate at LOS “B” or better.

As the table shows, the unsignalized intersections operate acceptably under 20-year Total traffic conditions. The weekend Devereux northbound left turn movement delay increased beyond 25 seconds into the LOS D threshold.

The percent increase in delay at the study intersections, based on the peak hour background plus site-generated traffic versus 2018 volumes are summarized in the table below.

Table 17 - Percentage Increase in Delay (s)

	Weekday PM	Weekend
Centennial / Devereux	145%	273%

Devereux / US 6

131%

109%

20-year Total Intersection Queuing Analysis

The following table shows the baseline analysis 95th percentile queue lengths estimated by the SimTraffic model. Five simulation runs for 1 hour were conducted to estimate the queue lengths in the table below.

Table 18 - 20-year Total 95th Percentile Queue Summary

Approach	Weekday PM (ft)	Weekend (ft)	Available Length (ft)
US 6 WB Left Turn	59	78	200
Devereux Rd NB Right Turn	48	29	60
Centennial SB approach	57	51	-

The 95th percentile queue length represents the maximum queue length that occurs during 95% of the hour, so there may be times when it is exceeded for a short period of time during peak hours. The summary above shows that all movements operate at an acceptable level based upon existing storage length. The Centennial approach is provided as reference to understand another measure of effectiveness. Since the approach operates at an acceptable LOS, a two-lane approach is not recommended at this time.

State Highway Access Permit Evaluation

The need for an access permit at the US 6 and Devereux Road intersection is based on the proposed project generated development traffic volume entering US 6 at the Devereux Road leg divided by the existing volume of traffic on the Devereux Road approach (205 DHV), as provided for in CDOT Access Permit 314157, dated January 27, 2015. The result is $98 \text{ DHV} / 205 \text{ DHV} = 47\%$, an increase greater than 20% requires a new access permit.

Also, per CDOT, when the volume exceeds 205 DHV, a new permit is required. The existing (2018) traffic on that leg is 147 during the AM peak and 305 during the PM peak. Existing PM peak hour traffic currently exceeds the permit volume.

Project generated traffic is projected to add 98 DHV to that leg, while 20-year Total traffic including seasonal and annual growth is projected at 453 DHV.

The passenger car equivalent (PCE) DHV for permit purposes is calculated by applying the 2018 heavy vehicle percentage on the Devereux Road leg of the US 6 intersection, 3.2% to the projected DHV and assuming 2 PCE's per Heavy Vehicle, for a total of 468 PCE's.

20-year Total Traffic Turn Lane Analysis

Auxiliary turn lane requirements for accesses along Colorado State Highways are based on the projected DHV's, the speed limit and geometry of the highway adjacent to the access, and the classification of the highway. For design purposes, the speed limit along US 6 adjacent to the access is 25 mph for the eastbound approach and 25 mph for the westbound approach on an NR-B highway. Based on the State Highway Access Code (Code), a right turn deceleration lane is required when the peak hour turning volume exceeds 50 vph, a left turn deceleration lane is required when the peak hour turning volume exceeds 25 vph.

For the Devereux Road approach, and the Devereux Road and Centennial Drive intersection a similar analysis was completed. Low volume, IVow speed intersections may not typically be analyzed in this fashion, as LOS and Delay are usually indicators of unsignalized intersection operations. Because of the high volume of EB right and SB left turns at the 20-year Total traffic volumes, this provides a method to determine the need for auxiliary lanes. Devereux Road was assumed be classified as a NR-C highway, with a speed limit of 25 mph at each approach.

The Table 19 summarizes the existing conditions, DHV and warrant volumes by intersection and movement.

Table 19 - CDOT SHAC Auxiliary Lane Warrant Analysis

CDOT SHAC Auxiliary Lane Warrant Analysis								
Auxiliary Turn Lane		Existing (2018)		20-year Total		Existing Turn Lane		Design Waiver
Type (Decel)	Required when DHV >	DHV (vph)	Warrant met?	DHV (vph)	Warrant met?	?	Length (ft)	<100 DHV
<i>Devereux & US 6 (NR-B highway)</i>								
WB Left	25	68	Yes	130	Yes	Yes	200	
EB Right	50	47	No	61	Yes	Yes	250	
NB Left	25	37	Yes	50	Yes	Yes	-	
NB Right	50	153	Yes	212	Yes	Yes	60	
<i>Centennial & Devereux (use NR-C highway)</i>								
EB Right	50	3	No	3	No	No		
EB Left	25	13	No	20	No	No		
WB Right	50	31	No	93	Yes	No		29
WB Left	25	11	No	11	No	No		
SB Left	25	66	Yes	124	Yes	No		48

The design of the turn lanes should accommodate 20-year Total traffic turning volumes shown in Table 19 above. The design of NR-B lanes for speeds under 40 mph includes the length of the taper distance plus the length required for storage. Taper rate or distance is based on speed and width of the turn lane, while storage is based on peak hour traffic demand. Both US 6 lanes are currently 12' wide and the required taper rates are 7.5:1. Therefore, the taper distances for required by the Code to be 90 feet. The storage distances would be 50 feet and 130 feet for the eastbound and westbound lanes, respectively.

The eastbound right turn lane should be designed 140 feet long by Code. This lane is currently striped about 250 feet full width (plus 125-foot taper) and develops west of the Traver Trail intersection to the north. The westbound left turn lane should be 220 feet long by Code. This lane is currently striped about 150 feet full width (plus 125-foot taper) and develops east of Linden. The turn lanes of the US 6 corridor of intersection are currently sized to accommodate 20-year Total traffic DHV turning movements projected for the IMHS Phase 2 development plus background traffic.

The northbound Devereux Road approach consists of two unstriped 11 to 12-foot wide approach lanes with a full width length of about 60 feet, the existing taper (5.5:1) is approximately 60 feet long transitioning back to a single 12-foot travel lane width. Based upon the posted speed limit, the 95th percentile queue length and LOS of the northbound movements the operation of the approach is acceptable.

At the Centennial Drive and Devereux Road intersection, westbound right and southbound left turn lanes would be warranted based upon 20-year Total traffic DHV's. The SHAC (3.5 (5)) allows a design waiver for volume warranted movements that have low opposing traffic volumes, generally less than 100 DHV, which applies to these movements. Based upon the 95th percentile queue length, LOS of the intersection, and low opposing volumes to the southbound left and west bound right movements, the operation of the intersection is acceptable.

Sight Distance and Access Conditions

The Devereux Road and US 6 intersection is controlled by a stop sign on the Devereux Road approach. This approach lies within the inside of a horizontal curve and allows about 350 feet of clear sight line to the east and about 550 feet of clear sight line to the west. The Code requires 250 feet for passenger vehicles, 325 feet for single-unit trucks and 425 feet for a Multi-Unit truck for each approach (25 mph) to access the highway. Although almost all users of the proposed development will be driving passenger vehicles, the design vehicle for the proposed development would be an RV or Bus.

Based on the Code, the sight distance meets the Code for the user and design vehicles but is slightly below the minimum standard under today's conditions for multi-unit trucks. Additionally, observations concluded that the vehicles stopping at the stop sign on Devereux sometimes pull forward into the pedestrian crossing area in order to safely view eastbound oncoming traffic before turning onto US 6.

At the Centennial Street intersection with Devereux Road there is adequate sight distance (>425 feet) for traffic entering Devereux Road, a 25-mph roadway, although the westbound sight distance may be obscured by low-lying branches during the summer months. The access is very wide at the throat of the intersection (120 feet). Following the initial development of IMHS, a revised striping plan was implemented to provide for a 90° intersection and 50-foot entering radii. The intersection approach provided better delineation, while maintain access for larger CDOT maintenance vehicles at the intersection.

7.0 Conclusions and Proposed Mitigation Measures

Summary of Conclusions

- A new access permit is required. The new permit volume is calculated to be a DHV of 468 in passenger car equivalents.
- The study intersections operate at an acceptable LOS at the 20-year Total traffic volume condition.
- The study intersections have acceptable auxiliary turn lanes, in terms of width, storage length and taper rates.
- The study intersections generally provide acceptable sight distance, except as noted in Section 7.2 below.
- The study intersections are both at skewed angles and enhanced delineation could be implemented for improved turning and pedestrian movements.
- The Traver Trail and US 6 intersection is not significantly impacted by the 20-year Total traffic volumes.

Proposed Mitigation Measures

- Provide improved delineation at Devereux Road / US 6 and Centennial Drive / Devereux Road intersections. The improvements could include signage, striping, raised medians that would serve to improve the intersection safety for pedestrians and vehicles.
- Provide continuous pedestrian connection from Centennial Drive to Two Rivers Plaza Drive to serve future overflow GCAP parking.
- Remove trees, branches, bushes within the City right-of-way adjacent to the Hampton Inn to improve sight distance to the east at the Devereux Road and US 6 intersection.

Appendix A

Methodology / Assumptions (Correspondence with CDOT)

Appendix #1

Emailed to CDOT 2/4/19 – Dan R and Mark B

Iron Mountain Hot Springs - 2019

Level III Traffic Impact Study Assumptions

This memo documents the initial assumptions that SGM intends to use for the Level 3 traffic analysis for IMHS proposed development at the north end of Centennial Drive in Glenwood Springs.

The proposed development will consist the following mix of potential uses:

- ___ Additional hot springs pools.
- ___ Hotel.
- ___ Parking Lot spaces

Study Intersections and day

This Level 3 study will provide current and future-year operational analysis of the intersections of Devereux & Centennial and Devereux and US 6. Both Weekday AM and PM peak hour plus Saturday peak hour will be analyzed.

Baseline Traffic

SGM collected Saturday peak hour volumes at the study intersections on 9/29/18. Weekday peak hour counts will be performed Wednesday and Thursday February 6-7, 2019 at the study intersection. Counts were collected from 7-9 AM and from 4-6 PM. Counts will be adjusted based upon seasonal variations at nearby CDOT count station.

- Added ADT traffic count from 6/13/18 completed by ADT for Stolfus for CoGS which included Traver Trail intersection

Analysis Years and Growth Rate

Operational analysis of Opening Day traffic (2020) and Future (2040) traffic will be conducted. All baseline traffic volumes at all study intersections will be factored by CDOT's 20-year factor of 1.04 will be used based upon the previous IMHS study in 2014.

Specific Development Trip Generation Rates

- ___ Hot springs pools. (Use trip generation rate based on existing traffic and existing pools)
- ___ Hotel. (Use ITE Trip Generation Rate for Resort Hotel, ITE Code 330)
- ___ Parking Lot spaces (trip generation rate to be determined)

SH 82 Project Traffic Distribution

Follows distributions established in *Iron Mountain PUD Initial Traffic Evaluation by Turnkey, 4/2013*; and previous IMHS Study.

- 73% site traffic east on US 6
- 15% site traffic west on US 6
- 10% site traffic west on Devereux Road
- 2% site traffic north on Traver Trail

Access Permits

The existing Access Permit is 314157

Comments back from Mark Bunnell, 2/4/19

- Study Area Intersections - due to the close proximity of the Devereux Rd and Traver Trail intersections on US-6, the study will need to include the US-6 and Traver Trail intersection.
- Baseline Traffic - The adjustments to the traffic counts will need to specifically include summer season traffic to/from the Glenwood Caverns Adventure Park, not just a seasonal factor from a nearby CDOT count station.

Other Items:

- The traffic analysis for the two US-6 intersections (Devereux Rd and Traver Trail) will need to be conducted using SimTraffic due to the close proximity of these intersections.
- Please provide a traffic signal warrant analysis for US-6 and Devereux Rd/Traver Trail (consider both intersections as one for the warrant analysis)

I would suggest developing the traffic volumes and submitting those for review prior to conducting the traffic analysis.

Following submittal of Trip Generation estimates on 3/4/19, phone call with Mark B on 3/8/19

Email from Mark B 3/8/19

As a follow up to our conversation, here is my recollection of the decisions:

- * Use the 30th highest day (assumes one peak hour per day) of combined GCAP & IMHS attendance, which is Friday, March 30, to represent the weekday.
- * Use the 31st highest day, which is Saturday, February 12, to represent the weekend.
- * Seasonal adjustment - use the CDOT permanent traffic counters on SH-82 south of GWS and I-70 east of GWS to determine an appropriate seasonal factor; use a weekly factor instead of monthly.
- * Seasonal adjustment will only be applied to thru traffic on US-6, since traffic to/from Devereux Rd is fairly consistent year-round (except for GCAP and IMHS, which is reflected in other calculations).

Email to Mark B o 4/1/19 regarding the below

Seasonal and Growth Factor Calculation Summary

US 6 through traffic

- CDOT growth factor at Sta 000391, using a 20 yr factor of 1.04
- In reviewing the 2018 SH 82 volumes at Sta 000214, June is the highest monthly average volume. I am using a weekday count from 6/13/18, and would not adjust those counts volumes for a seasonal variation.
- Seasonal adjustment - use the CDOT permanent traffic counters on SH-82 south of GWS and I-70 east of GWS to determine an appropriate seasonal factor; ~~use a weekly factor instead of monthly.~~
- Seasonal adjustment will only be applied to thru traffic on US-6, since traffic to/from Devereux Rd is fairly consistent year-round (except for GCAP and IMHS, which is reflected in other calculations).

IMHS / GCAP Seasonal Factors

DHV / Attendance Data - IMHS / GCAP Traffic				Attendance			(131%)	Seasonal Factors			2018/19 Intersection Volumes						CDOT
				IMHS	GCAP	Combined	20 yr ADT	IMHS	GCAP	Overall	IMHS	Cent/Dev AM	Cent/Dev PM	Dev/US6 AM	Dev/US6 PM	Traver/US6	000214
	Weekend/Holiday (31st highest, 2/17/18)			1804	721	2525	3308										19961
	Weekday (30th highest, 3/30/18, PM only)			1448	1087	2535	3321										26870
	Saturday SGM Traffic Count 9/29/18			1342	998	2340		134%	72%	141%	73		191		751		21884
	Wednesday ADT Traffic Count 6/13/18			503	752	1255		288%	145%	265%				560	846	651	29461
	Thursday SGM Traffic Count 2/7/19			370	0	370						115	186	690	722		23258
																	27013
Site-based Attend.	WEEKEND/HOLIDAY 30th HIGHEST at 2 PM/Day			1350	1680	3030											
	WEEKEND/HOLIDAY 30th HIGHEST at 1 PM/Day			1237	1277	2514		92%	128%	107% of Saturday 9/29/18 count							
Attend.	WEEKDAY 30th HIGHEST			772	1032	1804		153%	137%	144% of Wednesday 6/13/18 count							
20 yr Growth	WEEKEND/HOLIDAY 30th HIGHEST at 2 PM/Day			1462	2456	4058		119%	146%	131% 20 yr growth factor							
	WEEKEND/HOLIDAY 30th HIGHEST at 1 PM/Day			1468	1867	3335		109%	187%	143% of Saturday 9/29/18 count							
	WEEKDAY 30th HIGHEST			916	1509	2425		182%	201%	193% of Wednesday 6/13/18 count							

IMHS Pool Rate & Seasonal Factor Calculation																					
Saturday 9/29/18				Weekend (or Holiday)				Weekday				6/13/18 Weekday Data				Seasonal/Growth Adjust (ADT->30th)					
Peak Hour Traffic	IN	OUT	Attend.	# Pools	IN	OUT	Highest Attend.*	IN	OUT	Attend.*	IN	OUT	PM	PM	IN	OUT	PM	PM	PM	PM	
	45	28	1342	16	2.81	1.75	1237	2.59	1.61	772	1.62	1.01	26	16	503	1.05	0.66	17	10	9	6
20 YEAR (119%)	119%	Growth	1597	16	3.34	2.08	1468	3.07	1.91	916	1.92	1.19	31	19	503	1.05	0.65	17	10	14	9
*Based on IMHS attendance																					

On-Site Parking				Parking Need				GCAP Rate & Seasonal/Growth Factor				Directional Distribution				Directional Distribution					
Existing	Proposed	Change		Existing	Need Below	Weekday	Weekend	Count	6/13/18	752	280	12	137%	GCAP Seasonal + Growth Factor				IMHS Seasonal + Growth Factor			
GCAP	345	251	-94	297	367	46	116	Seasonal	1092	1092	280	12	137%	Weekday Traffic				Weekday Traffic			
GCAP	345	251	-94	297	367	46	116	Growth	146%	1509	757	33	201%	PM Design Hour				PM Design Hour			
Hotel	100	100						*GCAP arrival rate				Cent at Devereux				Cent at Devereux					
Housing	40	40						3.48 visitors per vehicle				Devereux at US 6				Devereux at US 6					
Office	32	32						Seasonal Adjust				East (US 6)				East (US 6)					
Total	489	780	291					ADT->30th				West (US 6)				West (US 6)					
								PM				Traver Trail				Traver Trail					
								IN				IN				IN					
								OUT				OUT				OUT					
								16				16				14					

*GCAP arrival rate				3.48 visitors per vehicle				Additional to Devereux / US 6 Intersection				16				16			
*Based on operations observations at full parking -> attendance				WALKING DISTANCE															
Walking Distance				20 year				434				537				183			
				20 year				434				537				183			

This provides the data for the seasonal and growth factor's to be used for IMHS/GCAP background DHV's.

It varies from the 30th highest direction we had decided upon previously. To apply these factors to the data available, I thought they should be applied based on attendance at each venue (IMHS & GCAP). In addition, I do not believe that using the highest weekday attendance (3/30/18) is representative of the 30th highest weekday volume.

I went back to the original thought of 30th highest volumes for each Weekend/Holiday and Weekday (including all weekdays less holidays). I've shown those on the attached image, I can send xls if you would like to see that. The total weekend attendance numbers are very close to the 30th highest numbers in the above scenario. The weekday numbers are more representative of a typical busy weekday, with typical weekday commuter traffic along US 6 and on Devereux to/from Centennial Dr office buildings.

That information is presented with associated seasonal and 20 yr growth factors.

That data is then used to adjust the IMHS traffic data using the Sat 9/29/18 count at the IMHS entrance.

The data is used to adjust the additional GCAP background traffic using attendance, and assumes 3.48 persons/vehicle to generate vehicles. Those vehicles are added to existing counted volume at the GCAP intersection between Centennial and US 6 using the same IMHS directional distribution.

From Mark B 4/3/19

I reviewed the information in your emails and agree with the traffic volume forecast method. Please include a detailed discussion of the forecast method in the TIS.

Appendix B

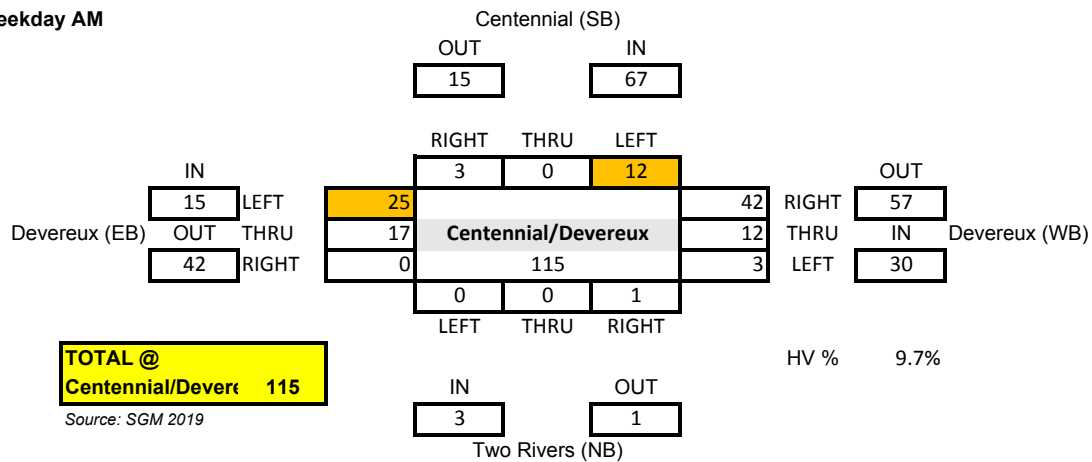
Intersection Traffic Volume Figures (2018, 20-year Background, 20-year Total)

Appendix #2

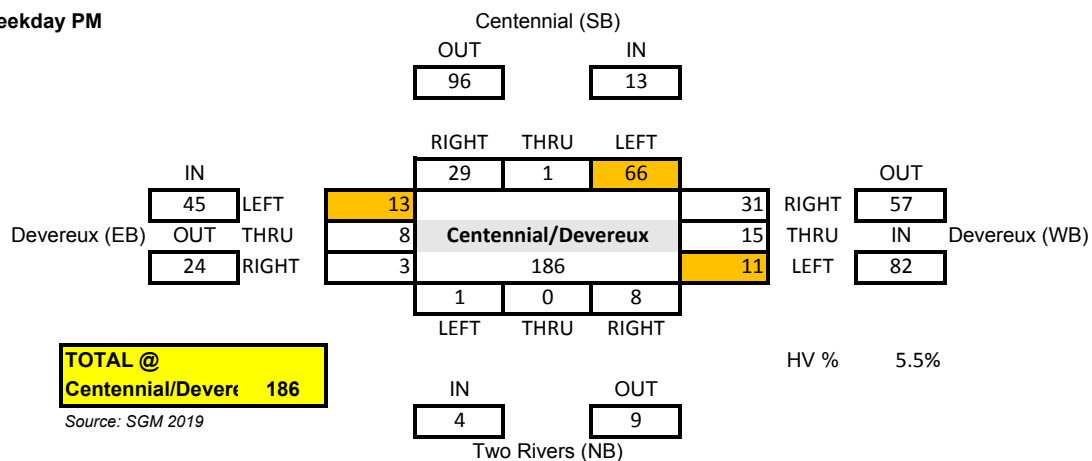
CENTENNIAL & DEVEREUX

Existing - 2/7/19

Weekday AM



Weekday PM



Left "waiver" for conflicting volume < 100 vph (PM)

EB left -> WB conflicting	57
SB left -> EB/WB/NB conflicting	42
WB left -> EB conflicting	24

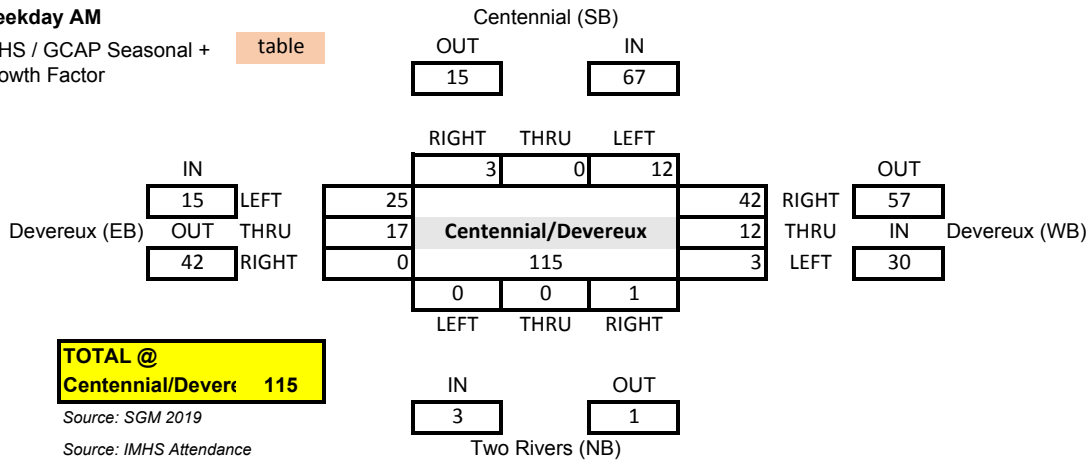
CENTENNIAL & DEVEREUX

20 yr Background Traffic - Seasonal & Growth Factor

Weekday AM

IMHS / GCAP Seasonal +
Growth Factor

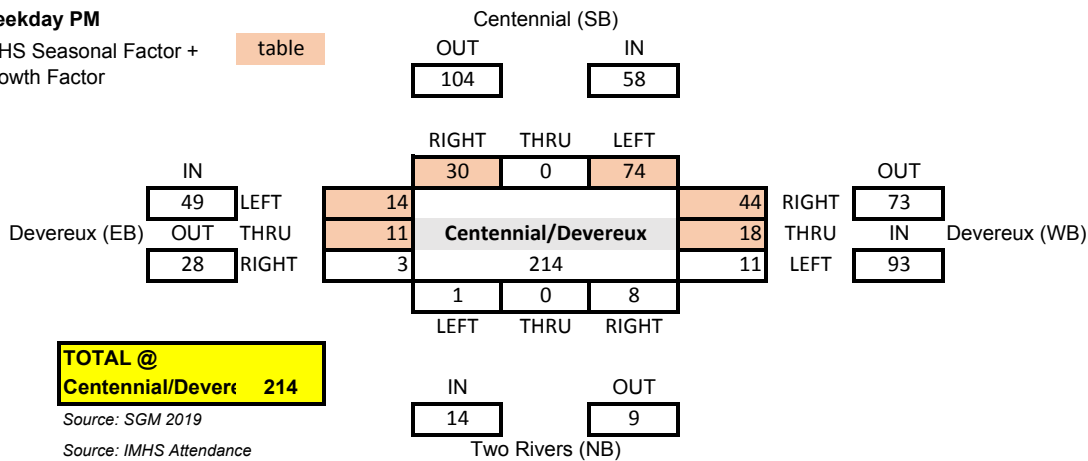
table



Weekday PM

IMHS Seasonal Factor +
Growth Factor

table

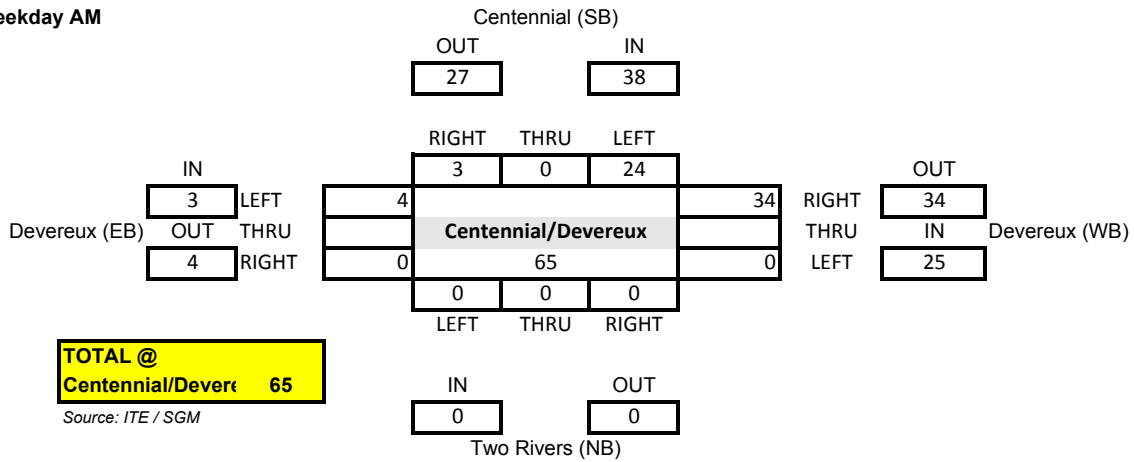


IMHS Seasonal + Growth Factor				IMHS	GCAP
Distribution				182%	201%
Weekday Traffic		PM Design Hour			
Cent / Devereux		In	Out		
East	0.9	13	8		
West	0.1	1	1		
From GCAP		2	2		
East	0.73				
West	0.15	4	3		
Traver Trail	0.02	0	0		

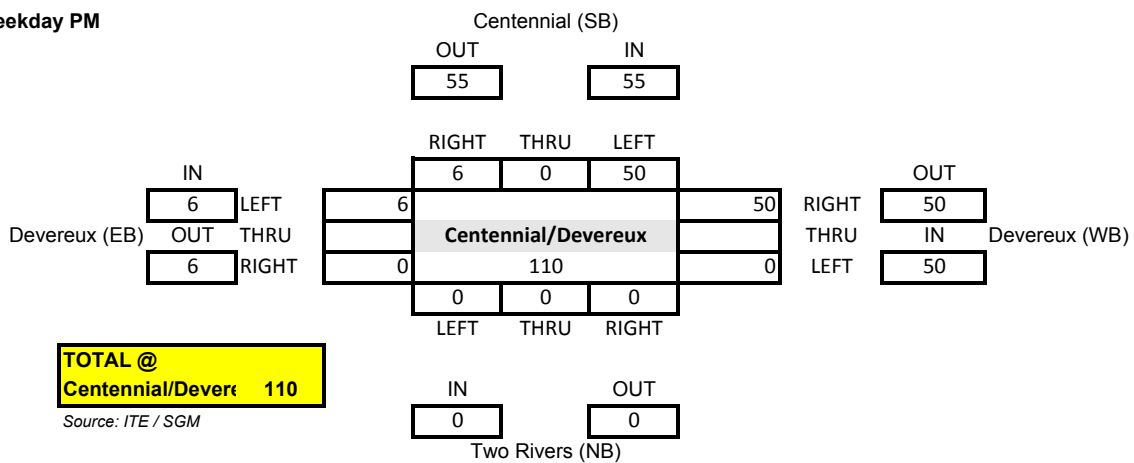
CENTENNIAL & DEVEREUX

Project Generated

Weekday AM



Weekday PM



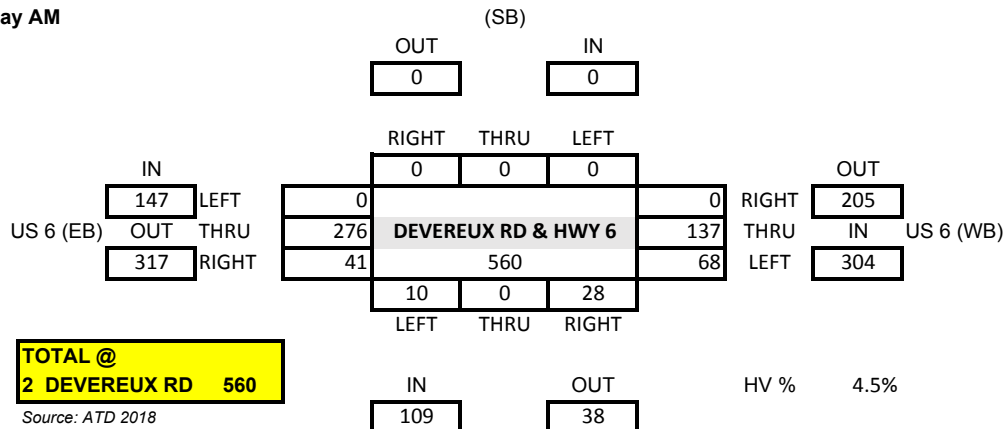
20 yr Traffic - Background + Project

Left "waiver" for conflicting volume < 100 vph (PM)	
EB left -> WB conflicting	122
SB left -> EB/WB/NB conflicting	48
WB left -> EB conflicting	34
WB Right -> WB conflicting	29

DEVEREUX RD & HWY 6

Existing - 6/13/18

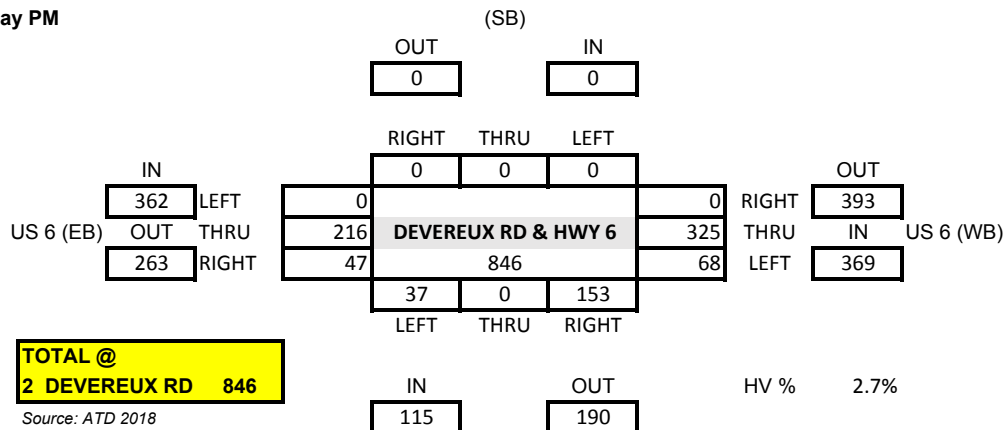
Weekday AM



Devereux (NB)

147

Weekday PM



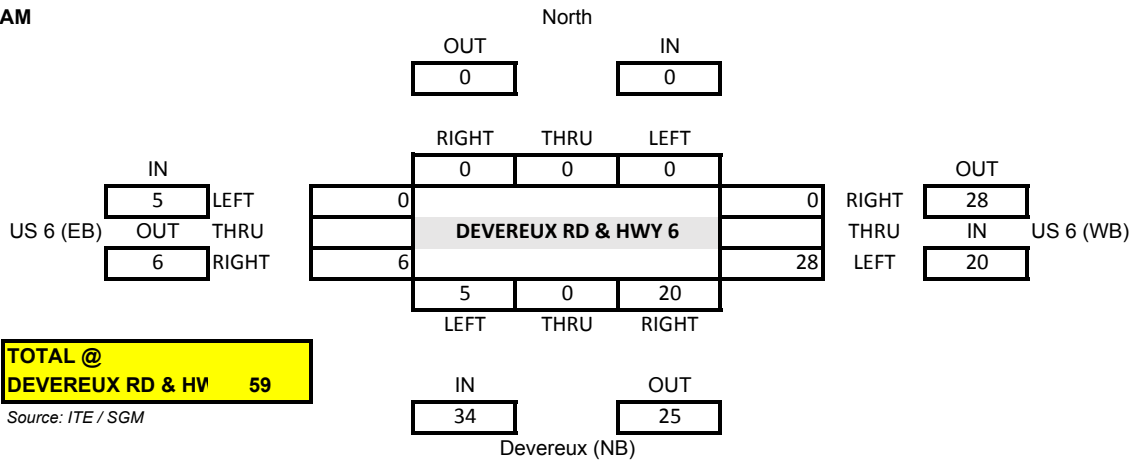
Devereux (NB)

305

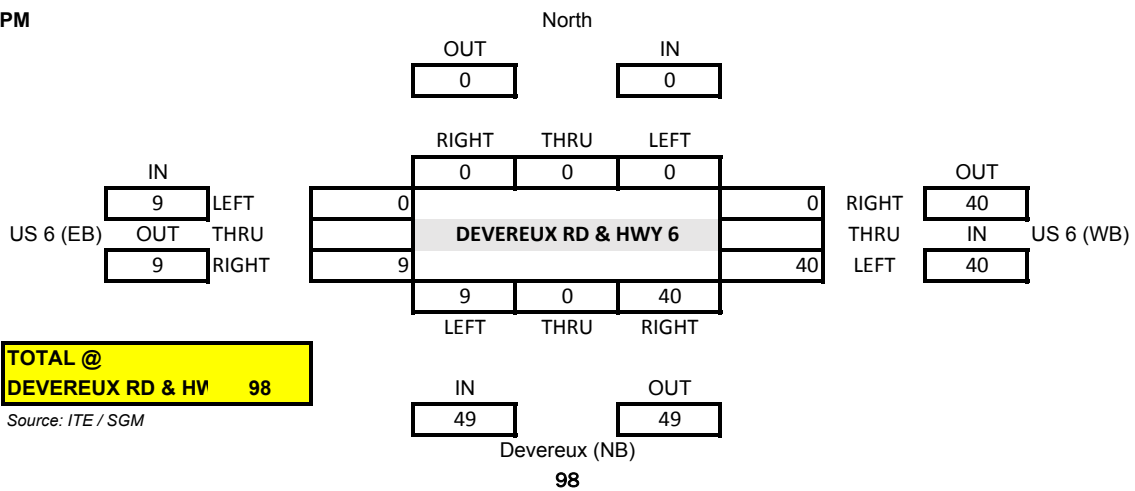
DEVEREUX RD & HWY 6

Project Generated Traffic

Weekday AM



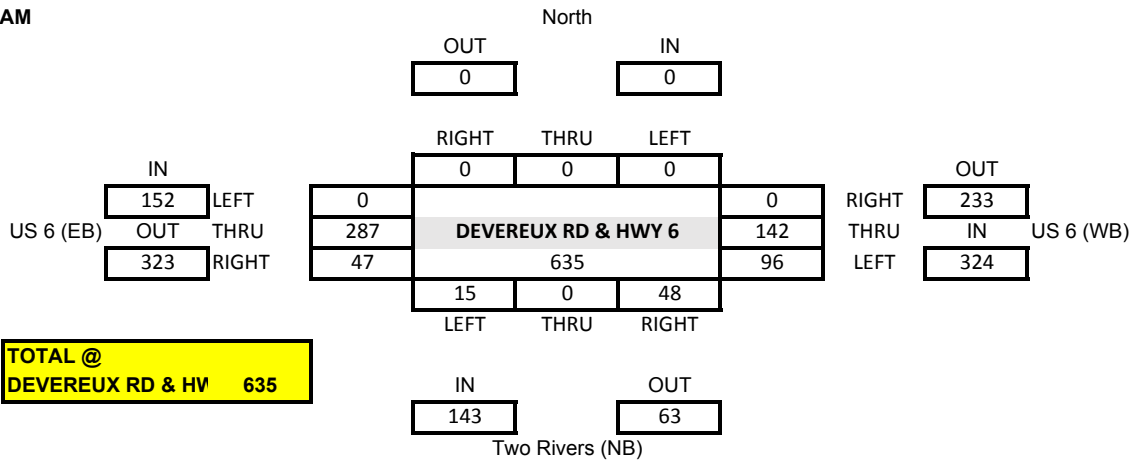
Weekday PM



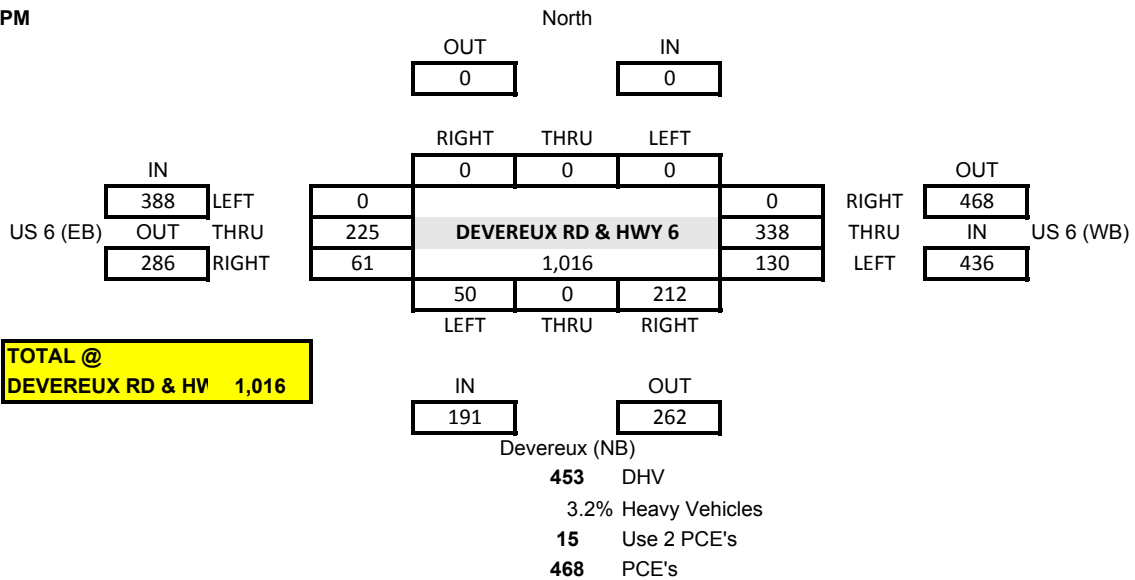
DEVEREUX RD & HWY 6

20 yr Traffic - Background + Project

Weekday AM



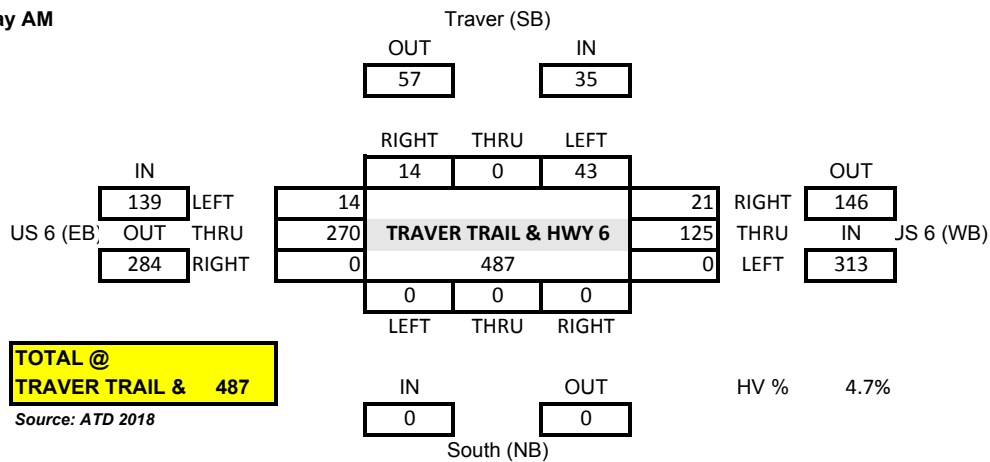
Weekday PM



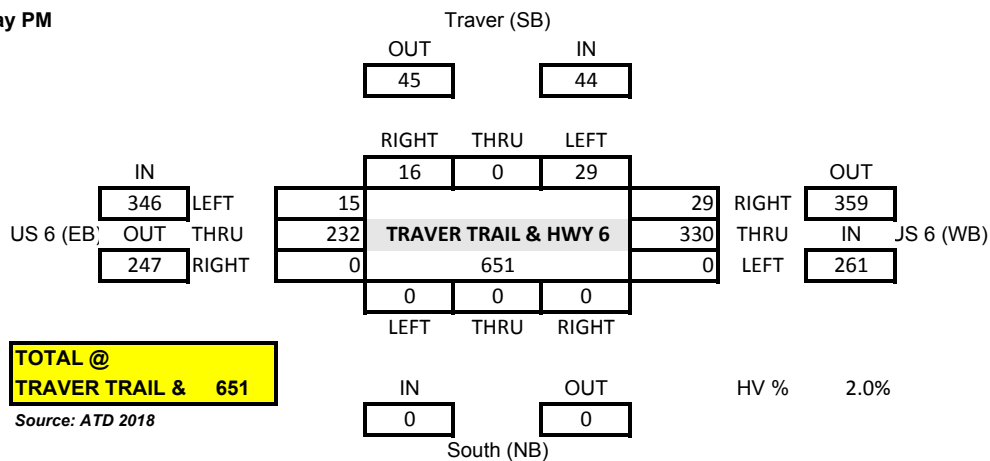
TRAVER TRAIL & HWY 6

Existing - 6/13/18

Weekday AM



Weekday PM



TRAVER TRAIL & HWY 6

20 yr Background Traffic - Seasonal & Growth Factor

Weekday AM

US 6 Seasonal Factor

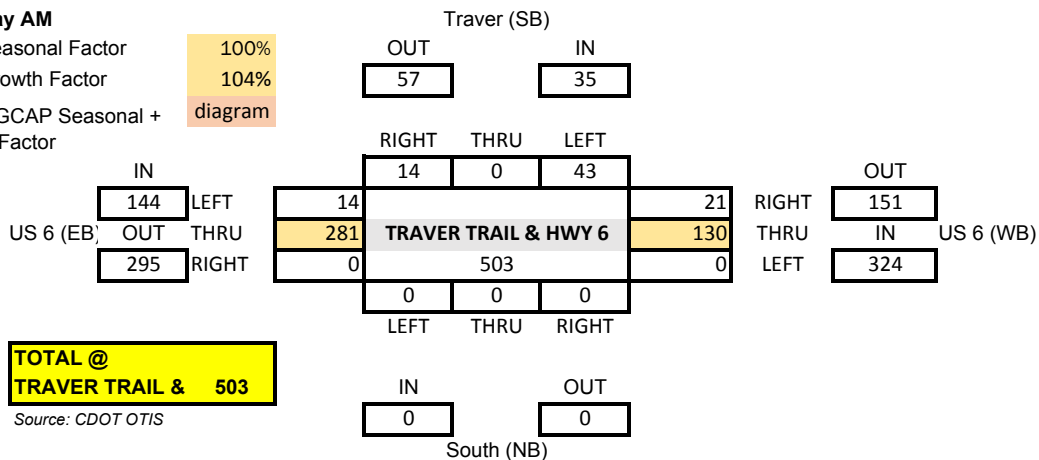
100%

US 6 Growth Factor

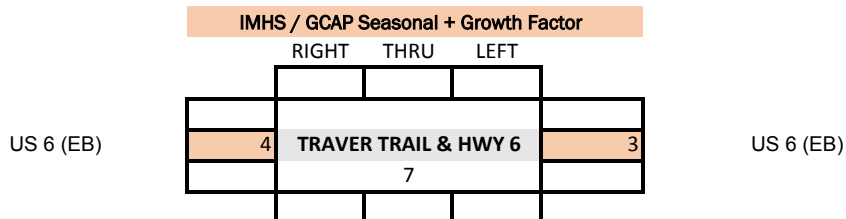
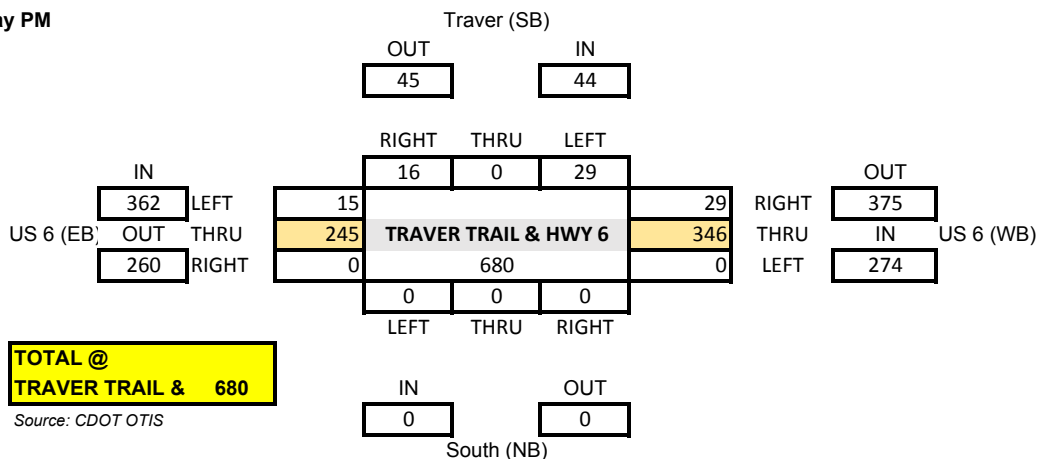
104%

IMHS / GCAP Seasonal +
Growth Factor

diagram



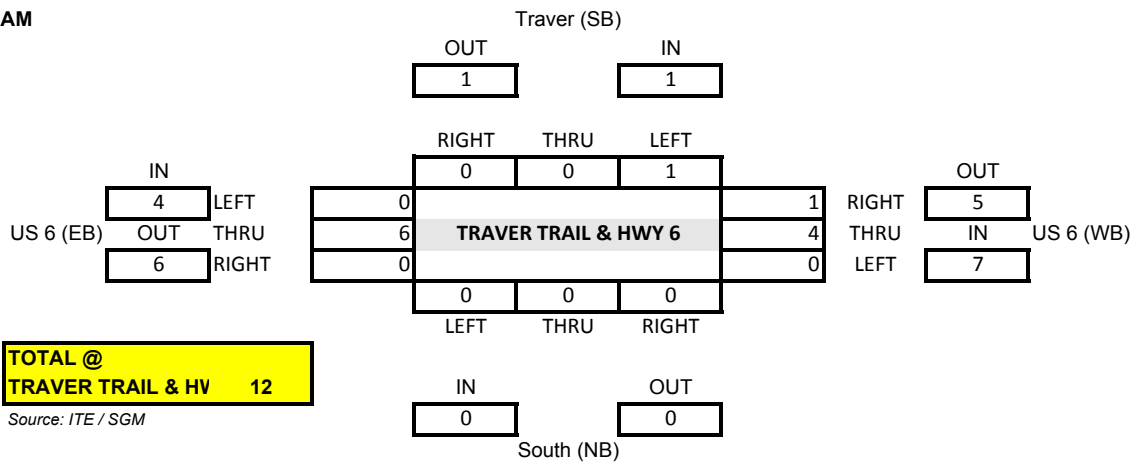
Weekday PM



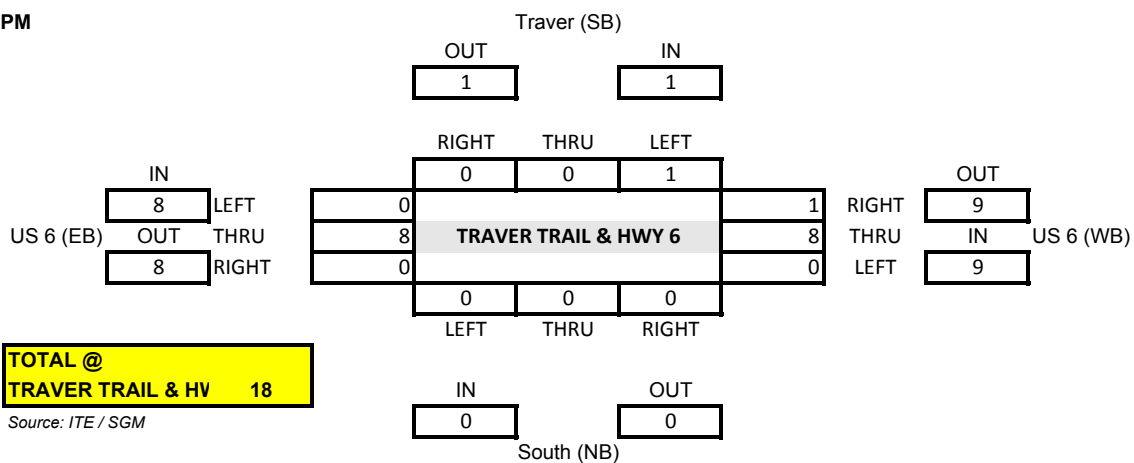
TRAVER TRAIL & HWY 6

Project Traffic

Weekday AM



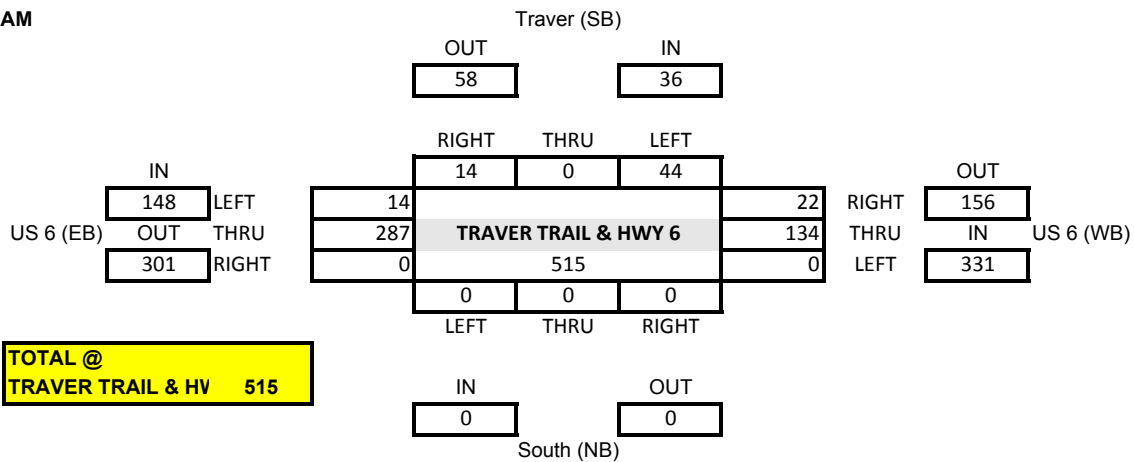
Weekday PM



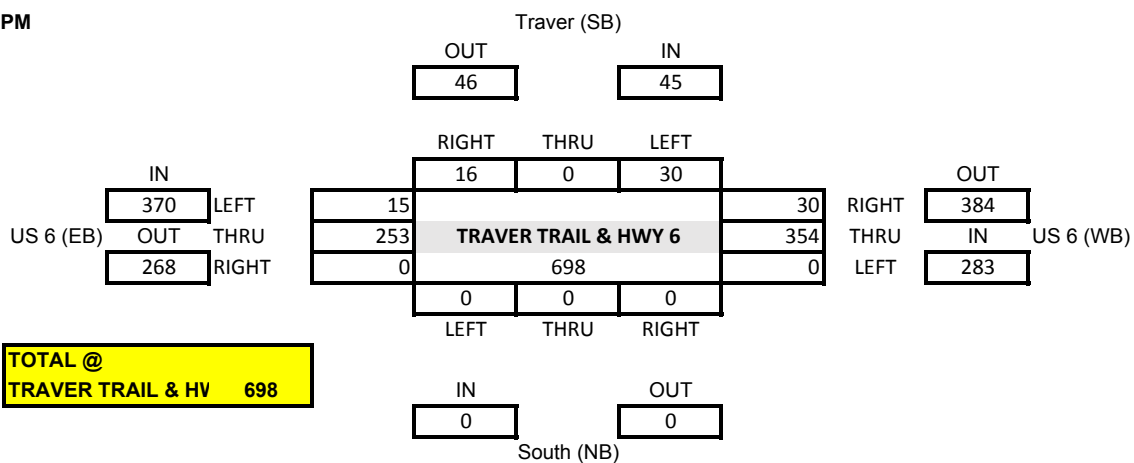
TRAVER TRAIL & HWY 6

20 yr Traffic - Background + Project

Weekday AM



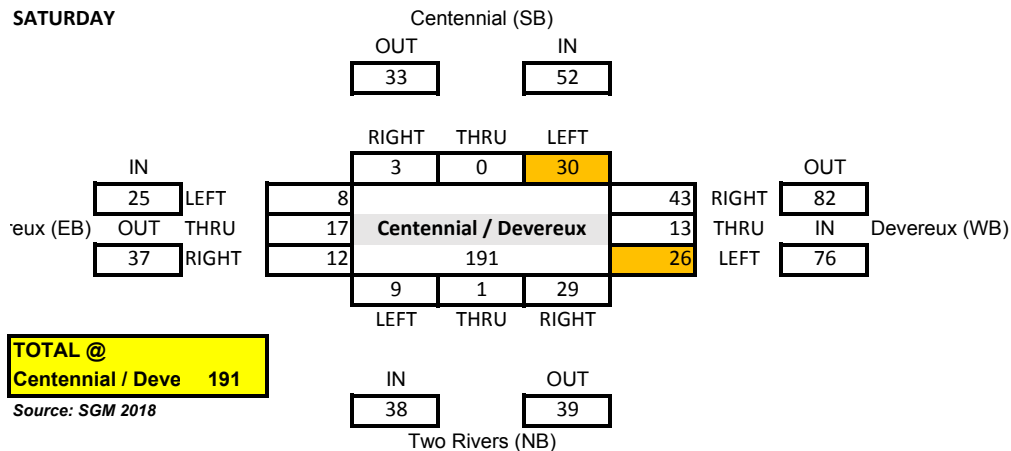
Weekday PM



CENTENNIAL & DEVEREUX

Existing - 9/29/18

SATURDAY



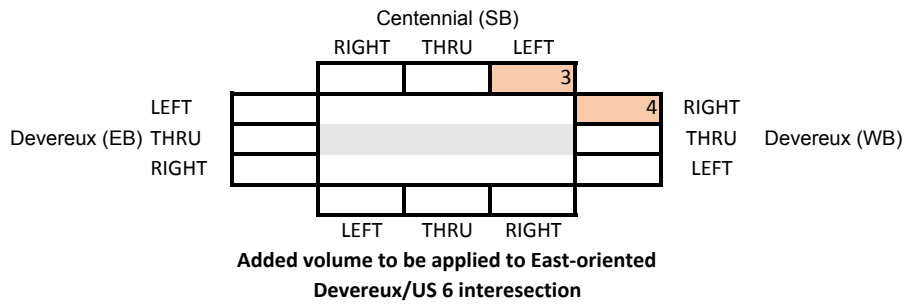
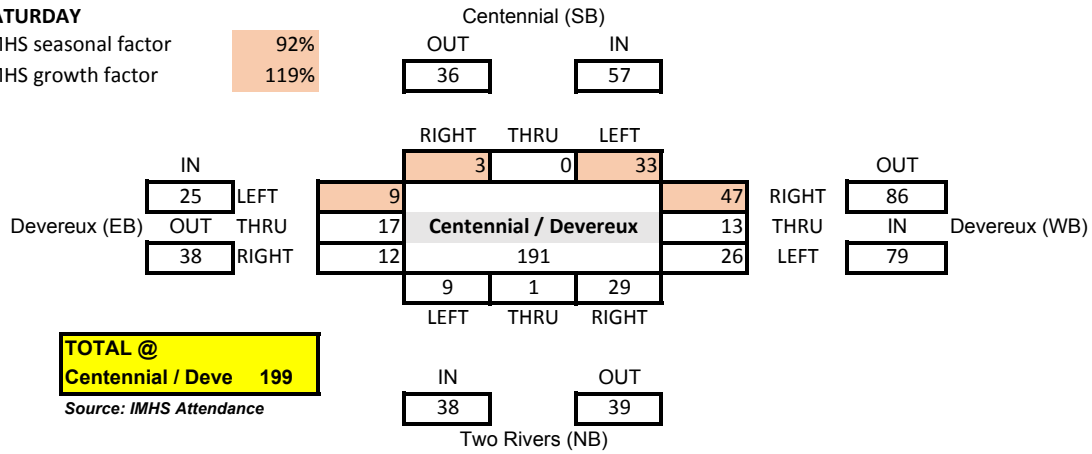
Left "waiver" for conflicting volume < 100 vph (PM)	
EB left -> WB conflicting	82
SB left -> EB/WB/NB conflicting	85
WB left -> EB conflicting	37

CENTENNIAL & DEVEREUX

Background Traffic - Seasonal / Growth Factor Adjustment

SATURDAY

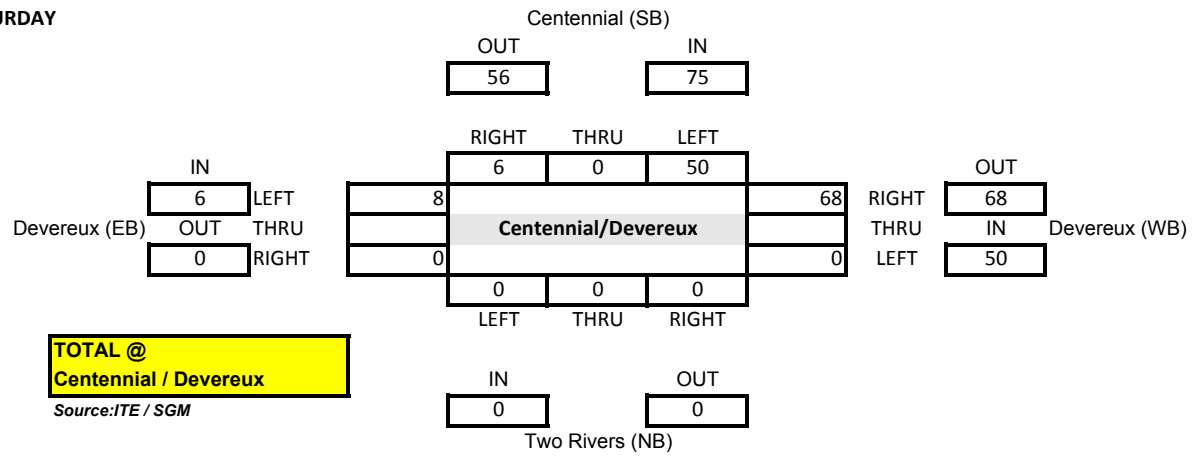
IMHS seasonal factor	92%
IMHS growth factor	119%



CENTENNIAL & DEVEREUX

Project Generated Traffic

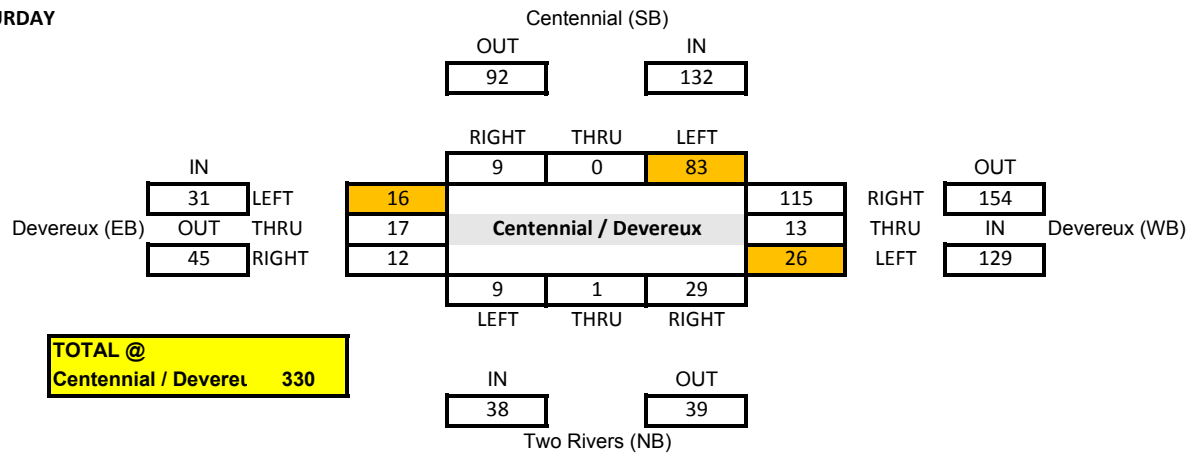
SATURDAY



CENTENNIAL & DEVEREUX

20 yr Traffic Volume

SATURDAY



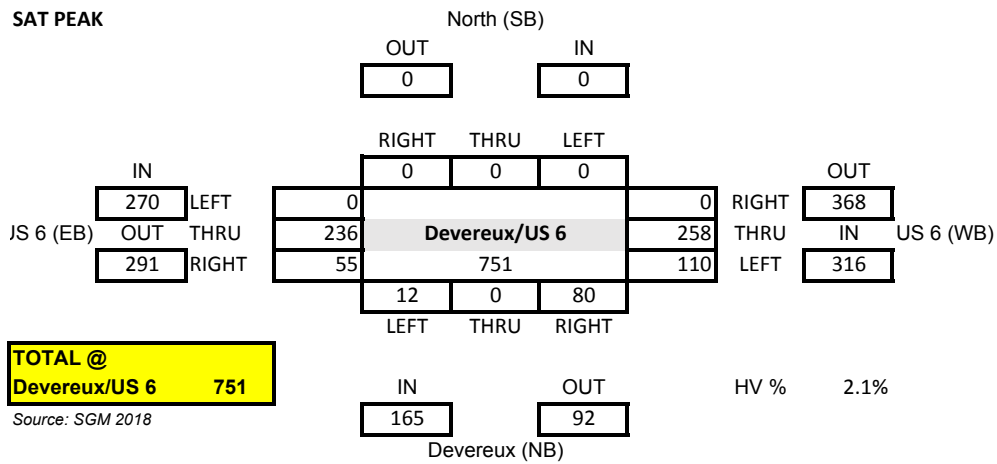
Left "waiver" for conflicting volume < 100 vph (PM)

EB left --> WB conflicting	154
SB left --> EB/WB/NB conflicting	85
WB left --> EB conflicting	45

DEVEREUX RD & HWY 6

Existing - 9/29/18

SAT PEAK

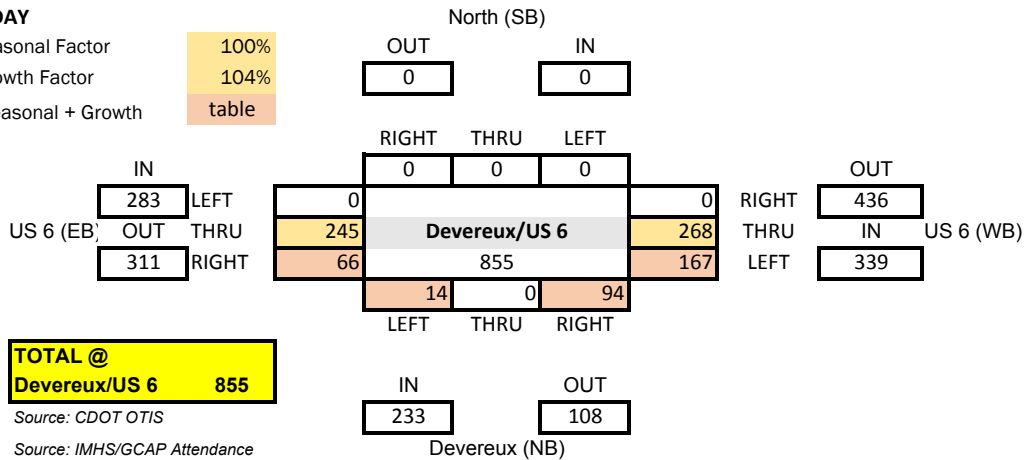


DEVEREUX RD & HWY 6

20 yr Background Traffic - Seasonal / Growth Factor Adjustment

SATURDAY

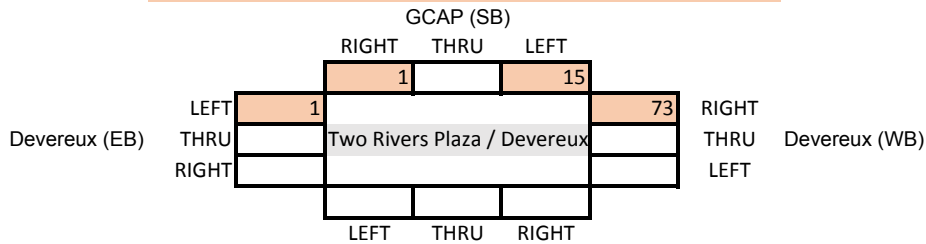
US 6 seasonal Factor	100%
US 6 Growth Factor	104%
GCAP Seasonal + Growth Factor	table



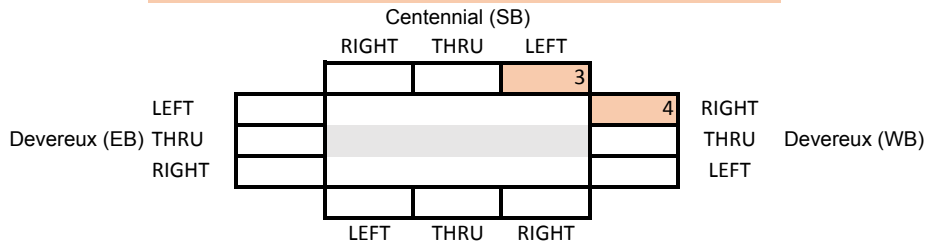
Weekend - GCAP Rate & Seasonal/Growth Factor

Devereux at US 6	165	92	Existing Sat volume at US 6
Devereux at Centennial	82	76	Existing Sat volume at Centennial
Apply to exist GCAP	83	16	9/29/18 From GCAP (Δ Intersections)
		128%	GCAP Seasonal Factor
		146%	GCAP Growth Factor

Background (Future - Existing) GCAP intersection



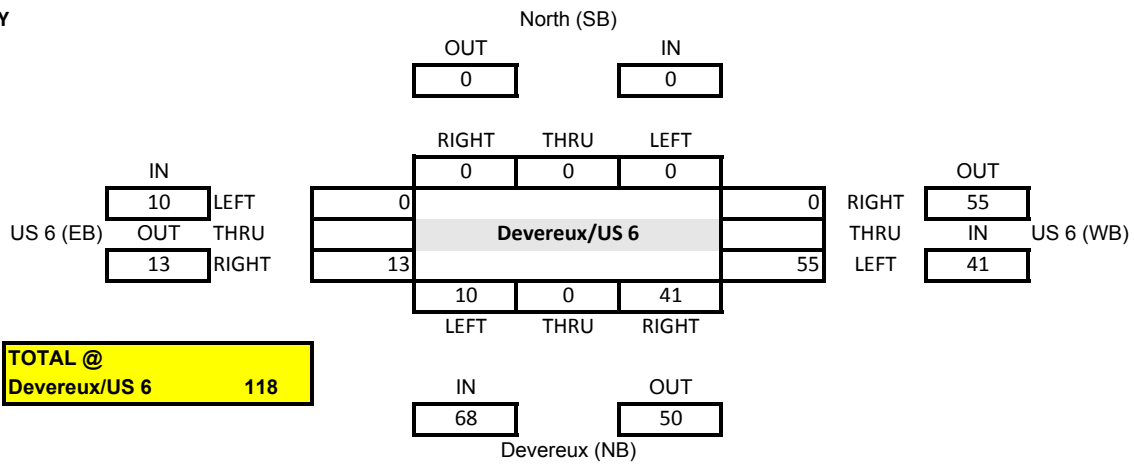
IMHS volume from Cent/Dev applied to Dev/US 6 intersection



DEVEREUX RD & HWY 6

Project Generated Traffic

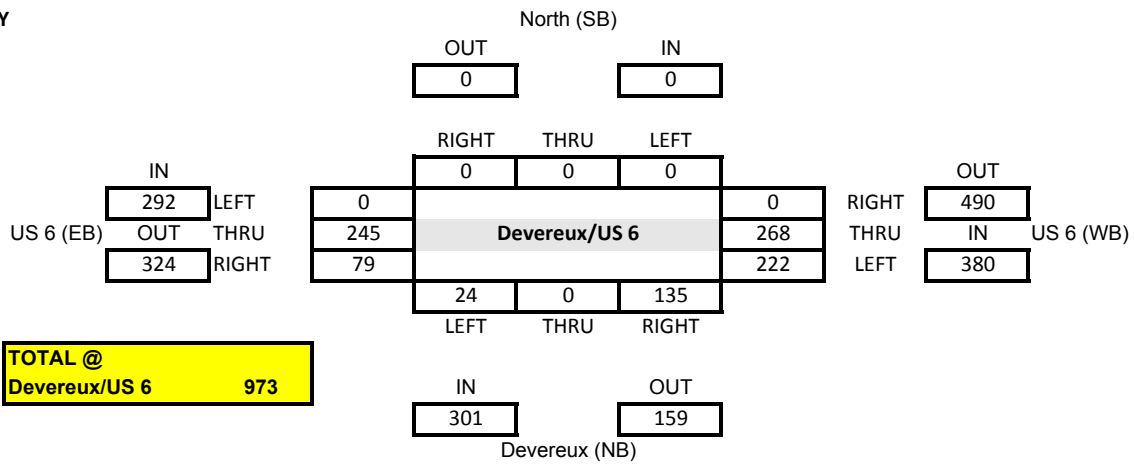
SATURDAY



DEVEREUX RD & HWY 6

20 yr Traffic Volume

SATURDAY



Appendix C

Seasonal and Growth Factor, Parking Diversion

Appendix #3

Growth Factor (131%)									
DHV / Attendance Data - IMHS / GCAP Traffic									
	Attendance			Attendance-based Factors			Seasonal		
	IMHS	GCAP	Combined	IMHS	GCAP	Overall	IMHS	GCAP	Overall
Weekend/Holiday (31st highest, 2/17/18)	1804	721	2525	20 yr Att	3308				
Weekday (30th highest, 3/30/18 PM only)	1448	1087	2535	3321					
Saturday SGM Traffic Count 9/29/18	1342	998	2340						
Wednesday ADT Traffic Count 6/13/18	503	752	1255						
Thursday SGM Traffic Count 2/7/19	370	0	370						
Attendance Data - IMHS / GCAP									
	Attendance			Count Day			Seasonal Factors		
	IMHS	GCAP	Combined	IMHS	GCAP		IMHS	GCAP	Combined
2018 WEEKEND/HOLIDAY 30th HIGHEST at 1 PM/Day	1237	1277	2514	1342	998		92%	128%	107%
WEEKDAY 30th HIGHEST	772	1032	1804	503	752		153%	137%	144%
20 yr Growth WEEKEND/HOLIDAY 30th HIGHEST at 1 PM/Day	1468	1867	3335				109%	187%	143%
WEEKDAY 30th HIGHEST	916	1509	2425				182%	201%	193%

2018/19 Intersection Volumes									
	Cent/Dev AM			Cent/Dev PM			Dev/US6 AM		
	Cent	Dev	AM	Cent	Dev	PM	Dev	US6	AM
Traver/US6									
CDOT	000214	19961							
	19961	0.742873							
	26870								
	21884	0.742813							
	29461								
	23258								
	27013								

IMHS Pool Rate & Seasonal Factor Calculation													
Saturday 9/29/18				Weekend (or Holiday)				Weekday				Seasonal/Growth	
Peak Hour Traffic		Per Pool Rate		30th Highest Attend		30th Highest Attend		30th Highest Attend		Per Pool Rate		IMHS Attendance	
IN	OUT	IN	OUT	IN	OUT	IN	OUT	IN	OUT	IN	OUT	IN	OUT
45	28	2.81	1.75	1237	2.59	1.61	1.91	1.62	1.01	26	16	503	6
119%	Growth	1597	16	3.34	2.08	1468	3.07	916	1.92	1.19	31	503	9
*Based on IMHS attendance													
EXIST.													
20 YEAR													
119% Growth													
153%													
182%													

ON-SITE PARKING ANALYSIS													
Location	Existing	Proposed	Change	Pkg Need (veh)	Pkg Diversion (veh)	Design Year	Seasonal Adjust						
GCAP	345	251	-94	297	367	46	Additional to Devereux / US 6 Intersection						
"CDOT"	144	186	42	434	537	183	Additional to Devereux / US 6 Intersection						
IMHS	171	27	-144	297	367	46	Additional to Devereux / US 6 Intersection						
Hotel	100	40	-60	297	367	46	Additional to Devereux / US 6 Intersection						
Housing	40	40	0	297	367	46	Additional to Devereux / US 6 Intersection						
Office	32	32	0	297	367	46	Additional to Devereux / US 6 Intersection						
Total	489	780	291	434	537	183	Additional to Devereux / US 6 Intersection						

GCAP Rate & Seasonal/Growth Factor													
Count	Weekday	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend	30th Highest Attend
Seasonal	6/13/18	752	1032	1509	757	33	12	12	12	12	12	12	12
Growth	30th GCAP	146%	1509	757	33	12	12	12	12	12	12	12	12
*GCAP arrival rate 3.48 visitors per vehicle													
Seasonal Adjust ADT-->30th													
Additional to Devereux / US 6 Intersection													
16													
16													

Directional Distribution													
GCAP Seasonal + Growth Factor							IMHS Seasonal + Growth Factor						
Weekday Traffic							Weekday Traffic						
Cent at Devereux							Cent at Devereux						
In							In						
Out							Out						
East (Devereux)							East (Devereux)						
0.10							0.10						
Devereux at US 6							Devereux at US 6						
0.73							0.73						
East (US 6)							East (US 6)						
0.15							0.15						
West (US 6)							West (US 6)						
0.02							0.02						
Traver Trail							Traver Trail						
0							0						
16							16						
16							16						

Weekend GCAP Rate & Seasonal/Growth Factor													
Devereux at US 6							Devereux at US 6						
Existing Sat volume at US 6							Existing Sat volume at US 6						
92							92						
Devereux at Centennial							Devereux at Centennial						
82							82						
Apply to exist GCAP							Apply to exist GCAP						
83							83						
GCAP Seasonal Factor							GCAP Seasonal Factor						
128%							128%						
GCAP Growth Factor							GCAP Growth Factor						
146%							146%						

Weekend-GCAP Rate & Seasonal/Growth Factor

Devereux at US 6	165	92	Existing Sat volume at US 6
Devereux at Centennial	82	76	Existing Sat volume at Centennial
Apply to exist GCAP	83	16	9/29/18 From GCAP (Δ Intersections)
		128%	GCAP Seasonal Factor
		146%	GCAP Growth Factor

Background (Future-Existing) GCAP intersection

			GCAP (SB)					
			RIGHT	THRU	LEFT			
				1			15	
LEFT		1	Two Rivers Plaza / Devereux				73	RIGHT
THRU								THRU
RIGHT								LEFT
			LEFT	THRU	RIGHT			

Existing GCAP intersection

			GCAP (SB)					
			RIGHT	THRU	LEFT			
				1			17	
LEFT		1	Two Rivers Plaza / Devereux				84	RIGHT
THRU								THRU
RIGHT								LEFT
			LEFT	THRU	RIGHT			

Future GCAP intersection

			GCAP (SB)					
			RIGHT	THRU	LEFT			
				2			32	
LEFT		2					157	RIGHT
THRU								THRU
RIGHT								LEFT
			LEFT	THRU	RIGHT			

Appendix D

Trip Generation

Appendix #4

Iron Mountain Hot Springs (Phase 2) - TRIP GENERATION TABLE

Land Use	Number of Units	ITE Code	Weekday				212				Weekday Design Hour Traffic			
			Rate	AM Rate	AM Entering	AM Exiting	PM Rate	PM Entering	PM Exiting	Weekday Traffic	AM IN	AM OUT	PM IN	PM OUT
IMHS pool	3	n/a								0	0	0	5	3
Employee Housing	20	220	5.5	0.50	0.12	0.39	0.70	0.44	0.26	110	2	8	9	5
Hotel	100	310	7.02	0.45	0.27	0.18	0.49	0.25	0.24	702	27	18	25	24
Office	9	710	11.5	1.16	1.00	0.16	1.16	0.19	0.97	104	9	1	2	9
GCAP OF Parking	186	n/a	1.97	0.00	0.00	0.00	0.15	0.08	0.08	366	0	0	14	14
TOTAL TRIPS:										1,282	38	27	55	55

ITE Trip Generation Manual, 10th Edition									
Weekday Design Hour Distribution									
			AM IN	AM OUT	PM IN	PM OUT			
IMHS Pool - based upon 2018 Sat. Count (factored)					62%	38%			
ITE Code 220 - Multi-Family Housing (Low-Rise) (20 apartments)			23%	77%	63%	37%			
ITE Code 310 - Hotel (100 rooms)			59%	41%	51%	49%			
ITE Code 710 - General Office (8900 sf)			86%	14%	16%	84%			
GCAP Overflow Parking (occurs at 1200 visitors)					50%	50%			

Iron Mountain Hot Springs (Phase 2) - TRIP GENERATION TABLE

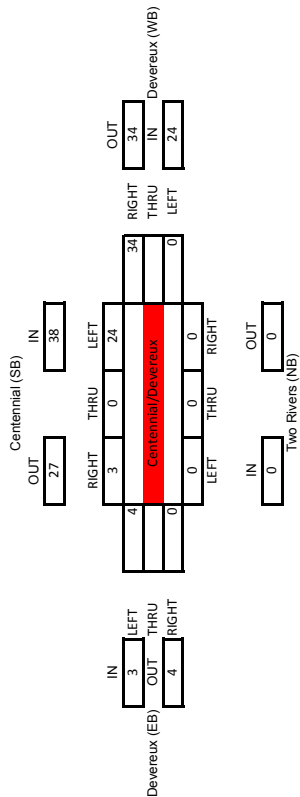
Land Use	Number of Units	ITE Code	Saturday Design Hour Rates				Saturday Design Hour Traffic			
			Sat. Peak Rate	Enter	Exit	OUT	Saturday Traffic	IN	OUT	IN
IMHS pool	3	n/a	4.21	2.59	1.61	8		8	5	
Employee Housing	20	220	0.35	0.35	0.35	7		7	7	
Hotel	100	310	0.72	0.40	0.32	40		40	32	
Office	8	710	0.53	0.29	0.24	2		2	2	
GCAP OF Parking	186	n/a	1.00	0.10	0.05	18	372	18	10	
TOTAL TRIPS:										75
										56

ITE Trip Generation Manual, 10th Edition									
Saturday Design Hour Distribution									
			IN	OUT					
IMHS Pool - based upon 2018 Saturday Count			62%	38%					
ITE Code 220 - MF Housing (20 apartments)			50%	50%					
ITE Code 310 - Hotel (100 rooms)			56%	44%					
ITE Code 710 - General Office (8900 sf)			54%	46%					
GCAP Overflow Parking (occurs at 1200 visitors)			65%	35%					

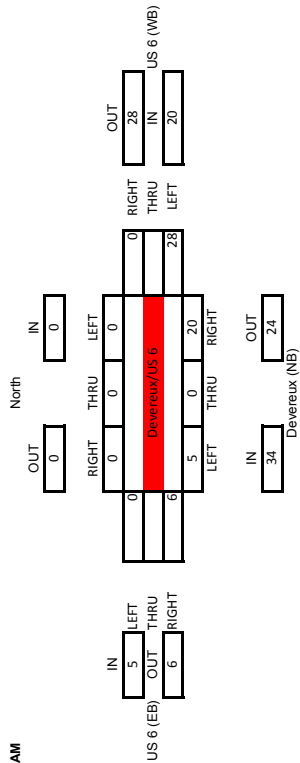
Directional Distribution			Weekday Traffic				Saturday Traffic			
			AM Design Hour	PM Design Hour	Design Hour		AM Design Hour	PM Design Hour	Design Hour	
Centennial at Devereux			In	Out	In	Out	In	Out	In	Out
East	0.9	34	24	50	50	68	50			
	West	0.1	4	3	6	8	6			
Devereux at US 6										
East	0.73	28	20	40	40	55	41			
	West	0.15	6	4	8	11	8			
Traver Trail	0.02	1	1	1	1	2	1			
	TOTAL	38	27	55	55	75	56			

Project Trip Distribution

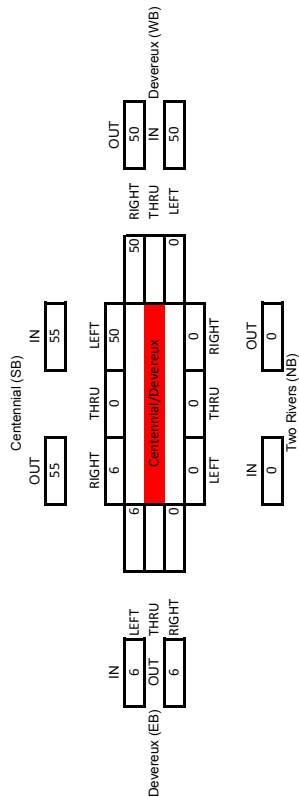
Weekday AM



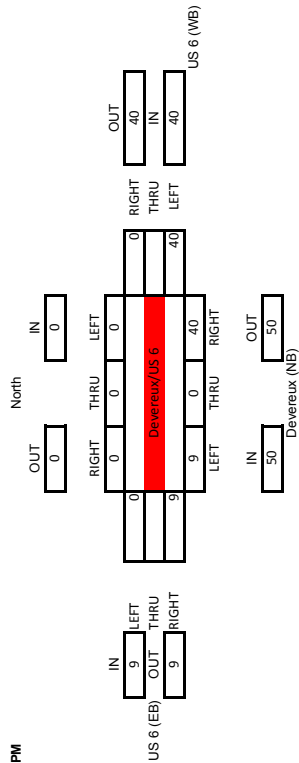
Weekday AM



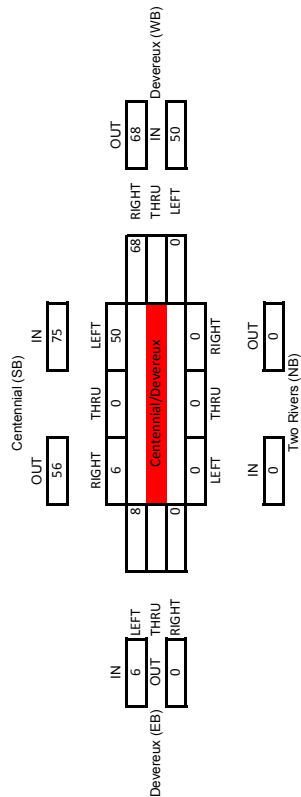
Weekday PM



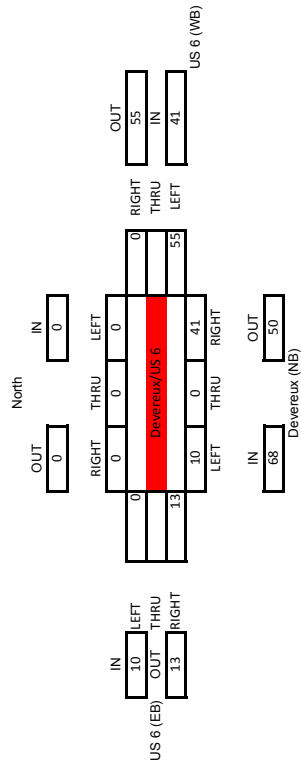
Weekday PM



Saturday



Saturday



Appendix E

Synchro /SimTraffic output

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/02/2019

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	13	8	3	11	15	31	1	0	8	66	1	29
Future Vol, veh/h	13	8	3	11	15	31	1	0	8	66	1	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	9	3	12	16	34	1	0	9	72	1	32

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	50	0	0	12	0	0	97	113	11	100	97	33
Stage 1	-	-	-	-	-	-	39	39	-	57	57	-
Stage 2	-	-	-	-	-	-	58	74	-	43	40	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1557	-	-	1607	-	-	885	777	1070	881	793	1041
Stage 1	-	-	-	-	-	-	976	862	-	955	847	-
Stage 2	-	-	-	-	-	-	954	833	-	971	862	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1557	-	-	1607	-	-	846	764	1070	862	780	1041
Mov Cap-2 Maneuver	-	-	-	-	-	-	846	764	-	862	780	-
Stage 1	-	-	-	-	-	-	967	854	-	946	840	-
Stage 2	-	-	-	-	-	-	917	826	-	954	854	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	4			1.4			8.5			8.2		
HCM LOS							A			A		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	1039	1557	-	-	1607	-	-	1233
HCM Lane V/C Ratio	0.009	0.009	-	-	0.007	-	-	0.085
HCM Control Delay (s)	8.5	7.3	0	-	7.3	0	-	8.2
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.3

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/02/2019

Intersection

Int Delay, s/veh 3.5

Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	216	47	68	325	37	153
Future Vol, veh/h	216	47	68	325	37	153
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	237	60	80	342	48	174

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	297
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.12
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.218
Pot Cap-1 Maneuver	-	-	1264
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1264
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	0	1.5	12
HCM LOS			B

Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	356	800	-	-	1264	-
HCM Lane V/C Ratio	0.135	0.217	-	-	0.063	-
HCM Control Delay (s)	16.7	10.7	-	-	8	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	0.5	0.8	-	-	0.2	-

HCM 2010 TWSC
18: Traver Tr & US 6

04/10/2019

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
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Lane Configurations		↔	↑	↗	↘	
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Traffic Vol, veh/h	15	232	330	29	29	16
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Future Vol, veh/h	15	232	330	29	29	16
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Conflicting Peds, #/hr	0	0	0	0	0	0
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Sign Control	Free	Free	Free	Free	Stop	Stop
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RT Channelized	-	None	-	None	-	None
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Storage Length	-	-	-	0	0	-
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Veh in Median Storage, #	-	0	0	-	0	-
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Grade, %	-	1	0	-	0	-
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Peak Hour Factor	92	92	92	92	92	92
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Heavy Vehicles, %	2	2	2	2	2	2
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Mvmt Flow	16	252	359	32	32	17
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Major/Minor	Major1	Major2	Minor2
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Conflicting Flow All	391	0	0	643	359
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Stage 1	-	-	-	359	-
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Stage 2	-	-	-	284	-
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Critical Hdwy	4.12	-	-	6.42	6.22
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Critical Hdwy Stg 1	-	-	-	5.42	-
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Critical Hdwy Stg 2	-	-	-	5.42	-
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Follow-up Hdwy	2.218	-	-	3.518	3.318
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Pot Cap-1 Maneuver	1168	-	-	438	685
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Stage 1	-	-	-	707	-
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Stage 2	-	-	-	764	-
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Platoon blocked, %	-	-	-	-	-
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Mov Cap-1 Maneuver	1168	-	-	431	685
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Mov Cap-2 Maneuver	-	-	-	431	-
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Stage 1	-	-	-	696	-
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Stage 2	-	-	-	764	-
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Approach	EB	WB	SB
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HCM Control Delay, s	0.5	0	13.1
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HCM LOS			B
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Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
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Capacity (veh/h)	1168	-	-	-	496
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HCM Lane V/C Ratio	0.014	-	-	-	0.099
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HCM Control Delay (s)	8.1	0	-	-	13.1
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HCM Lane LOS	A	A	-	-	B
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HCM 95th %tile Q(veh)	0	-	-	-	0.3
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Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	WB	NE	NE
Directions Served	R	L	L	R
Maximum Queue (ft)	9	40	35	61
Average Queue (ft)	0	16	9	5
95th Queue (ft)	4	43	24	28
Link Distance (ft)	282			
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	250	200		60
Storage Blk Time (%)	0			
Queuing Penalty (veh)	0			

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	10	8	26	59
Average Queue (ft)	0	0	6	30
95th Queue (ft)	5	6	24	52
Link Distance (ft)	268	240	217	249
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/03/2019

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	8	17	12	26	13	43	9	1	29	30	0	3
Future Vol, veh/h	8	17	12	26	13	43	9	1	29	30	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	9	18	13	28	14	47	10	1	32	33	0	3
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	61	0	0	31	0	0	137	160	25	153	143	38
Stage 1	-	-	-	-	-	-	43	43	-	94	94	-
Stage 2	-	-	-	-	-	-	94	117	-	59	49	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1542	-	-	1582	-	-	834	732	1051	814	748	1034
Stage 1	-	-	-	-	-	-	971	859	-	913	817	-
Stage 2	-	-	-	-	-	-	913	799	-	953	854	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1542	-	-	1582	-	-	816	714	1051	774	730	1034
Mov Cap-2 Maneuver	-	-	-	-	-	-	816	714	-	774	730	-
Stage 1	-	-	-	-	-	-	965	854	-	908	802	-
Stage 2	-	-	-	-	-	-	894	785	-	918	849	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.6			2.3			8.9			9.4		
HCM LOS							A			A		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	974	1542	-	-	1582	-	-	851				
HCM Lane V/C Ratio	0.044	0.006	-	-	0.018	-	-	0.042				
HCM Control Delay (s)	8.9	7.3	0	-	7.3	0	-	9.4				
HCM Lane LOS	A	A	A	-	A	A	-	A				
HCM 95th %tile Q(veh)	0.1	0	-	-	0.1	-	-	0.1				

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/03/2019

Intersection

Int Delay, s/veh 2.7

Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	236	55	110	258	12	80
Future Vol, veh/h	236	55	110	258	12	80
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	259	71	129	272	16	91

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	330
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.12
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.218
Pot Cap-1 Maneuver	-	-	1229
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1229
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	0	2.7	11.2
HCM LOS			B

Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	318	777	-	-	1229	-
HCM Lane V/C Ratio	0.049	0.117	-	-	0.105	-
HCM Control Delay (s)	16.9	10.2	-	-	8.3	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	0.2	0.4	-	-	0.4	-

Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	WB	NE	NE
Directions Served	R	L	L	R
Maximum Queue (ft)	4	62	23	15
Average Queue (ft)	0	25	4	1
95th Queue (ft)	3	55	14	7
Link Distance (ft)	282			
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	250	200		60
Storage Blk Time (%)	0			
Queuing Penalty (veh)	0			

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	20	3	53	58
Average Queue (ft)	1	0	21	20
95th Queue (ft)	8	3	45	47
Link Distance (ft)	268	240	217	249
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/05/2019

Intersection												
Int Delay, s/veh	5.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	14	11	3	11	18	44	1	0	8	74	0	30
Future Vol, veh/h	14	11	3	11	18	44	1	0	8	74	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	15	12	3	12	20	48	1	0	9	80	0	33
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	68	0	0	15	0	0	112	136	14	116	113	44
Stage 1	-	-	-	-	-	-	44	44	-	68	68	-
Stage 2	-	-	-	-	-	-	68	92	-	48	45	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1533	-	-	1603	-	-	866	755	1066	861	777	1026
Stage 1	-	-	-	-	-	-	970	858	-	942	838	-
Stage 2	-	-	-	-	-	-	942	819	-	965	857	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1533	-	-	1603	-	-	827	741	1066	842	763	1026
Mov Cap-2 Maneuver	-	-	-	-	-	-	827	741	-	842	763	-
Stage 1	-	-	-	-	-	-	960	849	-	933	831	-
Stage 2	-	-	-	-	-	-	905	812	-	948	848	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.7			1.1			8.5			8.4		
HCM LOS							A			A		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	1033	1533	-	-	1603	-	-	1183				
HCM Lane V/C Ratio	0.009	0.01	-	-	0.007	-	-	0.096				
HCM Control Delay (s)	8.5	7.4	0	-	7.3	0	-	8.4				
HCM Lane LOS	A	A	A	-	A	A	-	A				
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.3				

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/05/2019

Intersection						
Int Delay, s/veh	4.1					
Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	225	52	90	338	41	172
Future Vol, veh/h	225	52	90	338	41	172
Conflicting Peds, #/hr	0	20	0	0	2	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	247	67	106	356	53	195
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	334	0	837	267
Stage 1	-	-	-	-	267	-
Stage 2	-	-	-	-	570	-
Critical Hdwy	-	-	4.12	-	6.45	6.23
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.218	-	3.545	3.327
Pot Cap-1 Maneuver	-	-	1225	-	333	769
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	560	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1205	-	298	756
Mov Cap-2 Maneuver	-	-	-	-	298	-
Stage 1	-	-	-	-	691	-
Stage 2	-	-	-	-	559	-
Approach	EB		WB		NE	
HCM Control Delay, s	0		1.9		13.2	
HCM LOS	B					
Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	298	756	-	-	1205	-
HCM Lane V/C Ratio	0.179	0.259	-	-	0.088	-
HCM Control Delay (s)	19.7	11.4	-	-	8.3	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	0.6	1	-	-	0.3	-

HCM 2010 TWSC
18: Traver Tr & US 6

04/10/2019

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	15	245	346	29	29	16
Future Vol, veh/h	15	245	346	29	29	16
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	0	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	1	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	266	376	32	32	17
Major/Minor	Major1	Major2		Minor2		
Conflicting Flow All	408	0	-	0	674	376
Stage 1	-	-	-	-	376	-
Stage 2	-	-	-	-	298	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1151	-	-	-	420	670
Stage 1	-	-	-	-	694	-
Stage 2	-	-	-	-	753	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1151	-	-	-	413	670
Mov Cap-2 Maneuver	-	-	-	-	413	-
Stage 1	-	-	-	-	683	-
Stage 2	-	-	-	-	753	-
Approach	EB	WB		SB		
HCM Control Delay, s	0.5	0		13.4		
HCM LOS	B					
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1151	-	-	-	-	478
HCM Lane V/C Ratio	0.014	-	-	-	-	0.102
HCM Control Delay (s)	8.2	0	-	-	-	13.4
HCM Lane LOS	A	A	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	-	0.3

Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	EB	WB	WB	NE	NE
Directions Served	T	R	L	T	L	R
Maximum Queue (ft)	15	17	58	11	77	72
Average Queue (ft)	1	1	19	0	12	10
95th Queue (ft)	8	9	48	8	38	40
Link Distance (ft)	296			267	282	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)		250	200			60
Storage Blk Time (%)					0	0
Queuing Penalty (veh)					1	0

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	NB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	10	26	62
Average Queue (ft)	1	7	29
95th Queue (ft)	6	26	51
Link Distance (ft)	268	217	249
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 1

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/05/2019

Intersection

Int Delay, s/veh 4.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	9	17	12	26	13	47	9	1	29	33	0	3
Future Vol, veh/h	9	17	12	26	13	47	9	1	29	33	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	10	18	13	28	14	51	10	1	32	36	0	3

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	65	0	0	31	0	0	141	166	25	157	147	40
Stage 1	-	-	-	-	-	-	45	45	-	96	96	-
Stage 2	-	-	-	-	-	-	96	121	-	61	51	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1537	-	-	1582	-	-	829	727	1051	809	744	1031
Stage 1	-	-	-	-	-	-	969	857	-	911	815	-
Stage 2	-	-	-	-	-	-	911	796	-	950	852	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1537	-	-	1582	-	-	811	709	1051	769	725	1031
Mov Cap-2 Maneuver	-	-	-	-	-	-	811	709	-	769	725	-
Stage 1	-	-	-	-	-	-	962	851	-	905	800	-
Stage 2	-	-	-	-	-	-	892	782	-	914	846	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.7			2.2			8.9			9.5		
HCM LOS							A			A		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	973	1537	-	-	1582	-	-	839
HCM Lane V/C Ratio	0.044	0.006	-	-	0.018	-	-	0.047
HCM Control Delay (s)	8.9	7.4	0	-	7.3	0	-	9.5
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0.1	0	-	-	0.1	-	-	0.1

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/05/2019

Intersection

Int Delay, s/veh 3.3

Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	245	66	167	268	14	94
Future Vol, veh/h	245	66	167	268	14	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	269	85	196	282	18	107

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	354
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.12
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.218
Pot Cap-1 Maneuver	-	-	1205
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1205
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	0	3.5	12.1
HCM LOS			B

Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	241	767	-	-	1205	-
HCM Lane V/C Ratio	0.075	0.139	-	-	0.163	-
HCM Control Delay (s)	21.2	10.5	-	-	8.6	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	0.2	0.5	-	-	0.6	-

Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	WB	NE	NE
Directions Served	R	L	L	R
Maximum Queue (ft)	9	82	21	12
Average Queue (ft)	0	34	3	1
95th Queue (ft)	6	66	14	7
Link Distance (ft)	282			
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	250	200	60	
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	20	10	54	50
Average Queue (ft)	1	0	21	21
95th Queue (ft)	10	4	44	45
Link Distance (ft)	268	240	217	249
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/02/2019

Intersection

Int Delay, s/veh 5.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	20	11	3	11	18	93	1	0	8	124	0	35
Future Vol, veh/h	20	11	3	11	18	93	1	0	8	124	0	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	12	3	12	20	101	1	0	9	135	0	38

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	121	0	0	15	0	0	153	203	14	157	154	71
Stage 1	-	-	-	-	-	-	58	58	-	95	95	-
Stage 2	-	-	-	-	-	-	95	145	-	62	59	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1467	-	-	1603	-	-	814	693	1066	809	738	991
Stage 1	-	-	-	-	-	-	954	847	-	912	816	-
Stage 2	-	-	-	-	-	-	912	777	-	949	846	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1467	-	-	1603	-	-	769	677	1066	789	721	991
Mov Cap-2 Maneuver	-	-	-	-	-	-	769	677	-	789	721	-
Stage 1	-	-	-	-	-	-	940	834	-	898	809	-
Stage 2	-	-	-	-	-	-	870	771	-	927	833	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.4			0.7			8.6			9.3		
HCM LOS							A			A		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	1022	1467	-	-	1603	-	-	1012
HCM Lane V/C Ratio	0.01	0.015	-	-	0.007	-	-	0.171
HCM Control Delay (s)	8.6	7.5	0	-	7.3	0	-	9.3
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.6

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/03/2019

Intersection						
Int Delay, s/veh	5					
Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	225	61	130	338	50	212
Future Vol, veh/h	225	61	130	338	50	212
Conflicting Peds, #/hr	0	20	0	0	2	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	247	78	153	356	65	241
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	345	0	931	267
Stage 1	-	-	-	-	267	-
Stage 2	-	-	-	-	664	-
Critical Hdwy	-	-	4.12	-	6.45	6.23
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.218	-	3.545	3.327
Pot Cap-1 Maneuver	-	-	1214	-	293	769
Stage 1	-	-	-	-	771	-
Stage 2	-	-	-	-	506	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1194	-	251	756
Mov Cap-2 Maneuver	-	-	-	-	251	-
Stage 1	-	-	-	-	661	-
Stage 2	-	-	-	-	505	-
Approach	EB		WB		NE	
HCM Control Delay, s	0		2.5		14.6	
HCM LOS	B					
Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	251	756	-	-	1194	-
HCM Lane V/C Ratio	0.259	0.319	-	-	0.128	-
HCM Control Delay (s)	24.3	12	-	-	8.5	-
HCM Lane LOS	C	B	-	-	A	-
HCM 95th %tile Q(veh)	1	1.4	-	-	0.4	-

HCM 2010 TWSC
18: Traver Tr & US 6

04/10/2019

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↑	↗	↘	
Traffic Vol, veh/h	15	253	354	30	30	16
Future Vol, veh/h	15	253	354	30	30	16
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	0	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	1	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	275	385	33	33	17

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	418	0	0 692 385
Stage 1	-	-	- 385 -
Stage 2	-	-	- 307 -
Critical Hdwy	4.12	-	- 6.42 6.22
Critical Hdwy Stg 1	-	-	- 5.42 -
Critical Hdwy Stg 2	-	-	- 5.42 -
Follow-up Hdwy	2.218	-	- 3.518 3.318
Pot Cap-1 Maneuver	1141	-	- 410 663
Stage 1	-	-	- 688 -
Stage 2	-	-	- 746 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1141	-	- 403 663
Mov Cap-2 Maneuver	-	-	- 403 -
Stage 1	-	-	- 676 -
Stage 2	-	-	- 746 -

Approach	EB	WB	SB
HCM Control Delay, s	0.5	0	13.6
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1141	-	-	-	467
HCM Lane V/C Ratio	0.014	-	-	-	0.107
HCM Control Delay (s)	8.2	0	-	-	13.6
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.4

Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	EB	WB	WB	NE	NE
Directions Served	T	R	L	T	L	R
Maximum Queue (ft)	11	20	70	25	94	82
Average Queue (ft)	1	1	29	1	17	13
95th Queue (ft)	10	11	59	14	53	48
Link Distance (ft)	296			267	282	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)		250	200			60
Storage Blk Time (%)					1	1
Queuing Penalty (veh)					1	0

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	NB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	20	26	70
Average Queue (ft)	2	6	36
95th Queue (ft)	13	24	57
Link Distance (ft)	268	217	249
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 2

HCM 2010 TWSC

10: Devereux Rd & Centennial Dr

04/05/2019

Intersection												
Int Delay, s/veh	4.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	16	17	12	26	13	115	9	1	29	83	0	9
Future Vol, veh/h	16	17	12	26	13	115	9	1	29	83	0	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Stop
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	18	13	28	14	125	10	1	32	90	0	10

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	139	0	0	31	0	0	192	254	25	208	198	77
Stage 1	-	-	-	-	-	-	59	59	-	133	133	-
Stage 2	-	-	-	-	-	-	133	195	-	75	65	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1445	-	-	1582	-	-	768	650	1051	749	698	984
Stage 1	-	-	-	-	-	-	953	846	-	870	786	-
Stage 2	-	-	-	-	-	-	870	739	-	934	841	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1445	-	-	1582	-	-	743	630	1051	709	676	984
Mov Cap-2 Maneuver	-	-	-	-	-	-	743	630	-	709	676	-
Stage 1	-	-	-	-	-	-	942	836	-	860	771	-
Stage 2	-	-	-	-	-	-	845	725	-	894	831	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.7			1.2			9			10.2		
HCM LOS							A			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	944	1445	-	-	1582	-	-	786
HCM Lane V/C Ratio	0.045	0.012	-	-	0.018	-	-	0.127
HCM Control Delay (s)	9	7.5	0	-	7.3	0	-	10.2
HCM Lane LOS	A	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0.1	-	-	0.4

HCM 2010 TWSC

6: Devereux & US 6/6th St

04/02/2019

Intersection

Int Delay, s/veh 4.4

Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑	↗	↘	↑	↘	↗
Traffic Vol, veh/h	245	79	222	268	24	135
Future Vol, veh/h	245	79	222	268	24	135
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	250	200	-	0	60
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	78	85	95	77	88
Heavy Vehicles, %	2	2	2	3	5	3
Mvmt Flow	269	101	261	282	31	153

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	370
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.12
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.218
Pot Cap-1 Maneuver	-	-	1189
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1189
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	0	4.3	13.8
HCM LOS			B

Minor Lane/Major Mvmt	NELn1	NELn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	188	767	-	-	1189	-
HCM Lane V/C Ratio	0.166	0.2	-	-	0.22	-
HCM Control Delay (s)	27.9	10.9	-	-	8.9	-
HCM Lane LOS	D	B	-	-	A	-
HCM 95th %tile Q(veh)	0.6	0.7	-	-	0.8	-

Queuing and Blocking Report

04/06/2019

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Intersection: 6: Devereux & US 6/6th St

Movement	EB	EB	WB	NE	NE
Directions Served	T	R	L	L	R
Maximum Queue (ft)	11	22	96	21	66
Average Queue (ft)	0	1	41	7	5
95th Queue (ft)	5	9	78	19	29
Link Distance (ft)	296			282	
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		60
Storage Blk Time (%)				0	0
Queuing Penalty (veh)				0	0

Intersection: 10: Devereux Rd & Centennial Dr

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	25	8	55	58
Average Queue (ft)	2	0	19	33
95th Queue (ft)	13	5	43	51
Link Distance (ft)	268	240	217	249
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Network Summary

Network wide Queuing Penalty: 0

Appendix F

2018 Traffic Counts

Appendix #6



(303) 216-2439
www.alltrafficdata.net

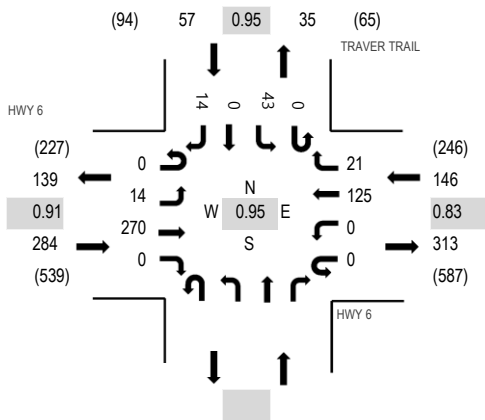
Location: 1 TRAVER TRAIL & HWY 6 AM

Date and Start Time: Wednesday, June 13, 2018

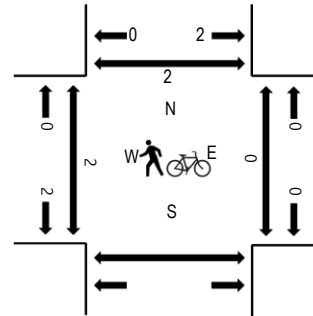
Peak Hour: 07:45 AM - 08:45 AM

Peak 15-Minutes: 08:00 AM - 08:15 AM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Traffic Counts

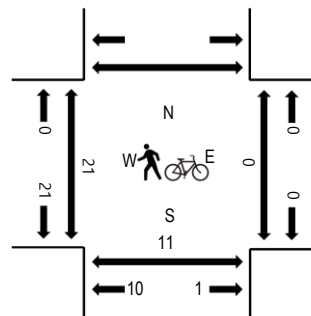
Interval Start Time	HWY 6 Eastbound				HWY 6 Westbound				Northbound				TRAVER TRAIL Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
7:00 AM	0	2	57	0	0	0	0	17	2				0	5	0	2	85	394	0	0		0
7:15 AM	0	1	71	0	0	0	0	22	3				0	7	0	0	104	437	0	0		2
7:30 AM	0	3	53	0	0	0	0	16	4				0	10	0	1	87	460	0	0		0
7:45 AM	0	2	76	0	0	0	0	25	1				0	7	0	7	118	487	0	0		0
8:00 AM	0	2	64	0	0	0	0	40	7				0	13	0	2	128	485	1	0		0
8:15 AM	0	4	74	0	0	0	0	29	5				0	11	0	4	127		0	0		0
8:30 AM	0	6	56	0	0	0	0	31	8				0	12	0	1	114		0	0		1
8:45 AM	0	6	62	0	0	0	0	27	9				0	9	0	3	116		0	0		0
Count Total	0	26	513	0	0	0	0	207	39				0	74	0	20	879		1	0		3
Peak Hour	0	14	270	0	0	0	0	125	21				0	43	0	14	487		1	0		1



www.alltrafficdata.net

Peak 15-Minutes: 08:00 AM - 08:15 AM

Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Traffic Counts

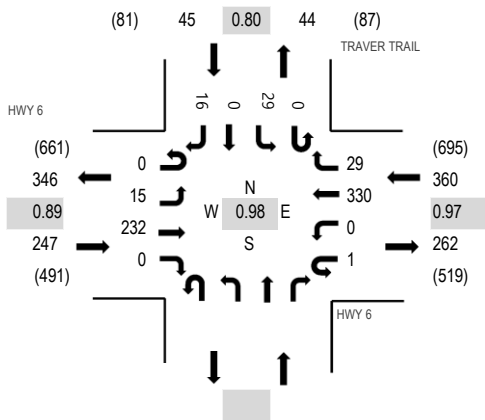
Interval Start Time	HWY 6 Eastbound				HWY 6 Westbound				DEVEREUX RD Northbound				Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
7:00 AM	0	0	61	2	0	9	18	0	0	1	0	7					98	469	0	0	4	
7:15 AM	0	0	72	3	0	6	24	0	0	1	0	8					114	525	3	0	4	
7:30 AM	0	0	57	8	0	15	19	0	0	1	0	9					109	545	0	0	3	
7:45 AM	0	0	70	14	0	30	26	0	0	0	0	8					148	560	6	0	0	
8:00 AM	0	0	71	9	0	17	45	0	0	5	0	7					154	544	2	0	1	
8:15 AM	0	0	78	7	0	12	31	0	0	1	0	5					134		0	0	0	
8:30 AM	0	0	57	11	0	9	35	0	0	4	0	8					124		11	0	9	
8:45 AM	0	0	61	10	0	19	33	0	0	3	0	6					132		8	0	3	
Count Total	0	0	527	64	0	117	231	0	0	16	0	58					1,013		30	0	24	
Peak Hour	0	0	276	41	0	68	137	0	0	10	0	28					560		19	0	10	



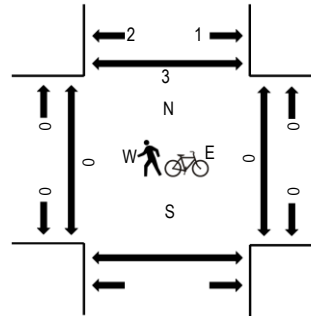
(303) 216-2439
www.alltrafficdata.net

Location: 1 TRAVER TRAIL & HWY 6 PM
Date and Start Time: Wednesday, June 13, 2018
Peak Hour: 04:45 PM - 05:45 PM
Peak 15-Minutes: 04:45 PM - 05:00 PM

Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

Traffic Counts

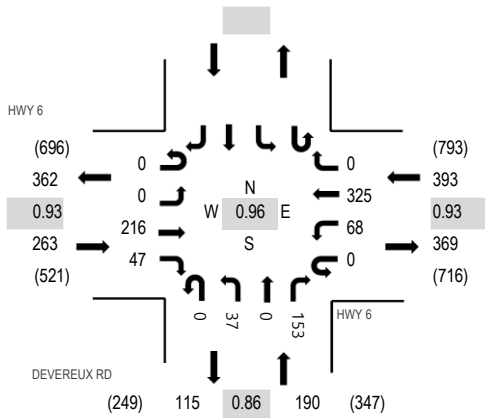
Interval Start Time	HWY 6 Eastbound				HWY 6 Westbound				Northbound				TRAVER TRAIL Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
4:00 PM	0	4	66	0	1	0	74	9					0	5	0	3	162	619	0	0		0
4:15 PM	0	3	64	0	1	0	67	8					0	2	0	4	149	617	0	0		1
4:30 PM	0	3	45	0	0	0	78	5					0	8	0	3	142	633	2	0		0
4:45 PM	0	4	61	0	0	0	81	5					0	11	0	4	166	652	0	0		0
5:00 PM	0	6	52	0	1	0	87	6					0	5	0	3	160	648	0	0		0
5:15 PM	0	2	60	0	0	0	81	8					0	9	0	5	165		0	0		3
5:30 PM	0	3	59	0	0	0	81	10					0	4	0	4	161		0	0		0
5:45 PM	0	2	57	0	0	0	83	9					0	8	0	3	162		0	0		1
Count Total	0	27	464	0	3	0	632	60					0	52	0	29	1,267		2	0		5
Peak Hour	0	15	232	0	1	0	330	29					0	29	0	16	652		0	0		3



(303) 216-2439
www.alltrafficdata.net

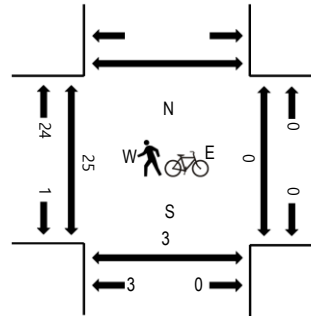
Location: 2 DEVEREUX RD & HWY 6 PM
Date and Start Time: Wednesday, June 13, 2018
Peak Hour: 04:45 PM - 05:45 PM
Peak 15-Minutes: 05:15 PM - 05:30 PM

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.

Peak Hour - Pedestrians/Bicycles on Crosswalk



Traffic Counts

Interval Start Time	HWY 6 Eastbound				HWY 6 Westbound				DEVEREUX RD Northbound				Southbound				Total	Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right			West	East	South	North
4:00 PM	0	0	60	11	0	23	77	0	0	7	0	28					206	819	0	0	5	
4:15 PM	0	0	55	12	0	27	68	0	0	8	0	29					199	814	8	0	0	
4:30 PM	0	0	45	10	0	22	76	0	0	7	0	38					198	835	9	0	3	
4:45 PM	0	0	59	12	0	20	83	0	0	5	0	37					216	846	16	0	0	
5:00 PM	0	0	43	16	0	11	81	0	0	12	0	38					201	842	0	0	2	
5:15 PM	0	0	60	9	0	18	77	0	0	11	0	45					220		1	0	0	
5:30 PM	0	0	54	10	0	19	84	0	0	9	0	33					209		4	0	0	
5:45 PM	0	0	57	8	0	21	86	0	0	5	0	35					212		1	0	0	
Count Total	0	0	433	88	0	161	632	0	0	64	0	283					1,661		39	0	10	
Peak Hour	0	0	216	47	0	68	325	0	0	37	0	153					846		21	0	2	

Study Name Iron Mountain Hot Spring

Start Date 09/29/2018

Start Time 10:00 AM

Site Code 2014-339.001

Location IMHS Entrance

Channel	Direction Outbound		Direction Inbound		15-min Totals	60-min Totals
	Southbound		Northbound			
10:00 AM	3		13		16	62
10:15 AM	1		12		13	58
10:30 AM	5		12		17	69
10:45 AM	7		9		16	73
11:00 AM	1		11		12	69
11:15 AM	13		11		24	
11:30 AM	7		14		21	
11:45 AM	3		9		12	
Peak Hour	28		45		73	
2-hour Total	40		91			

Study Name Iron Mountain Hot Springs

Start Date 09/29/2018

Start Time 10:00 AM

Site Code 2014-339.001

Project

Type Road

Classification Totals

Start Time	Centennial Dr Southbound				Devereux - Westbound				Two Rivers Park Entrance Northbound				Devereux - Eastbound Eastbound				15-min Totals	60-min Totals	Truck/Bus %
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn			
10:00 AM	1	0	2	0	9	2	8	0	5	0	0	2	0	3	6	0	38	150	1.3%
10:15 AM	0	0	1	0	12	0	6	0	1	1	0	0	0	4	2	0	26	147	1.4%
10:30 AM	2	0	5	0	4	6	6	1	3	0	1	0	0	1	5	0	39	180	1.7%
10:45 AM	1	0	8	0	12	5	6	0	6	0	0	2	0	1	4	2	47	192	1.6%
11:00 AM	0	0	4	0	12	3	4	0	5	0	0	2	0	1	3	1	35	186	0.5%
11:15 AM	0	0	14	0	10	0	11	1	8	1	0	2	0	6	4	2	59		
11:30 AM	2	0	4	0	9	5	5	0	10	0	0	3	0	4	6	3	51		
11:45 AM	0	0	4	0	9	5	9	1	6	0	1	1	0	2	3	0	41		
12:00 PM	3	0	30	0	43	13	26	1	29	1	9	0	0	12	17	8	192		
Peak Hour																			

Study Name Iron Mountain Hot Springs

Start Date 09/29/2018

Start Time 10:00 AM

Site Code 2014-339.001

Project

Type Road

Classification Totals

Start Time	HWY-6- Northwest Bound Westbound				Devereux Rd Northbound				HWY-6-Southeast Bound Eastbound				15-min Totals	60-min Totals	Truck/Bus %
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn			
10:00 AM	47	25	0	0	10	3	0	0	12	40	0	0	137	626	2.1%
10:15 AM	61	22	0	0	9	1	0	0	9	50	0	0	152	674	2.2%
10:30 AM	61	24	0	0	14	1	0	0	9	61	0	0	170	732	2.0%
10:45 AM	42	27	0	0	15	12	0	0	15	56	0	0	167	736	1.9%
11:00 AM	67	25	0	0	17	1	0	0	13	62	0	0	185	752	2.3%
11:15 AM	69	30	0	0	29	2	0	0	16	64	0	0	210		
11:30 AM	58	23	0	0	15	5	0	0	15	58	0	0	174		
11:45 AM	64	32	0	0	19	4	1	1	11	52	0	0	183		
12:00 PM															
Peak Hour	258	110	0	0	80	0	12	1	55	236	0	0	752		

Study Name Iron Mtn Hot Springs

Start Date 02/06/2019

Start Time 4:00 PM

Site Code Devereux/US6

Project

Type Road

Classification Totals

Start Time	6th St (SB) Southbound				6th St (NB) Northbound				Devereux Rd Eastbound				15-min Totals	60-min Totals	Truck/Bus %
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn			
7:00 AM	1	45	0	0	0	18	5	0	4	4	0	0	77	595	5.2%
7:15 AM	12	74	0	0	0	33	5	0	6	1	0	0	131	673	3.7%
7:30 AM	5	101	0	0	0	42	13	0	9	1	0	0	171	690	3.0%
7:45 AM	14	126	0	0	0	47	16	0	9	4	0	0	216	632	3.3%
8:00 AM	10	82	0	0	0	38	12	0	12	1	0	0	155	544	4.8%
8:15 AM	6	89	0	0	0	34	12	0	5	2	0	0	148		
8:30 AM	6	67	0	0	0	29	6	0	4	1	0	0	113		
8:45 AM	4	67	0	0	0	34	15	0	6	2	0	0	128		4.0%
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0			
AM Peak Hour	35	398	0	0	0	161	53	0	35	0	8	0	690		
4:00 PM	0	49	0	0	0	61	13	0	13	6	0	0	142	658	3.2%
4:15 PM	5	33	0	0	0	91	10	0	20	4	0	0	163	722	2.6%
4:30 PM	3	56	0	0	0	88	12	0	17	5	0	0	181	715	2.0%
4:45 PM	5	45	0	0	0	82	17	0	20	3	0	0	172	702	2.6%
5:00 PM	4	54	0	0	0	92	17	0	34	5	0	0	206	701	2.7%
5:15 PM	3	48	0	0	0	70	9	0	25	1	0	0	156		
5:30 PM	1	41	0	0	0	91	16	0	14	5	0	0	168		
5:45 PM	4	59	0	0	0	72	13	0	18	5	0	0	171		2.6%
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0			
PM Peak Hour	17	188	0	0	0	353	56	0	91	0	17	0	722		

Study Name Iron Mtn Hot Springs
Start Date 02/06/2019
Start Time 4:00 PM
Site Code Centennial/Devereux
Project

Type Road

Classification Totals

Start Time	Centennial St Southbound			Devereux (WB) Westbound			Two Rivers Access Northbound			Devereux (EB) Eastbound			15-min Totals	60-min Totals	Truck/Bus %
	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left			
7:00 AM	1	0	3	0	5	0	1	0	0	0	0	1	3	0	93
7:15 AM	1	0	1	0	5	2	0	1	0	0	0	5	6	0	114
7:30 AM	0	0	2	0	4	3	1	0	1	0	0	4	7	0	22
7:45 AM	0	0	5	0	13	4	1	0	0	0	0	5	7	0	22
8:00 AM	1	0	3	0	16	2	1	0	0	0	0	7	5	0	35
8:15 AM	2	0	2	0	9	3	0	0	0	0	0	1	6	0	35
8:30 AM	2	0	1	0	8	2	0	0	0	0	0	2	3	0	23
8:45 AM	0	0	5	0	14	0	0	0	0	1	0	0	4	0	19
9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24
AM Peak Hour	3	0	12	0	42	12	3	0	1	0	0	17	25	0	115
4:00 PM	2	0	9	0	8	5	2	0	4	0	0	1	4	0	35
4:15 PM	2	0	14	0	6	3	1	0	1	0	0	3	2	0	33
4:30 PM	6	0	12	0	7	2	3	0	3	0	0	2	2	0	186
4:45 PM	6	0	13	0	7	6	3	0	4	0	1	1	4	0	37
5:00 PM	15	1	27	0	11	4	4	0	0	0	0	2	5	0	46
5:15 PM	6	0	14	0	3	1	1	0	3	0	1	1	1	0	70
5:30 PM	3	0	7	0	5	8	1	0	3	0	1	6	4	0	31
5:45 PM	0	0	11	0	5	5	0	0	2	0	0	2	0	0	38
6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	25
PM Peak Hour	29	1	66	0	31	15	11	0	8	0	1	3	8	13	186

Appendix G

CDOT Access Permit 314157

Appendix #7



COLORADO

Department of Transportation

Transportation Systems
Management & Operations

Region 3 Traffic Section
222 S. Sixth St., Room 100
Grand Junction, Colorado 81501
(970) 683-6284 Fax:(970) 683-6290

February 10, 2015

Lee Barger
SGM, Inc.
118 West 6th, Suite 200
Glenwood Springs, CO 81601

RE: State Highway Access Permit No. 314157, Located on Highway 006, Milepost 0.0, in Garfield County.

Dear Permittee or Applicant:

The Colorado Department of Transportation (CDOT) has received your signed permit and application fee. A copy of the issued permit is enclosed. The next step in the CDOT access permitting process is for you (Applicant) to obtain a Notice to Proceed (NTP). ***Failure to obtain an approved Notice to Proceed prior to any construction will be a violation of the State Highway Access Code (2 CCR 601-1,"the Code") § 2.4.***

Notice to Proceed Information

Well in advance of construction, the Applicant shall request a NTP in writing along with submitting other items, such as construction drawings, specifications, and other required documents to CDOT. The Applicant must submit a complete packet of this information to CDOT with their written request. If the Applicant chooses not to request the NTP, the permit expires pursuant to subsection 2.3(11)(d).

CDOT has seven days to determine if the NTP submittal is complete for review and then notify the applicant of any deficiencies. If complete, CDOT will review and comment on the submitted information within 30-days. If CDOT determines the information is unacceptable, missing, or in need of correction, the Applicant shall correct their submittal and resubmit the complete request for NTP.

Once resubmitted, CDOT will review the revised NTP documents within 10-days. If the revised documents are satisfactory, CDOT will issue a NTP. If further corrections are necessary, the cycle of submittal, review and comments will repeat itself until approval is granted and the NTP is issued.

The request for NTP shall include the following documents, along with any other items specified in the Terms and Conditions of your permit:

- 1) **Cover Letter Requesting a NTP** (include firm name, PE name and contact number)
- 2) **Traffic Control Plan (2 copies)**

The traffic control plan must be:

- A. Consistent with CDOT Standard Plans Manual for Maintenance and Signing
- B. Consistent with the MUTCD



- C. Prepared by individual with American Traffic Safety Services Association (ATSSA) or Colorado Contractors Association certification – or sealed (stamped) by a Colorado registered professional engineer
- D. Acceptable to CDOT prior to any construction within the right-of-way
- E. Presented in a manner that provides a method of handling traffic (MHT) for each different phase of construction. The MHT will describe proposed construction phasing and will include dimensioned diagrams of work zone elements.

The final traffic control plan must be submitted a minimum of three working days in advance of construction. Such plans may be revised as necessary with CDOT concurrence.

3) Insurance Liability Certification

The Applicant or contractor shall be required to provide a comprehensive general liability and property damage insurance naming CDOT as an additional insured party, in the amounts of not less than \$1,000,000 per occurrence and automobile liability insurance of \$1,000,000 combined single limit bodily injury and property damage for each accident, during the period of access construction.

4) Complete Construction Plans

The Applicant shall provide two copies of 11" x 17" construction plans and specifications for the proposed improvements. The plans shall:

- A. Address, as applicable, geometry, drainage, striping, signing, and signalization
- B. Include, but not limited to, layout of the access, highway improvements, utility locations, present and proposed drainage, present and proposed right-of-way lines, present and proposed traffic control devices, and clear zone analysis
- C. Sealed by a Colorado Professional Engineer in accordance with CRS 12-25-117
- D. Conform to the requirement of the permit terms and conditions
- E. If applicable, include the following statement on the cover page of the plans: "This design is in full compliance with Section 4 of the State Highway Access Code, 2 CCR 601-1 except for the following approved design waivers:"

Feel free to contact me with any questions you might have.

Respectfully,



Dan Roussin
Region 3 Access Manager

COLORADO DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ACCESS PERMIT

CDOT Permit No. **314157**

ck# 1076

State Highway No/Mp/Side
006 K / .000 / R

Permit Fee
\$100.00

Date of transmittal
1/27/2015

Region/Section/Patrol
3 / / 12-2 Alan Hayes

Local Jurisdiction
Glenwood Springs

The Permittee(s);

Iron Mountain Hot Springs, LLC
Steve Beckley
51000 Two Rivers Plaza Road
Glenwood Springs, CO 81601
970-618-7127

Applicant:

SGM, Inc.
Lee Barger
118 West 6th, Suite 200
Glenwood Springs, CO 81601
970-945-1004

Ref No.: I

is hereby granted permission to have an access to the state highway at the location noted below. The access shall be constructed, maintained and used in accordance with this permit, including the State Highway Access Code and any attachments, terms, conditions and exhibits. This permit may be revoked by the issuing authority if at any time the permitted access and its use violate any parts of this permit. The issuing authority, the Department and their duly appointed agents and employees shall be held harmless against any action for personal injury or property damage sustained by reason of the exercise of the permit.

Location: It is the start of SH 006K also known as Devereux Road.

Access to Provide Service to: (Land Use Code:)
999 - City Street - Devereux Road

(Size or Count) (Units)

Additional Information:

MUNICIPALITY OR COUNTY APPROVAL

Required only when the appropriate local authority retains issuing authority.

Signature	Print Name	Title	Date

Upon the signing of this permit the permittee agrees to the terms and conditions and referenced attachments contained herein. All construction shall be completed in an expeditious and safe manner and shall be finished within 45 days from Initiation. The permitted access shall be completed in accordance with the terms and conditions of the permit prior to being used.

The permittee shall notify Les Stanton 2K2 with the Colorado Department of Transportation in Grand Junction, Colorado at (970) 384-3357, at least 48 hours prior to commencing construction within the State Highway right-of-way.

The person signing as the permittee must be the owner or legal representative of the property served by the permitted access and have full authority to accept the permit and its terms and conditions.

Permittee Signature	Print Name	Date
<i>Yael Cooper, Manager</i>	Yael Cooper	2/6/15
Iron Mountain Hot Springs LLC		

This permit is not valid until signed by a duly authorized representative of the Department.
COLORADO DEPARTMENT OF TRANSPORTATION

Signature	Print Name	Title	Date (of issue)
<i>David Robinson</i>	David Robinson	Permit Manager	2-10-15

Copy Distribution:

Required:
1.Region
2.Applicant

3.Staff Access Section
4.Central Files

Make copies as necessary for:
Local Authority
MTCE Patrol
Inspector
Traffic Engineer

Previous editions are obsolete and may not be used
Page 1 of 3 CDOT Form #101 5/07

The following paragraphs are excerpts of the State Highway Access Code. These are provided for your convenience but do not alleviate compliance with all sections of the Access Code. A copy of the State Highway Access Code is available from your local issuing authority (local government) or the Colorado Department of Transportation (Department). When this permit was issued, the issuing authority made its decision based in part on information submitted by the applicant, on the access category which is assigned to the highway, what alternative access to other public roads and streets is available, and safety and design standards. Changes in use or design not approved by the permit or the issuing authority may cause the revocation or suspension of the permit.

APPEALS

1. Should the permittee or applicant object to the denial of a permit application by the Department or object to any of the terms or conditions of a permit placed there by the Department, the applicant and permittee (appellant) have a right to appeal the decision to the [Transportation] Commission [of Colorado]. To appeal a decision, submit a request for administrative hearing to the Transportation Commission of Colorado within 60 days of transmittal of notice of denial or transmittal of the permit for signature. Submit the request to the Transportation Commission of Colorado, 4201 East Arkansas Avenue, Denver, Colorado 80222-3400. The request shall include reasons for the appeal and may include changes, revisions, or conditions that would be acceptable to the permittee or applicant.

2. Any appeal by the applicant or permittee of action by a local issuing authority shall be filed with the local authority and be consistent with the appeal procedures of the local authority.

3. In submitting the request for administrative hearing, the appellant has the option of including within the appeal a request for a review by the Department's internal administrative review committee pursuant to [Code] subsection 2.10. When such committee review is requested, processing of the appeal for formal administrative hearing, 2.9(5) and (6), shall be suspended until the appellant notifies the Commission to proceed with the administrative hearing, or the appellant submits a request to the Commission or the administrative law judge to withdraw the appeal. The two administrative processes, the internal administrative review committee, and the administrative hearing, may not run concurrently.

4. Regardless of any communications, meetings, administrative reviews or negotiations with the Department or the internal administrative review Committee regarding revisions or objections to the permit or a denial, if the permittee or applicant wishes to appeal the Department's decision to the Commission for a hearing, the appeal must be brought to the Commission within 60 days of transmittal of notice of denial or transmittal of the permit.

PERMIT EXPIRATION

1. A permit shall be considered expired if the access is not under construction within one year of the permit issue date or before the expiration of any authorized extension. When the permittee is unable to commence construction within one year after the permit issue date, the permittee may request a one year extension from the issuing authority. No more than two one-year extensions may be granted under any circumstances. If the access is not under construction within three years from date of issue the permit will be considered expired. Any request for an extension must be in writing and submitted to the issuing authority before the permit expires. The request should state the reasons why the extension is necessary, when construction is anticipated, and include a copy of page 1 (face of permit) of the access permit. Extension approvals shall be in writing. The local issuing authority shall obtain the concurrence of the Department prior to the approval of an extension, and shall notify the Department of all denied extensions within ten days. Any person wishing to reestablish an access permit that has expired may begin again with the application procedures. An approved Notice to Proceed, automatically renews the access permit for the period of the Notice to Proceed.

CONSTRUCTION

1. Construction may not begin until a Notice to Proceed is approved. (Code subsection 2.4)

2. The construction of the access and its appurtenances as required by the terms and conditions of the permit shall be completed at the expense of the permittee except as provided in subsection 2.14. All materials used in the construction of the access within the highway right-of-way or on permanent easements, become public property. Any materials removed from the highway right-of-way will be disposed of only as directed by the Department. All fencing, guard rail, traffic control devices and other equipment and materials removed in the course of access construction shall be given to the Department unless otherwise instructed by the permit or the Department inspector.

3. The permittee shall notify the individual or the office specified on the permit or Notice to Proceed at least two working days prior to any construction within state highway right-of-way. Construction of the access shall not proceed until both the access permit and the Notice to Proceed are issued. The access shall be completed in an expeditious and safe manner and shall be finished within 45 days from initiation of construction within the highway right-of-way. A construction time extension not to exceed 30 working days may be requested from the individual or office specified on the permit.

4. The issuing authority and the Department may inspect the access during construction and upon completion of the access to ensure that all terms and conditions of the permit are met. Inspectors are authorized to enforce the conditions of the permit during construction and to halt any activities within state right-of-way that do not comply with the provisions of the permit, that conflict with concurrent highway construction or maintenance work, that endanger

highway property, natural or cultural resources protected by law, or the health and safety of workers or the public.

5. Prior to using the access, the permittee is required to complete the construction according to the terms and conditions of the permit. Failure by the permittee to abide by all permit terms and conditions shall be sufficient cause for the Department or issuing authority to initiate action to suspend or revoke the permit and close the access. If in the determination of the Department or issuing authority the failure to comply with or complete the construction requirements of the permit create a highway safety hazard, such shall be sufficient cause for the summary suspension of the permit. If the permittee wishes to use the access prior to completion, arrangements must be approved by the issuing authority and Department and included in the permit. The Department or issuing authority may order a halt to any unauthorized use of the access pursuant to statutory and regulatory powers. Reconstruction or improvement of the access may be required when the permittee has failed to meet required specifications of design or materials. If any construction element fails within two years due to improper construction or material specifications, the permittee shall be responsible for all repairs. Failure to make such repairs may result in suspension of the permit and closure of the access.

6. The permittee shall provide construction traffic control devices at all times during access construction, in conformance with the M.U.T.C.D. as required by section 42-4-104, C.R.S., as amended.

7. A utility permit shall be obtained for any utility work within highway right-of-way. Where necessary to remove, relocate, or repair a traffic control device or public or private utilities for the construction of a permitted access, the relocation, removal or repair shall be accomplished by the permittee without cost to the Department or issuing authority, and at the direction of the Department or utility company. Any damage to the state highway or other public right-of-way beyond that which is allowed in the permit shall be repaired immediately. The permittee is responsible for the repair of any utility damaged in the course of access construction, reconstruction or repair.

8. In the event it becomes necessary to remove any right-of-way fence, the posts on either side of the access shall be securely braced with an approved end post before the fence is cut to prevent any slacking of the remaining fence. All posts and wire removed are Department property and shall be turned over to a representative of the Department.

9. The permittee shall ensure that a copy of the permit is available for review at the construction site at all times. The permit may require the contractor to notify the individual or office specified on the permit at any specified phases in construction to allow the field inspector to inspect various aspects of construction such as concrete forms, subbase, base course compaction, and materials specifications. Minor changes and additions may be ordered by the Department or local authority field inspector to meet unanticipated site conditions.

10. Each access shall be constructed in a manner that shall not cause water to enter onto the roadway or shoulder, and shall not interfere with the existing drainage system on the right-of-way or any adopted municipal system and drainage plan.

11. The Permittee is responsible for obtaining any necessary additional Federal, State and/or City/County permits

or clearances required for construction of the access. Issuance of this access permit does not constitute verification of the above required actions by the Permittee.

By accepting the permit, the permittee stipulates and agrees to fully protect, save, defend, indemnify, and hold harmless, to the extent allowed by law, the issuing Authority, and each of the Authority's directors, officers, employees, agents and representatives, from and against any and all claims, costs (including but not limited to all reasonable fees and charges of engineers, architects, attorneys, and other professionals or expert witnesses and all court or other dispute resolution costs directly incurred by reason of claims directly brought against the Authority), losses, damages, pre- or post-judgment interest, causes of action, suits, or liability of any nature whatsoever by reason of liability imposed due to Permittee's failure to obtain, or disregard of, any applicable federal, state or local environmental permits, approvals, authorizations, or clearances, or in meeting or complying with any applicable federal, state or local environmental law, regulation, condition or requirements in connection with any activities authorized by this Access Permit.

CHANGES IN ACCESS USE AND PERMIT VIOLATIONS

1. It is the responsibility of the property owner and permittee to ensure that the use of the access to the property is not in violation of the Code, permit terms and conditions or the Act. The terms and conditions of any permit are binding upon all assigns, successors-in-interest, heirs and occupants. If any significant changes are made or will be made in the use of the property which will affect access operation, traffic volume and or vehicle type, the permittee or property owner shall contact the local issuing authority or the Department to determine if a new access permit and modifications to the access are required.

2. When an access is constructed or used in violation of the Code, section 43-2-147(5)(c), C.R.S., of the Act applies. The Department or issuing authority may summarily suspend an access permit and immediately order closure of the access when its continued use presents an immediate threat to public health, welfare or safety. Summary suspension shall comply with article 4 of title 24, C.R.S.

MAINTENANCE

1. The permittee, his or her heirs, successors-in-interest, assigns, and occupants of the property serviced by the access shall be responsible for meeting the terms and conditions of the permit, the repair and maintenance of the access beyond the edge of the roadway including any cattle guard and gate, and the removal or clearance of snow or ice upon the access even though deposited on the access in the course of Department snow removal operations. Within unincorporated areas the Department will keep access culverts clean as part of maintenance of the highway drainage system. However, the permittee is responsible for the repair and replacement of any access-related culverts within the right-of-way. Within incorporated areas, drainage responsibilities for municipalities are determined by statute and local ordinance. The Department will maintain the roadway including auxiliary lanes and shoulders, except in those cases where the access installation has failed due to improper access construction and/or failure to follow permit requirements and specifications in which case the permittee shall be responsible for such repair. Any significant repairs such as culvert replacement, resurfacing, or changes in design or specifications, requires authorization from the Department.

STATE HIGHWAY ACCESS PERMIT 314157

January 27, 2015

Located on Highway 006K near RP 0.0 Right
Issued to Iron Mountain Hot Springs, LLC

TERMS AND CONDITIONS

1. This access is approved as constructed in accordance with Section 2.6(3) of the Access Code.
2. This permit replaces #300025 and all additional access permits that may be in existence for this intersection.
3. This permitted access is only for the use and purpose stated in the Application and Permit. This Permit is issued in accordance with the State Highway Access Code (2 CCR 601-1), and is based in part upon the information submitted by the Permittee. Any subsequent relocation, reconstruction, or modifications to the access or changes in the traffic volume or traffic nature using the access shall be requested for by means of a new application. Any changes causing non-compliance with the Access Code may render this permit void, requiring a new permit.
4. The traffic limitations listed on the face of this permit shall be adhered to. The traffic volumes are listed in Design Hourly Volume (DHV) trips or Average Daily Trips (ADT) where entering the site and returning counts as two trips.
5. The traffic volume for driveway is 205 DHV. This does include passenger car equivalents. The access is for Devereux Road and US 6K.
6. When signalization of the intersection becomes justified as per the Manual on Uniform Traffic Control Devices (MUTCD), the Permittee shall apply for a new access permit. Signal design, construction, installation, and operation shall be conducted in accordance with the State Highway Access Code (2 CCR 601-1) and the MUTCD. All cost associated with the installation of traffic signal for this access are the responsibility of the owner of the public road (Devereux Road). This includes but is not limited to the design, construction, utility relocation, testing of materials, and inspection.
7. The Permittee is responsible for obtaining any necessary additional Federal, State and/or City/County permits or clearances required for construction of the access. Approval of this access permit does not constitute verification of this action by the Permittee. Permittee is also responsible for obtaining all necessary utility permits in addition to this access permit.
8. The permittee, his or her heirs, successors-in-interest, assigns, and occupants of the property serviced by the access shall be responsible for meeting the terms and conditions of the permit, the repair and maintenance of the access beyond the edge of the roadway including any cattle guard and gate, and the removal or clearance of snow or ice upon the access even though deposited on the access in the course of Department snow removal operations. Within unincorporated areas the Department will keep access culverts clean as part of maintenance of the highway drainage system. However, the permittee is responsible for the repair and replacement of any access-related culverts within the right-of-way. Within incorporated areas, drainage responsibilities for municipalities are determined by statute and local ordinance. The Department will maintain the roadway including auxiliary lanes and shoulders, except in those cases where the access installation has failed due to improper access construction and/or failure to follow permit requirements and specifications in which case the permittee shall be responsible for such repair. Any significant repair such as culvert replacement, resurfacing, or changes in design or specifications, requires authorization from the Department.

Appendix H

Turnkey Traffic Memo



MEMORANDUM

TO: Steve Beckley, Glenwood Caverns Adventure Park
 FROM: Mark Bunnell, PE, PTOE
 DATE: 12 April 2013
 RE: Iron Mountain PUD – Initial Traffic Evaluation



This memo documents the initial traffic evaluation conducted for the Iron Mountain PUD (Project). The purpose of the initial traffic evaluation was to determine the highway/intersection improvements needed for the full Project build-out. A traffic impact study will be prepared at a later date. The sections below discuss the initial traffic evaluation.

Year 2035 Background Traffic Volumes

- Used CDOT traffic data to develop year 2035 background thru traffic volumes on SH-6.
- Refer to the Attachments to see the CDOT traffic data for SH-6.

Direction	Year 2011 <i>[from July 13, 2011 traffic count conducted by CDOT]</i>		Year 2035 <i>[forecasted using the CDOT 20 year factor of 1.27 (1.20% AGR)]</i>	
	AM	PM	AM	PM
Eastbound	387 vph	442 vph	516 vph	589 vph
Westbound	677 vph	754 vph	902 vph	1,004 vph

AGR – annual growth rate (based on annual compound growth formula)

- Used the CDOT 20 year factor of 1.27 (1.20% AGR) to forecast year 2035 traffic volumes on Devereux Rd and Traver Trail.

Year 2035 Project Traffic

The Project traffic was estimated using the following methodology:

- With the addition of the proposed Project, the maximum attendance capacity for the Glenwood Caverns Adventure Park (GCAP) is 300,000 people per year.
- Used year 2012 attendance numbers to forecast the peak day attendance in year 2035.
- Refer to the Attachments for more Project traffic details and calculations.

Year	GCAP Attendance (# of people)			Growth Factor (Year 2012 to 2035)
	Daily Average <i>(whole year)</i>	Peak Day <i>(single highest day)</i>	Yearly <i>(total)</i>	
2012	1,209	1,770 ¹	165,000	1.82
2035	2,199	3,218	300,000	

¹ – August 4 (Saturday) was the highest attendance day in year 2012

Year 2035 Total Traffic (Background plus Project)

- Year 2035 total traffic consists of background traffic plus Project traffic.
- Refer to the Attachments for details and calculations for Year 2035 Total Traffic Volumes.

Year 2035 Total Traffic (Background Plus Project) SH-6 & Devereux/Traver Trail												
Peak Hour	Eastbound			Westbound			Northbound			Southbound		
	LT	Thru	RT	LT	Thru	RT	LT	Thru	RT	LT	Thru	RT
AM	29	516	38	105	902	96	16	4	71	42	5	12
PM	23	589	19	58	1004	58	30	9	128	62	9	17

Turn Lane Warrants (per CDOT SHAC)

- Used the CDOT State Highway Access Code (SHAC) to determine which turn lanes are warranted based on traffic volumes.
- Speed limit on SH-6 changes just west of Traver Trail; used the following:
 - Eastbound direction – 40 mph
 - Westbound direction – 25 mph

CDOT Turn Lane Warrants (NR-B Access Class, Speed Limit: EB = 40 mph, WB = 25 mph)								
Turn Lane	Turn Lane Required IF DHV is greater than:	Year 2013 (Existing)		Year 2015 (Opening Year)		Year 2035 (Project Build-out)		Turn Lane Already Exists?
		DHV (vph)	Turn Lane Warranted	DHV (vph)	Turn Lane Warranted	DHV (vph)	Turn Lane Warranted	
Deceleration Lanes								
EB Left-turn	25	11	NO	19	NO	29	YES	NO
EB Right-turn	50	36	NO	26	NO	38	NO	YES
WB Left-turn	25	101	YES	74	YES	105	YES	YES ¹
WB Right-turn	50	27	NO	67	YES	96	YES	YES ¹
Acceleration Lanes								
NB to WB Left-turn	Acceleration lanes are generally not required for this access classification, unless it would provide a safety and/or operational benefit.							NO
NB to EB Right-turn								NO
SB to EB Left-turn								NO
SB to WB Right-turn								NO

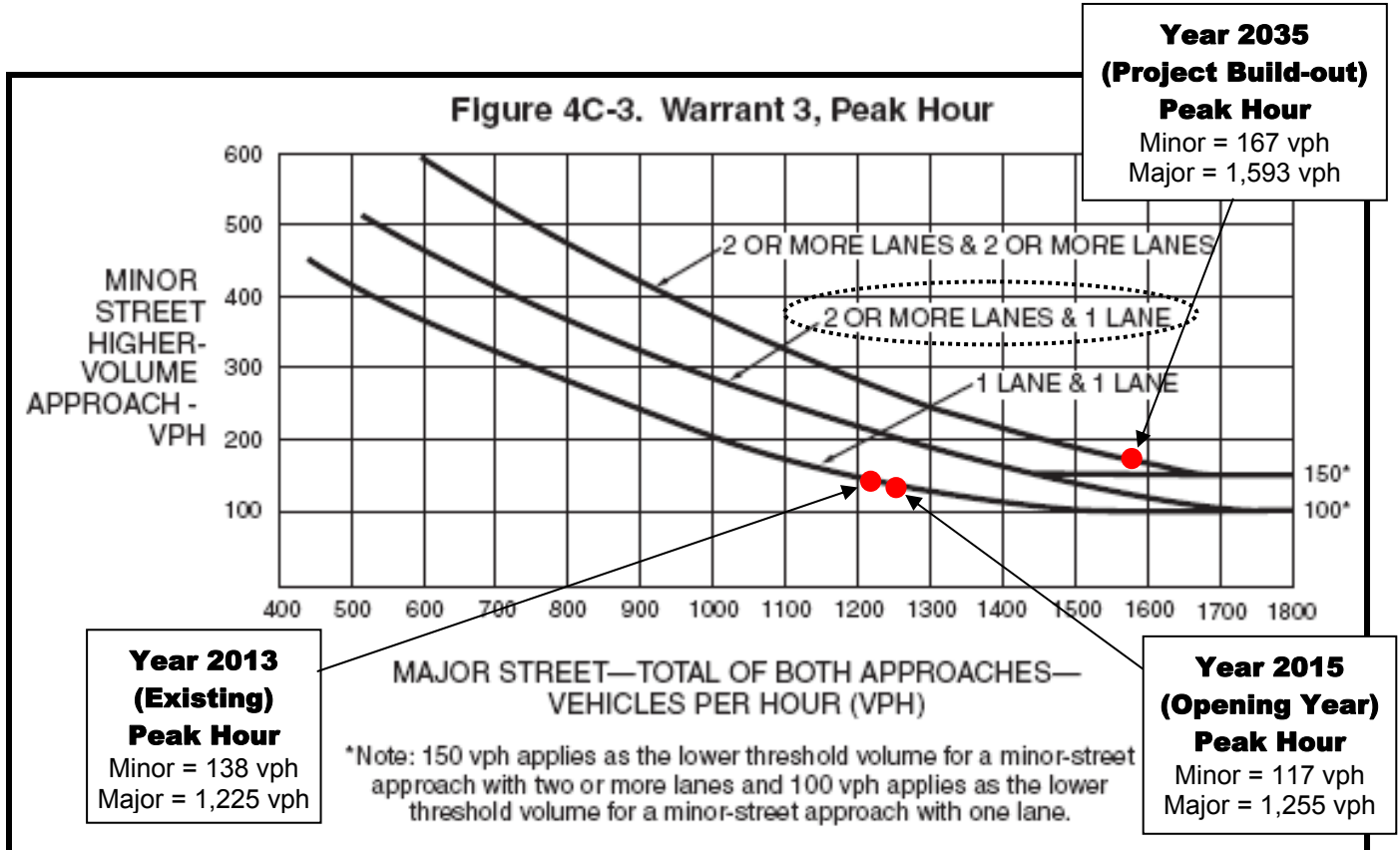
Note

DHV – Design Hour Volume (highest AM or PM peak hour volume)

1 – The existing turn lanes may need to be reconstructed to meet current CDOT standards

Signal Warrants (per MUTCD)

- Peak Hour signal warrant chart below from the Manual on Uniform Traffic Control Devices (MUTCD) shows the peak hour traffic volumes at the SH-6 & Devereux/Traver Trail intersection. Based on the three plotted points it is anticipated that a traffic signal would NOT be warranted (based on the Peak Hour signal warrant) until sometime around year 2025.



Year 2035 Traffic Analysis

- Used Synchro and SimTraffic traffic analysis software to analyze year 2035 traffic volumes for the intersection options listed in the table.
- Refer to the Attachments to see the Synchro and SimTraffic analysis output.

Year 2035 Traffic Analysis Results <i>(Year 2035 Total Traffic – Background Plus Project Traffic)</i>			
Option & Description	LOS – Delay (sec) AM Peak / PM Peak		
	Overall Intersection	Worst Movement (All Mvmts)	SH-6 Thru Mvmt (worst case of EB or WB)
# 1 – Existing Location • Traver Trail & Devereux Rd remain in existing location • RT & LT deceleration lanes on SH-6 as required per CDOT requirements • Install traffic signals (2) at both intersections	B-14 / B-15	C-29 / C-34	B-19 / C-22
#2 – Relocate Traver Trail • Relocate Traver Trail to align with Devereux Rd • RT & LT deceleration lanes on SH-6 as required per CDOT requirements • Install traffic signal (1), which operates with Split Phasing on the side roads • No change to Devereux Rd or Traver Trail	C-26 / C-34	D-43 / D-47	C-31 / D-47
#3 – Relocate & Widen Traver Trail and Restripe Devereux Rd • Relocate Traver Trail to align with Devereux Rd • Widen Traver Trail for a left-turn lane and shared thru/right-turn lane (eliminates the signal Split Phasing) • Restripe Devereux Rd for a left-turn lane and shared thru/right-turn lane (eliminates the signal Split Phasing) • RT & LT deceleration lanes on SH-6 as required per CDOT requirements • Install traffic signal (1), which operates with standard phasing	B-15 / C-23	C-24 / C-31	B-17 / C-31

Conclusions & Recommendations

Turn Lanes on SH-6

- Per the CDOT SHAC, the turn lanes listed below are warranted on SH-6. Two of these lanes currently exist, but may need to be reconstructed to meet current CDOT standards.
 - EB left-turn deceleration lane [reaches warrant threshold sometime around yr 2025]
 - WB left-turn deceleration lane [exceeds warrant threshold now]
 - WB right-turn deceleration lane [exceeds warrant threshold opening year]

Signalization

- It is anticipated that a traffic signal would not be warranted and justified by CDOT until sometime around year 2025.

Intersection Alignment & Traffic Operations

- All three options shown in the previous table provide LOS D or better for all movements.
- Option 1 (Existing Location) operates better than the two relocation options (#2 & #3) because it operates as a three-phase signal instead of a standard four-phase signal.
- Offset intersections allow the northbound (Devereux Rd) and southbound (Traver Trail) traffic to utilize the same signal phase, rather than two phases (NB & SB left-turns followed by NB & SB thru movements) for a standard four-leg intersection.

Recommendations

- Discuss the three options with the City to determine the City's preference.
- Develop construction cost estimates for the three options.

Attachments

CDOT Traffic Data for SH-6



OTIS Online Transportation Information System

Highway Data

Traffic Data

Data Catalog

Reports

Map View

Help

Traffic Data Explorer



Search by city / county

Search by highway segment

Select a route and begin and end ref points. You can select a route from the drop down list or click the pencil icon then click the map. You can enter the ref points into the text boxes or click the pencil icon then click the map.

Zoom to County (optional):

select a county

CDOT Route Number:



006K



Find by Name

Begin Reference:



0



End Reference:



0.338



Count type:



All

Search by traffic station

Stations

AADT

Future Traffic

ESAL

Found 1 stations. Click the magnifying glass icon in in front of a station to see count data below.

Export to Excel

	Route	Start	End	AADT	Year	Derivation	Single Unit	Combination Trucks	% Trucks	20 Year Factor	DHV	DVMT	DD
	006K	0	0.338	12,000	2011	Current Yr	620	140	6.4	1.27	9	3,900	62

ON SH 6, WEST ST W/O SH 82, GLENWOOD SPRINGS (Station Id: 100391)

Daily

Monthly Summaries

07/13/2011

View Entire Month

Export to Excel

Dir	0h	1h	2h	3h	4h	5h	6h	7h	8h	9h	10h	11h	12h	13h	14h	15h	16h	17h	18h	19h	20h	21h	22h	23h
P	28	7	7	14	17	80	231	279	373	298	285	329	387	325	320	318	372	442	365	309	312	422	196	52
S	42	14	13	15	12	81	219	369	490	456	482	588	677	620	581	597	628	754	751	618	476	467	254	84
C	70	21	20	29	29	161	450	648	863	754	767	917	1,064	945	901	915	1,000	1,196	1,116	927	788	889	450	136

P = Primary direction S = Secondary direction C = Combined traffic counts

- System Classification

Export to Excel

Route	Begin Ref	End Ref	Length	Access Control	Admin Class	Functional Class	NHS Designation	Special System
006K	0	0.338	0.325	NR-B: Non Rural Arterial	CDOT Highway	5 Major Collector	0 Not on NHS	NON-STRAHNET

Year 2035 Project Traffic – Calculations & Assumptions

- Used the traffic data collected on August 4, 2012 to forecast year 2035 traffic volumes. The August 4th data was collected or estimated hourly and included the items listed below.
 - Tram ridership
 - Available and occupied parking spaces
 - People arriving and leaving via vehicles or walking
 - Vehicles to the GCAP and hotel

Saturday, August 4, 2012 Trip Data (Year 2012 Peak Day)							
Trips		AM Peak Hour (11 am to 12 pm)			PM Peak Hour (5 pm to 6 pm)		
		GCAP	Hotel	Total	GCAP	Hotel	Total
In	Vehicles	95	0	95	39	12	51
	Pedestrians	86		86	35		35
Out	Vehicles	9	24	33	66	0	66
	Pedestrians	6		6	45		45

- Forecasted year 2035 trips In & Out using the 1.82 growth factor.
- Assumed that patrons arriving by vehicle would split 50/50 between:
 - Existing parking lot (275 parking spaces) & Tram at the Base, and
 - Proposed parking lot (260 parking spaces) & Funicular up at the Quarry.

Year 2035 Peak Day – Vehicles IN & OUT (factor year 2012 peak day to year 2035 using 1.82 growth factor)							
Vehicles	Total		Base / Quarry Split	Tram – Base		Funicular – Quarry	
	AM	PM		AM	PM	AM	PM
IN	173	93	50% / 50%	87	47	86	46
OUT	60	121		30	61	30	60

- Trip distribution percentages for year 2035 Project trips based on traffic counts collected during Spring break on March 26, 2013.

Trip Distribution Percentages (Based on March 26, 2013 traffic counts)						
Period	Inbound			Outbound		
	SH-6		Devereux	SH-6		Devereux
	EB	WB	NB	EB	WB	SB
AM	25%	70%	5%	70%	15%	15%
PM	20%	60%	20%	70%	15%	15%

Traffic Volumes - Existing (2013), Opening Year (2015), and Project Build-out (2035)

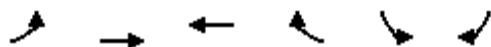
Condition		SH-6 & Devereux/Transfer Trail												Devereux & Two Rivers Plaza					
		Eastbound				Westbound				Northbound				Eastbound		Northbound		Southbound	
		LT	Thru	RT		LT	Thru	RT		LT	Thru	RT		LT	RT	LT	Thru	Thru	RT
AM	Dist %	25.0%				70.0%				15.0%				5.0%				85.0%	
	IN																		
	OUT																		
	Project Trips	24				67				5				5				28	
	Background	5				396				12				47				5	
PM	Dist %	20.0%				60.0%				15.0%				20.0%				85.0%	
	IN																		
	OUT																		
	Trips	10				31				10				10				56	
	Background	11				453				7				82				31	
AM	Dist %	12.5%				35.0%				7.5%				2.5%				42.5%	
	IN																		
	OUT																		
	Project Trips	14				39				3				3				17	
	Background	5				406				12				48				3	
PM	Dist %	10.0%				30.0%				10.0%				10.0%				42.5%	
	IN																		
	OUT																		
	Trips	6				18				6				6				33	
	Background	11				464				8				84				32	
AM	Dist %	12.5%				35.0%				7.5%				2.5%				42.5%	
	IN																		
	OUT																		
	Project Trips	22				61				5				4				26	
	Background	7				516				16				61				5	
PM	Dist %	10.0%				30.0%				10.0%				10.0%				42.5%	
	IN																		
	OUT																		
	Trips	9				28				9				9				51	
	Background	14				589				10				107				40	

Option #1
Existing Location

Year 2035 - Existing Alignment
AM Peak Hour

Iron Mountain PUD - Glenwood Caverns

1: SH-6 & Traver Trail



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø1	ø2	ø3	ø4	ø5
Lane Configurations											
Volume (vph)	29	554	918	100	47	12					
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900					
Storage Length (ft)	150			100	0	0					
Storage Lanes	1			1	1	0					
Taper Length (ft)	25				25						
Right Turn on Red				Yes		Yes					
Link Speed (mph)		25	25		25						
Link Distance (ft)		1000	220		1000						
Travel Time (s)		27.3	6.0		27.3						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92					
Shared Lane Traffic (%)											
Turn Type	custom	NA	NA	custom	NA						
Protected Phases	7	4 7	8 5		6		1	2	3	4	5
Permitted Phases	4			8							
Detector Phase	7	4 7	8 5	8	6						
Switch Phase											
Minimum Initial (s)	4.0			4.0	4.0		4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	8.0			20.0	20.0		8.0	20.0	8.0	20.0	8.0
Total Split (s)	8.0			54.0	20.0		8.0	20.0	8.0	54.0	8.0
Total Split (%)	8.9%			60.0%	22.2%		9%	22%	9%	60%	9%
Yellow Time (s)	3.5			3.5	3.5		3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	0.5			0.5	0.5		0.5	0.5	0.5	0.5	0.5
Lost Time Adjust (s)	0.0			0.0	0.0						
Total Lost Time (s)	4.0			4.0	4.0						
Lead/Lag	Lead			Lag	Lag		Lead	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes			Yes	Yes		Yes	Yes	Yes	Yes	Yes
Recall Mode	None			None	Max		None	Max	None	None	None

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 1: SH-6 & Traver Trail

#2 	#2 	#2 	#1 #2
ø1	ø2	ø3	ø4
8 s	20 s	8 s	54 s
#1 	#1 	#1 	#1 #2
ø5	ø6	ø7	ø8
8 s	20 s	8 s	54 s

Year 2035 - Existing Alignment
AM Peak Hour

Iron Mountain PUD - Glenwood Caverns

2: Devereux Rd & SH-6

							ø1	ø5	ø6	ø7	ø8
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR					
Lane Configurations	↑	↑	↑	↑	↑	↑					
Volume (vph)	558	43	105	998	20	71					
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900					
Storage Length (ft)		0	165		0	25					
Storage Lanes		1	1		1	1					
Taper Length (ft)			85		75						
Right Turn on Red		Yes				Yes					
Link Speed (mph)	25			25	25						
Link Distance (ft)	220			1000	375						
Travel Time (s)	6.0			27.3	10.2						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92					
Shared Lane Traffic (%)											
Turn Type	NA	custom	custom	NA	NA	Perm					
Protected Phases	4 1		3	8 3	2		1	5	6	7	8
Permitted Phases		4	8			2					
Detector Phase	4 1	4	3	8 3	2	2					
Switch Phase											
Minimum Initial (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)		20.0	8.0		20.0	20.0	8.0	8.0	20.0	8.0	20.0
Total Split (s)		54.0	8.0		20.0	20.0	8.0	8.0	20.0	8.0	54.0
Total Split (%)		60.0%	8.9%		22.2%	22.2%	9%	9%	22%	9%	60%
Yellow Time (s)		3.5	3.5		3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)		0.5	0.5		0.5	0.5	0.5	0.5	0.5	0.5	0.5
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					
Total Lost Time (s)		4.0	4.0		4.0	4.0					
Lead/Lag		Lag	Lead		Lag	Lag	Lead	Lead	Lag	Lead	Lag
Lead-Lag Optimize?		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode		None	None		Max	Max	None	None	Max	None	None

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd & SH-6

#2 → ø1 8 s	#2 ↘ ø2 20 s	#2 ↙ ø3 8 s	#1 #2 → ø4 54 s
#1 ← ø5 8 s	#1 ↘ ø6 20 s	#1 ↙ ø7 8 s	#1 #2 ← ø8 54 s

Year 2035 - Existing Alignment
AM Peak Hour

Iron Mountain PUD - Glenwood Caverns

3: Devereux Rd & Two Rivers Plaza Rd

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	26	5	4	65	65	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

1: Sh-6 & Traver Trail Performance by movement

Movement	EBL	EBT	WBT	WBR	SBL	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	28.7	9.3	3.5	1.9	27.5	19.3	6.6

2: Devereux Rd & Sh-6 Performance by movement

Movement	EBT	EBR	WBL	WBT	NBL	NBT	NBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	5.2	1.7	19.4	18.6	26.7	0.6	2.2	13.6

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	4.4	2.6	2.7	0.2	1.2	1.5	1.4

Total Network Performance

Denied Del/Veh (s)				0.9			
Total Del/Veh (s)				22.9			

Intersection: 1: Sh-6 & Traver Trail

Movement	EB	EB	EB	WB	WB	SB
Directions Served	L	T	T	T	R	LR
Maximum Queue (ft)	132	323	112	161	90	86
Average Queue (ft)	27	127	14	49	12	31
95th Queue (ft)	79	240	62	129	50	68
Link Distance (ft)		950		152		924
Upstream Blk Time (%)				0	0	
Queuing Penalty (veh)				3	0	
Storage Bay Dist (ft)	150		100		100	
Storage Blk Time (%)		11		2	0	
Queuing Penalty (veh)		32		2	0	

Intersection: 2: Devereux Rd & Sh-6

Movement	EB	WB	WB	NB	NB
Directions Served	T	L	T	L	R
Maximum Queue (ft)	166	250	657	55	64
Average Queue (ft)	66	92	319	16	9
95th Queue (ft)	147	241	561	45	40
Link Distance (ft)	152		950	307	
Upstream Blk Time (%)	1				
Queuing Penalty (veh)	2				
Storage Bay Dist (ft)		165			25
Storage Blk Time (%)			20	13	1
Queuing Penalty (veh)			21	9	0

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB	SB
Directions Served	LR	LT	R
Maximum Queue (ft)	46	26	4
Average Queue (ft)	16	2	0
95th Queue (ft)	37	14	3
Link Distance (ft)	430	950	307
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 69

Year 2035 - Existing Alignment
PM Peak Hour

Iron Mountain PUD - Glenwood Caverns

1: SH-6 & Traver Trail

											
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø1	ø2	ø3	ø4	ø5
Lane Configurations											
Volume (vph)	23	608	1034	67	71	17					
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900					
Storage Length (ft)	150			100	0	0					
Storage Lanes	1			1	1	0					
Taper Length (ft)	25				25						
Right Turn on Red				Yes		Yes					
Link Speed (mph)		25	25		25						
Link Distance (ft)		1000	220		1000						
Travel Time (s)		27.3	6.0		27.3						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92					
Shared Lane Traffic (%)											
Turn Type	custom	NA	NA	custom	NA						
Protected Phases	7	4 7	8 5		6		1	2	3	4	5
Permitted Phases	4			8							
Detector Phase	7	4 7	8 5	8	6						
Switch Phase											
Minimum Initial (s)	4.0			4.0	4.0		4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	8.0			20.0	20.0		8.0	20.0	8.0	20.0	8.0
Total Split (s)	8.0			64.0	20.0		8.0	20.0	8.0	64.0	8.0
Total Split (%)	8.0%			64.0%	20.0%		8%	20%	8%	64%	8%
Yellow Time (s)	3.5			3.5	3.5		3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	0.5			0.5	0.5		0.5	0.5	0.5	0.5	0.5
Lost Time Adjust (s)	0.0			0.0	0.0						
Total Lost Time (s)	4.0			4.0	4.0						
Lead/Lag	Lead			Lag	Lag		Lead	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes			Yes	Yes		Yes	Yes	Yes	Yes	Yes
Recall Mode	None			None	Max		None	Max	None	None	None

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 1: SH-6 & Traver Trail

#2 	#2 	#2 	#1 #2 
ø1	ø2	ø3	ø4
8 s	20 s	8 s	64 s
#1 	#1 	#1 	#1 #2 
ø5	ø6	ø7	ø8
8 s	20 s	8 s	64 s

Year 2035 - Existing Alignment
PM Peak Hour

Iron Mountain PUD - Glenwood Caverns

2: Devereux Rd & SH-6

							ø1	ø5	ø6	ø7	ø8
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR					
Lane Configurations	↑	↑	↑	↑	↑	↑					
Volume (vph)	651	28	58	1062	39	128					
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900					
Storage Length (ft)		0	165		0	25					
Storage Lanes		1	1		1	1					
Taper Length (ft)			85		75						
Right Turn on Red		Yes				Yes					
Link Speed (mph)	25			25	25						
Link Distance (ft)	220			1000	375						
Travel Time (s)	6.0			27.3	10.2						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92					
Shared Lane Traffic (%)											
Turn Type	NA	custom	custom	NA	NA	Perm					
Protected Phases	4 1		3	8 3	2		1	5	6	7	8
Permitted Phases		4	8			2					
Detector Phase	4 1	4	3	8 3	2	2					
Switch Phase											
Minimum Initial (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)		20.0	8.0		20.0	20.0	8.0	8.0	20.0	8.0	20.0
Total Split (s)		64.0	8.0		20.0	20.0	8.0	8.0	20.0	8.0	64.0
Total Split (%)		64.0%	8.0%		20.0%	20.0%	8%	8%	20%	8%	64%
Yellow Time (s)		3.5	3.5		3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)		0.5	0.5		0.5	0.5	0.5	0.5	0.5	0.5	0.5
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					
Total Lost Time (s)		4.0	4.0		4.0	4.0					
Lead/Lag		Lag	Lead		Lag	Lag	Lead	Lead	Lag	Lead	Lag
Lead-Lag Optimize?		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode		None	None		Max	Max	None	None	Max	None	None

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd & SH-6

#2 → ø1	#2 ↘ ø2	#2 ↙ ø3	#1 #2 → ø4
8 s	20 s	8 s	64 s
#1 ← ø5	#1 ↘ ø6	#1 ↙ ø7	#1 #2 ← ø8
8 s	20 s	8 s	64 s

Year 2035 - Existing Alignment
PM Peak Hour

Iron Mountain PUD - Glenwood Caverns

3: Devereux Rd & Two Rivers Plaza Rd

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	51	9	6	116	49	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

1: SH-6 & Traver Trail Performance by movement

Movement	EBL	EBT	WBT	WBR	SBL	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	31.0	9.6	3.9	1.8	34.3	27.9	7.5

2: Devereux Rd & SH-6 Performance by movement

Movement	EBT	EBR	WBL	WBT	NBL	NBT	NBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	5.8	1.6	23.5	21.7	32.8	0.2	4.4	15.3

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	4.7	2.4	2.1	0.3	1.3	1.5	1.6

Total Network Performance

Denied Del/Veh (s)				1.1			
Total Del/Veh (s)				26.1			

Intersection: 1: SH-6 & Traver Trail

Movement	EB	EB	EB	WB	WB	SB
Directions Served	L	T	T	T	R	LR
Maximum Queue (ft)	95	332	127	166	97	124
Average Queue (ft)	21	140	12	67	10	49
95th Queue (ft)	68	254	65	156	51	98
Link Distance (ft)		950		152		924
Upstream Blk Time (%)				1	0	
Queuing Penalty (veh)				9	0	
Storage Bay Dist (ft)	150		100		100	
Storage Blk Time (%)		12		4		
Queuing Penalty (veh)		41		2		

Intersection: 2: Devereux Rd & SH-6

Movement	EB	WB	WB	B203	NB	NB
Directions Served	T	L	T	T	L	R
Maximum Queue (ft)	167	249	794	30	103	98
Average Queue (ft)	96	57	386	2	30	36
95th Queue (ft)	183	185	701	26	76	87
Link Distance (ft)	152		950	72	307	
Upstream Blk Time (%)	2		1	1		
Queuing Penalty (veh)	5		0	0		
Storage Bay Dist (ft)		165				25
Storage Blk Time (%)			22		22	4
Queuing Penalty (veh)			13		28	2

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (ft)	57	12
Average Queue (ft)	22	1
95th Queue (ft)	43	10
Link Distance (ft)	430	950
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Option #2
Relocate Traver Trail

Year 2035 - Realigned with Signal
AM Peak Hour

Iron Mountain PUD - Glenwood Caverns

2: Devereux Rd/Traver Trail & SH-6

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	29	516	38	105	902	96	16	4	71	42	5	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		300	165		150	0		25	0		0
Storage Lanes	1		1	1		1	0		1	0		0
Taper Length (ft)	25			85			75			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		1220			1000			375			1000	
Travel Time (s)		33.3			27.3			10.2			27.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases	4		4	8		8			2			
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Split (s)	8.0	20.0	20.0	8.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	
Total Split (s)	8.0	70.0	70.0	10.0	72.0	72.0	20.0	20.0	20.0	20.0	20.0	
Total Split (%)	6.7%	58.3%	58.3%	8.3%	60.0%	60.0%	16.7%	16.7%	16.7%	16.7%	16.7%	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 110.8

Natural Cycle: 100

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd/Traver Trail & SH-6

 Ø2	 Ø6	 Ø3	 Ø4
20 s	20 s	10 s	70 s
		 Ø7	 Ø8
		8 s	72 s

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	26	5	4	65	65	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	30.6	17.9	3.1	31.7	31.0	22.7	42.7	26.1	2.9	38.7	31.4	29.7

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	All
Denied Del/Veh (s)	0.0
Total Del/Veh (s)	25.6

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	4.2	2.6	2.5	0.2	1.5	1.8	1.6

Total Network Performance

Denied Del/Veh (s)	1.0
Total Del/Veh (s)	29.8

Intersection: 2: Devereux Rd/Traver Trail & SH-6

Movement	EB	EB	EB	WB	WB	WB	B203	NB	NB	SB
Directions Served	L	T	R	L	T	R	T	LT	R	LTR
Maximum Queue (ft)	162	395	96	250	927	175	35	72	66	96
Average Queue (ft)	31	192	4	107	505	72	3	19	12	37
95th Queue (ft)	101	316	64	268	862	194	31	54	45	79
Link Distance (ft)		1164			944		72	307		924
Upstream Blk Time (%)					1		1			
Queuing Penalty (veh)					0		0			
Storage Bay Dist (ft)	150		300	165		150			25	
Storage Blk Time (%)		13	0	0	31	0		18	1	
Queuing Penalty (veh)		9	0	0	61	0		13	0	

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (ft)	40	15
Average Queue (ft)	15	1
95th Queue (ft)	35	8
Link Distance (ft)	431	950
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 84

Year 2035 - Realigned with Signal
PM Peak Hour

Iron Mountain PUD - Glenwood Caverns

2: Devereux Rd/Traver Trail & SH-6

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	23	589	19	58	1004	58	30	9	128	62	9	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		300	165		150	0		25	0		0
Storage Lanes	1		1	1		1	0		1	0		0
Taper Length (ft)	25			85			75			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		1220			1000			375			1000	
Travel Time (s)		33.3			27.3			10.2			27.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases	4		4	8		8			2			
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Split (s)	8.0	20.0	20.0	8.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	
Total Split (s)	8.0	72.0	72.0	8.0	72.0	72.0	20.0	20.0	20.0	20.0	20.0	
Total Split (%)	6.7%	60.0%	60.0%	6.7%	60.0%	60.0%	16.7%	16.7%	16.7%	16.7%	16.7%	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 116.8

Natural Cycle: 120

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd/Traver Trail & SH-6

			
ø2	ø6	ø3	ø4
20 s	20 s	8 s	72 s
			
		ø7	ø8
		8 s	72 s

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	51	9	9	116	49	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	36.3	16.8	3.0	46.5	46.5	37.8	40.7	38.6	8.1	40.1	36.2	28.6

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	All
Denied Del/Veh (s)	0.0
Total Del/Veh (s)	33.9

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	4.9	2.7	1.9	0.4	1.6	1.5	1.7

Total Network Performance

Denied Del/Veh (s)	7.6
Total Del/Veh (s)	39.2

Intersection: 2: Devereux Rd/Traver Trail & SH-6

Movement	EB	EB	EB	WB	WB	WB	B203	NB	NB	SB
Directions Served	L	T	R	L	T	R	T	LT	R	LTR
Maximum Queue (ft)	164	379	64	250	978	175	65	136	97	133
Average Queue (ft)	31	212	2	73	692	52	29	39	40	53
95th Queue (ft)	105	343	45	222	1146	169	102	104	94	105
Link Distance (ft)		1164			944		72	307		924
Upstream Blk Time (%)					14		13	0		
Queuing Penalty (veh)					0		0	0		
Storage Bay Dist (ft)	150		300	165		150			25	
Storage Blk Time (%)		16	0		34	0		32	8	
Queuing Penalty (veh)		7	0		39	0		41	3	

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (ft)	50	20
Average Queue (ft)	22	1
95th Queue (ft)	42	10
Link Distance (ft)	431	950
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 91

Option #3
**Relocate & Widen Traver Trail
and Restripe Devereux Rd**

Year 2035 - Realigned with Signal & No Split Phasing Iron Mountain PUD - Glenwood Caverns
AM Peak Hour

2: Devereux Rd/Traver Trail & SH-6

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	29	516	38	105	902	96	16	4	71	42	5	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		300	165		150	100		0	50		0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (ft)	25			85			75			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		1220			1000			375			1000	
Travel Time (s)		33.3			27.3			10.2			27.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	3	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Minimum Split (s)	8.0	20.0	20.0	8.0	20.0	20.0	8.0	20.0		8.0	20.0	
Total Split (s)	8.0	53.0	53.0	9.0	54.0	54.0	8.0	20.0		8.0	20.0	
Total Split (%)	8.9%	58.9%	58.9%	10.0%	60.0%	60.0%	8.9%	22.2%		8.9%	22.2%	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5		3.5	3.5	
All-Red Time (s)	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	Max		None	Max	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 78.8

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd/Traver Trail & SH-6

			
ø1	ø2	ø3	ø4
8 s	20 s	9 s	53 s
			
ø5	ø6	ø7	ø8
8 s	20 s	8 s	54 s

Year 2035 - Realigned with Signal & No Split Phasing Iron Mountain PUD - Glenwood Caverns
 AM Peak Hour

3: Devereux Rd & Two Rivers Plaza Rd

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	26	5	4	65	65	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	0			100
Storage Lanes	1	0	0			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	23.4	13.1	2.6	18.0	17.3	11.2	20.1	17.0	7.9	23.8	19.7	13.1

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	All
Denied Del/Veh (s)	0.0
Total Del/Veh (s)	15.4

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	4.2	2.6	2.6	0.2	2.0	2.4	1.9

Total Network Performance

Denied Del/Veh (s)	0.9
Total Del/Veh (s)	19.4

Intersection: 2: Devereux Rd/Traver Trail & SH-6

Movement	EB	EB	EB	WB	WB	WB	NB	NB	SB	SB
Directions Served	L	T	R	L	T	R	L	TR	L	TR
Maximum Queue (ft)	108	327	105	250	610	175	39	74	59	44
Average Queue (ft)	25	151	4	75	297	66	12	33	23	10
95th Queue (ft)	74	265	56	204	520	183	37	62	53	33
Link Distance (ft)	1158				938			307		925
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	150		300	165		150	100		50	
Storage Blk Time (%)		7	0		18	0		0	5	0
Queuing Penalty (veh)		5	0		37	0		0	1	0

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB	SB
Directions Served	LR	LT	R
Maximum Queue (ft)	37	18	2
Average Queue (ft)	15	1	0
95th Queue (ft)	34	9	2
Link Distance (ft)	425	950	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			100
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 43

Year 2035 - Realigned with Signal & No Split Phasing Iron Mountain PUD - Glenwood Caverns
PM Peak Hour

2: Devereux Rd/Traver Trail & SH-6

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	23	589	19	58	1004	58	30	9	128	62	9	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		300	165		150	100		0	50		0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (ft)	25			85			75			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		1220			1000			375			1000	
Travel Time (s)		33.3			27.3			10.2			27.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	3	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Minimum Split (s)	8.0	20.0	20.0	8.0	20.0	20.0	8.0	20.0		8.0	20.0	
Total Split (s)	8.0	54.0	54.0	8.0	54.0	54.0	8.0	20.0		8.0	20.0	
Total Split (%)	8.9%	60.0%	60.0%	8.9%	60.0%	60.0%	8.9%	22.2%		8.9%	22.2%	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5		3.5	3.5	
All-Red Time (s)	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	Max		None	Max	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 83.6

Natural Cycle: 90

Control Type: Semi Act-Uncoord

Splits and Phases: 2: Devereux Rd/Traver Trail & SH-6

			
8 s	20 s	8 s	54 s
			
8 s	20 s	8 s	54 s

Year 2035 - Realigned with Signal & No Split Phasing Iron Mountain PUD - Glenwood Caverns
PM Peak Hour

3: Devereux Rd & Two Rivers Plaza Rd

						
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (vph)	51	9	9	116	49	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	0			100
Storage Lanes	1	0	0			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	500			1000	375	
Travel Time (s)	13.6			27.3	10.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Del/Veh (s)	25.1	13.0	2.6	29.8	30.5	22.2	22.7	24.3	10.9	24.9	20.7	14.9

2: Devereux Rd/Traver Trail & SH-6 Performance by movement

Movement	All
Denied Del/Veh (s)	0.0
Total Del/Veh (s)	23.0

3: Devereux Rd & Two Rivers Plaza Rd Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Denied Del/Veh (s)	0.0	0.0	0.0	0.0	0.0	0.1	0.0
Total Del/Veh (s)	4.9	2.6	2.0	0.3	1.9	1.8	1.8

Total Network Performance

Denied Del/Veh (s)	1.7
Total Del/Veh (s)	27.3

Intersection: 2: Devereux Rd/Traver Trail & SH-6

Movement	EB	EB	EB	WB	WB	WB	B203	NB	NB	SB	SB
Directions Served	L	T	R	L	T	R	T	L	TR	L	TR
Maximum Queue (ft)	159	360	32	250	876	175	37	88	126	68	82
Average Queue (ft)	24	173	1	72	473	49	10	23	55	31	16
95th Queue (ft)	81	290	32	219	912	160	59	62	100	61	51
Link Distance (ft)		1158			938		72		307		925
Upstream Blk Time (%)					4		4				
Queuing Penalty (veh)					0		0				
Storage Bay Dist (ft)	150		300	165		150		100		50	
Storage Blk Time (%)		9			27	0		0	1	8	0
Queuing Penalty (veh)		4			31	0		0	0	2	0

Intersection: 3: Devereux Rd & Two Rivers Plaza Rd

Movement	EB	NB
Directions Served	LR	LT
Maximum Queue (ft)	50	20
Average Queue (ft)	22	1
95th Queue (ft)	42	10
Link Distance (ft)	425	950
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 38



DEPARTMENT OF THE ARMY
U.S. ARMY CORPS OF ENGINEERS, SACRAMENTO DISTRICT
1325 J STREET
SACRAMENTO CA 95814-2922

December 22, 2017

Regulatory Division (SPK-2017-00468)

Recreation Engineering & Planning
Attn: Mr. Gary Lacy
485 Arapahoe Avenue
Boulder, Colorado 80302

Dear Mr. Lacy:

We are responding to your request, on behalf of Iron Mountain Hot Springs, LLC, for a Department of the Army permit for the Iron Mountain Hot Springs Riverbank Improvements project. This project involves activities, including discharges of fill material, in waters of the United States to stabilize 1,000 linear-feet of streambank. The project site is located along the north bank (river right) of the Colorado River, within Government Lot 7 of Section 9 and Government Lot 8 of Section 4, Township 6 South, Range 89 West, 6th Principal Meridian, between Latitude 39.552808°, Longitude -107.336577° and Latitude 39.556465°, Longitude -107.337230°, in the City of Glenwood Springs, Garfield County, Colorado.

Based on the information you provided to this office, the proposed activity, including discharges of dredge and fill material in waters of the United States, involves the removal of 4,760 cubic-yards of river cobble and installation of 1,850 cubic-yards of triangular current deflectors and vegetated stone terracing to stabilize approximately 1,000 linear-feet of streambank, in accordance with the Permit Set drawings, dated May 25, 2017. These activities will result in permanent impacts to approximately 0.1 acre of perennial non-wetland waters.

We have determined activities associated with this project are authorized by Regional General Permit (RGP) 37 - Stream Stabilization Projects in Colorado. Your work must comply with the general terms and conditions listed on the enclosed RGP information sheets. Additionally, it is your responsibility to ensure that all contract personnel are familiar with permit requirements. Please note that you are required to sign the enclosed Compliance Certification and return it to this office within 30 days after completion of the remaining authorized work.

This verification is valid until October 22, 2022. Failure to comply with the General Conditions of this Regional General Permit may result in the suspension or revocation of your authorization.

Please refer to identification number SPK-2017-00468 in any correspondence concerning this project. If you have any questions, please contact me at the Colorado West Regulatory Branch, 400 Rood Avenue, Room 224, Grand Junction, Colorado 81501, by email at w.travis.morse@usace.army.mil, or telephone at (970) 243-1199 ext. 1014.

For more information regarding our program, please visit our website at www.spk.usace.army.mil/Missions/Regulatory.aspx. We would appreciate your feedback. At your earliest convenience, please tell us how we are doing by completing the customer survey from the link on our website, listed above.

Sincerely,



Travis Morse
Senior Project Manager
Colorado West Section
Regulatory Division

Enclosures

1. RGP 37
2. Compliance Certification

cc:

Mr. Gary Lacy, Recreation Engineering & Planning, gary@boaterparks.com
Mr. Steve Beckley, Iron Mountain Hot Springs, LLC, sbeckley@glenwoodcaverns.com
Mr. Tom Barnes, City of Glenwood Springs, tom.barnes@cogs.us
Ms. Sheryl Bower, Garfield County Building & Planning Department, sbower@garfield-county.com
Ms. Kendall Backich, Colorado Parks and Wildlife, kendall.backich@state.co.us



Regional General Permit 37

U.S. ARMY CORPS OF ENGINEERS

BUILDING STRONG®

STREAM STABILIZATION PROJECTS IN COLORADO

EFFECTIVE DATE: October 20, 2017

EXPIRATION DATE: October 20, 2022

In accordance with Section 404 of the Clean Water Act (33U.S.C. 1344) and Section 10 of the Rivers and Harbors Act of 1899 (33 U.S.C. 403), the U.S. Army Corps of Engineers (Corps) hereby authorizes certain limited discharges of dredged and fill material associated with stream stabilization in the State of Colorado. The purpose of this Regional General Permit (RGP) is to authorize stream bed and bank stabilization activities.

LOCATION: This RGP is applicable to waters of the United States within the State of Colorado, including Tribal Lands.

SCOPE OF WORK: This RGP authorizes stream stabilization activities necessary for erosion control or prevention, such as vegetative stabilization, bioengineering, sills, rip rap, revetment, barbs, jetties, weirs, or combinations of bank stabilization techniques, provided the activity meets all of the following criteria:

1. No material is placed in excess of the minimum needed for erosion protection;
2. The activity is no more than 1,000 feet in length along the bank;
3. The activity will not exceed an average of two cubic yard per running foot, as measured along the length of the treated bank, below the plane of the ordinary high water mark (OHWM), unless the district engineer waives this criterion by making a written determination concluding that the discharge will result in no more than minimal adverse environmental effects;
4. The average stream width measured at the OHWM is no less than 20 feet; and
5. The activity will not exceed 0.5 acre of permanent impacts to wetlands.

NOTIFICATION AND APPROVAL PROCEDURES:

Written notification requesting approval and concurrence with the requirements of the RGP must be sent to the Corps office that services the area of the project location. For assistance in determining the appropriate regulatory office, please contact one of the Regulatory offices below or visit the Colorado Regulatory website at <http://www.nwo.usace.army.mil/Missions/RegulatoryProgram/Colorado/GeographicResponsibility.aspx>.

Denver Regulatory Office
(Omaha District)
9307 South Wadsworth Blvd.

Grand Junction Regulatory Office
(Sacramento District)
400 Rood Ave., Room 224

Littleton, CO 80128-6901
Phone: (303) 979-4120

Grand Junction, CO 81501-2563
Phone: (970) 243-1199

Southern Colorado Regulatory Office
(Albuquerque District)
200 S. Santa Fe Ave., Suite 301
Pueblo, CO 81003
Phone: (719) 543-9459

Durango Regulatory Office
(Albuquerque & Sacramento Districts)
1970 E 3rd Ave., Suite 109
Durango, CO 81301
Phone: (970) 259-1764

This RGP does not authorize activities that would adversely affect important spawning areas, including Gold Medal Waters, or that would be conducted in these waters during designated spawning seasons. In order to determine if projects would result in adverse effects to important spawning areas, the Corps will consult with Colorado Parks and Wildlife (CPW) for all projects proposed in Gold Medal Waters and other fisheries of concern identified by CPW on a case-by-case basis. Pre-application consultation with CPW, preferably on-site, is highly recommended. Providing documentation of pre-application consultation with CPW and their response(s) will satisfy the coordination requirements of this permit resulting in quicker processing times. For assistance in determining the appropriate CPW office and point-of-contact, please visit the following state website to determine the appropriate office for coordination:

<http://wildlife.state.co.us>, or call one of the following:

Northeast Region Manager
6060 Broadway
Denver, CO 80216
(303) 291-7227

Northwest Region Manager
711 Independent Ave.
Grand Junction, CO 81505
(970) 255-6100

Southeast Region Manager
4255 Sinton Rd.
Colorado Springs, CO 80907
(719) 227-5200

Southwest Region Manager
415 Turner Dr.
Durango, CO 81303
(970) 375-6702

The applicant must first notify the Corps in writing according to the Notification procedures of the RGP as described below. Work cannot proceed until the Corps has provided written approval to the applicant. The applicant must provide the following information:

1. Name, address and telephone number of the applicant responsible for the work, the owner of the affected lands (if different than the applicant), and the contractor(s) that will be performing the work, if applicable. The applicant must provide written permission allowing the Corps, CPW, and/or tribal representative to enter upon and perform work on property not belonging to the applicant. The applicant should be the entity retaining long-term maintenance responsibility for constructed features. The notification must include a letter, signed and dated by the applicant, stating that they certify that the information in the notification is complete and accurate and that they will abide by the terms and conditions of this permit;
2. A description of the project location including section, township, range, and coordinates (latitude/longitude, UTM, etc.) at both ends of the work area;

3. A written well-defined purpose and need for the work, including a description/assessment of the existing stream conditions (i.e., baseline information) and an explanation for why the project is needed, including a description of how the project will result in bank stabilization;
4. Quantity of waters of the U.S to be impacted by the proposed activity in terms of wetlands and other aquatic resources. Units of measure should be provided in acres or square feet, and also linear feet, if appropriate;
5. If the proposed activity will result in the loss of greater than 1/10-acre of wetlands, the prospective permittee must submit a proposal describing how the wetland loss will be mitigated, or explaining why the adverse environmental effects are no more than minimal and why compensatory mitigation should not be required. As an alternative, the prospective permittee may submit a conceptual or detailed mitigation plan;
6. Pre-construction photos depicting the physical setting (to be compared to post-construction site conditions from the same photo points). Photos should contain figure labels with time, date, bearing, and a general description of the site;
7. A set of drawings per established standards and guidelines for maps and drawings (refer to the map standards located at http://www.spd.usace.army.mil/Portals/13/docs/regulatory_standards/MapStand020816.pdf). Additionally, the drawings must include the following elements:
 - a. Location map, including name of the waterway, nearest town, county, and coordinates (NAD83) of each structure and at both ends of the work area;
 - b. Plan view of all work, including staging area(s) and access point(s), clearly identifying types and locations of structures/impacts, along with dimensions, a scale bar, a north arrow, and approximate extent of aquatic resources within the project area. To aid in visual understanding, this plan can be overlaid on a recent aerial image of the project site;
 - c. Cross-sectional and profile views to scale of the existing stream channel and the proposed structures, including dimensions (length, width and height of the structures or work), and clear delineation of the limits of the OHWM; and
 - d. A delineation of the OHWM and all special aquatic sites at the project site, including wetlands and riffle and pool complexes. The aquatic resource delineation report must be submitted in accordance with the wetland delineations protocols for the individual Corps District that covers the project area;
8. A complete description of the work, including the composition, source, and volume in cubic yards of all material to be placed or redistributed in waters of the U.S. The description should contain a proposed procedure to be taken to reduce sedimentation and the basis used in determining the volume of fill to be placed or redistributed in waters of the U.S.:
 - a. For projects involving grade control, the project description must also include a detailed explanation and criteria for the following design features:
 - (1) Number and spacing of structures;

- (2) Any changes in base flow elevations; and
 - (3) For projects located in perennial streams, a justification for any structure heights that exceed 18 inches and/or do not include a fish passage element with a maximum drop of 12 inches;
 - b. For projects that involve deflectors, such as jetties and weirs, the project description must also include a detailed explanation and criteria for the following design features:
 - (1) Height: Deflectors are generally designed to be exposed during low flows and submerged during high flows. Depending on the site and the project goals, deflectors may be constructed with the height of the structure at the stream bank equal to the bankfull elevation sloping down to the low flow elevation at the toe or end of the structure;
 - (2) Length: The maximum extension of any deflector into a waterway generally should not exceed $\frac{1}{3}$ of the channel width as measured at bankfull elevation (approximately 1-2 year event);
 - (3) Angle: Deflectors are designed to redirect flows away from the stream bank and back toward the center of the channel. In order to best accomplish this goal, the angle of deflectors is generally perpendicular to the desired direction of flow downstream of the deflector. Incorrect deflector angles can result in scouring behind the structure, particularly during low flow conditions, or erosion of the opposite stream bank. In most cases, the angle of the deflector should be between 0 and 30 degrees upstream from perpendicular to the stream bank; and
 - (4) Spacing: In many cases, a series of deflectors are designed to work together as a single unit. In such applications, deflectors should be spaced appropriately along the bank in order to prevent scouring or scalloping of the bank between the structures;
 - c. For projects involving work on multiple reaches or sections of stream, the project description must also include detailed explanation of the independent utility of each section of work; and
 - d. If the proposal does not incorporate bioengineering techniques, the project description must also include a discussion on the alternative techniques that were considered and why they were found not to be practicable. The description of the proposed activity and any proposed mitigation measures should be sufficiently detailed to allow the District Engineer to determine that the adverse environmental effects of the activity will be no more than minimal and to determine the need for compensatory mitigation or other mitigation measures;
9. If the project is located on the following Indian Lands, the applicant must obtain an individual water quality certification (WQC) under Section 401 of the Clean Water Act and provide a copy of the WQC to the Corps (see also General Condition 20):
- a. Southern Ute Indian Lands. WQCs for projects located on Southern Ute Indian Lands must be obtained from the U.S. Environmental Protection Agency (EPA). For assistance, contact the EPA at: EPA, EPR-EP, Aquatic Resources Protection and Accountability

Unit, 1595 Wynkoop Street, Denver, Colorado 80202-1129, 303-312-6909, or visit the webpage: <http://www.epa.gov/region8/water/wqc.html>; or

- b. Ute Mountain Ute Indian Lands. WQCs for projects located on Ute Mountain Ute Indian Lands must be obtained from the Ute Mountain Ute Tribe. For assistance, contact the Ute Mountain Ute Environmental Programs Department, 520 Sunset Blvd. or P.O. Box 448, Towaoc, Colorado 81334, 970-564-5430 (fax 970-565-2651), or visit the webpage: <http://www.utemountainuteenvironmental.org>;
10. For non-federal permittees, if any listed species or designated critical habitat might be affected or is in the vicinity of the activity, or if the activity is located in designated critical habitat, the notification package must include the name(s) of those endangered or threatened species that might be affected by the proposed activity or utilize the designated critical habitat that might be affected by the proposed activity. Federal permittees must provide documentation demonstrating compliance with the Endangered Species Act (see General Condition 16);
11. For non-federal permittees, if the RGP activity may have the potential to cause effects to a historic property listed on, determined to be eligible for listing on, or potentially eligible for listing on, the National Register of Historic Places, the notification package must state which historic property may have the potential to be affected by the proposed activity or include a vicinity map indicating the location of the historic property. Federal permittees must provide documentation demonstrating compliance with Section 106 of the National Historic Preservation Act (see General Condition 18); and
12. Applicants are encouraged to provide the Corps with electronic files to expedite agency coordination and permit review. Such submittals may be limited by size so the applicant should request confirmation of delivery receipt.

The information should reference "Regional General Permit No. 37" and be sent to the appropriate Corps office. The Corps will review the applicant's request for authorization under this RGP and will inform the applicant in writing that the work may proceed. The applicant may not start work until notified by the Corps. If the work is not authorized under this RGP, the applicant must apply to the Corps for a Standard Individual Permit or other appropriate Department of the Army permit(s).

For projects located within tribal trust lands, coordination is required with the appropriate tribal entity. Pre-application consultation with the tribe, preferably on-site, is highly recommended.

For projects that have the potential to impact vested water rights, it is recommended that the applicant provide evidence or coordination with the local Water Commissioner. For assistance, contact the Colorado Division of Water Resources (DWR), 1313 Sherman St., Suite 821, Denver, CO 80203, (303) 866-3581, or you may contact the appropriate Water Commissioner using the directory found at: <http://water.state.co.us/DivisionsOffices/Pages/default.aspx>.

GENERAL CONDITIONS: To qualify for this RGP authorization, the prospective permittee must comply with the following general conditions (GC), in addition to any regional or case-specific conditions imposed by the Division Engineer or District Engineer:

1. Upon receiving approval to perform work under this RGP, the permittee will have three years to complete the work or until the permit expires or is revoked, unless specified otherwise in a Corps verification letter. If the permittee commenced work on the authorized activity or is under contract to commence work when the permit expires or is revoked, the permittee will have one more year or until the original expiration, whichever is less, to complete the construction. Upon completion of the work, the permittee shall submit a signed Certification of Compliance form to the Corps;
2. For all projects that include a design-build component, the permittee shall submit a complete set of as-built drawings to the Corps within 90 days following the completion of work;
3. Structures or fill authorized by this permit shall not impede waterborne navigation, including rafting and canoeing, or create a hazard to navigation in waterways. Any interference with navigation may require removal or modification of the structure at the permittee's expense;
4. Activities authorized by this permit shall be designed to withstand expected high flows and maintain preconstruction surface flow rates from the site to the maximum extent practicable;
5. This RGP does not authorize stream stabilization projects that are a necessary component of residential, commercial, and institutional developments;
6. The construction of recreational features (e.g., water parks, kayak courses) and flood control projects are not authorized by this permit;
7. Material may not be placed in any location or manner that will impair surface water flows into or out of any special aquatic sites. The permittee must take precautions to avoid and minimize temporary impacts and protect wetlands and riffle-pool complexes from damage during access and construction. Temporary fills must be removed in their entirety and the affected areas returned to pre-construction conditions, including the revegetation of affected areas, as appropriate. This permit does not authorize the loss of greater than 0.5 acre of wetland;
8. The activity must comply with applicable FEMA-approved state or local floodplain management requirements;
9. Heavy equipment working in wetlands or mudflats must be placed on mats, or other measures must be taken to minimize soil disturbance;
10. The District Engineer may designate, after notice and opportunity for public comment, additional waters officially designated by the State of Colorado as having particular environmental or ecological significance, such as outstanding national resource waters or state natural heritage sites. The District Engineer may also designate additional critical resource waters after notice and opportunity for public comment;
11. The District Engineer may determine on a case-by-case basis that compensatory mitigation is required to ensure that the activity results in no more than minimal adverse environmental effects. The District Engineer will consider the following factors when determining appropriate and practicable mitigation necessary to ensure that the individual and cumulative adverse environmental effects are no more than minimal:

- a. The activity must be designed and constructed to avoid and minimize adverse effects, both temporary and permanent, to waters of the United States to the maximum extent practicable at the project site (i.e., on site);
- b. Mitigation in all its forms (avoiding, minimizing, rectifying, reducing, or compensating for resource losses) will be required to the extent necessary to ensure that the individual and cumulative adverse environmental effects are no more than minimal;
- c. Compensatory mitigation plans for RGP activities in or near streams or other open waters will normally include a requirement for the restoration or enhancement, maintenance, and legal protection (e.g., conservation easements) of riparian areas next to open waters. In some cases, the restoration of riparian areas may be the only compensatory mitigation required. Restored riparian areas should consist of native species. The width of the required riparian area will address documented water quality or aquatic habitat loss concerns. Normally, the riparian area will be 25 to 50 feet wide on each side of the stream, but the District Engineer may require slightly wider riparian areas to address documented water quality or habitat loss concerns. Where both wetlands and open waters exist on the project site, the District Engineer will determine the appropriate compensatory mitigation (e.g., riparian areas and/or wetlands compensation) based on what is best for the aquatic environment on a watershed basis. In cases where riparian areas are determined to be the most appropriate form of compensatory mitigation, the District Engineer may waive or reduce any requirement to provide wetland compensatory mitigation for wetland losses;
- d. Compensatory mitigation projects provided to offset losses of aquatic resources must comply with the applicable provisions of 33 CFR part 332;
 - (1) The prospective permittee is responsible for proposing an appropriate compensatory mitigation option if compensatory mitigation is necessary to ensure that the activity results in no more than minimal adverse environmental effects;
 - (2) If permittee-responsible mitigation is the proposed option, the prospective permittee is responsible for submitting a mitigation plan. A conceptual or detailed mitigation plan may be used by the District Engineer to make the decision on the RGP verification request, but a final mitigation plan that addresses the applicable requirements of 33 CFR parts 332.4(c)(2) – (14) must be approved by the District Engineer before the permittee begins work in waters of the United States, unless the District Engineer determines that prior approval of the final mitigation plan is not practicable or not necessary to ensure timely completion of the required compensatory mitigation (see 33 CFR part 332.3(k)(3));
 - (3) If mitigation bank or in-lieu fee program credits are the proposed option, the mitigation plan only needs to address the baseline conditions at the impact site and the number of credits to be provided; and
 - (4) Compensatory mitigation requirements (e.g., resource type and amount to be provided as compensatory mitigation, site protection, ecological performance standards, monitoring requirements) may be added through special conditions added to the RGP authorization;

- e. Compensatory mitigation will not be used to increase the acreage losses allowed by the acreage limits of this RGP. For example, RGP 37 cannot be used to authorize any activity resulting in permanent impacts to greater than 0.5 acre of wetlands, even if compensatory mitigation is provided to replace or restore some of the lost waters. However, compensatory mitigation can and should be used, as necessary, to ensure that an activity already meeting the established acreage limits also satisfies the no more than minimal impact requirement for this RGP; and
- f. Permittees may propose the use of mitigation banks, in-lieu fee programs, or permittee-responsible mitigation. For permittee-responsible mitigation, the special conditions of the RGP verification must clearly indicate the party or parties responsible for the implementation and performance of the compensatory mitigation project, and, if required, its long-term management.

When mitigation is required, no work in waters of the United States may occur until the District Engineer has approved a specific mitigation plan or has determined that prior approval of a final mitigation plan is not practicable or not necessary to ensure timely completion of the required compensatory mitigation. For projects requiring compensatory mitigation, please refer to the Mitigation and Monitoring Guidelines located at:

<http://www.spd.usace.army.mil/Portals/13/docs/regulatory/mitigation/MitMon.pdf>;

- 12. No activity may substantially disrupt the necessary life cycle movements of those species of aquatic life indigenous to the waterbody, including those species that normally migrate through the area;
- 13. Destruction of riparian or riverine vegetation, especially mature cottonwoods, shall be avoided to the maximum extent practicable. The permittee is cautioned that cottonwoods may be locally very important for several protected species, including the yellow-billed cuckoo, and bald eagles, which are protected under the Bald and Golden Eagle Act. If the authorized work causes damage to riparian vegetation that is not directly covered by a permanent feature, these scarred areas shall be replanted with a mixture of native trees, shrubs, forbs and grasses. Seeding, sprigging, or other means of planting native woody and herbaceous plants is highly recommended and advantageous to further stabilize stream banks. For further information on planting, please contact the local Natural Resources Conservation Service or Corps office;
- 14. Appropriate soil erosion and sediment controls must be used and maintained in effective operating condition during construction, and all exposed soil and other fills, as well as any work below the OHWM, must be permanently stabilized at the earliest practicable date. Erosion control fabrics/blankets may be required on a case-by-case basis. All disturbed areas, including the riparian and upland buffer zones, shall be revegetated with native species to prevent erosion. These areas shall be monitored and maintained to ensure vegetative success;
- 15. Activities in breeding areas for migratory waterfowl must be avoided to the maximum extent practicable. On a case-by-case basis, the Corps may restrict the timing of construction in order to avoid and minimize impacts to migratory waterfowl during the breeding season;
- 16. Endangered Species:

- a. No activity is authorized under any RGP which is likely to directly or indirectly jeopardize the continued existence of a threatened or endangered species or a species proposed for such designation, as identified under the Federal Endangered Species Act (ESA), or which will directly or indirectly destroy or adversely modify the critical habitat of such species. No activity is authorized under any RGP which “may affect” a listed species or critical habitat, unless ESA section 7 consultation addressing the effects of the proposed activity has been completed. Direct effects are the immediate effects on listed species and critical habitat caused by the RGP activity. Indirect effects are those effects on listed species and critical habitat that are caused by the RGP activity and are later in time, but still are reasonably certain to occur;
- b. Federal agencies should follow their own procedures for complying with the requirements of the ESA. The federal permittee must provide the district engineer with the appropriate documentation to demonstrate compliance with those requirements. The district engineer will verify that the appropriate documentation has been submitted. If the appropriate documentation has not been submitted, additional ESA section 7 consultation may be necessary for the activity and the respective federal agency would be responsible for fulfilling its obligation under section 7 of the ESA;
- c. For activities that might affect federally-listed endangered or threatened species or designated critical habitat, the non-federal applicant’s notification must include the name(s) of the endangered or threatened species that might be affected by the proposed activity or that utilize the designated critical habitat that might be affected by the proposed activity. The district engineer will determine whether the proposed activity “may affect” or will have “no effect” to listed species and designated critical habitat and will notify the non-federal applicant of the Corps’ determination within 45 days of receipt of a complete notification. In cases where the non-federal applicant has identified listed species or critical habitat that might be affected or is in the vicinity of the activity, and has so notified the Corps, the applicant shall not begin work until the Corps has provided notification that the proposed activity will have “no effect” on listed species or critical habitat, or until ESA section 7 consultation has been completed;
- d. As a result of formal or informal consultation with the FWS the district engineer may add species specific permit conditions to the RGP;
- e. Authorization of an activity by the RGP does not authorize the “take” of a threatened or endangered species as defined under the ESA. In the absence of separate authorization (e.g., an ESA Section 10 Permit, a Biological Opinion with “incidental take” provisions, etc.) from the FWS or the NMFS, the Endangered Species Act prohibits any person subject to the jurisdiction of the United States to take a listed species, where “take” means to harass, harm, pursue, hunt, shoot, wound, kill, trap, capture, or collect, or to attempt to engage in any such conduct. The word “harm” in the definition of “take” means an act which actually kills or injures wildlife. Such an act may include significant habitat modification or degradation where it actually kills or injures wildlife by significantly impairing essential behavioral patterns, including breeding, feeding or sheltering;
- f. If the non-federal permittee has a valid ESA section 10(a)(1)(B) incidental take permit with an approved Habitat Conservation Plan for a project or a group of projects that includes the proposed RGP activity, the non-federal applicant should provide a copy of that ESA section 10(a)(1)(B) permit with the notification required by paragraph (c) of this

general condition. The district engineer will coordinate with the agency that issued the ESA section 10(a)(1)(B) permit to determine whether the proposed RGP activity and the associated incidental take were considered in the internal ESA section 7 consultation conducted for the ESA section 10(a)(1)(B) permit. If that coordination results in concurrence from the agency that the proposed RGP activity and the associated incidental take were considered in the internal ESA section 7 consultation for the ESA section 10(a)(1)(B) permit, the district engineer does not need to conduct a separate ESA section 7 consultation for the proposed RGP activity. The district engineer will notify the non-federal applicant within 45 days of receipt of a complete pre-construction notification whether the ESA section 10(a)(1)(B) permit covers the proposed RGP activity or whether additional ESA section 7 consultation is required; and

- g. Information on the location of threatened and endangered species and their critical habitat can be obtained directly from the offices of the FWS or their World Wide Web pages at <http://www.fws.gov/> or <http://www.fws.gov/ipac>;
17. The permittee is responsible for ensuring their action complies with the Migratory Bird Treaty Act and the Bald and Golden Eagle Protection Act. The permittee is responsible for contacting appropriate local office of the USFWS to determine applicable measures to reduce impacts to migratory birds or eagles, including whether "incidental take" permits are necessary and available under the Migratory Bird Treaty Act or Bald and Golden Eagle Protection Act for a particular activity;
18. Historic Properties:
- a. In cases where the District Engineer determines that the activity may affect properties listed, or eligible for listing, in the National Register of Historic Places, the activity is not authorized, until the requirements of Section 106 of the National Historic Preservation Act (NHPA) have been satisfied;
 - b. Federal permittees should follow their own procedures for complying with the requirements of Section 106 of the National Historic Preservation Act. Federal permittees must provide the District Engineer with the appropriate documentation to demonstrate compliance with those requirements. The District Engineer will verify that the appropriate documentation has been submitted. If the appropriate documentation is not submitted, then additional consultation under Section 106 may be necessary. The respective federal agency is responsible for fulfilling its obligation to comply with Section 106;
 - c. Non-federal permittees must identify in their notification which historic properties may be affected by the proposed work or include a vicinity map indicating the location of the historic properties or the potential for the presence of historic properties.

Assistance regarding information on the location of or potential for the presence of historic resources can be sought from the State Historic Preservation Officer or Tribal Historic Preservation Officer, as appropriate, and the National Register of Historic Places (see 33 CFR part 330.4(g)). When reviewing notification packages, District Engineers will comply with the current procedures for addressing the requirements of Section 106 of the National Historic Preservation Act. The District Engineer shall make a reasonable and good faith effort to carry out appropriate identification efforts, which may include

background research, consultation, oral history interviews, sample field investigation, and field survey. Based on the information submitted and these efforts, the District Engineer shall determine whether the proposed activity has the potential to cause an effect on the historic properties. Where the non-federal applicant has identified historic properties on which the activity may have the potential to cause effects and so notified the Corps, the non-federal applicant shall not begin the activity until notified by the District Engineer either that the activity has no potential to cause effects or that consultation under Section 106 of the NHPA has been completed;

- d. The District Engineer will notify the prospective permittee within 45 days of receipt of a complete notification package whether NHPA Section 106 consultation is required. Section 106 consultation is not required when the Corps determines that the activity does not have the potential to cause effects on historic properties (see 36 CFR part 800.3(a)). If NHPA Section 106 consultation is required and will occur, the District Engineer will notify the non-federal applicant that he or she cannot begin work until Section 106 consultation is completed. If the non-federal applicant has not heard back from the Corps within 45 days, the applicant must still wait for notification from the Corps; and
- e. Prospective permittees should be aware that Section 110k of the NHPA (16 U.S.C. 470h-2(k)) prevents the Corps from granting a permit or other assistance to an applicant who, with intent to avoid the requirements of Section 106 of the NHPA, has intentionally significantly adversely affected a historic property to which the permit would relate, or having legal power to prevent it, allowed such significant adverse effect to occur, unless the Corps, after consultation with the Advisory Council on Historic Preservation (ACHP), determines that circumstances justify granting such assistance despite the adverse effect created or permitted by the applicant. If circumstances justify granting the assistance, the Corps is required to notify the ACHP and provide documentation specifying the circumstances, the degree of damage to the integrity of any views obtained from the applicant, SHPO/THPO, appropriate Indian tribes if the undertaking occurs on or affects historic properties on tribal lands or affects properties of interest to those tribes, and other parties known to have a legitimate interest in the impacts to the permitted activity on historic properties.

If any previously unknown historic, cultural or archeological remains and artifacts are discovered while accomplishing the activity authorized by this permit, the permittee must immediately notify the District Engineer of what was found, and to the maximum extent practicable, avoid construction activities that may affect the remains and artifacts until the required coordination has been completed. The District Engineer will initiate the federal, tribal and state coordination required to determine if the items or remains warrant a recovery effort or if the site is eligible for listing in the National Register of Historic Places;

- 19. To ensure that all structures are safely designed, the District Engineer may require non-federal applicants to demonstrate that the structures have been designed by qualified persons. The District Engineer may also require documentation that the design has been independently reviewed by similarly qualified persons, and appropriate modifications made to ensure safety;

20. If a conditioned water quality certification is issued for the project, (i.e., for projects located on Southern Ute or Ute Mountain Ute Tribal Lands) the permittee must comply with the conditions specified in the certification as special conditions to this permit. In Colorado, excluding Indian lands, regional general permits are unconditionally certified by statute;
21. This permit does not authorize discharges of dredged or fill material associated with channelization, ditching, mechanized land clearing, cutting off meanders, or blocking off channels;
22. All construction debris (including excess dredged or fill materials, wood, cleared vegetation, concrete, and all other materials not specifically authorized by the permit) shall be disposed of in an upland area in such a manner that it cannot enter a waterway or wetland;
23. Dredged or fill material shall not consist of unsuitable material (e.g., trash, debris, waste metal products, asphalt, car bodies, tires, etc.) and must be free from toxic pollutants in toxic amounts (see Section 307 of the Clean Water Act);
24. The use of grout is not allowed by this permit unless, after a case-by-case review, the Corps makes a written determination that the use of grout would not cause more than minor impacts to the aquatic resource;
25. Activities that result in the physical destruction (e.g., through excavation, fill, or downstream smothering by substantial turbidity) of an important spawning area are not authorized by this RGP;
26. Discharges of dredged or fill material shall not occur in the proximity of a water supply intake, shall not limit the ability of any existing diversion structure to appropriate water, shall not impair the ability to deliver vested water rights, and shall not adversely impact a stream gauging station;
27. Activities shall not impair reserved tribal rights including, but not limited to, reserved water rights and treaty fishing and hunting rights;
28. National Wild and Scenic River System:
 - a. No activity may occur in a component of the National Wild and Scenic River System, or in a river officially designated by Congress as a "study river" for possible inclusion in the system while the river is in an official study status, unless the appropriate federal agency with direct management responsibility for such river, has determined in writing that the proposed activity will not adversely affect the Wild and Scenic River designation or study status;
 - b. If a proposed RGP activity will occur in a component of the National Wild and Scenic River System, or in a river officially designated by Congress as a "study river" for possible inclusion in the system while the river is in an official study status, the District Engineer will coordinate with the federal agency with direct management responsibility for that river. The permittee shall not begin the RGP activity until notified by the District Engineer that the federal agency with direct management responsibility for that river has determined in writing that the proposed RGP activity will not adversely affect the Wild and Scenic River designation or study status; and

- c. Information on Wild and Scenic Rivers may be obtained from the appropriate federal land management agency responsible for the designated Wild and Scenic River or study river (e.g., National Park Service, U.S. Forest Service, Bureau of Land Management, U.S. Fish and Wildlife Service). Information on these rivers is also available at: <http://www.rivers.gov/>;
29. The permittee must allow representatives from the Corps to inspect the authorized work at any time deemed necessary to ensure that it is being or has been accomplished in compliance with the terms and conditions of the permit;
30. The permittee shall maintain the activity authorized by this permit in good condition and in conformance with the terms and conditions of this permit, including maintenance to ensure public safety and compliance with applicable RGP general conditions, as well as any activity-specific conditions added by the District Engineer to an RGP authorization. The permittee is not relieved of this requirement if construction of the permitted activity is abandoned, although the permittee may make a good faith transfer to a third party in compliance with General Condition 31 below. Should the permittee wish to cease maintenance of the authorized activity, or should the permittee desire to abandon it without a good faith transfer, the permittee shall obtain a modification of the permit from the Corps, which may require restoration of the area; and
31. If the permittee sells the property associated with the permit verification, the permittee may transfer the permit verification to the new owner by submitting a letter to the Corps to validate the transfer. A copy of the permit verification must be attached to the letter, and the letter must contain the following statement and signature: *"When the structures or work authorized by this permit are still in existence at the time the property is transferred, the terms and conditions of this permit, including any special conditions, shall continue to be binding on the new property owner(s). To validate the transfer of this permit and the liabilities associated with compliance with its terms and conditions, have the transferee sign and date below."*

BEST MANAGEMENT PRACTICES:

1. In order to prevent the spread of invasive and/or nuisance species (e.g., Asian Clam, Grand Valley Asian Tapeworm, Green River Mud Snail, New Zealand Mud Snail), the permittee is strongly encouraged to clean heavy equipment prior to and after construction if the equipment was previously used in another stream, river, lake, pond, or wetland within 10 days of initiating work. The following are recommended methods for preventing the spread of invasive aquatic organisms:
 - a. Remove all mud and debris from equipment (tracks, turrets, buckets, drags, teeth, etc.) and spray/soak equipment with a 1:15 solution of disinfection solution containing the following ingredients:
 - (1) Dialkyl dimethyl ammonium chloride (5-10% by weight);
 - (2) Alkyl dimethyl benzyl ammonium chloride (5-10% by weight);
 - (3) Nonyl phenol ethoxylate (5-10% by weight);

- (4) Sodium sesquicarbonate (1-5%); and
- (5) Tetrasodium ethylene diaminetetraacetate (1-15%); and
- b. The equipment should be kept moist for at least 10 minutes, and rinsate should be managed as a solid waste in accordance with local, county, state, or federal regulations. Alternately, equipment, hand tools, boots and any other equipment that was previously used in a river, stream, lake, pond, or wetland prior to moving the equipment to another water body may be disinfected using either of the following methods:
 - (1) Spray/soak equipment with water greater than 140 degrees Fahrenheit for at least 10 minutes; or
 - (2) Sanitize water suction hoses and water transportation tanks (using methods described above) and discard rinse water at an appropriately permitted disposal facility; and
- 2. No petroleum products, chemicals, or other deleterious materials should be allowed to enter or be disposed of in such a manner in which they could enter the waterway or adjacent wetlands. Accordingly, it is recommended that oil absorbent “booms” be installed downstream of the project site during construction activities.

DEFINITIONS:

Bankfull: Bankfull, as defined by Dunne and Leopold (1978), is the stream stage corresponding to the discharge at which channel maintenance is most effective. That is, the discharge at which moving sediment, forming or removing bars, forming or changing bends and meanders, and generally doing work that result in the average morphological characteristics of channels.

Best management practices (BMPs): Policies, practices, procedures, or structures implemented to mitigate the adverse environmental effects on surface water quality resulting from development. BMPs are categorized as structural or non-structural.

Bioengineering Techniques: The combination of biological, mechanical, and ecological concepts to control erosion and stabilize soil through the use of native vegetation or a combination of vegetative and traditional construction materials. Bioengineering techniques include both living and non-living plants. Non-living plants (trees, logs, etc.) are used as construction materials, similar to or in combination with engineered materials. Planted vegetation controls erosion, stabilizes soil, and serves as wildlife and fisheries habitat in riparian areas, between the stream channel and adjacent upland habitats.

Compensatory mitigation: The restoration (re-establishment or rehabilitation), establishment (creation), enhancement, and/or in certain circumstances preservation of aquatic resources for the purposes of offsetting unavoidable adverse impacts which remain after all appropriate and practicable avoidance and minimization has been achieved.

Direct effects: Effects that are caused by the activity and occur at the same time and place.

Discharge: The term “discharge” means any discharge of dredged or fill material into waters of the United States.

Enhancement: The manipulation of the physical, chemical, or biological characteristics of an aquatic resource to heighten, intensify, or improve a specific aquatic resource function(s). Enhancement results in

the gain of selected aquatic resource function(s), but may also lead to a decline in other aquatic resource function(s). Enhancement does not result in a gain in aquatic resource area.

Ephemeral stream: An ephemeral stream has flowing water only during, and for a short duration after, precipitation events in a typical year. Ephemeral stream beds are located above the water table year-round. Groundwater is not a source of water for the stream. Runoff from rainfall is the primary source of water for stream flow.

Establishment (creation): The manipulation of the physical, chemical, or biological characteristics present to develop an aquatic resource that did not previously exist at an upland site. Establishment results in a gain in aquatic resource area.

Historic Property: Any prehistoric or historic district, site (including archaeological site), building, structure, or other object included in, or eligible for inclusion in, the National Register of Historic Places maintained by the Secretary of the Interior. This term includes artifacts, records, and remains that are related to and located within such properties. The term includes properties of traditional religious and cultural importance to an Indian tribe or Native Hawaiian organization and that meet the National Register criteria (36 CFR part 60).

Independent utility: A test to determine what constitutes a single and complete non-linear project in the Corps Regulatory Program. A project is considered to have independent utility if it would be constructed absent the construction of other projects in the project area. Portions of a multi-phase project that depend upon other phases of the project do not have independent utility. Phases of a project that would be constructed even if the other phases were not built can be considered as separate single and complete projects with independent utility.

Indirect effects: Effects that are caused by the activity and are later in time or farther removed in distance, but are still reasonably foreseeable.

Intermittent stream: An intermittent stream has flowing water during certain times of the year, when groundwater provides water for stream flow. During dry periods, intermittent streams may not have flowing water. Runoff from rainfall is a supplemental source of water for stream flow.

Loss of waters of the United States: Waters of the United States that are permanently adversely affected by filling, flooding, excavation, or drainage because of the regulated activity. Permanent adverse effects include permanent discharges of dredged or fill material that change an aquatic area to dry land, increase the bottom elevation of a waterbody, or change the use of a waterbody. The acreage of loss of waters of the United States is a threshold measurement of the impact to jurisdictional waters for determining whether a project may qualify for a general permit; it is not a net threshold that is calculated after considering compensatory mitigation that may be used to offset losses of aquatic functions and services. The loss of stream bed includes the acres or linear feet of stream bed that are filled or excavated as a result of the regulated activity. Waters of the United States temporarily filled, flooded, excavated, or drained, but restored to pre-construction contours and elevations after construction, are not included in the measurement of loss of waters of the United States. Impacts resulting from activities that do not require Department of the Army authorization, such as activities eligible for exemptions under section 404(f) of the Clean Water Act, are not considered when calculating the loss of waters of the United States.

Navigable waters: Waters subject to section 10 of the Rivers and Harbors Act of 1899. These waters are defined at 33 CFR part 329.

Open water: For purposes of this general permit, an open water is any area that in a year with normal patterns of precipitation has water flowing or standing above ground to the extent that an OHWM can be determined. Aquatic vegetation within the area of flowing or standing water is either non-emergent,

sparse, or absent. Vegetated shallows are considered to be open waters. Examples of “open waters” include rivers, streams, lakes, and ponds.

Ordinary High Water Mark: An OHWM is a line on the shore established by the fluctuations of water and indicated by physical characteristics, or by other appropriate means that consider the characteristics of the surrounding areas.

Perennial stream: A perennial stream has flowing water year-round during a typical year. The water table is located above the stream bed for most of the year. Groundwater is the primary source of water for stream flow. Runoff from rainfall is a supplemental source of water for stream flow.

Preservation: The removal of a threat to, or preventing the decline of, aquatic resources by an action in or near those aquatic resources. This term includes activities commonly associated with the protection and maintenance of aquatic resources through the implementation of appropriate legal and physical mechanisms. Preservation does not result in a gain of aquatic resource area or functions.

Re-establishment: The manipulation of the physical, chemical, or biological characteristics of a site with the goal of returning natural/historic functions to a former aquatic resource. Re-establishment results in rebuilding a former aquatic resource and results in a gain in aquatic resource area and functions.

Rehabilitation: The manipulation of the physical, chemical, or biological characteristics of a site with the goal of repairing natural/historic functions to a degraded aquatic resource. Rehabilitation results in a gain in aquatic resource function, but does not result in a gain in aquatic resource area.

Restoration: The manipulation of the physical, chemical, or biological characteristics of a site with the goal of returning natural/historic functions to a former or degraded aquatic resource. For the purpose of tracking net gains in aquatic resource area, restoration is divided into two categories: re-establishment and rehabilitation.

Riffle and pool complex: Riffle and pool complexes are special aquatic sites under the 404(b)(1) Guidelines. Riffle and pool complexes sometimes characterize steep gradient sections of streams. Such stream sections are recognizable by their hydraulic characteristics. The rapid movement of water over a coarse substrate in riffles results in a rough flow, a turbulent surface, and high dissolved oxygen levels in the water. Pools are deeper areas associated with riffles. A slower stream velocity, a streaming flow, a smooth surface, and a finer substrate characterize pools.

Riparian areas: Riparian areas are lands next to streams, lakes, and estuarine-marine shorelines. Riparian areas are transitional between terrestrial and aquatic ecosystems, through which surface and subsurface hydrology connects riverine, lacustrine, estuarine, and marine waters with their adjacent wetlands, non-wetland waters, or uplands. Riparian areas provide a variety of ecological functions and services and help improve or maintain local water quality. (See general condition 23.)

Single and complete project: For non-linear projects, such as those activities authorized by this RGP, the term “single and complete project” is defined at 33 CFR 330.2(i) as the total project proposed or accomplished by one owner/developer or partnership or other association of owners/developers. A single and complete non-linear project must have independent utility (see definition of “independent utility”). Single and complete non-linear projects may not be “piecemealed” to avoid the limits in this general permit.

Stream bed: The substrate of the stream channel between the OHWMs. The substrate may be bedrock or inorganic particles that range in size from clay to boulders. Wetlands contiguous to the stream bed, but outside of the OHWMs, are not considered part of the stream bed.

Stream channelization: The manipulation of a stream's course, condition, capacity, or location that causes more than minimal interruption of normal stream processes. A channelized stream remains a water of the United States.

Structure: An object that is arranged in a definite pattern of organization. Examples of structures include, without limitation, any pier, boat dock, boat ramp, wharf, dolphin, weir, boom, breakwater, bulkhead, revetment, riprap, jetty, artificial island, artificial reef, permanent mooring structure, power transmission line, permanently moored floating vessel, piling, aid to navigation, or any other manmade obstacle or obstruction.

Tribal lands: Any lands title to which is either: 1) held in trust by the United States for the benefit of any Indian tribe or individual; or 2) held by any Indian tribe or individual subject to restrictions by the United States against alienation.

Tribal rights: Those rights legally accruing to a tribe or tribes by virtue of inherent sovereign authority, unextinguished aboriginal title, treaty, statute, judicial decisions, executive order or agreement, and that give rise to legally enforceable remedies.

Waterbody: For purposes of this general permit, a waterbody is a jurisdictional water of the United States. If a wetland is adjacent to a waterbody determined to be a water of the United States, that waterbody and any adjacent wetlands are considered together as a single aquatic unit (see 33 CFR 328.4(c)(2)). Examples of "waterbodies" include streams, rivers, lakes, ponds, and wetlands.

FURTHER INFORMATION:

1. District Engineers have authority to determine if an activity complies with the terms and conditions of a RGP;
2. Congressional Authorities. This permit authorizes work in accordance with:
 - a. Section 10 of the Rivers and Harbors Act of 1899 (33 U.S.C. 403); and
 - b. Section 404 of the Clean Water Act (33 U.S.C. 1344);
3. Limits of this authorization:
 - a. This permit does not obviate the need to obtain other federal, state, or local authorizations required by law;
 - b. This permit does not grant any property rights or exclusive privileges; and
 - c. This permit does not authorize interference with any existing or proposed federal projects;
4. Limits of Federal Liability. In issuing this permit, the federal government does not assume any liability for the following:
 - a. Damages to the permitted project, or uses thereof, as a result of other permitted or unpermitted activities or from natural causes;
 - b. Damages to the permitted project, or uses thereof, as a result of current or future activities undertaken by or on behalf of the United States in the public interest;

- c. Damages to persons, property, or other permitted or unpermitted activities or structures caused by the activity authorized by this permit;
 - d. Design or construction deficiencies associated with the permitted work; and
 - e. Damage claims associated with any future modification, suspension, or revocation of this permit;
5. Reliance on applicant's data. The determination by the Corps that a verification based upon this permit is not contrary to the public interest will be made in reliance on the information provided by the applicant;
6. Re-evaluation of permit decisions. The Corps may reevaluate its decision on this permit at any time the circumstances warrant (33 CFR 325.7(a)). Circumstances that could require a reevaluation include, but are not limited to, the following:
- a. Failure to comply with the terms and conditions of this permit;
 - b. The information provided by you in support of your permit application proves to have been false, incomplete, or inaccurate; or
 - c. Significant new information surfaces that the Corps did not consider before verifying that the project is authorized by this permit.

Such re-evaluation may result in a determination that it is appropriate to use the suspension, modification, and revocation procedures contained in 33 CFR 325.7 or enforcement procedures such as those contained in 33 CFR 326.4 and 326.5. The referenced enforcement procedures provide for the issuance of an administrative order requiring the permittee to comply with the terms and conditions of the permit and for the initiation of legal action where appropriate;

7. Extensions. General Condition 1 establishes a time limit for the completion of the activity authorized by this permit. Unless there are circumstances requiring either a prompt completion of the authorized activity or a reevaluation of our decision, the Corps will normally give favorable consideration to a request for an extension of this time limit; and
8. Renewal of the RGP: This RGP may be reviewed for reissuance prior to its expiration date. Any reissuance will be processed in accordance with 33 CFR 325.2 including a public notice and environmental procedures and documentation required by the national Environmental Policy Act of 1969.

This permit becomes effective when the federal official, designated to act for the Sacramento, Albuquerque, and Omaha District Engineers, has signed below.

20 October 2017

Allan Steinle, Chief
Albuquerque District Regulatory Division

DATE

COMPLIANCE CERTIFICATION

Permit File Number: SPK-2017-00468

Regional General Permit Number: RGP 37 - Stream Stabilization Projects in Colorado

Permittee: Iron Mountain Hot Springs, LLC
Attn: Mr. Steve Beckley
281 Centennial Street
Glenwood Springs, Colorado 81601

County: Garfield

Date of Verification: December 22, 2017

Within 30 days after completion of the activity authorized by this permit, sign this certification and return it to the following address:

U.S. Army Corps of Engineers
Sacramento District
Colorado West Regulatory Branch
400 Rood Avenue, Room 224
Grand Junction, Colorado 81501
(970) 243-1199
Fax (970) 241-2358
DLL-CESPK-RD-Compliance@usace.army.mil

Please note that your permitted activity is subject to a compliance inspection by a U.S. Army Corps of Engineers representative. If you fail to comply with the terms and conditions of the permit your authorization may be suspended, modified, or revoked. If you have any questions about this certification, please contact the U.S. Army Corps of Engineers.

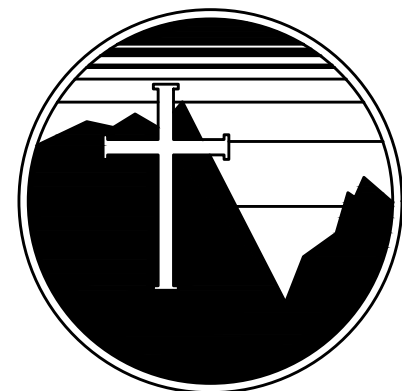
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I hereby certify that the work authorized by the above-referenced permit, including all the required mitigation, was completed in accordance with the terms and conditions of the permit verification.

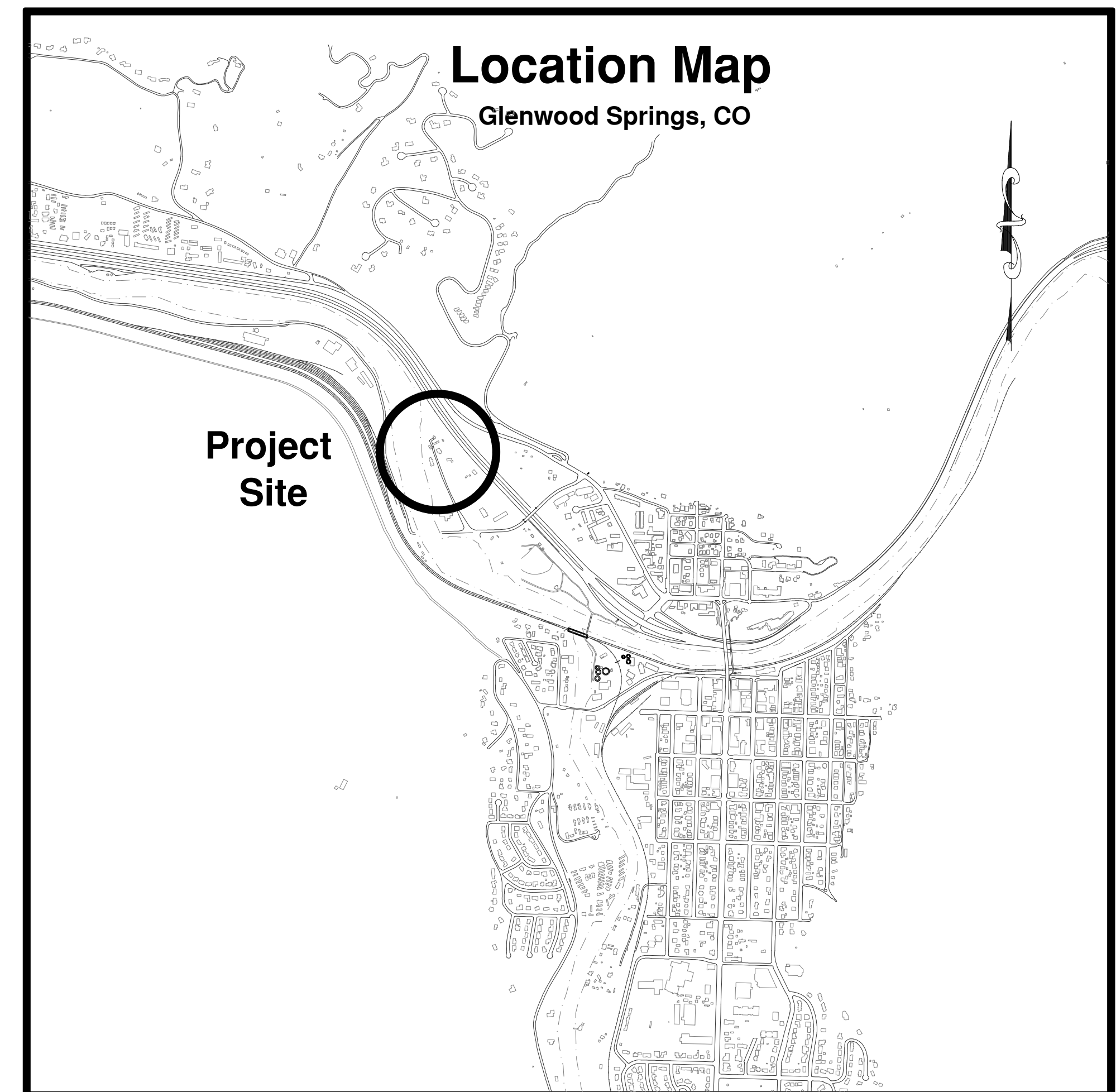
Signature of Permittee

Date

PREPARED BY:



Civil and Environmental Consulting and Design
826 1/2 Grand Avenue Glenwood Springs, CO 81601
ph 970.945.5544 fx 970.945.5558 www.mountaincross-eng.com



LEGEND & ABBREVIATIONS

●	FOUND PROPERTY CORNER	APPROX	APPROXIMATE
	= TREE	ARCH	ARCHITECT
---	GAS --- = GAS LINE	BLDG	BUILDING
---	FO --- = FIBER OPTIC	BM	BENCHMARK
---	SS --- = SANITARY SEWER	BW	BOTTOM OF WALL
---	W --- = WATER LINE	CF	CUBIC FEET
---	T --- = TELEPHONE LINE	CIP	CAST IN PLACE
---	P --- = ELECTRIC LINE	CJ	CONTROL JOINT
---	GAS --- = GAS	CL	CONTROL LINE
(M)	= MANHOLE	CLR	CLEAR
	= STORM INLET	C/O	CLEAN-OUT
T	= PHONE PEDESTAL	CONC	CONCRETE
CTV	= CABLE TELEVISION PEDESTAL	CY	CUBIC YARD
GM	= GAS METER	DEG	DEGREES
	= ELECTRIC METER	DEM0	DEMOLISH
● DS	= DOWN SPOUT	DM	DIMENSION
	= STORM CLEAN-OUT	DLS	DOWN SPOUT
○	= LIGHT POST	DWC	DRAWING
	= WATER VALVE	ELEV	ELEVATION
	= WATER CURB STOP VALVE	ENG	ENGINEER
	= FIRE HYDRANT	EOP	EDGE OF PAVEMENT
	= ELEVATION SPOT	EW	EACH WAY
		EXIST	EXISTING
		FG	FINISHED GRADE
		FL	FLOW LINE
		GAL	GALLON
		GALV	GALVANIZED
		GB	GRADE BREAK
		HORZ	HORIZONTAL
		HT	HEIGHT
		ID	INSIDE DIAMETER
		INV	INVERT
		IRR	IRRIGATION
		JT	JOINT
		MAX	MAXIMUM
		MECH	MECHANICAL
		MH	MANHOLE
		MIN	MINIMUM
		OD	OUTSIDE DIAMETER
		PC	POINT OF CURVATURE
		PERF	PERFORATED
		PE	PEDESTAL
		PI	POINT OF INTERSECTION
		PL	PROPERTY LINE
		PSF	POUNDS PER SQUARE FOOT
		PSI	POUNDS PER SQUARE INCH

1. Contractor responsible to coordinate locating all utilities, potholing if necessary. Call 811 72 hours prior to digging. Contractor responsible for any and all damage to utility lines.
2. Contractor responsible to obtain all necessary permits and/or approvals prior to beginning construction.
3. Contractor to coordinate with geotechnical engineer for recommendations concerning trench stability, safety, and shoring.
4. Maintain 18" separation between any utilities that may need to be crossed. If 18" can not be maintained notify engineer for alternatives.
5. The contractor shall verify all dimensions, elevations and site conditions before starting work and notify the architect and engineer immediately of any discrepancies.
6. All omissions and conflicts between the various elements of the working drawings and/or specifications shall be brought to the attention of the architect and engineer before proceeding with work.
7. The contractor shall review completely the soils report and follow all recommendations therein. Design assumes that all recommendations in the soils report will be implemented.
8. Finished excavation shall be neat and true to line with all loose material and standing water removed from excavations.
9. Due to corrosive soils, all concrete shall be Type S, sulfate resistive with Xypex crystalline additive.

SEWER: CITY OF GLENWOOD SPRINGS
MATT LANGHORST #970.384.6438

WATER: CITY OF GLENWOOD SPRINGS
MATT LANGHORST #970.384.6438

PHONE/INTERNET: CENTURY LINK
JASON SHARPE #970.384.0238

FIBER: CITY BROADBAND
ERIC ARNETTE #970.384.6416

CABLE TV/INTERNET: COMCAST
MICHAEL JOHNSON #970.930.4713

GAS: BLACK HILLS ENERGY
RYAN POGUE #719.208.3577

POWER: CITY OF GLENWOOD SPRINGS
DOUG HAZZARD #970.384.6353

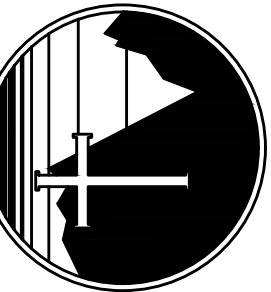
DRAWING LIST:

C1	Cover Sheet
C2	Demo, Site Layout, & Dimensions
C3	Utility Plan
C4	Grading and Drainage Plan
C5	Stormwater Management Plan
C6	Parking Lot B - Plan & Profile
C7	Parking Lot C - Plan & Profile
C8	Parking Lot D & E - Plan & Profile
C9	Details
C10	Details



Know what's **below**.
Call before you dig.

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ENGINEERING, INC.**
Civil and Environmental Consulting and Design
826 1/2 Grand Avenue Glenwood Springs, CO 81601
ph 970.945.5544 fx 970.945.5558 www.mountaincross-eng.com



Iron Mountain Hot Springs

Cover

PRELIMINARY
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[illegible]

TE 07/22/20

ALE $1''=20'$

GINEER
C HALF

WG FILE
IronM4n

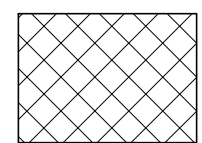
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SHEET

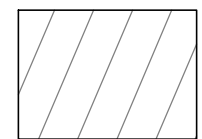
C1

- GENERAL NOTES**
1. Contractor responsible to coordinate locating all utilities, potholing if necessary. Call 811 72 hours prior to digging. Contractor responsible for any and all damage to utility lines.
 2. Contractor responsible to obtain all necessary permits and/or approvals prior to beginning construction.
 3. Contractor to coordinate with geotechnical engineer for recommendations concerning trench stability, safety, and shoring.
 4. Maintain 18" separation between any utilities that may need to be crossed. If 18" can not be maintained notify engineer for alternatives.
 5. The following codes and editions were used:
 - ACI Building Code (ACI 318-89)
 - Manual of Steel Construction (9th Edition)
 6. The contractor shall verify all dimensions, elevations and site conditions before starting work and notify the architect and engineer immediately of any discrepancies.
 7. All omissions and conflicts between the various elements of the working drawings and/or specifications shall be brought to the attention of the architect and engineer before proceeding with work.
 8. The contractor shall review completely the soils report and follow all recommendations therein. Design assumes that all recommendations in the soils report will be implemented.
 9. Finished excavation shall be neat and true to line with all loose material and standing water removed from excavations.
 10. Concrete work shall conform to the requirements of ACI 301, specifications for structural concrete for buildings.
 - Minimum strength: $f'c = 3500$ psi at 28 days.
 - Slump: $4" \pm 1"$
 - Maximum water/cement ratio $w/c = 0.45$.
 - Air = $4.0\% - 7.0\%$
 - Aggregate: per ASTM C33, maximum $1\frac{1}{2}"$
 - No water shall be added on site which will increase slump to above 5". Concrete to be mixed per ASTM C94 maximum temperature in mixer: $90^\circ F$. Due to corrosive soils, all concrete shall be Type S, sulfate resistive with Xypex crystalline additive.
 11. Curing shall immediately follow the finish operation. Keep concrete continuously moist for at least 7 days using a polyethylene film securely held in place or an alternate approved method.
 12. Place per ACI standard 614.
 13. Cement shall be per ASTM C150, Type I, low alkali.
 14. Submit mix design to architect for review by engineer a minimum of one week prior to first concrete delivery. No admixtures containing chloride salts are permitted.
 15. Concrete shall not be in contact with aluminum.
 16. Rebars: deformed steel, $f_y = 60$ ksi. ASTM spec. A615 grade 60. All rebar bends to be made cold.
 17. Reinforcing in concrete placed against earth without forms is to be supported with approved nonmetallic chairs, or another method approved by the engineer.
 18. Minimum cover on rebar where concrete is cast against earth is 3" and 2" in other locations.

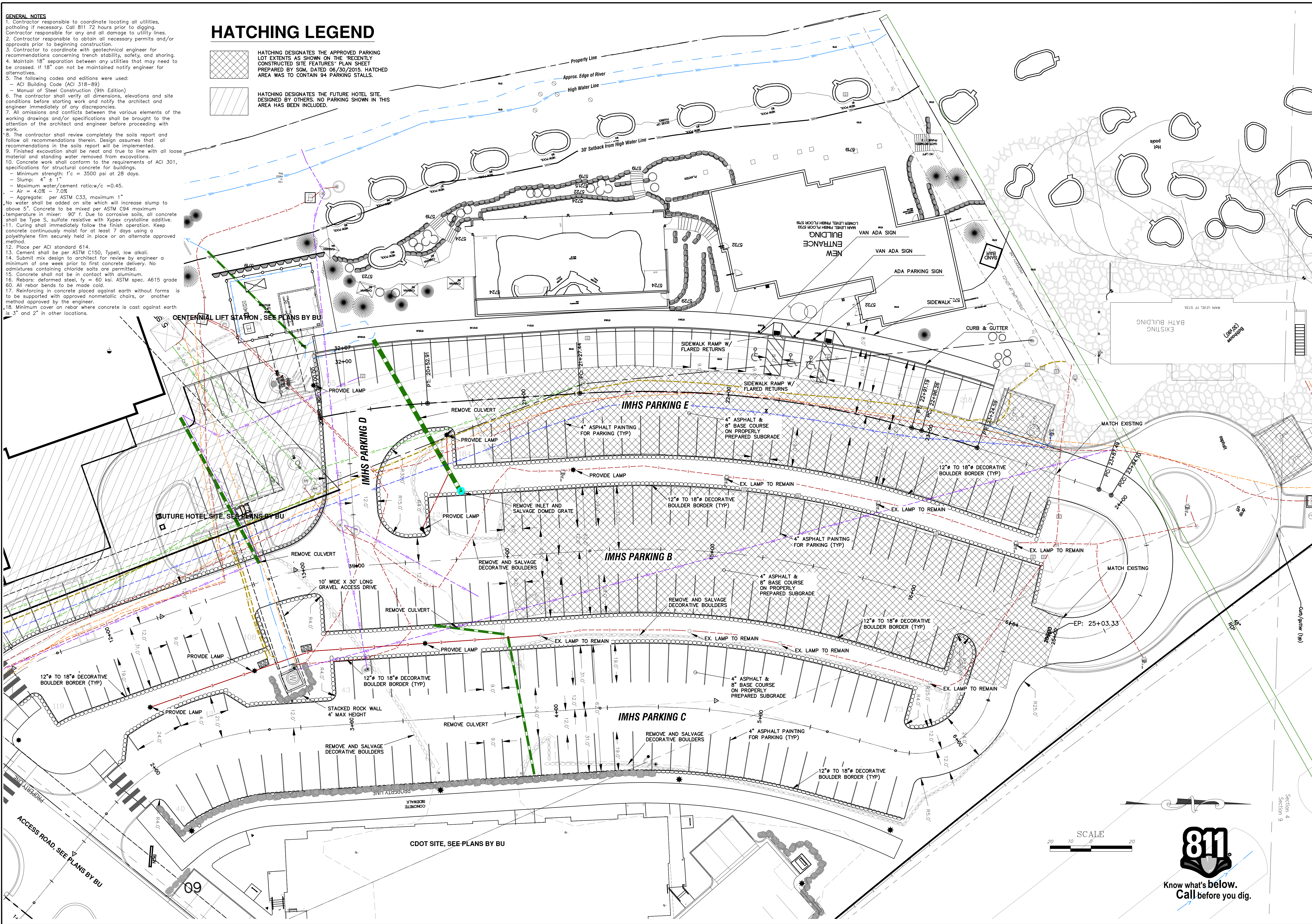
HATCHING LEGEND



HATCHING DESIGNATES THE APPROVED PARKING LOT EXTENTS AS SHOWN ON THE 'RECENTLY CONSTRUCTED SITE FEATURES' PLAN SHEET PREPARED BY SOM, DATED 06/30/2015. HATCHED AREA WAS TO CONTAIN 94 PARKING STALLS.



HATCHING DESIGNATES THE FUTURE HOTEL SITE. DESIGNED BY OTHERS. NO PARKING SHOWN IN THIS AREA HAS BEEN INCLUDED.



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Iron Mountain Hot Springs
Demo, Site Layout, & Dimensions

PRELIMINARY
NOT FOR CONSTRUCTION

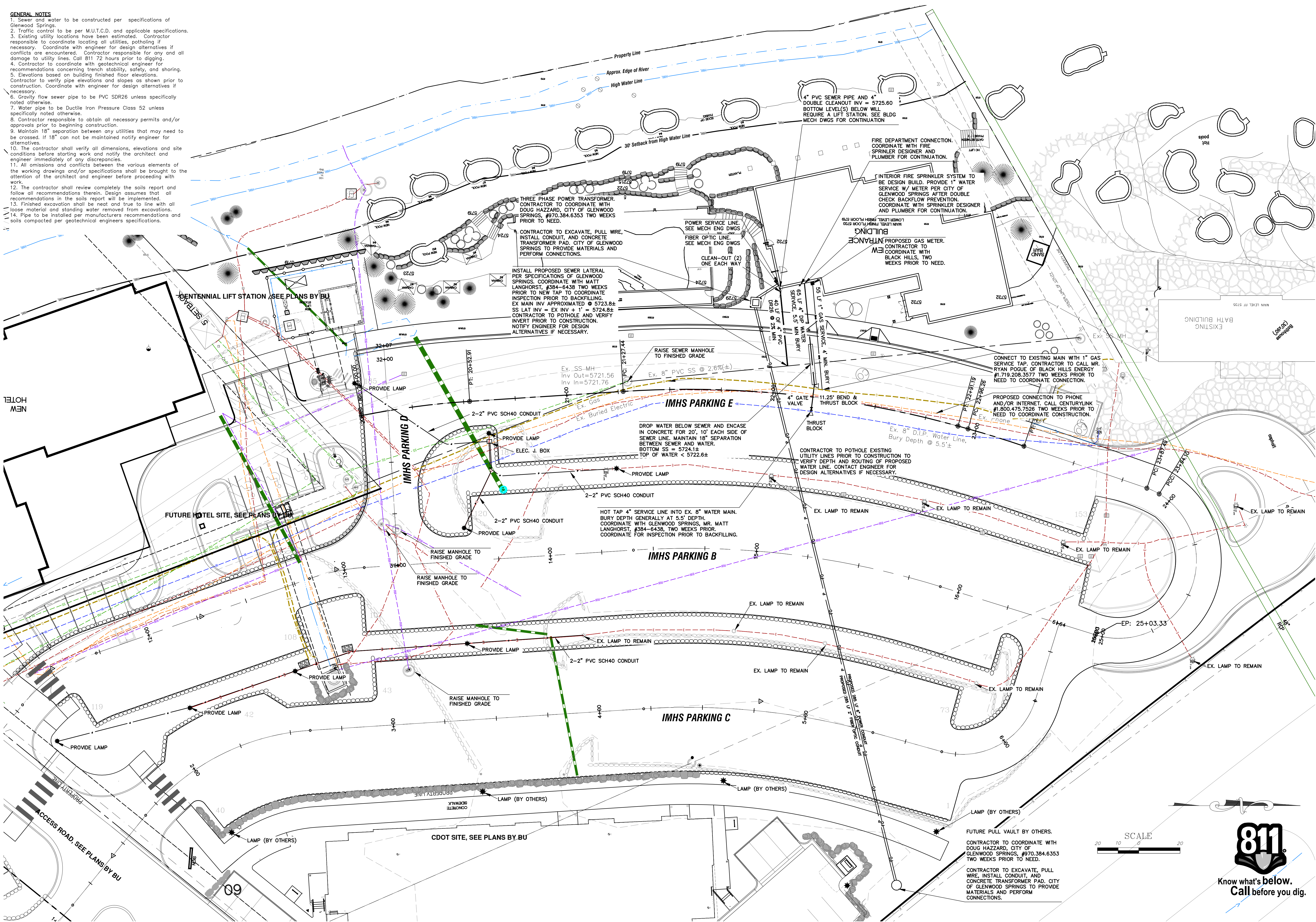
REVISIONS	DATE	BY	COMMENTS

DATE 08/04/20
SCALE 1"=20'
ENGINEER C. HALE
DWG FILE IronMtn
JOB 564-005
SHEET

811
Know what's below.
Call before you dig.

C2

- GENERAL NOTES**
1. Sewer and water to be constructed per specifications of Glenwood Springs.
 2. Traffic control to be per M.U.T.C.D. and applicable specifications.
 3. Existing utility locations have been estimated. Contractor responsible to coordinate locating all utilities, potholing if necessary. Coordinate with engineer for design alternatives if conflicts are encountered. Contractor responsible for any and all damage to utility lines. Call 811 72 hours prior to digging.
 4. Contractor to coordinate with geotechnical engineer for recommendations concerning trench stability, safety, and shoring.
 5. Elevations based on building finished floor elevations. Contractor to verify pipe elevations and slopes as shown prior to construction. Coordinate with engineer for design alternatives if necessary.
 6. Gravity flow sewer pipe to be PVC SDR26 unless specifically noted otherwise.
 7. Water pipe to be Ductile Iron Pressure Class 52 unless specifically noted otherwise.
 8. Contractor responsible to obtain all necessary permits and/or approvals prior to beginning construction.
 9. Maintain 18" separation between any utilities that may need to be crossed. If 18" can not be maintained notify engineer for alternatives.
 10. The contractor shall verify all dimensions, elevations and site conditions before starting work and notify the architect and engineer immediately of any discrepancies.
 11. All omissions and conflicts between the various elements of the working drawings and/or specifications shall be brought to the attention of the architect and engineer before proceeding with work.
 12. The contractor shall review completely the soils report and follow all recommendations therein. Design assumes that all recommendations in the soils report will be implemented.
 13. Finished excavation shall be neat and true to line with all loose material and standing water removed from excavations.
 14. Pipe to be installed per manufacturers recommendations and soils compacted per geotechnical engineers specifications.



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Iron Mountain Hot Springs
Utility Plan

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS	DATE	BY	COMMENTS

DATE 08/04/20

SCALE 1"=20'

ENGINEER C. HALE

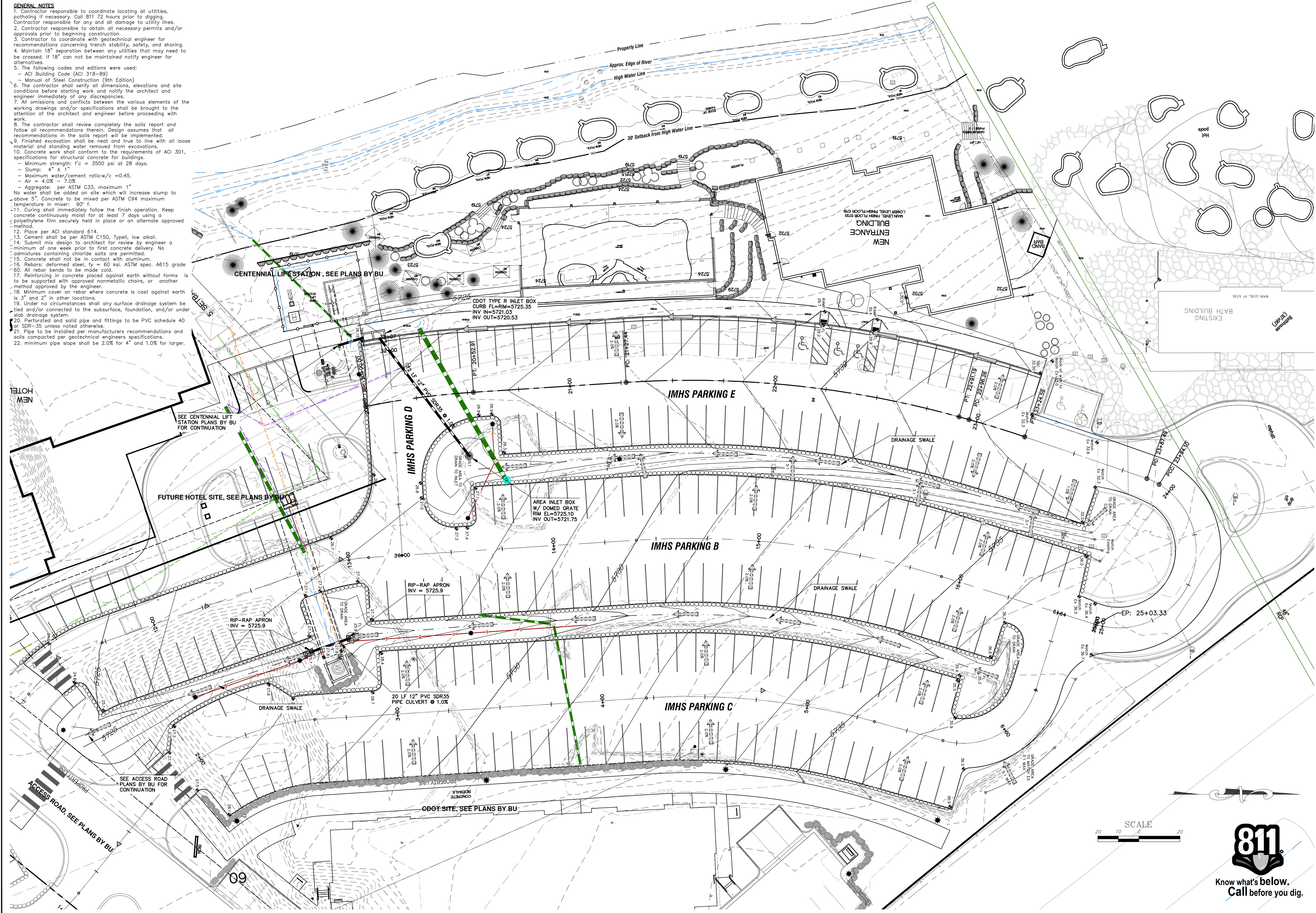
DWG FILE IronMtn

JOB 564-005

SHEET **C3**

Know what's below.
Call before you dig.

- GENERAL NOTES**
1. Contractor responsible to coordinate locating all utilities, potholing if necessary. Call 811 72 hours prior to digging. Contractor responsible for any and all damage to utility lines.
 2. Contractor responsible to obtain all necessary permits and/or approvals prior to beginning construction.
 3. Contractor to coordinate with geotechnical engineer for recommendations concerning trench stability, safety, and shoring.
 4. Maintain 18" separation between any utilities that may need to be crossed. If 18" can not be maintained notify engineer for alternatives.
 5. The following codes and editions were used:
 - ACI Building Code (ACI 318-89)
 - Manual of Steel Construction (9th Edition)
 6. The contractor shall verify all dimensions, elevations and site conditions before starting work and notify the architect and engineer immediately of any discrepancies.
 7. All omissions and conflicts between the various elements of the working drawings and/or specifications shall be brought to the attention of the architect and engineer before proceeding with work.
 8. The contractor shall review completely the soils report and follow all recommendations therein. Design assumes that all recommendations in the soils report will be implemented.
 9. Finished excavation shall be neat and true to line with all loose material and standing water removed from excavations.
 10. Concrete work shall conform to the requirements of ACI 301, specifications for structural concrete for buildings.
 - Minimum strength: $f'_c = 3500$ psi at 28 days.
 - Slump: $4" \pm 1"$
 - Maximum water/cement ratio/w/c = 0.45.
 - Air = 4.0% $\pm 0.5\%$
 - Aggregate: per ASTM C33, maximum 1"
 - No water shall be added on site which will increase slump to above 5". Concrete to be mixed per ASTM C94 maximum temperature in mixer: 90° F.
 11. Curing shall immediately follow the finish operation. Keep concrete continuously moist for at least 7 days using a polyethylene film securely held in place or an alternate approved method.
 12. Place per ACI standard 614.
 13. Cement shall be per ASTM C150, Type I, low alkali.
 14. Submit mix design to architect for review by engineer. A minimum of one week prior to first concrete delivery. No admixtures containing chloride salts are permitted.
 15. Concrete shall not be in contact with aluminum.
 16. Rebar: deformed steel, fy = 60 ksi, ASTM spec. A615 grade 60. All rebar bends to be made cold.
 17. Reinforcing in concrete placed against earth without forms is to be supported with approved nonmetallic chairs, or another method approved by the engineer.
 18. Minimum cover on rebar where concrete is cast against earth is 3" and 2" in other locations.
 19. Under no circumstances shall any surface drainage system be tied and/or connected to the subsurface, foundation, and/or under slab drainage system.
 20. Perforated and solid pipe and fittings to be PVC schedule 40 or SDR-35 unless noted otherwise.
 21. Pipe to be installed per manufacturers recommendations and soils compacted per geotechnical engineers specifications.
 22. minimum pipe slope shall be 2.0% for 4" and 1.0% for larger.



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Iron Mountain Hot Springs
Grading and Drainage

REVISIONS	
DATE	BY

DATE 08/04/20

SCALE 1"=20'

ENGINEER C. HALE

DWG FILE IronMtn

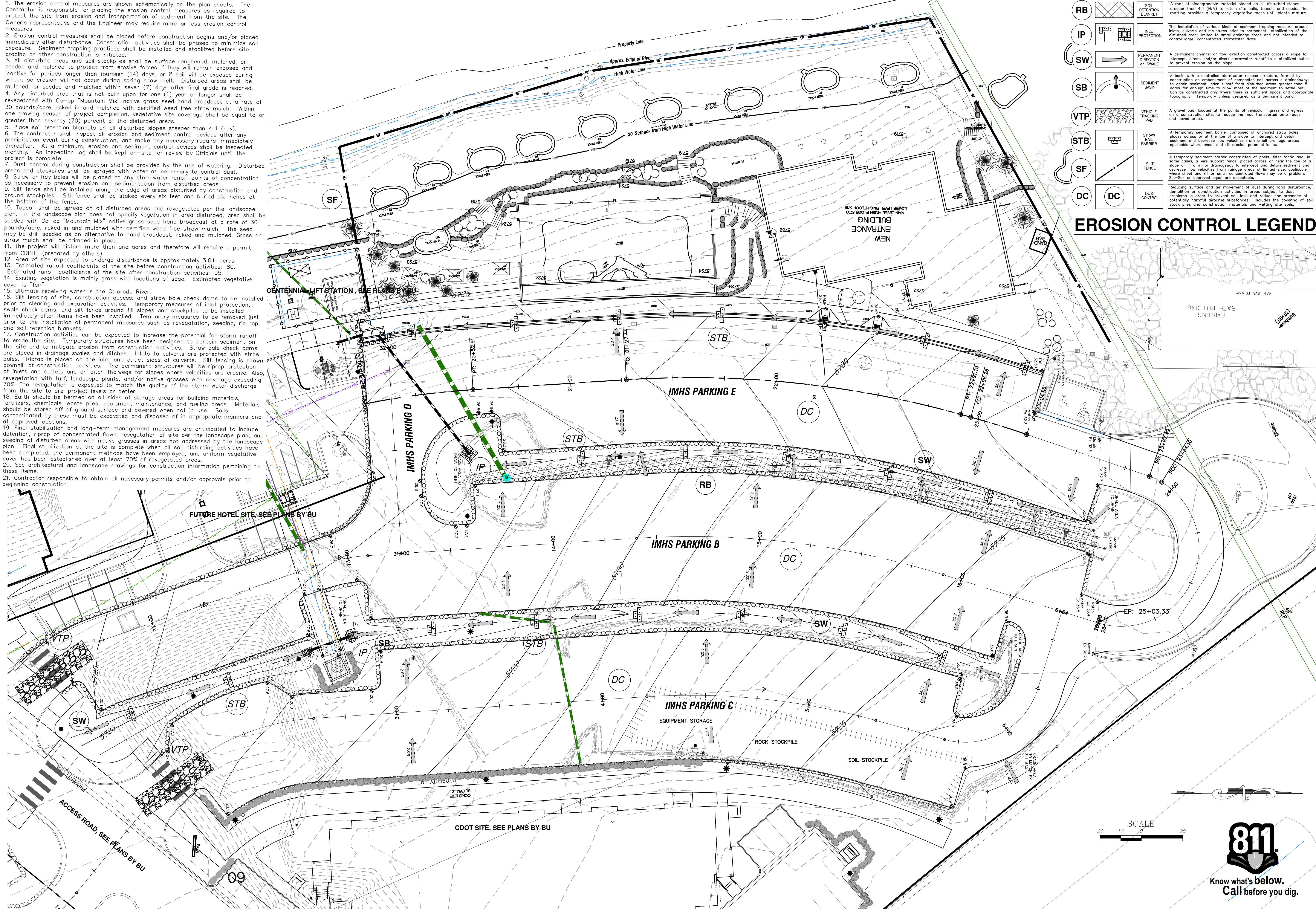
JOB 564-005

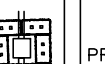
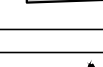
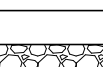
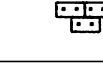
SHEET C4

811
Know what's below.
Call before you dig.

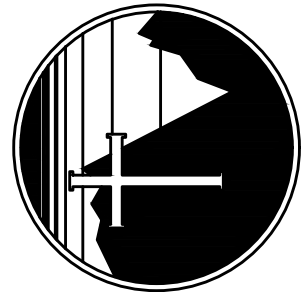
PRELIMINARY
NOT FOR CONSTRUCTION

1. The erosion control measures are shown schematically on the plan sheets. The Contractor is responsible for placing the erosion control measures as required to protect the site from erosion and transportation of sediment from the site. The Owner's representative and the Engineer may require more or less erosion control measures.
2. Erosion control measures shall be placed before construction begins and/or placed immediately after disturbance. Construction activities shall be phased to minimize soil exposure. Sediment trapping practices shall be installed and stabilized before site grading or other construction is initiated.
3. All disturbed areas and soil stockpiles shall be surface roughened, mulched, or seeded and mulched to protect from erosive forces if they will remain exposed during winter, so erosion will not occur during spring snow melt. Disturbed areas shall be mulched, or seeded and mulched within seven (7) days after final grade is reached.
4. Any disturbed area that is not built upon for one (1) year or longer shall be revegetated with Co-op "Mountain Mix" native grass seed hand broadcast at a rate of 10 pounds/acre, raked in and mulched with certified weed free straw mulch. Within one (1) month of construction, the revegetated area coverage shall be equal to or greater than seventy (70) percent of the disturbed areas.
5. Place soil retention blankets on all disturbed slopes steeper than 4:1 (h:v).
6. The contractor shall inspect all erosion and sediment control devices after any precipitation event during construction, and make any necessary repairs immediately thereafter. At a minimum, erosion and sediment control devices shall be inspected monthly. A construction log shall be kept on-site for review by Officials until the project is complete.
7. Dust control during construction shall be provided by the use of watering. Disturbed areas and stockpiles shall be sprayed with water as necessary to control dust.
8. Straw or hay bales will be placed at any stormwater runoff points of concentration as necessary to prevent erosion and sedimentation from disturbed areas.
9. Silt fence shall be installed along the edge of areas disturbed by construction and around stockpiles. Silt fence shall be staked every six feet and buried six inches at the bottom of the fence.
10. Topsoil shall be spread on all disturbed areas and revegetated per the landscape plan. The landscape plan does not specify vegetation in area disturbed, area shall be seeded with Co-op "Mountain Mix" native grass seed hand broadcast at a rate of 30 pounds/acre, raked in and mulched with certified weed free straw mulch. Grass may be drill seeded as an alternative to hand broadcast, raked and mulched. Grass or straw mulch shall be crimped in place.
11. The project will disturb more than one acre and therefore will require a permit from CDPHE (prepared by others).
12. Areas to be undisturbed by construction is approximately 3.04 acres.
13. Estimated runoff coefficients of the site before construction activities: 80.
14. Estimated runoff coefficients of the site after construction activities: 95.
14. Existing vegetation is mainly grass with locations of sage. Estimated vegetative cover is "fair".
15. Ultimate receiving water is the Colorado River.
16. Silt traps or silt ponds of concentrated flow, revegetation of raw bale check dams to be installed prior to clearing and excavation activities. Temporary measures of silt protection, straw check dams, and silt fence along hill slopes and stockpiles to be installed immediately after items have been installed. Temporary measures to be removed just prior to the installation of permanent measures such as revegetation, seeding, rip rap, and riprap placement.
17. Construction activities can be expected to increase the potential for storm runoff to erode the site. Temporary structures have been designed to contain sediment on the site and to mitigate erosion from construction activities. Straw bale check dams are placed in drainage swales and ditches. Inlets to culverts are protected with straw ditches. Riprap is placed on the inlet and outlet sides of culverts. Silt fencing is shown along the ditch and culvert. Straw piles, silt fences, and riprap protection at inlets and outlets and on ditch thresholds for slopes where velocities are erosive. Also, revegetation with turf, landscape plants, and/or native grasses with coverage exceeding 70%. The revegetation is expected to match the quality of the storm water discharge from the site to pre-project levels or better.
18. Earth shall be bermed on all sides of storage areas for building materials, fertilizers, chemicals, fuel, oil, and other materials. Materials shall be stored off of ground surface and covered when not in use. Soil contaminated by these items must be excavated and disposed of in appropriate manners and at approved locations.
19. Final stabilization and long-term management measures are anticipated to include, but not be limited to, revegetation of site per the landscape plan, and seeding of disturbed areas with native grasses in areas not addressed by the landscape plan. Final stabilization of the site is complete when all soil disturbing activities have been completed, the permanent methods have been employed, and uniform vegetative cover has been established over at least 70% of revegetated areas.
20. See the architectural and landscape drawings for construction information pertaining to these items.
21. Contractor responsible to obtain all necessary permits and/or approvals prior to beginning construction.



RB		A mat of biodegradable material placed on all disturbed slopes to prevent soil erosion and retain site soils, topsoil, and seedlings. The matting provides a temporary vegetative mesh until plants mature.
IP		The installation of various kinds of sediment trapping measure around inlets, culverts and structures prior to permanent stabilization of the disturbed area; limited to small drainage areas and not intended to control large, concentrated stormwater flows.
SW		A permanent channel or flow direction constructed across a slope to intercept, divert and/or divert stormwater runoff to a stabilized outlet to prevent erosion on the slope.
SB		A basin with a controlled stormwater release structure, formed by constructing an embankment of compacted soil across a drainageway to detain sediment-laden runoff from disturbed areas greater than 5 acres. It is used to trap most of the sediment to be settled. Sediment can be constructed only where there is sufficient space and appropriate topography. Temporary unless designed as a permanent pond.
VTP		A gravel pad, located at the points of vehicular ingress and egress on a construction site, to reduce the mud transported onto roads and paved areas.
STB		A temporary sediment barrier composed of anchored straw bales placed along all the toe of a slope to intercept and detain sediment and decrease flow velocities from small drainage areas; applicable where sheet and rill erosion potential is low.
SF		A temporary sediment barrier constructed of posts, filter fabric and, in some cases, a wire support fence, placed across the toe of a slope or in a minor drainageway to intercept and detain sediment and decrease velocities from small drainage areas of limited size, applicable where sheet and rill or small concentrated erosion may be a problem. Silt-Sniff or approved equal is acceptable.
DC		Reducing surface and air movement of dust during land disturbance, demolition or construction activities in areas subject to wind-blown problems in order to prevent soil loss and reduce the presence of potentially harmful airborne substances. Includes the covering of soil stock piles and construction materials and wetting site soils.

EROSION CONTROL LEGEND



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Phone 970.945.5544 Fax 970.945.5558 www.mountaincrossaer.com

Iron Mountain Hot Springs Stormwater Management Plan

Stormwater Management Plan

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS		
DATE	BY	COMMENTS

DATE 08/04/20

SCALE 1"=20'

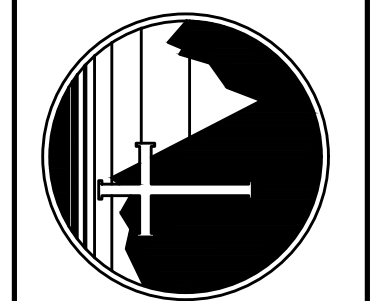
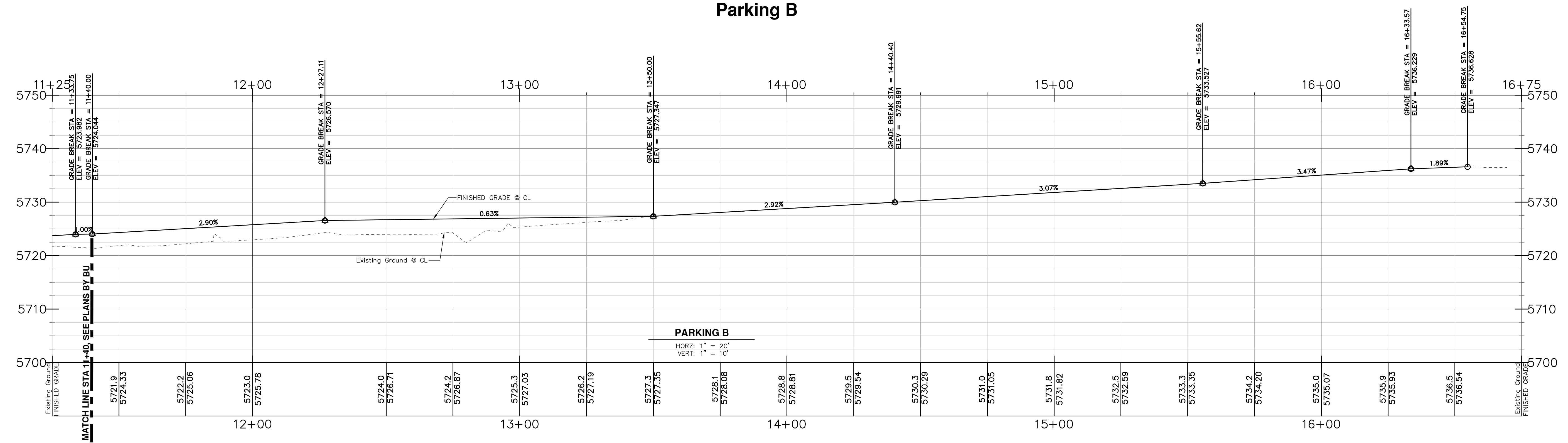
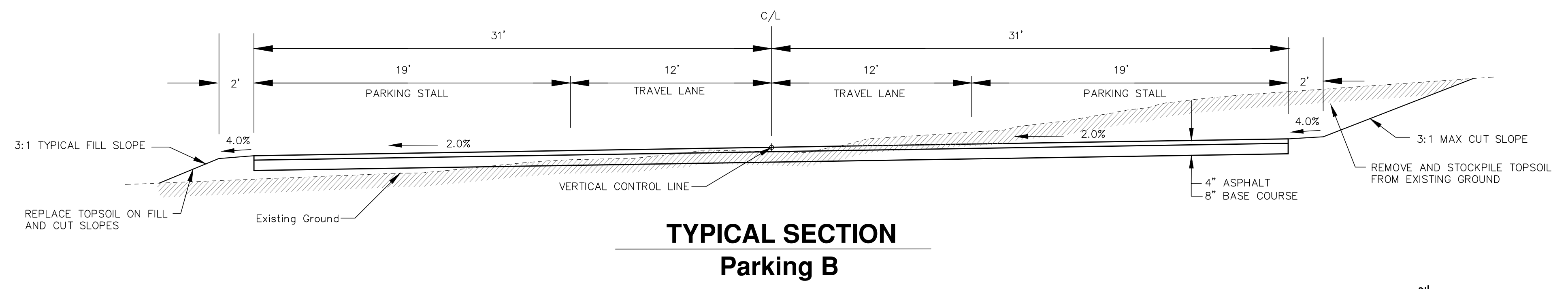
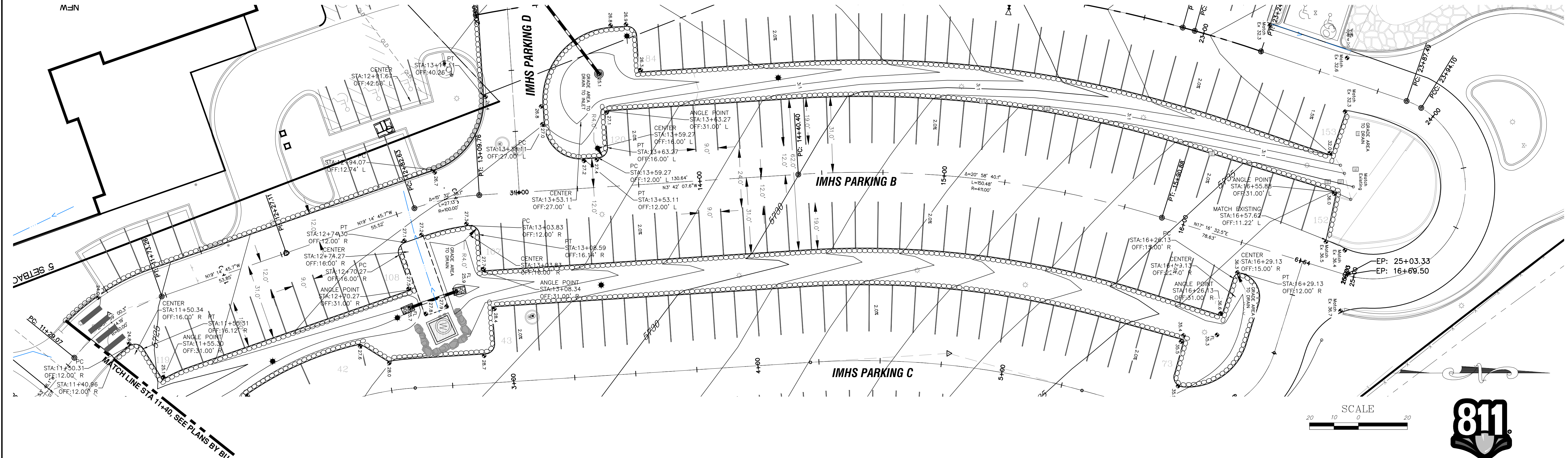
ENGINEER
C. HALE

DWG FILE IronMtn

JOB 564-005

SHEET

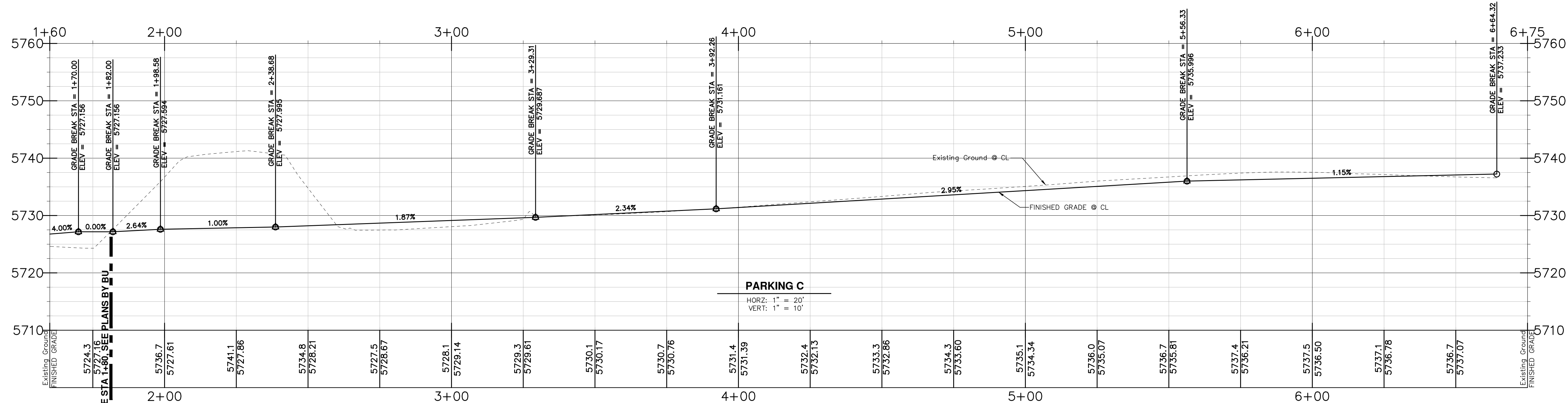
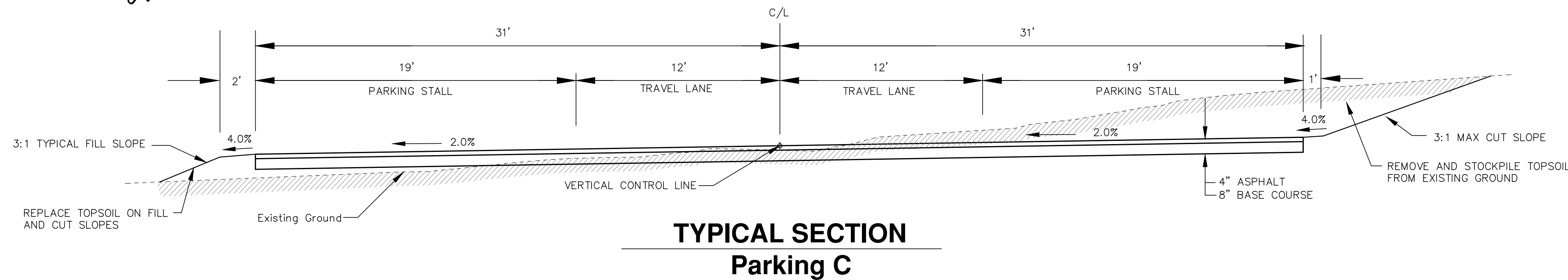
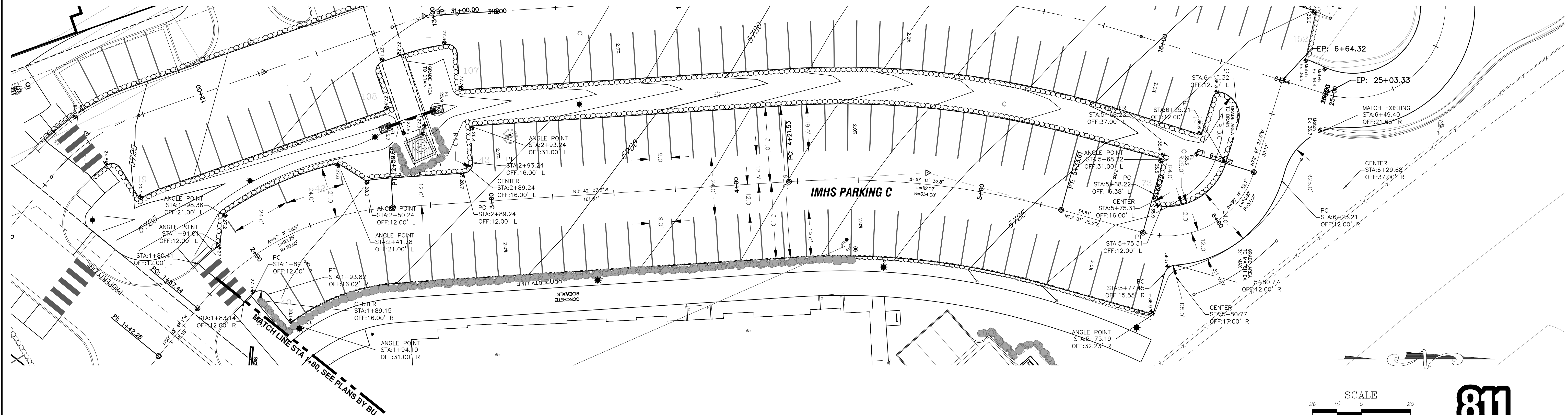
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Iron Mountain Hot Springs Parking Lot B Plan and Profile

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS		DATE	BY	COMMENTS
DATE		07/22/20		
SCALE		Horz: 1"=20' Vert: 1"=10'		
ENGINEER		C. HALE		
DWG FILE		IronMtn		
JOB		564-005		
SHEET		C6		

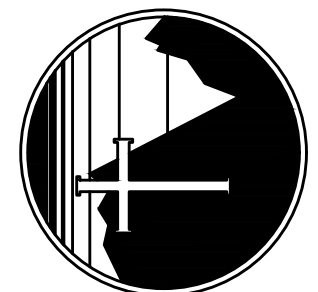


SCALE
20 10 0 20



Know what's below.
Call before you dig.

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ph 970.945.5544 fx 970.945.5588 www.mountaincross-eng.com



**Iron Mountain Hot Springs
Parking Lot C
Plan and Profile**

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS	DATE	BY	COMMENTS

DATE 07/22/20

SCALE Horz: 1"=20'
Vert: 1"=10'

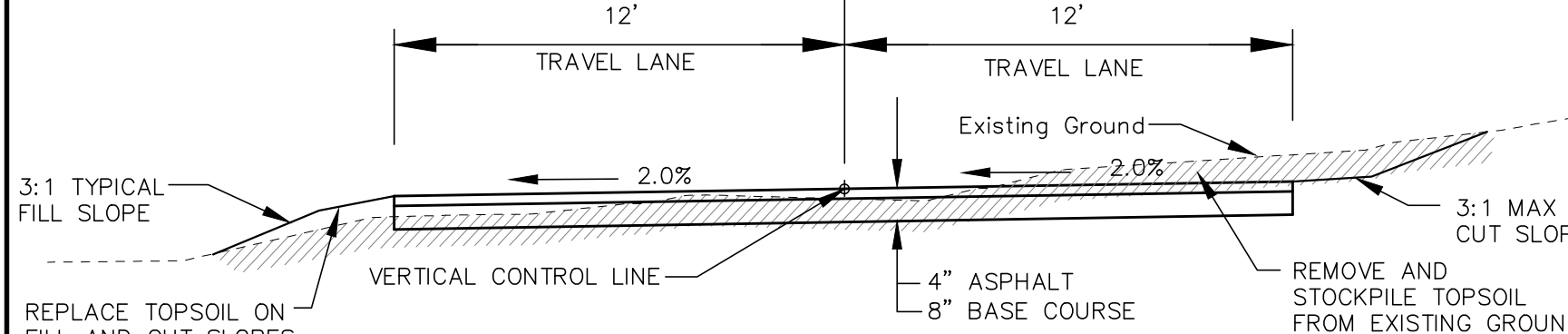
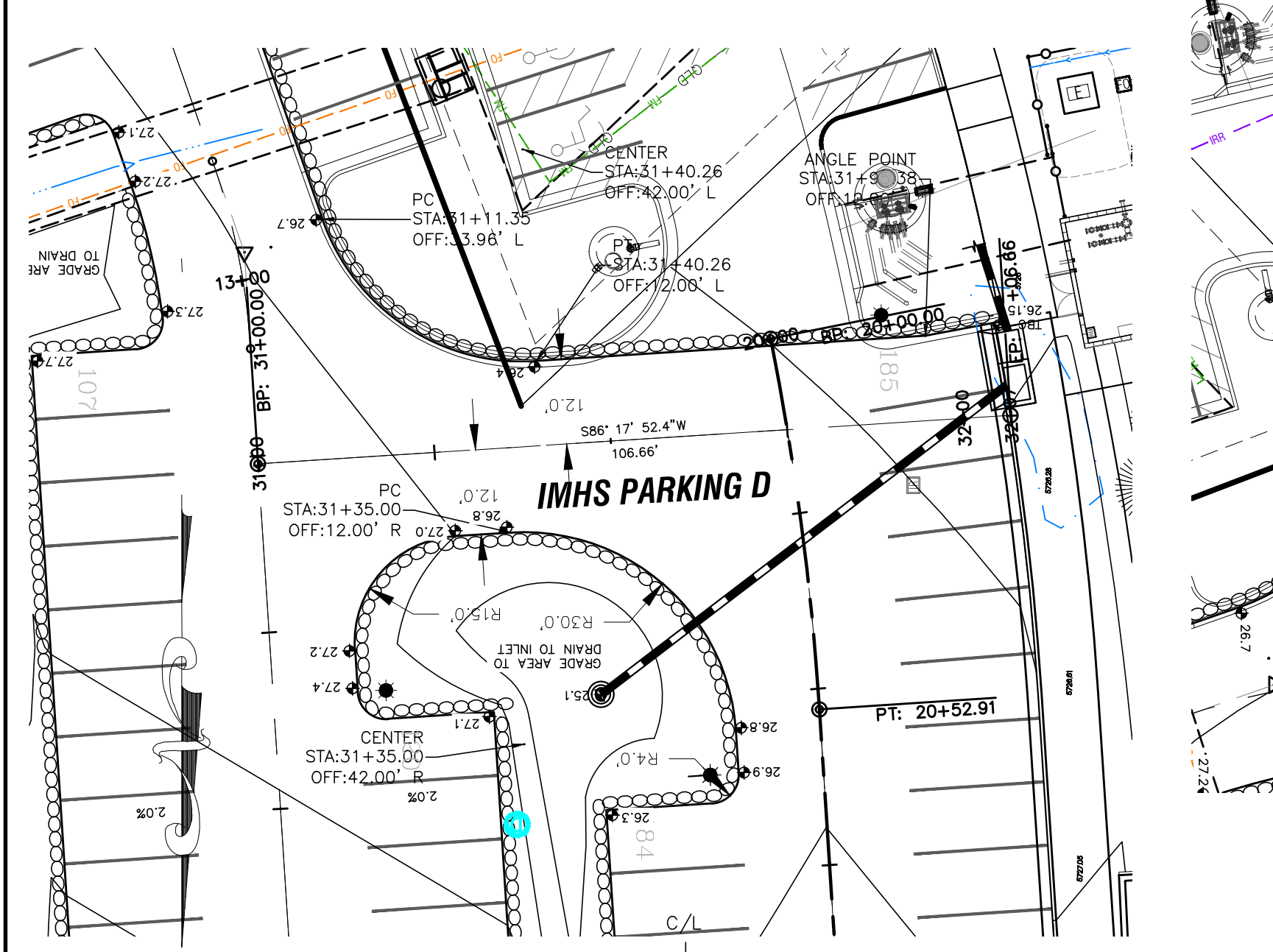
ENGINEER C. HALE

DWG FILE IronMtn

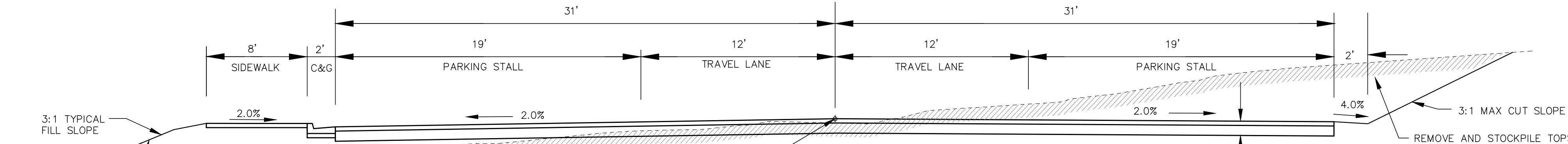
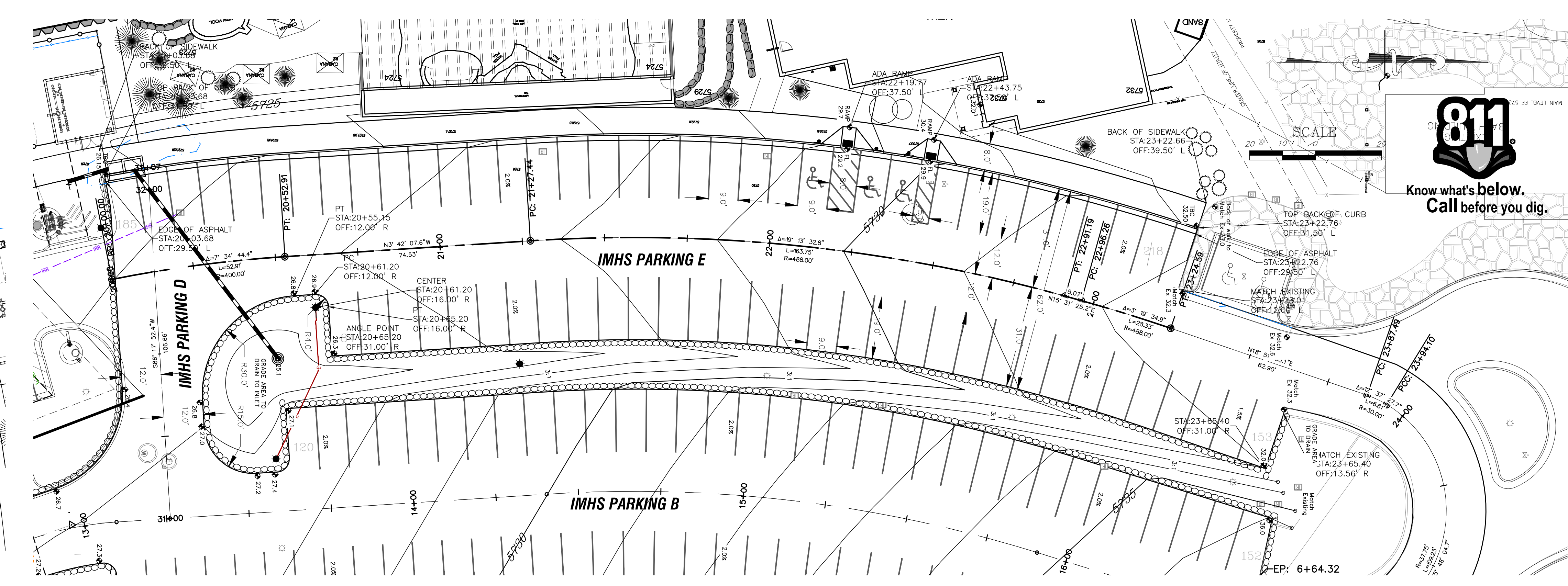
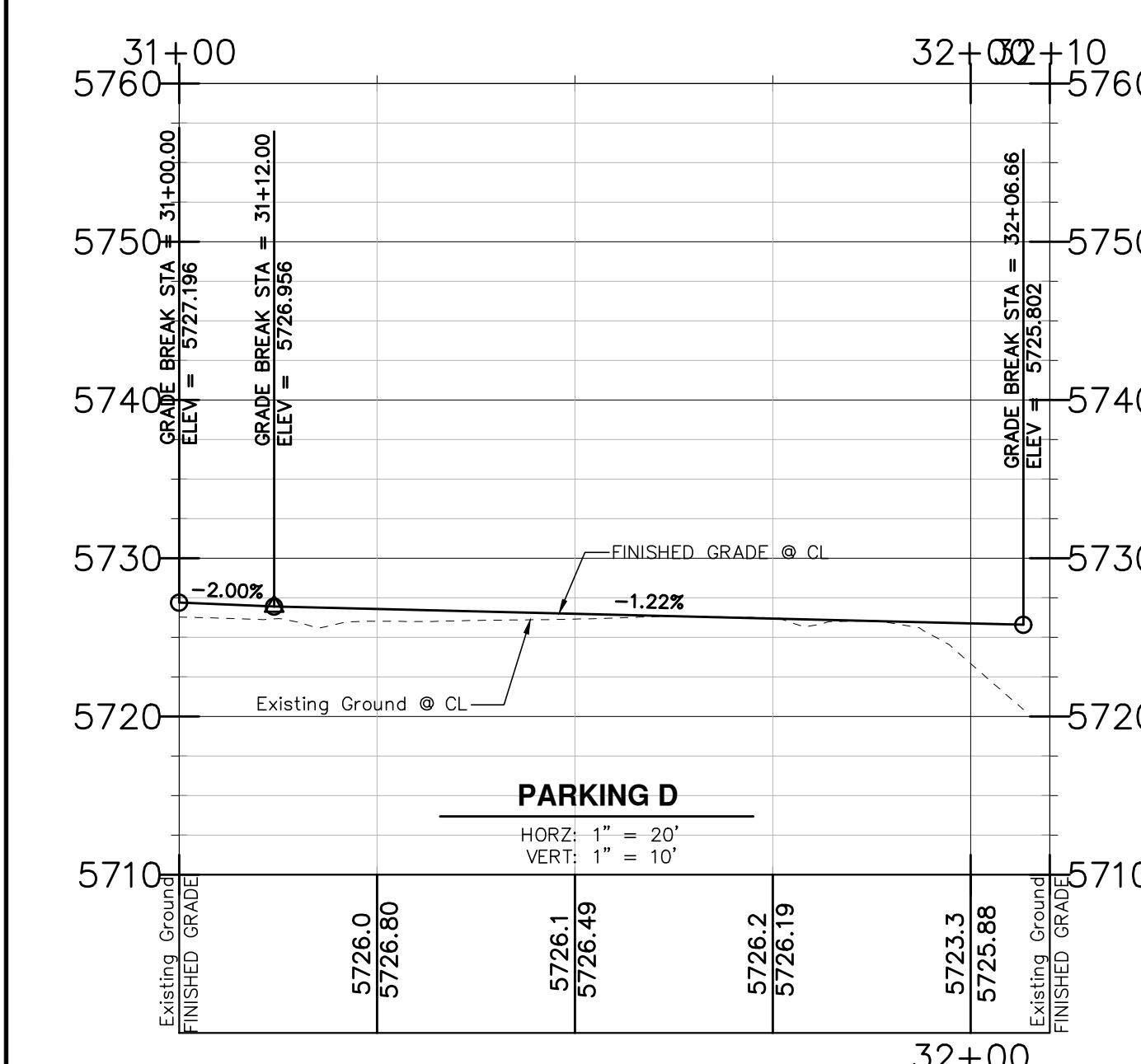
JOB 564-005

SHEET

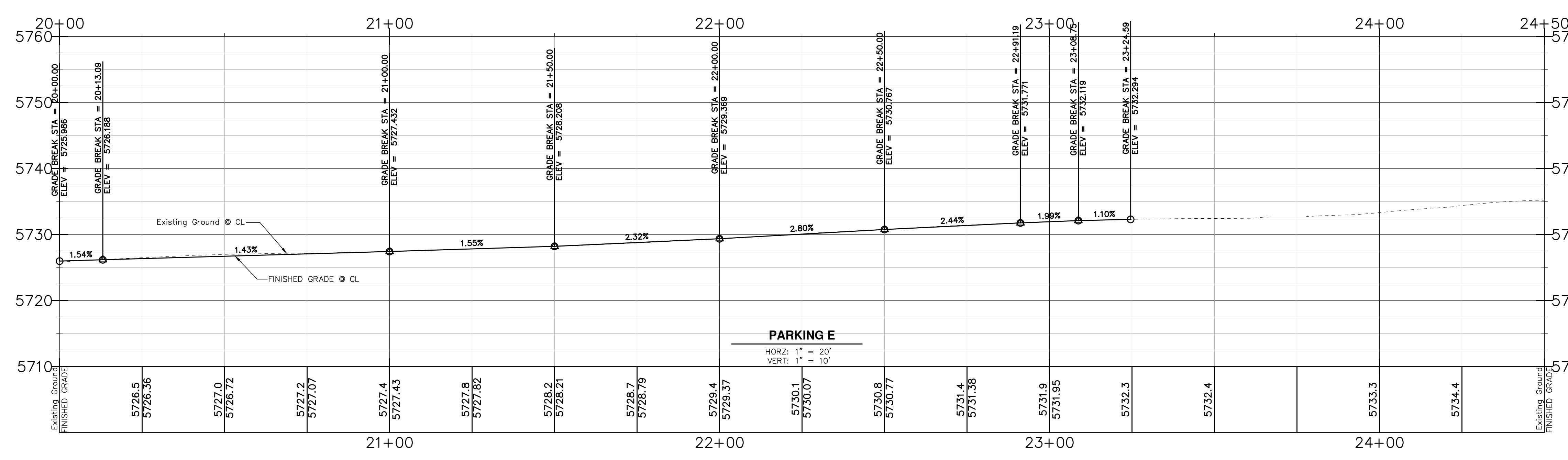
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TYPICAL SECTION
Parking D



TYPICAL SECTION
Parking E

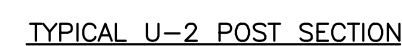
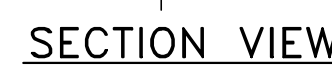
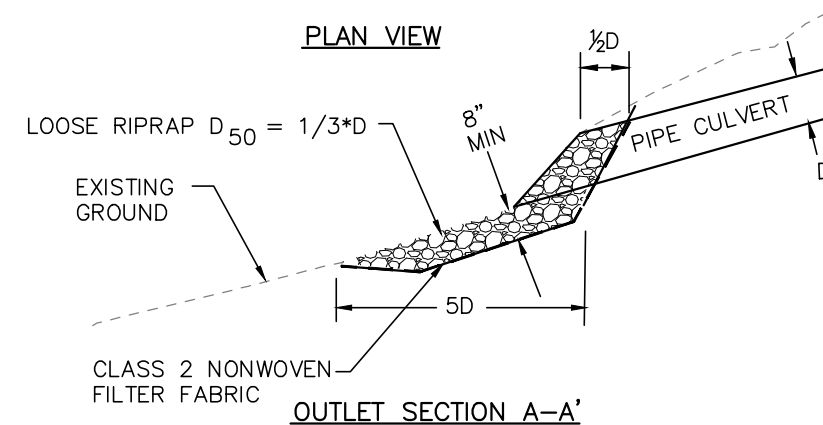
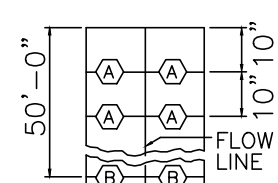
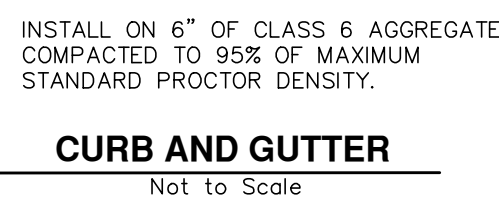


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Iron Mountain Hot Springs
Parking Lot D & E
Plan and Profile

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS		DATE	BY	COMMENTS
DATE		07/22/20		
SCALE		Horz: 1" = 20' Vert: 1" = 10'		
ENGINEER		C. HALE		
DWG FILE		IronMtn		
JOB		564-005		
SHEET		C8		



CLASS 1 SIGN PANELS SHALL BE SINGLE SHEET ALUMINUM, 0.100" MINIMUM THICKNESS AND SHALL BE FASTENED DIRECTLY TO THE POST. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED OR CADMIUM PLATED. FASTENERS SHALL BE DEGRADATED USING RETROREFLECTIVE SHEETING TO MATCH THE SIGN.

4. U-SHAPE POST SHALL BE A UNIFORM FLANGED CHANNEL SECTION MADE FROM HOT ROLLED STRUCTURAL STEEL, RE-ROLLED RAIL STEEL OR ALUMINUM. POST SHALL HAVE A MINIMUM YIELD STRENGTH OF AT LEAST 30,000 PSI, AND A MINIMUM TENSILE STRENGTH OF AT LEAST 50,000 PSI. POST SHALL WEIGH 2 LBS/FT.

5. U-SHAPE POSTS SHALL HAVE $\frac{5}{8}$ " HOLES DRILLED OR PUNCHED ON 1" OF 2" CENTERS FOR THE TOP 4 FEET OF THE POST AS A MINIMUM, WITH THE FIRST HOLE 1- $\frac{1}{2}$ " FROM THE TOP OF THE POST.

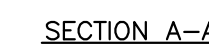
6. VERTICAL CLEARANCE BETWEEN SIGN PANELS ON THE SAME POST SHALL BE 1" MIN. TO 2" MAX.

NOT TO SCALE



INSTALL ON 6" OF CLASS 6 AGGREGATE COMPACTED
TO 95% OF MAXIMUM STANDARD PROCTOR DENSITY

NOT TO SCALE



NOT TO SCALE



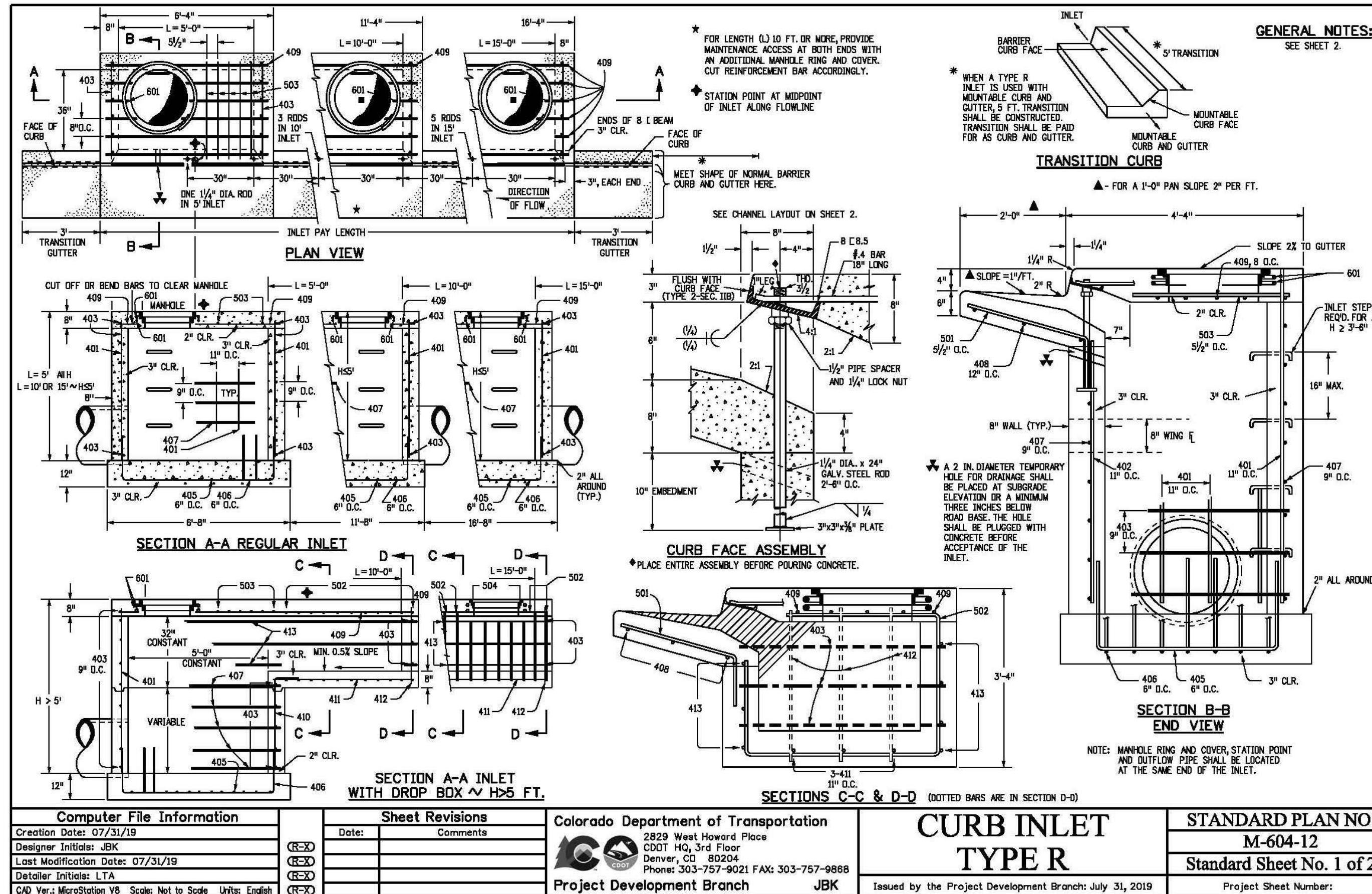
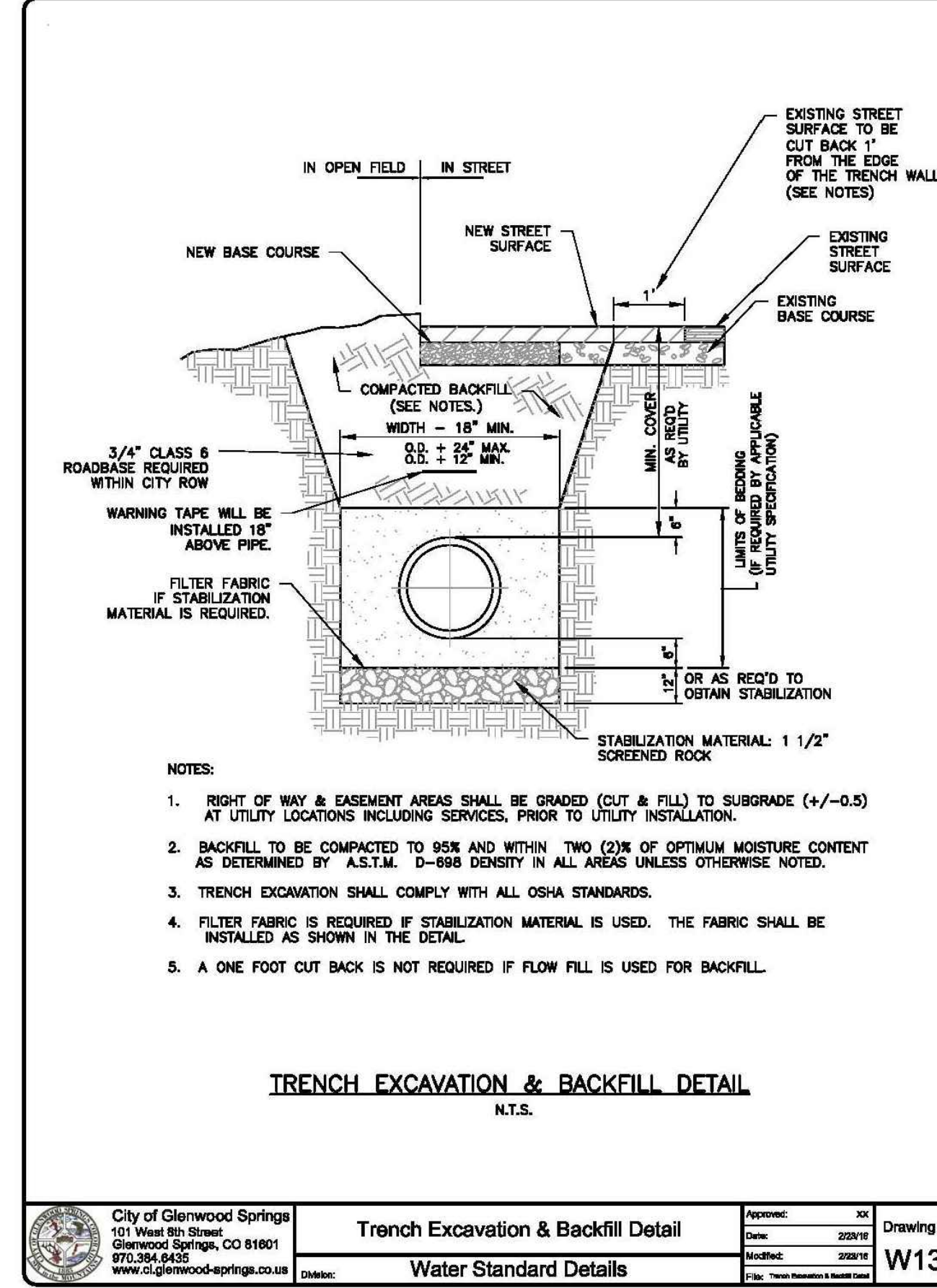
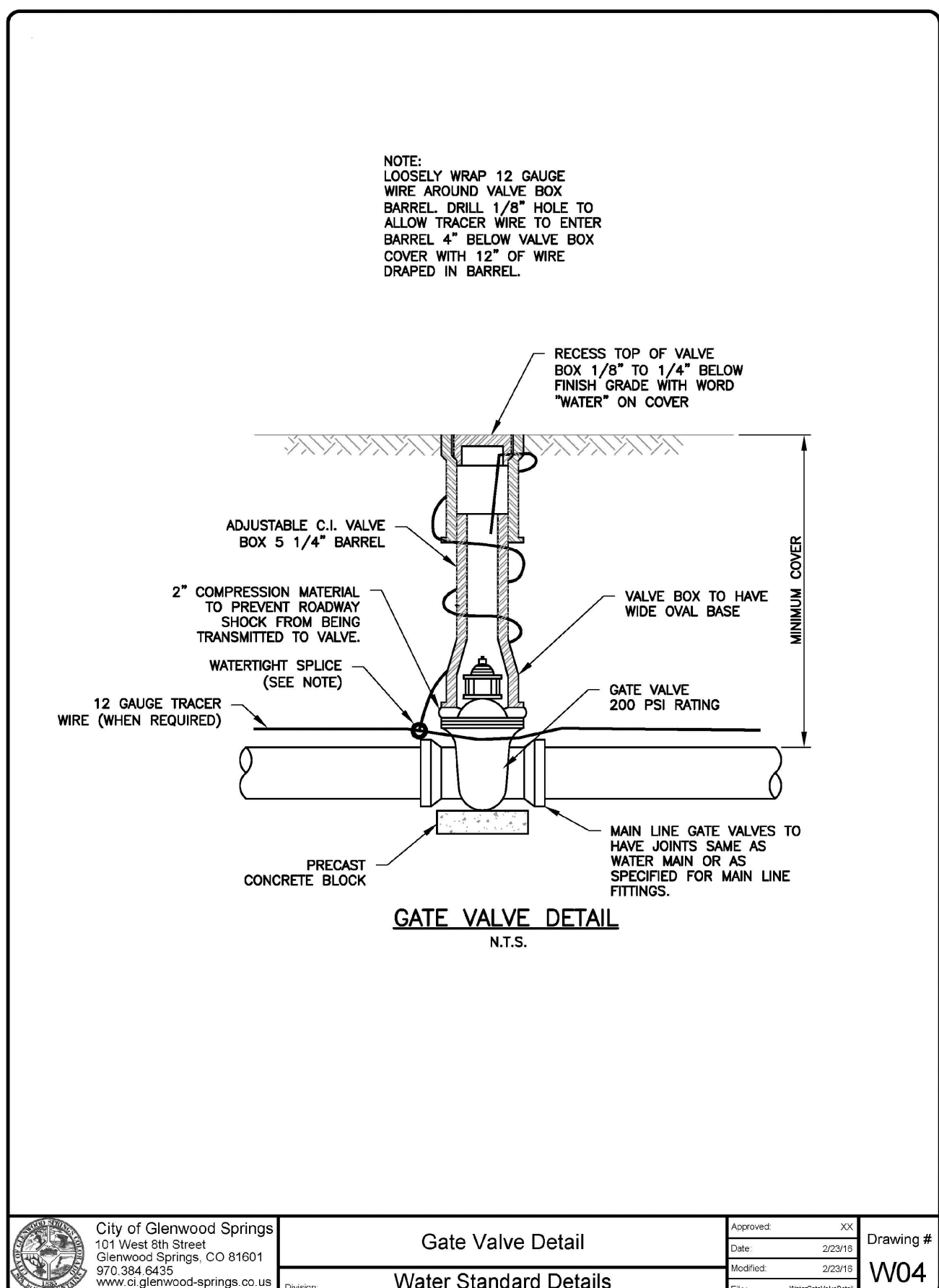
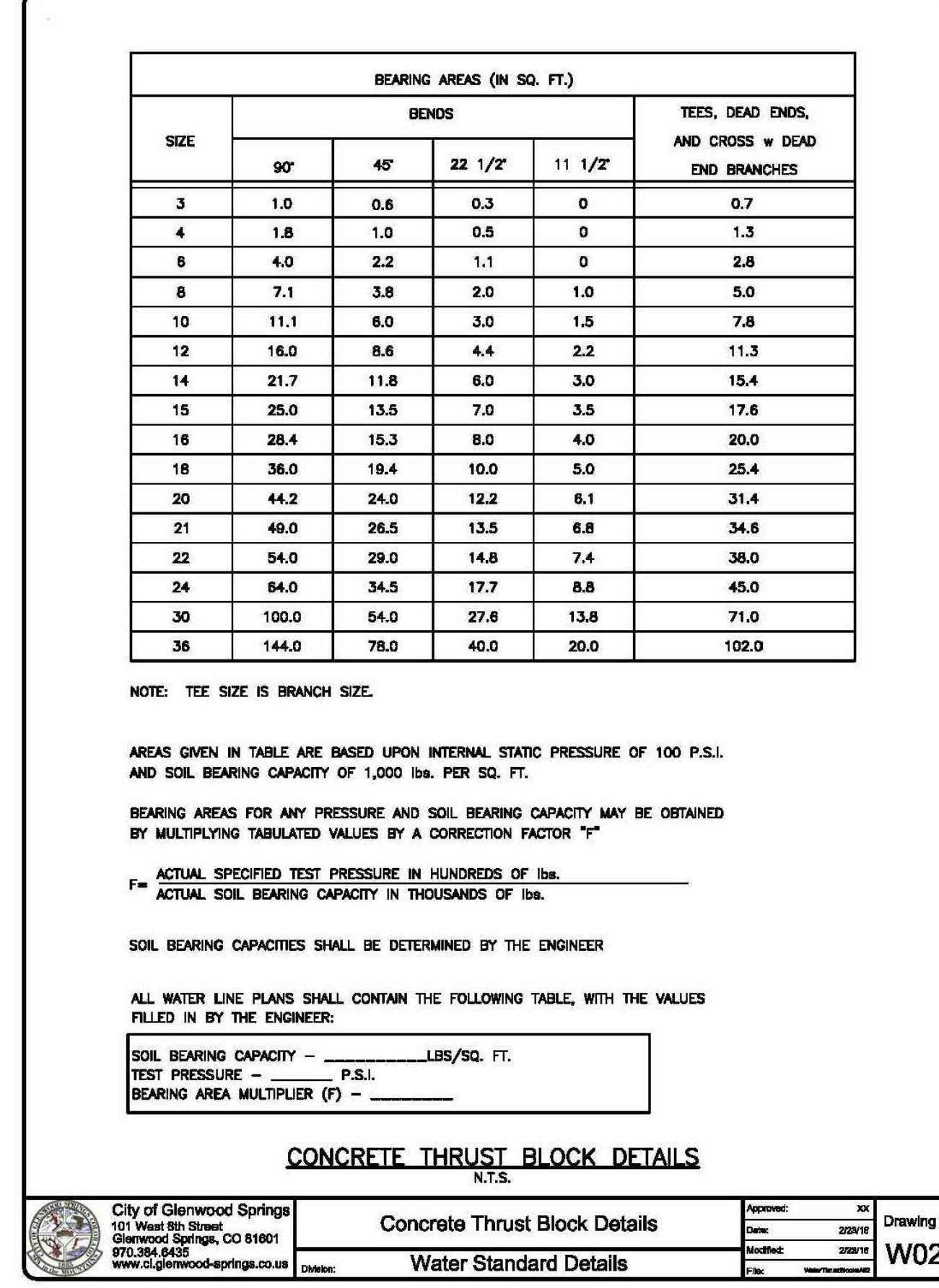
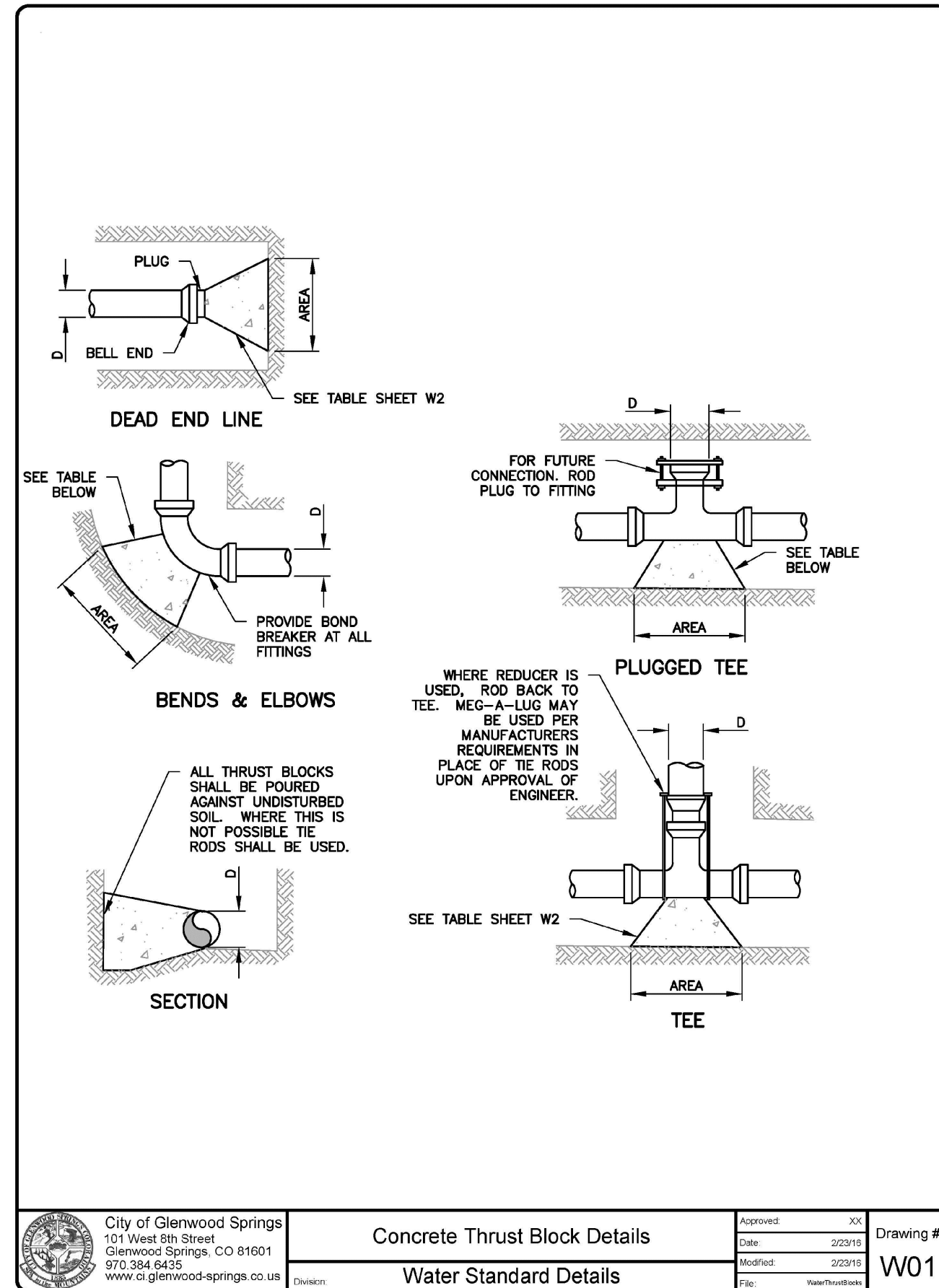
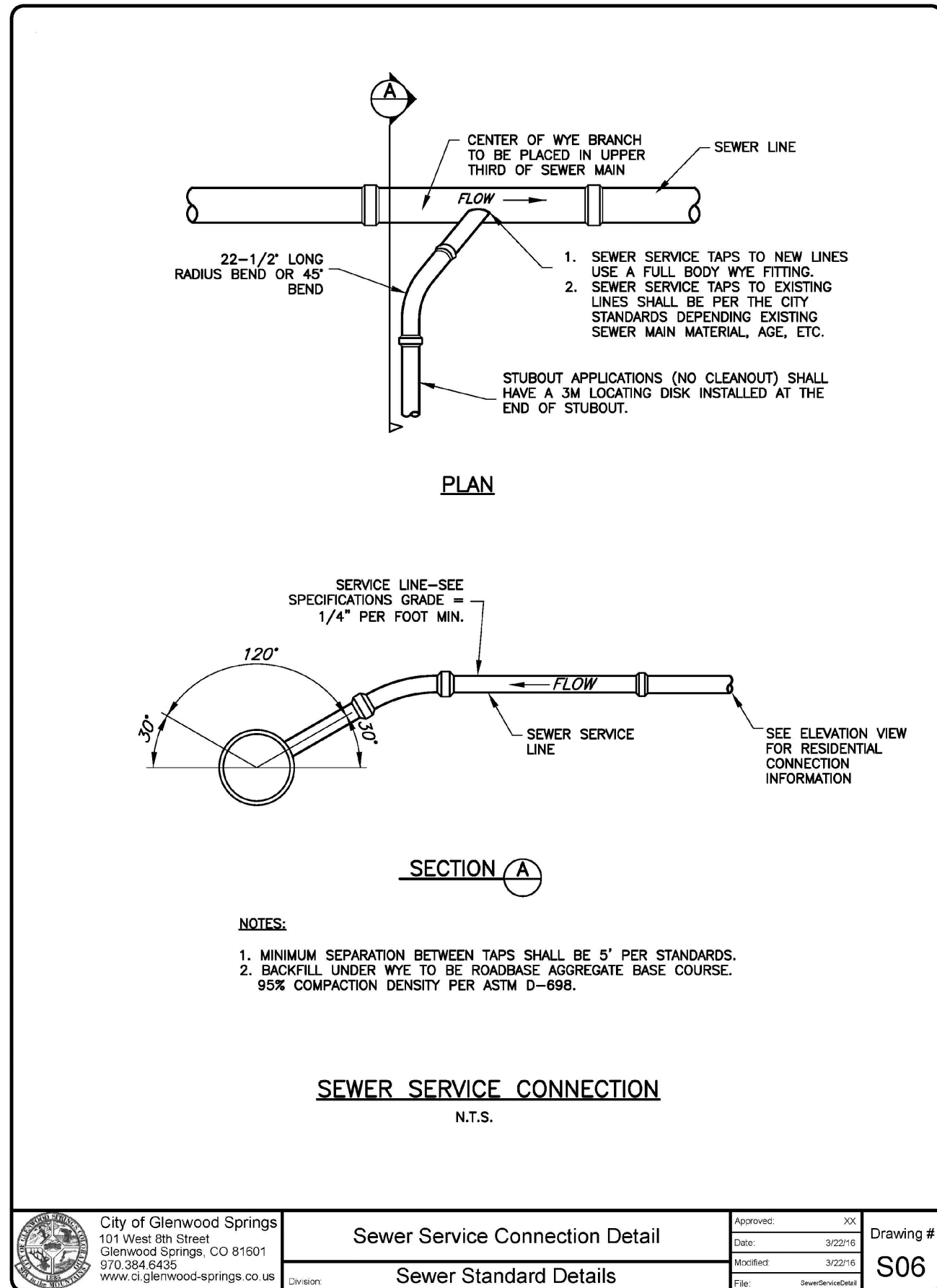
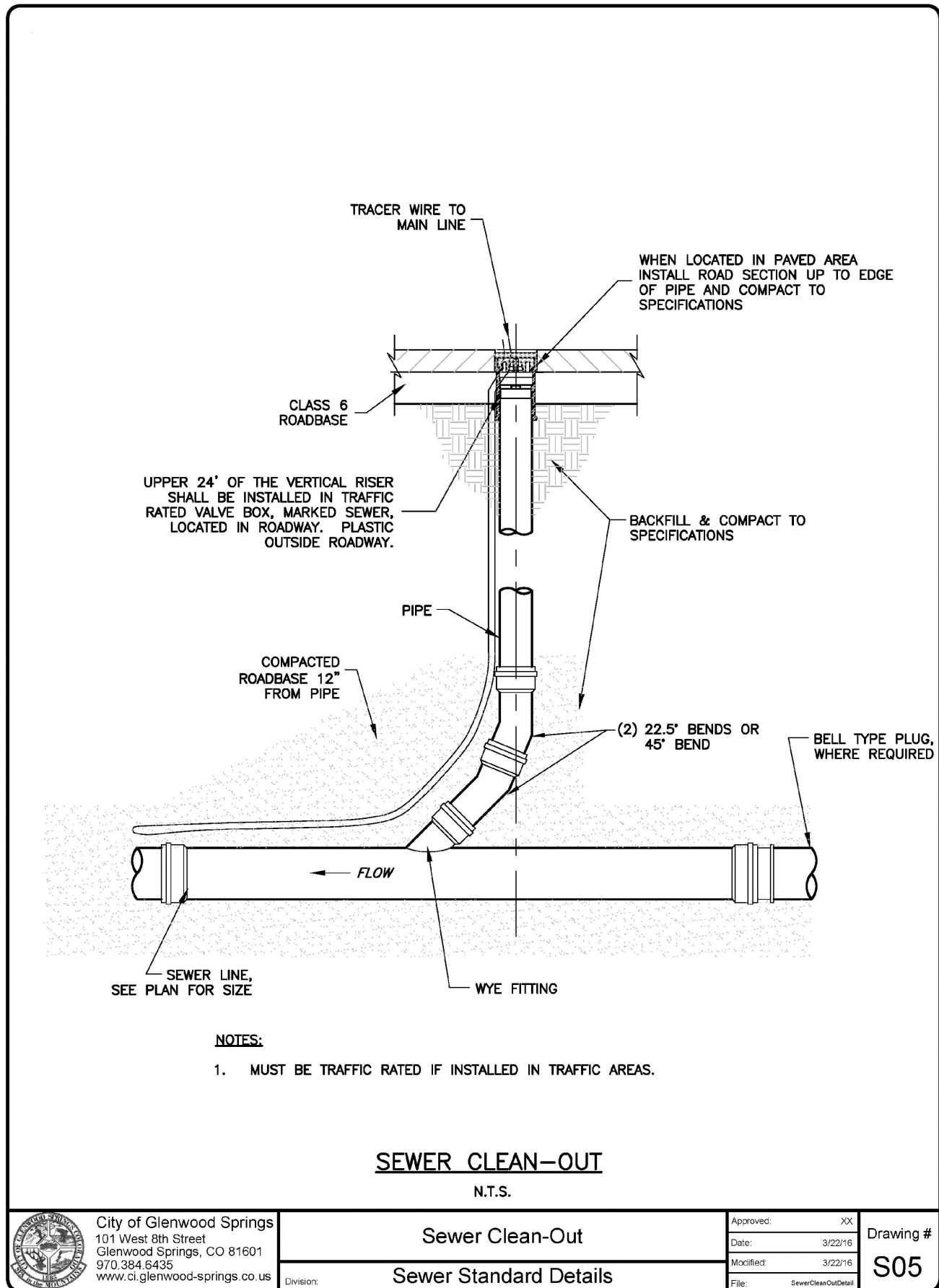
REVISIONS			COMMENTS
DATE	BY		

SCALE 1"=20'

DWG FILE

SHEET

C9



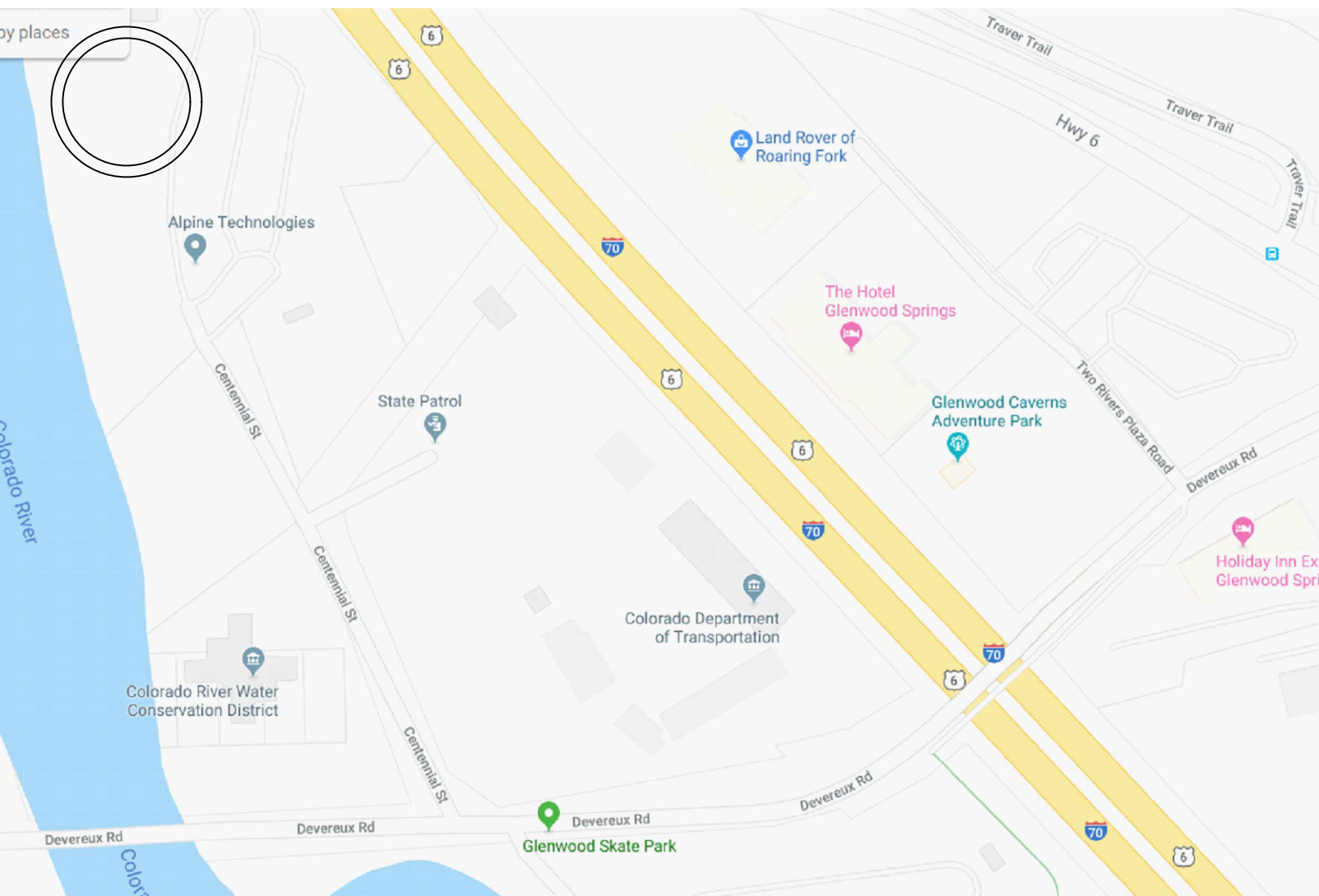


IRON MOUNTAIN HOT SPRINGS

ENTRANCE BUILDING

- CHECK IN
- RETAIL
- RESTAURANT
- MEN'S AND WOMENS' LOCKER ROOMS
 - one accessible unit
 - all others adaptable

GLENWOOD SPRINGS
GARFIELD COUNTY, COLORADO



VACINITY MAP

LIST OF SHEETS

ARCHITECTURAL

- A0 COVER SHEET, LIST OF SHEETS, PROJECT DATA
- A1 SITE PLAN
- A1.1 BLOW UP SITE PLAN

- AA2.0 LOWER ENTRANCE BUILDING FLOOR PLAN
- AA2.1 MAIN ENTRANCE BUILDING FLOOR PLAN
- AA2.2 UPPER ENTRANCE BUILDING FLOOR PLAN

- AA3.0 ROOF PLAN

- AA4.0 ENTRANCE BUILDING ELEVATIONS
- AA4.1 ENTRANCE BUILDING ELEVATIONS
- AA4.2 ENTRANCE BUILDING COLOR ELEVATIONS
- AA4.3 ENTRANCE BUILDING COLOR ELEVATIONS
- AA4.4 ENTRANCE BUILDING VIEWS
- AA4.5 ENTRANCE BUILDING RENDERING

- AB2.0 POOL BUILDING FLOOR PLAN

- AB4.0 BUILDING ELEVATIONS
- AB4.1 POOL BUILDING COLORED ELEVATIONS
- AB4.2 POOL BUILDING RENDERING

- AB5.0 POOL BUILDING SECTIONS

CIVIL

LANDSCAPE:

STRUCTURAL

MECHANICAL, ELECTRICAL, PLUMBING (MEP)

PROJECT DATA

OWNER: IRON MOUNTAIN HOT SPRINGS, LLC (970)945-4766

ARCHITECT: TRIUMPH CUSTOM HOMES (303-475-4413)

STRUCTURAL ENGINEERING:MONROE NEWELL STRUCTURAL ENGINEERS
GRAIG CARROLL (303) 807-8403

MEP ENGINEERING: BURGGRAAF AND ASSOCIATES
MARK BURGGRAAF (970) 240-5990

CONTRACTOR: TBD

SITE: PARCEL #:
MAILING ADDRESS: 281 CENTENINIAL STREET
GLENWOOD SPRINGS, CO 81601
PHYSICAL ADDRESS:

LEGAL: SECTION 9 TOWNSHIP: 6 RANGE 89 SUBDIVISION
TWO RIVERS PLAZA REPLAT OF 1A & 1B
BLOCK 2 / LOT 1

ZONING: RE-RESORT DISTRICT

BLDG. HT: 46'-0" AT HIGHEST RIDGE, WEST SIDE OF SECOND FLOOR TO NEW GRADE

DESCRIPTION: NEW ENTRANCE BUILDING FOR RESORT EXPANSION

NOTES:

1. BUILDING CODES:
ALL WORK SHALL COMPLY WITH THE CURRENT CODES, THE INTERNATIONAL BUILDING CODE 2015. INCLUDING THE INTERNATIONAL ENERGY CODE 2009, THE INTERNATIONAL PLUMBING AND MECHANICAL CODES, THE CURRENT NEC AND A LOCAL CODES HAVING JURISDICTION.

2. PROPOSED BUILDING TYPE:
TYPE V-A, NFPA 13R SPRINKLED BUILDING
OCCUPANT GROUP:
R-2 - 200 SF / OCCUPANT
BUSINESS - 100 SF / OCCUPANT
MINIMUM FIRE RESISTIVE REQUIREMENTS:
STRUCTURAL FRAME 1 HR
BEARING WALLS 1 HR
FLOOR CONSTRUCTION 1 HR
ROOF CONSTRUCTION 1 HR

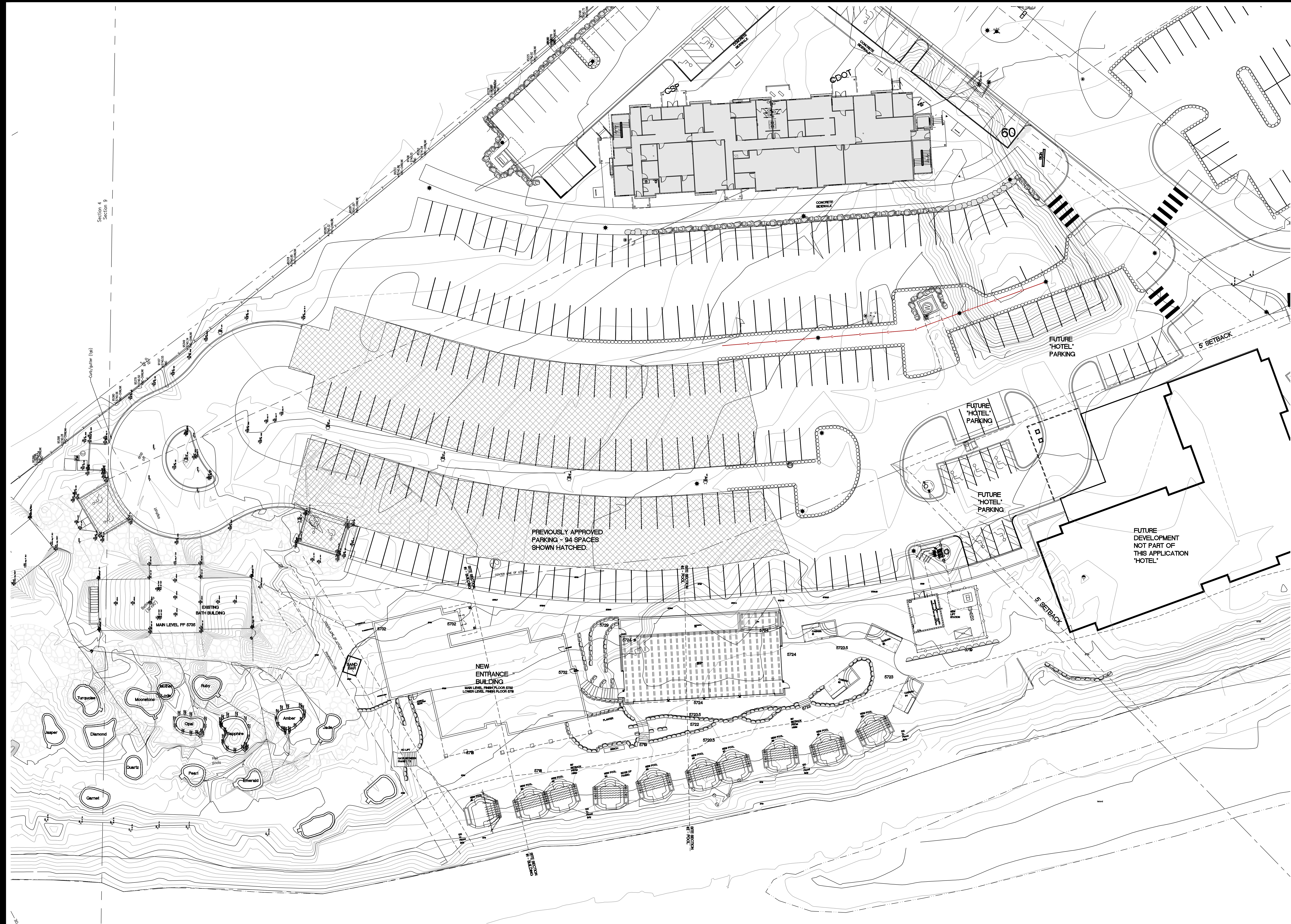
OCCUPANCY SEPARATION:

FIRE PROTECTION SYSTEM SUBMITTED SEPRATELY (SECTION 903.3)

SMOKE DETECTION SHALL BE PROVIDED THROUGHOUT.

FIRE ALARM SYSTEM (SECTION 907) PROVIDED THROUGHOUT

3. PROPOSED ENERGY COMPLIANCE:
WINDOW AND DOORS, U-0.35 OR LESS.
MINIMUM R-VALUES,
CEILINGS AND ROOFS, R-38
WALLS, R-23
FLOORS, R-11
SLAB PERIMETER AND DEPTH, R10/24-IN
CRAWL SPACE, R-10/13



1 SITE PLAN
1" = 20'-0"



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Sheet
Date: 8-6-20

Project No.

Drawn by: MEF

Checked by: MEF

A1.0

Sheet title
SITE PLAN

Revisions:

IRON MOUNTAIN HOT SPRINGS
NEW ENTRANCE BUILDING
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601

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1 SITE PLAN
1" = 20'-0"



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Sheet

A1.0B

Date: 8-04-20

Project No:

Drawn by: MEF

Checked by: MEF

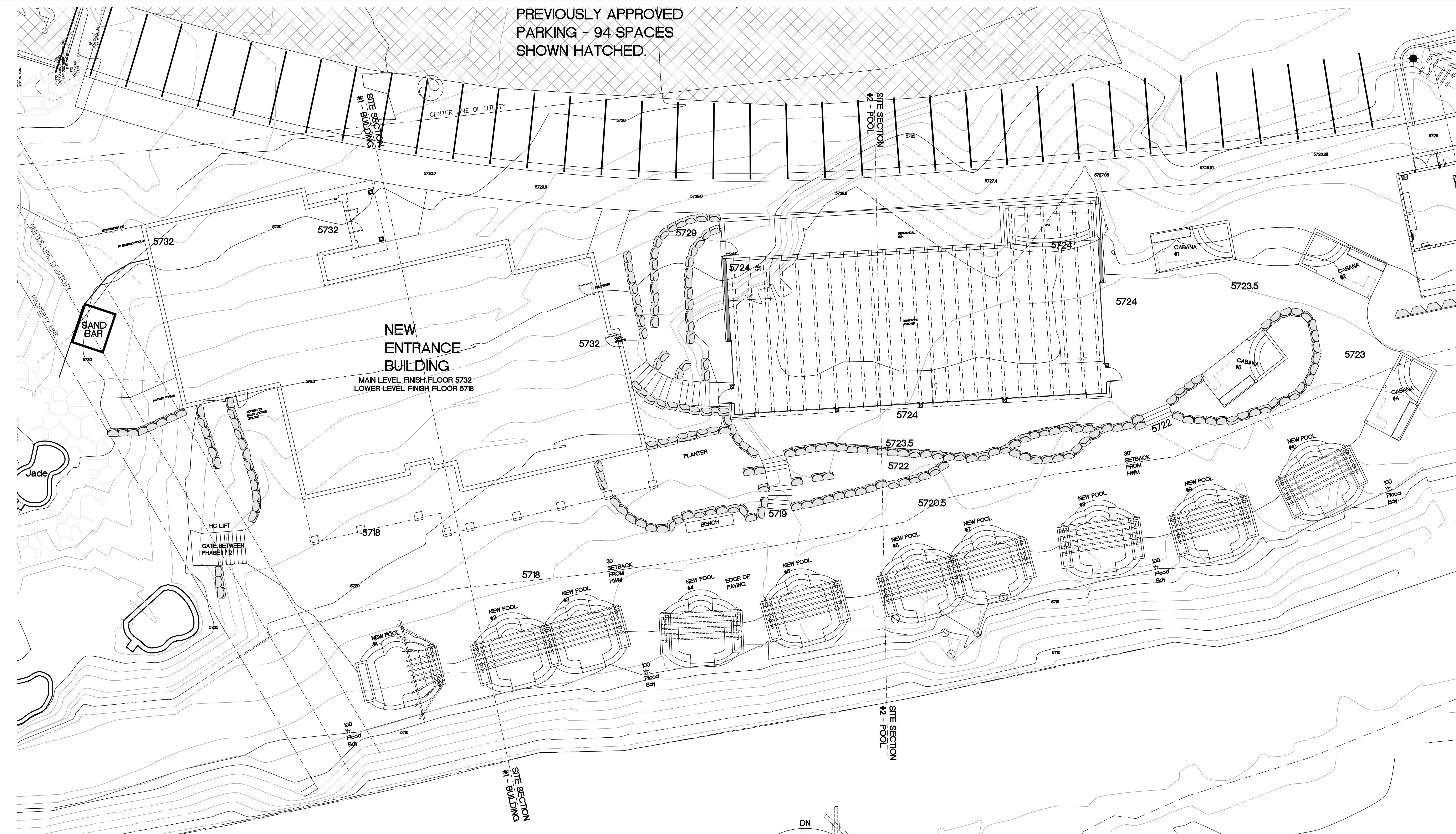
Stamp

Sheet title
EXISTING
SITE PLAN

Revisions:

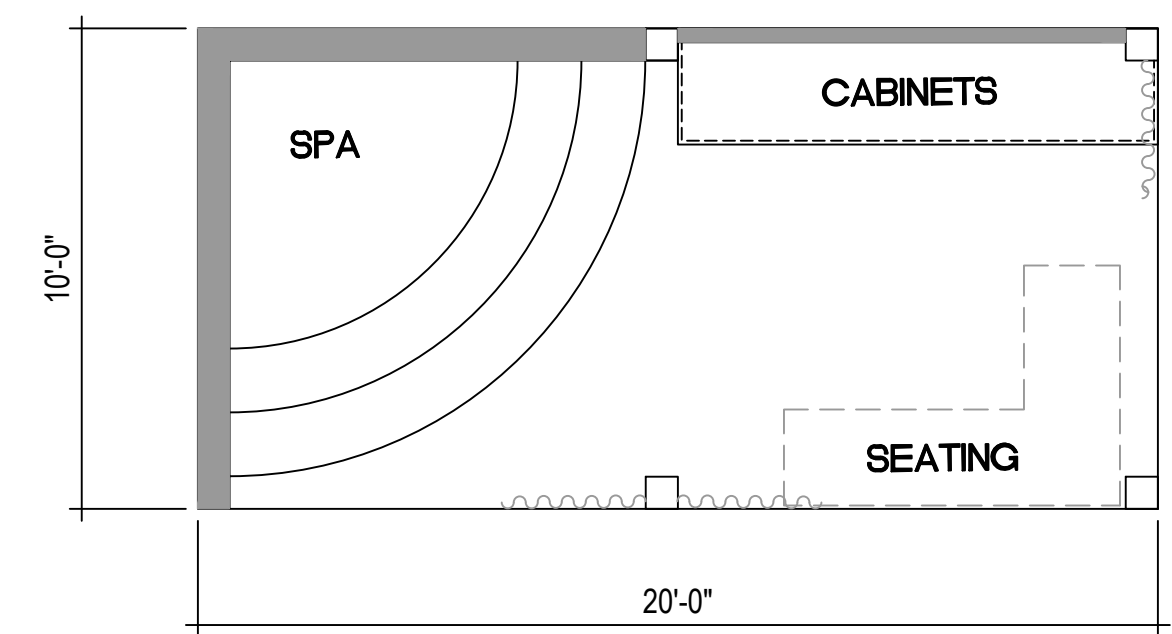
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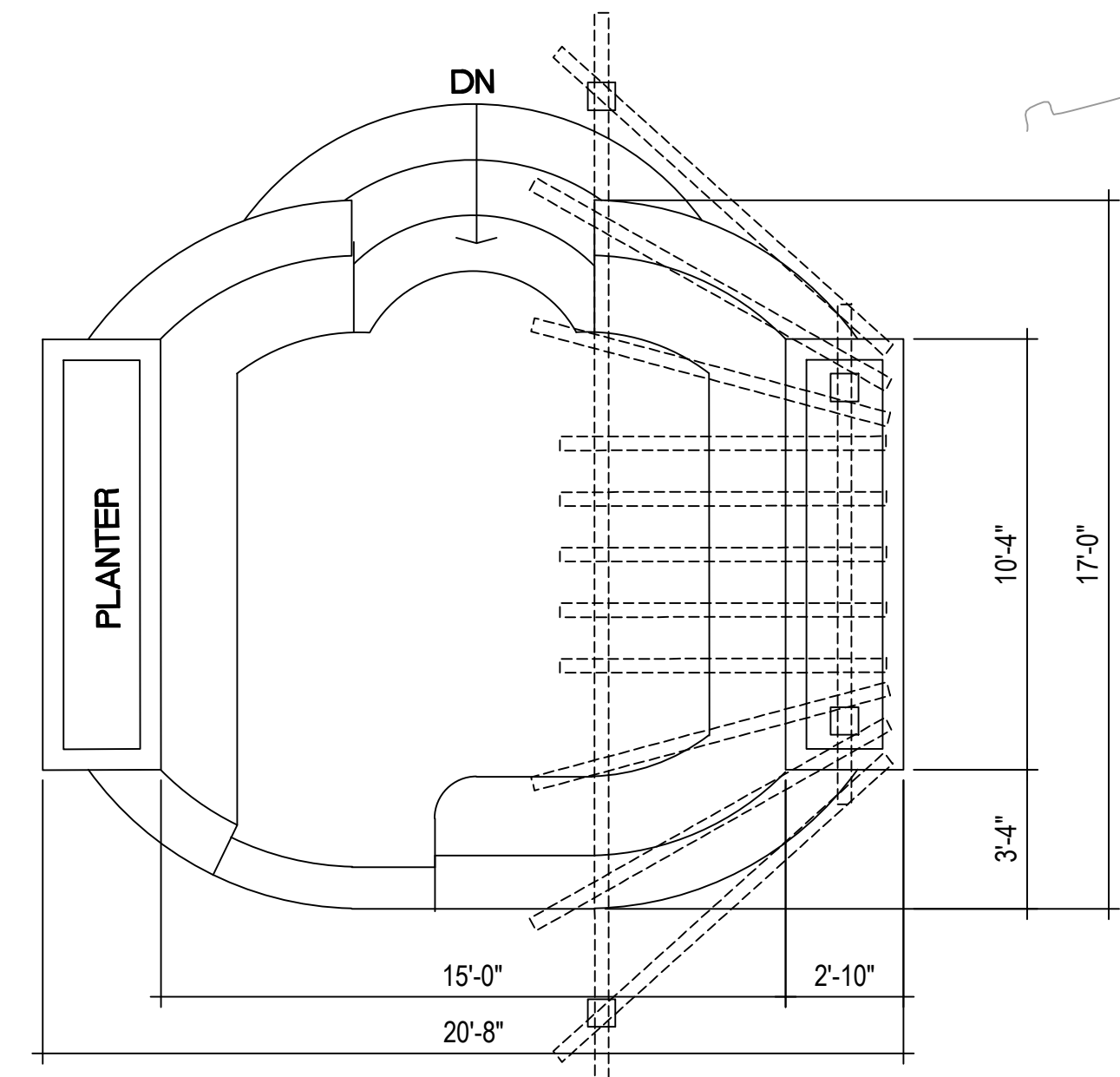


PREVIOUSLY APPROVED
PARKING - 94 SPACES
SHOWN HATCHED.

NEW
ENTRANCE
BUILDING
MAIN LEVEL FINISH FLOOR 5732
LOWER LEVEL FINISH FLOOR 5718



2 CABANA PLAN
1/4" = 1'-0"



3 SPA PLAN(S)
1/4" = 1'-0"

1 PARTIAL SITE PLAN
1" = 10'-0"

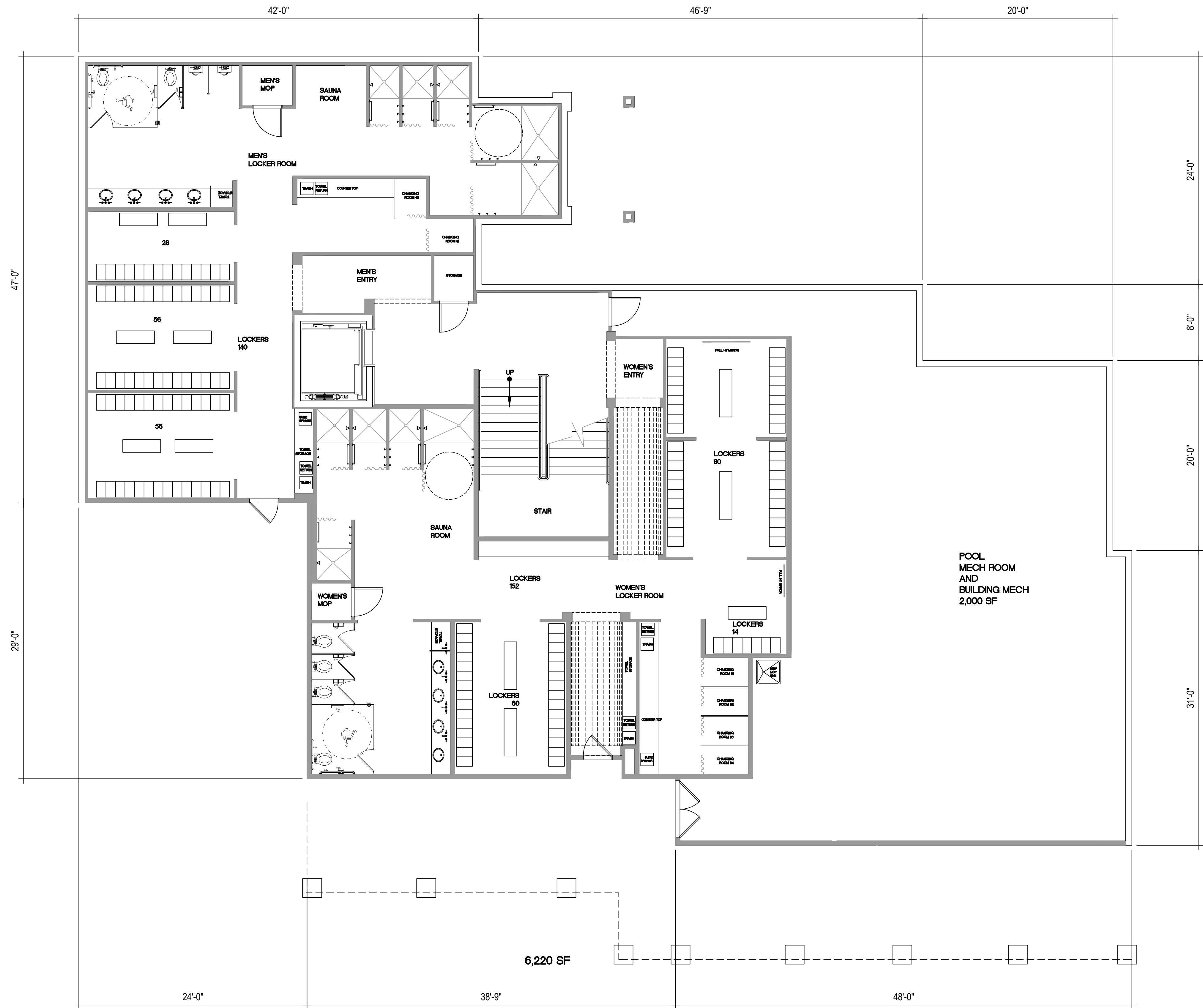


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NEW ENTRANCE BUILDING
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Sheet title PARTIAL SITE PLAN	Revisions:
Stamp	Date: 8-6-20
	Project No.
	Drawn by: MEF
	Checked by: MEF

A1.1



1 ENTRANCE BUILDING LOWER LEVEL
3/8" = 1'-0"



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Sheet

A2.0

Date: 6-22-20

Project No:

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title
BUILDING
PLANS

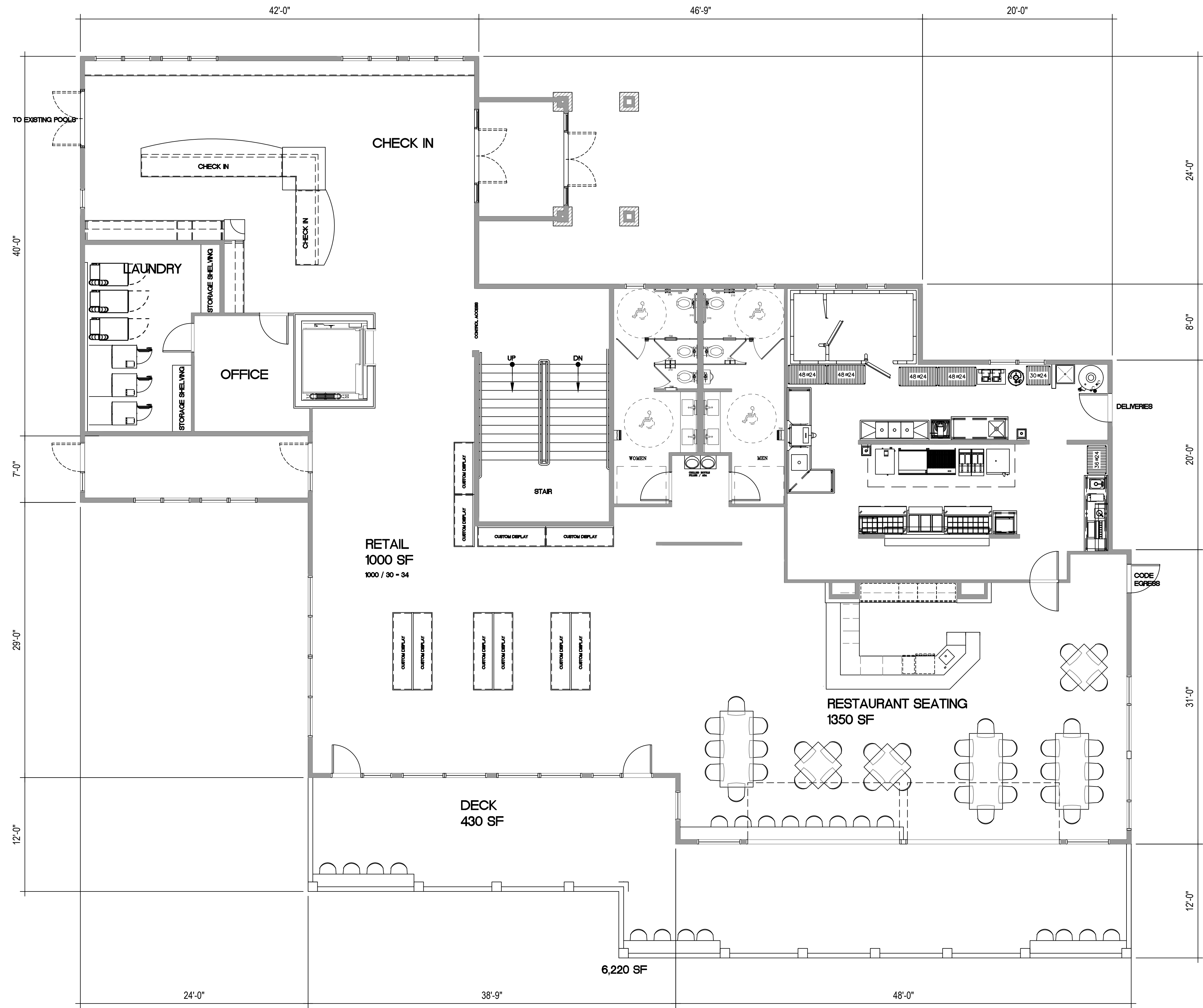
Revisions:

IRON MOUNTAIN HOT SPRINGS
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1 ENTRANCE BUILDING MAIN LEVEL
1/8" = 1'-0"



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Sheet

A2.1

Date: 6-22-20

Project No.

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title

BUILDING
PLANS

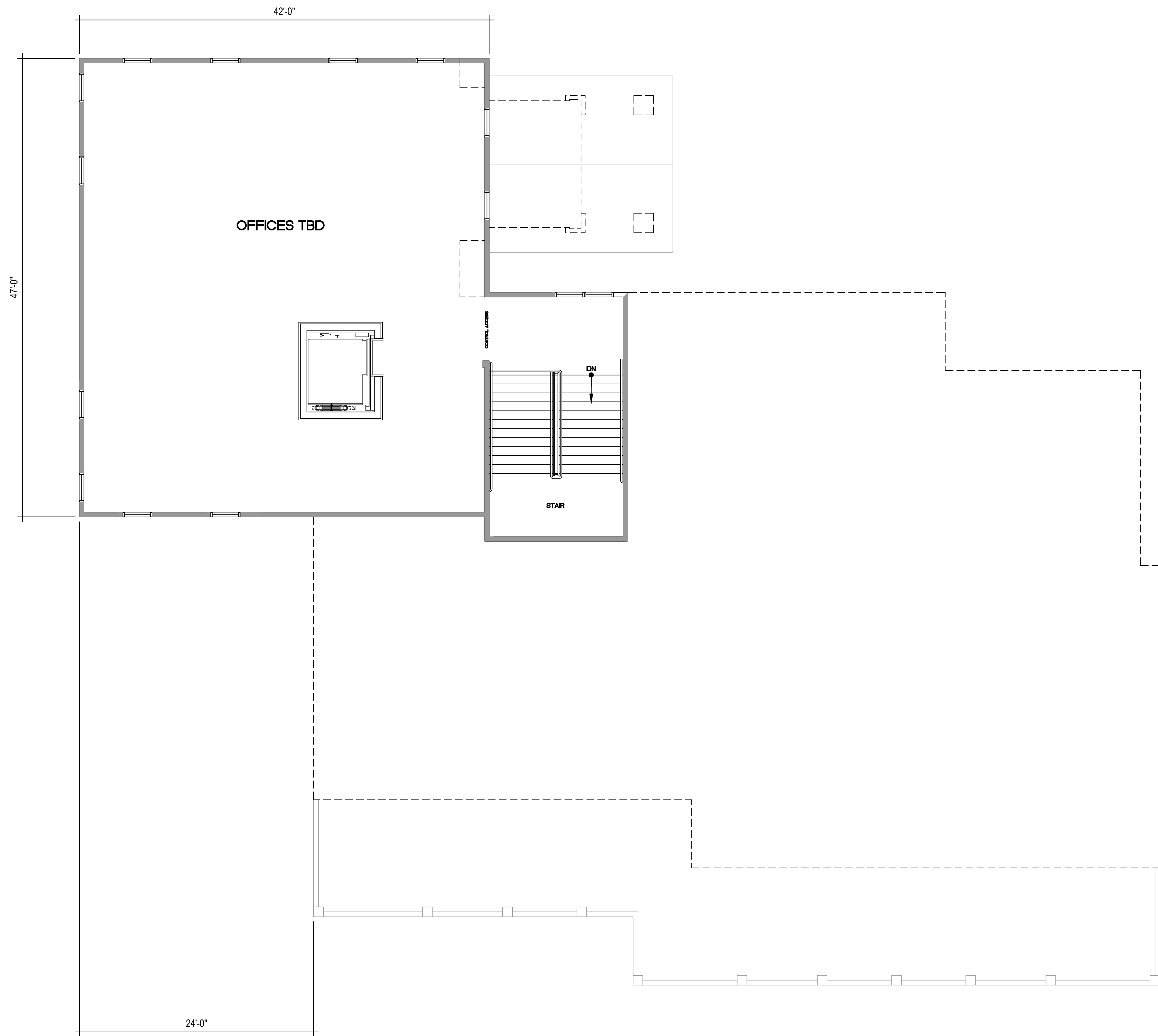
Revisions:

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1

ROOF PLAN
1/8" = 1'-0"



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Sheet

A2.2

Date: 7-23-20

Project No.

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title
BUILDING
PLANS

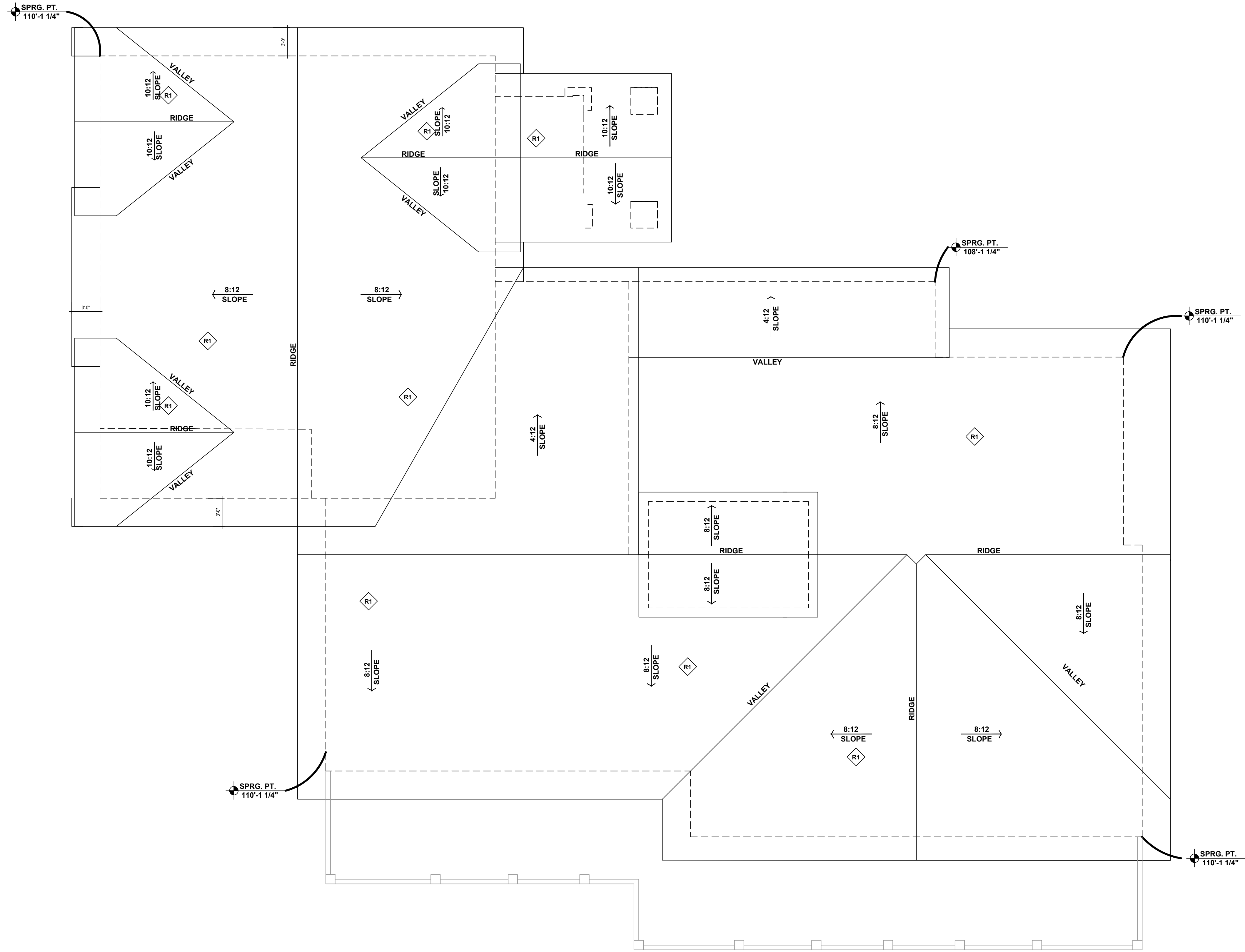
Revisions:

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1

ROOF PLAN
1/8" = 1'-0"



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Sheet	A3.0
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Date:	7-21-20
Project No.	
Drawn by:	MEF
Checked by:	MEF

Stamp

Sheet title
FLOOR
PLAN

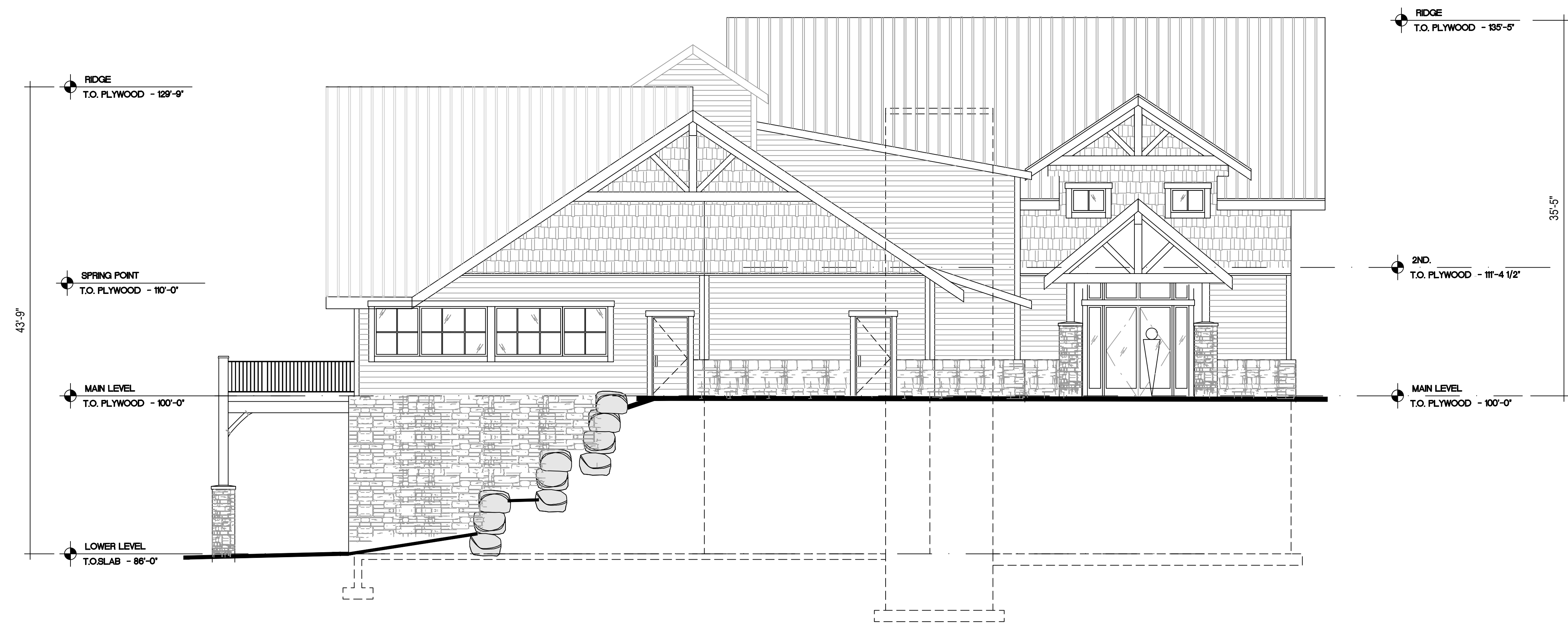
Revisions:

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1 EAST ELEVATION
3/16" = 1'-0"



2 SOUTH ELEVATION
3/16" = 1'-0"



1 WEST ELEVATION
3/16" = 1'-0"



2 NORTH ELEVATION
3/16" = 1'-0"



1 EAST ELEVATION



2 SOUTH ELEVATION
EXTERIOR MATERIALS AND COLORS TO MATCH
EXISTING BATH HOUSE.

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Sheet

A4.2

Date: 6-8-20

Project No.

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title

BUILDING
ELEVATIONS

Revisions:

IRON MOUNTAIN HOT SPRINGS
NEW ENTRANCE BUILDING

281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601

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1 WEST ELEVATION



2 NORTH ELEVATION

EXTERIOR MATERIALS AND COLORS TO MATCH
EXISTING BATH HOUSE.



ENTRANCE BUILDING

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A4.4

Date: 6-8-20

Project No.

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title
BUILDING
ELEVATIONS

Revisions:

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NEW ENTRANCE BUILDING
281 CENTENNIAL STREET
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ENTRANCE BUILDING

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A4.5

Date: 6-8-20

Project No.

Drawn by: MEF

Checked by: MEF

Stamp

Sheet title

BUILDING
ELEVATIONS

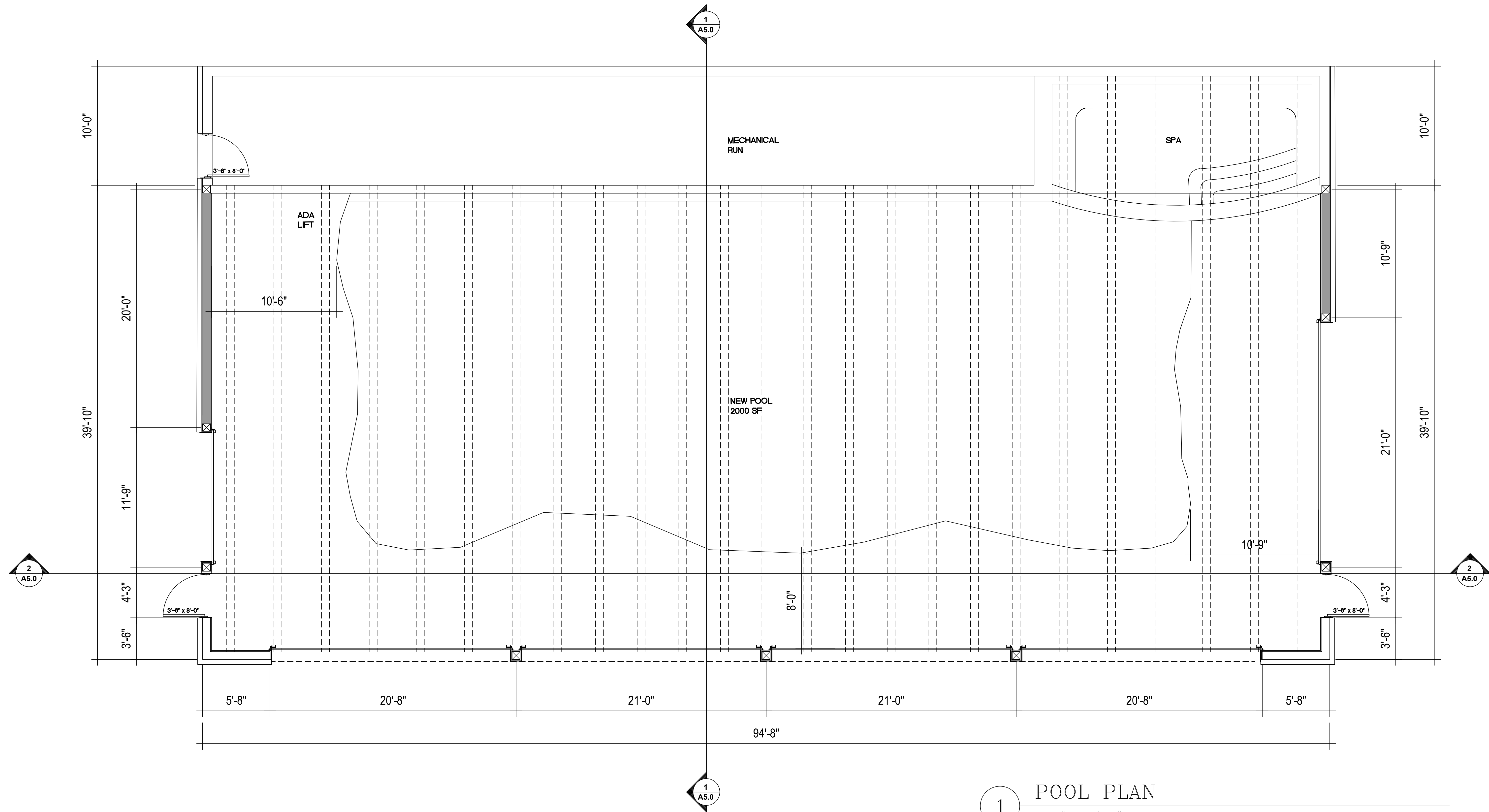
Revisions:

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NEW ENTRANCE BUILDING
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Tel: 970.479.9960

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1 POOL PLAN
1/4" = 1'-0"

POOL BUILDING



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Sheet
AB2.0

Date: 7-23-20

Project No:

Drawn by: MEF

Checked by: MEF

Sheet title
POOL
BUILDING
PLAN

Revisions:

IRON MOUNTAIN HOT SPRINGS
NEW ENTRANCE BUILDING
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601

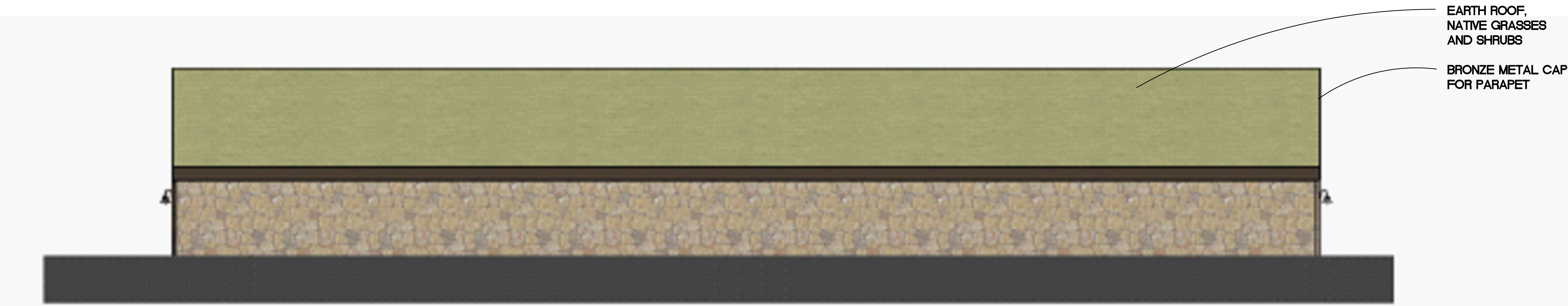
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Vail Office
12 Vail Road,
Suite 700
Vail, CO 81657
Tel: 970.479.9990

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POOL BUILDING



4 EAST ELEVATION
1/16" = 1'-0"



2 SOUTH ELEVATION
1/16" = 1'-0"



3 WEST ELEVATION
1/16" = 1'-0"



1 NORTH ELEVATION
1/16" = 1'-0"

POOL BUILDING

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AB4.1

Date: 8-6-20

Project No.

Drawn by: MEF

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Sheet title

POOL BUILDING ELEVATIONS

Revisions:

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NEW ENTRANCE BUILDING
281 CENTENNIAL STREET
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POOL BUILDING

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AB4.2

Date: 7-23-20

Project No.

Drawn by: MEF

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Sheet title

POOL
BUILDING
ELEVATIONS

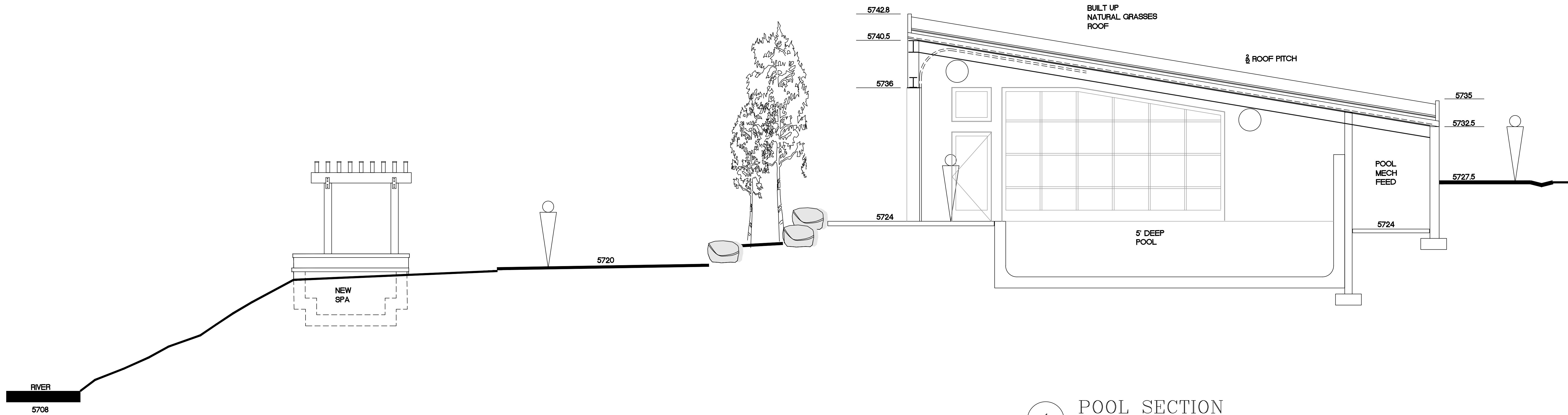
Revisions:

IRON MOUNTAIN HOT SPRINGS
NEW ENTRANCE BUILDING
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601

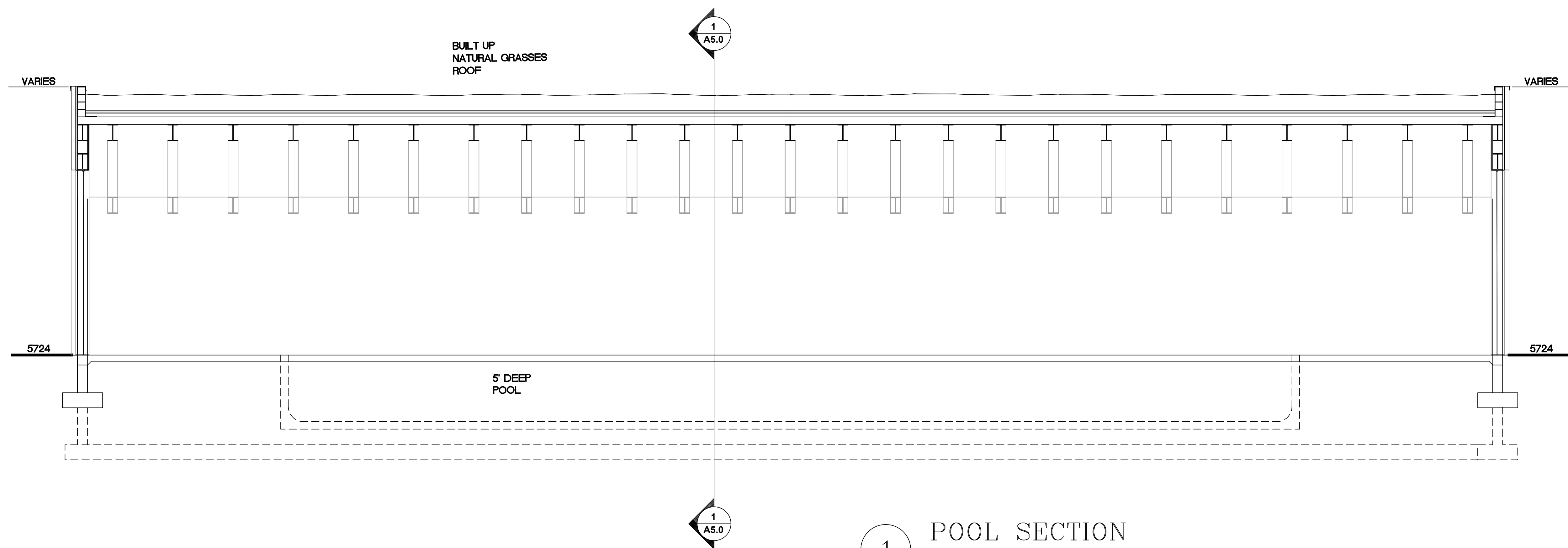
TRIUMPH

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12 Yall Road,
Suite 700
Yall, CO 81657
Tel: 970.479.9960

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1 POOL SECTION
1/4" = 1'-0"



1 POOL SECTION
1/4" = 1'-0"

POOL BUILDING



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IRON MOUNTAIN HOT SPRINGS
NEW ENTRANCE BUILDING

281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601

Revisions:

Sheet title
POOL
BUILDING
SECTIONS

Stamp

Date: 7-23-20

Project No:

Drawn by: MEF

Checked by: MEF

AB5.0

Sheet

Site Use Calculations
Hatched area #1 (parking area) = 91,379 sq. ft.
Hatched area #2 (building and pool area) = 55,561 sq. ft
Total site area = 146,940 sq. ft.

Common Open Space Area (10% of total site area)

Total site area = 146,940 sq. ft.
129,946 sq. ft x 10% = 14,694 sq. ft required Common Open Space
Actual Common Open Space = 44,927 sq. ft. (Area #2 less buildings and pool)

Minimum Landscaped Site Area (10% of total site area)

Total site area = 146,940 sq. ft
146,940 sq. ft x 10% = 14,694 sq. ft required landscaped site area
Actual Landscaped Site Area = 25,821 sq. ft. (not including parking landscape)

Parking Landscaped Area

Parking surface area = 70,984 sq. ft
70,984 sq. ft x 20% = 14,197 sq. ft required landscaped parking area
Actual Parking Landscaped Area = 19,151 sq. ft.

Landscape Requirements

Perennials: 1 per 50 s.f. of required landscape = 284
Shrubs: 1 per 100 s.f. of required landscape area = 142
Trees: 1 per 400 s.f. of required landscape area = 36

Coniferous Trees			
CS	6' Colorado Spruce 'Bachari'	Picea pungens	6
Deciduous Trees			
All trees to be 2.5" cal. when available			
RB	River Birch	Betula nigra	5
O	Swamp White Oak	Quercus bicolor	4
RH	Russian Hawthorn	Crataegus ambigua	5
GM	Ginnala Flame Maple	Acer ginnala	5
PC	Autumn Blaze Pear	Pyrus calleryana	2
H	Imperial Honeylocust	Gleditsia triacanthos inermis imperial	6
	Existing Trees		7
		Total Trees	40
Shrubs,			
All shrubs #5 container			
P	Pink Beauty Potentilla	Potentilla fruticosa 'Pink Beauty'	14
N	Little Devil Ninebark	Physocarpus opulifolius 'Donna May'	12
S	Spirea, Snowmound	Spirea nipponica 'Snowmound'	16
BS	Blue Mist Spirea	Caryopteris x clandonensis 'Blue Mist'	12
RB	Red Leaf Barberry	Berberis thunbergii 'Atropurpurea'	12
BT	Big Tuna Pine	Pinus mugo 'Big Tuna'	16
W	Bailey's Redtwig Dogwood	Cornus sericea 'Bailey'	25
MS	3' Montgomery Spruce	Picea pungens	12
R	Roses	Assorted Hardy Shrub Roses	17
PA	Pink Flowering Almond	Prunus glandulosa 'Rosea Plena'	6
		Total Shrubs	142
Perennials & Grasses			
#1 container			
	Blue Avena Grass	Helictotrichon sempervirens	20
	Heavy Metal Switch Grass	Panicum virgatum 'Heavy Metal'	20
	Karl Foerster Grass	Calamagrostis acutiflora 'Karl Foerster'	25
	Black-Eyed Susan	Rudbeckia fulgida 'Goldstrum'	20
	Firecracker Penstemon	Penstemon eatonii	20
	May Night Salvia	Salvia nemorosa 'May Night'	20
	Moonshine Yarrow	Achillea 'Moonshine'	20
	Walker's Low Catmint	Nepeta x faassenii	20
	Creeping Mahonia	Mahonia repens	20
	Dwarf Shasta Daisy	Leucanthemum x superbum 'Snow Lady'	20
	Delphinium x Pacific Giant		12
	Daylily	Hemerocallis variety	12
	Autumn Joy Sedum	Autumn Joy Stonecrop	20
	Sedum variety	Assorted Sedum ground cover plants	35
		Total Perennials	284

- Notes:
1. Wood mulch to be spread under and around all plant material in a uniform surface. Provide a sample of material for review and approval.
 2. Warranty plants and trees for one year after final acceptance. Replace dead or dying materials not in vigorous, thriving condition as soon as weather permits.
 3. Determine locations of underground utilities and perform work in a manner which will avoid possible damage.
 4. Plants shall be specimen quality, typical of their species or variety.
 5. Plant trees and shrubs in pits 12' larger than tree ball, backfill with a mix of 2 parts topsoil, 1 part Nutri-Mulch .



Landscape Plan
Iron Mountain Hot Springs Expansion
Parcel ID #218509200031
Glenwood Springs, Colorado
Updated August 6, 2020 by The Land Studio, Inc.

The LAND Studio
365 River Bend Way
Glenwood Springs, CO 81601
970.927.3690
landstudio2@comcast.net

Sheet
1 of 2

Site Use Calculations

Hatched area #1 (parking area) = 91,379 sq. ft.
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Shrubs: 1 per 100 s.f. of required landscape area = 142
Trees: 1 per 400 s.f. of required landscape area = 36

Hatched area #3 designates the approved parking and acces extents as shown on the "Recently Constructed Site Features" plan sheet prepared by SGM, dated 06/30/2015. Hatched area contains 94 parking stalls.



Landscape Plan

Iron Mountain Hot Springs Expansion

Parcel ID #218509200031

Glenwood Springs, Colorado
Updated August 6, 2020 by The Land Studio, Inc.

The **LAND** Studio
365 River Bend Way
Glenwood Springs, CO 81601
970.927.3690
landstudio2@comcast.net

Sheet
2 of 2

LIST OF NEIGHBORHOOD MEETING NOTICES SENT:

CITY OF GLENWOOD SPRINGS
101 W. 8TH STREET
GLENWOOD SPRINGS, CO
81601

COLO RIVER WATER CON DIST
PO BOX 1120
GLENWOOD SPRINGS, CO
81602

TRPP AT 300, LLC
PO BOX 2021
GLENWOOD SPRINGS, CO
81602

1303 GRAND, LLC
PO BOX 1576
GLENWOOD SPRINGS, CO
81602

TWO RIVERS PP OFFICE
CONDO ASSOC
PO BOX 1576
GLENWOOD SPRINGS, CO
81602

COLO ENERGY SYSTEMS LLC
274 MOUNTAIN SHADOWS DR
GLENWOOD SPRINGS, CO
81601

D&S FAMILY HOLDINGS LLC &
HALVERSON TRUST
PO BOX 3780
EAGLE, CO 81631

SRE COLORADO 4 RF LLC
4401 COLWICK ROAD
CHARLOTTE, NC 28211

CDOT
4201 E. ARKANSAS AVE
DENVER, CO 80222

LIST OF ATTENDEES AT MEETING:

GREG KELLER
JOHN NEIL
DAVID MCCONAUGHY
LYNN KLEAGER
AUDREY TURNER
STEVE BECKLEY
DOUG PRATTE
JULIE PRATTE

July 8, 2020

RE: PUBLIC NOTICE of a NEIGHBORHOOD MEETING to discuss the upcoming Major Site Plan Application for Iron Mountain Hot Springs.

Iron Mountain Hot Springs is submitting a request for Major Site/Architectural Plan Review to the City of Glenwood Springs for expansion of Iron Mountain Hot Springs.

Iron Mountain Hot Springs LLC wishes to expand the existing spa facility to include a new entry building with retail space and food service, including additional men's & women's locker rooms with sauna facilities. The expanded facility will have access control and be a separate experience from the existing pools. The expansion will provide a limited number of entries to keep the environment tranquil and restorative. The new expanded area includes 10 outdoor mineral pools, an enclosed heated fresh water pool w/ attached hot tub. There will also be a set of private cabanas that have small mineral pools within. The expansion project will also include a comprehensive riverbank restoration plan along the entire Iron Mountain Hot Springs property which will be incorporated into the new landscape features of the expanded facility. Lastly the parking area will be reconfigured to improve traffic flow, capacity and accessibility and the lot will be paved with asphalt.

A future hotel and the approved Caverns Village are also illustrated for context on the Master Plan concept drawing attached.

For more information about the project, please attend the outdoor meeting:

LOCATION: 281 CENTENNIAL, OUTSIDE OF EXISTING BUILDING

MEETING DATE: FRIDAY JULY 24, 2020

MEETING TIME: 1:00 PM (MST)

If you are unable to attend the meeting, please forward your questions to:
Landstudio2@comcast.net

Iron Mountain Hot Springs Master Plan

Glenwood Springs, Colorado

Updated July 23, 2020 by The Land Studio, Inc.

for City of Glenwood Springs Pre-Application Conference



0 60 120 180 300

river bank restoration

cafe
food service

mechanical

locker rooms
retail
offices

Iron Mountain
Hot Springs

Entry Building

locker rooms
food service
retail
mechanical

Iron Mountain
Hot Springs
Expansion

Enclosed
Pool

cabanas

new lift station

river bank restoration

future path connection to the LoVa trail

INTERSTATE 70

Reconfigured
Iron Mountain Hot Springs Parking

Caverns
Village
CDOT offices
(19) apartments

Caverns Village parking

Caverns Village parking

CDOT
maintenance
facility

entry
monument

Glenwood Caverns Adventure Park
additional parking



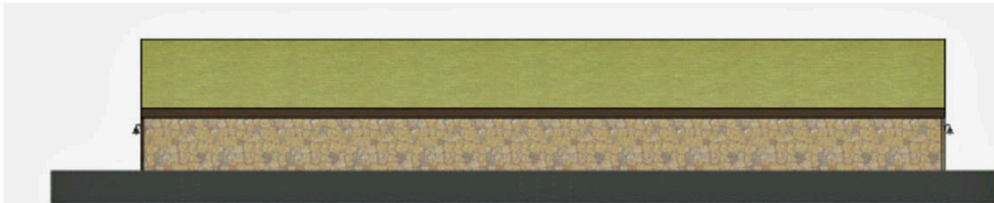
① EAST ELEVATION



① WEST ELEVATION



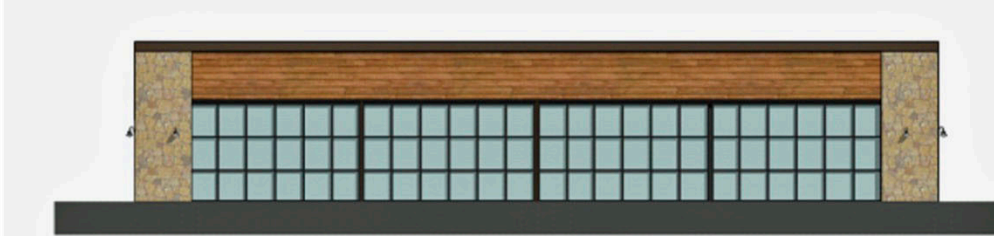
ENTRANCE BUILDING



4 EAST ELEVATION
1/16" = 1'-0"



2 SOUTH ELEVATION
1/16" = 1'-0"



3 WEST ELEVATION
1/16" = 1'-0"



1 NORTH ELEVATION
1/16" = 1'-0"

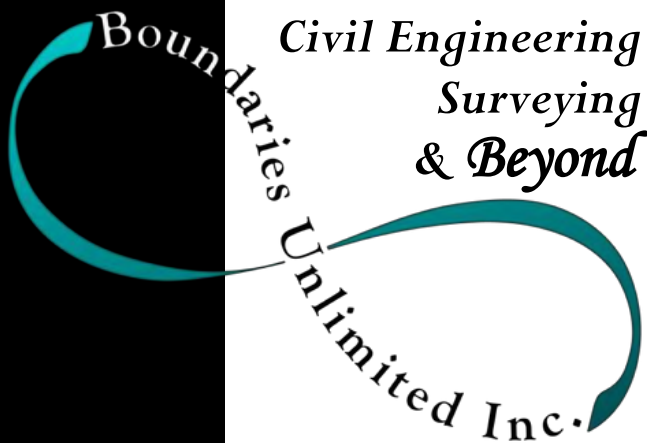


POOL BUILDING

Iron Mountain Hot Springs

Architectural/Site Plan Review

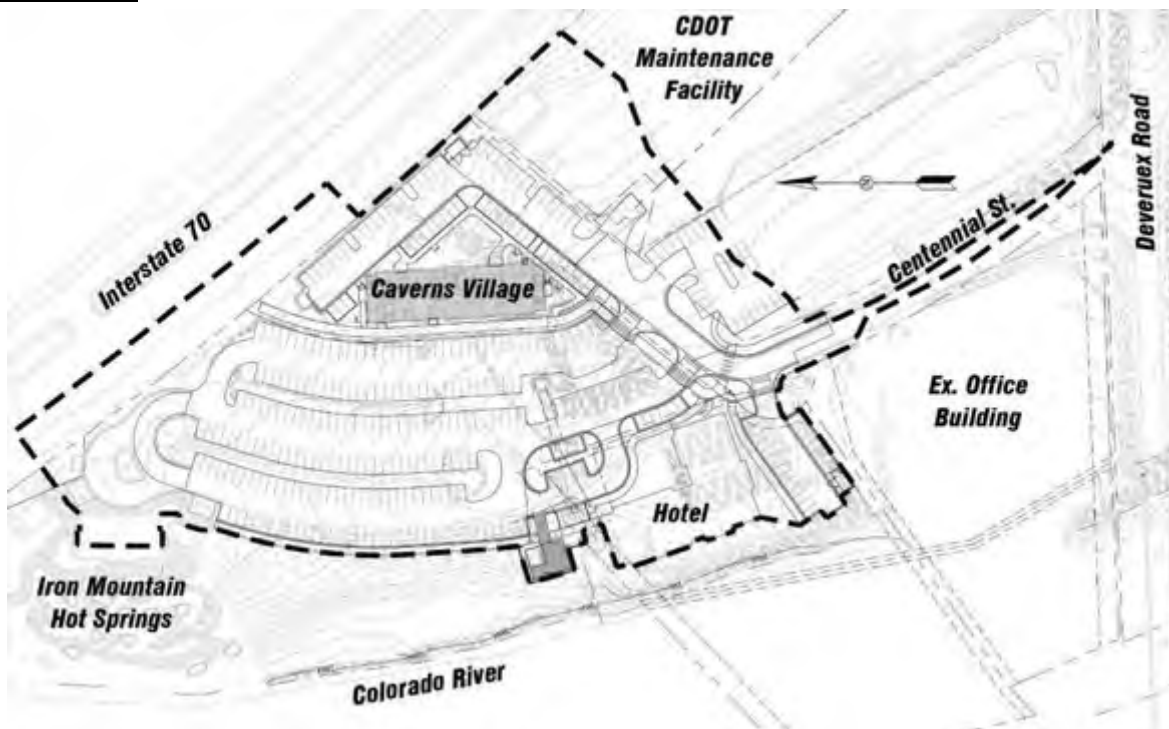
Glenwood Springs, Colorado
Prepared July 24, 2020 by The Land Studio, Inc.



Major Drainage Report

Iron Mountain Hot Springs Master Plan

July 6, 2020
BUI Project #17015



"I hereby affirm that this report and the accompanying plans for the Iron Mountain Hot Springs-Master Plan was prepared by me (or under my supervision) for the owners thereof in accordance with the provisions of the City of Glenwood Springs Stormwater, Drainage, Water Quality and Erosion Control Chapter and approved variances and exceptions listed thereto. I understand that it is the policy of the City of Glenwood Springs that the City of Glenwood Springs does not and will not assume liability for drainage facilities designed by others."

Deric Walter

Deric Walter
I am the author of this document
970.945.5252
2020.07.21 11:11:36-06'00'

License No. 37110

Licensed Professional Engineer, State of Colorado



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Appendix

Drainage Basin Summary Spreadsheet
Autodesk Storm and Sanitary Sewer Analysis (SSA) datasheets
NRCS Websoil Survey
NOAA Atlas 14-IDF Data
Letter-Dept. of the Army, U.S. Army Corps of Engineers (USACE)
USACE Regional General Permit #37 Application
BioClean/Forterra Deflective Screening Device Details
Design Plan & Details - Caverns Village (24"x36")

A. Introduction

The purpose of this report is to provide an outline and basis of design for the drainage improvements proposed for the Iron Mountain Hot Springs-Master Plan in Glenwood Springs, Colorado. Master Plan improvements include additional pools/buildings at Iron Mountain Hot Springs (IMHS), a lift station, a hotel, the Caverns Village multi-family/office complex and parking improvements.

B. Previous Studies

The property was previously analyzed by SGM, Inc. (Project #2014-339.001 dated July 29, 2014) for the construction of the existing Iron Mountain Hot Springs. The intent of that study was also to focus on stormwater quality improvements “to ensure that the quality of the Colorado River is not compromised due to stormwater runoff from this site (SGM-Inc, 2014).” The study considered six (6) drainage basins, A-F, across the property.

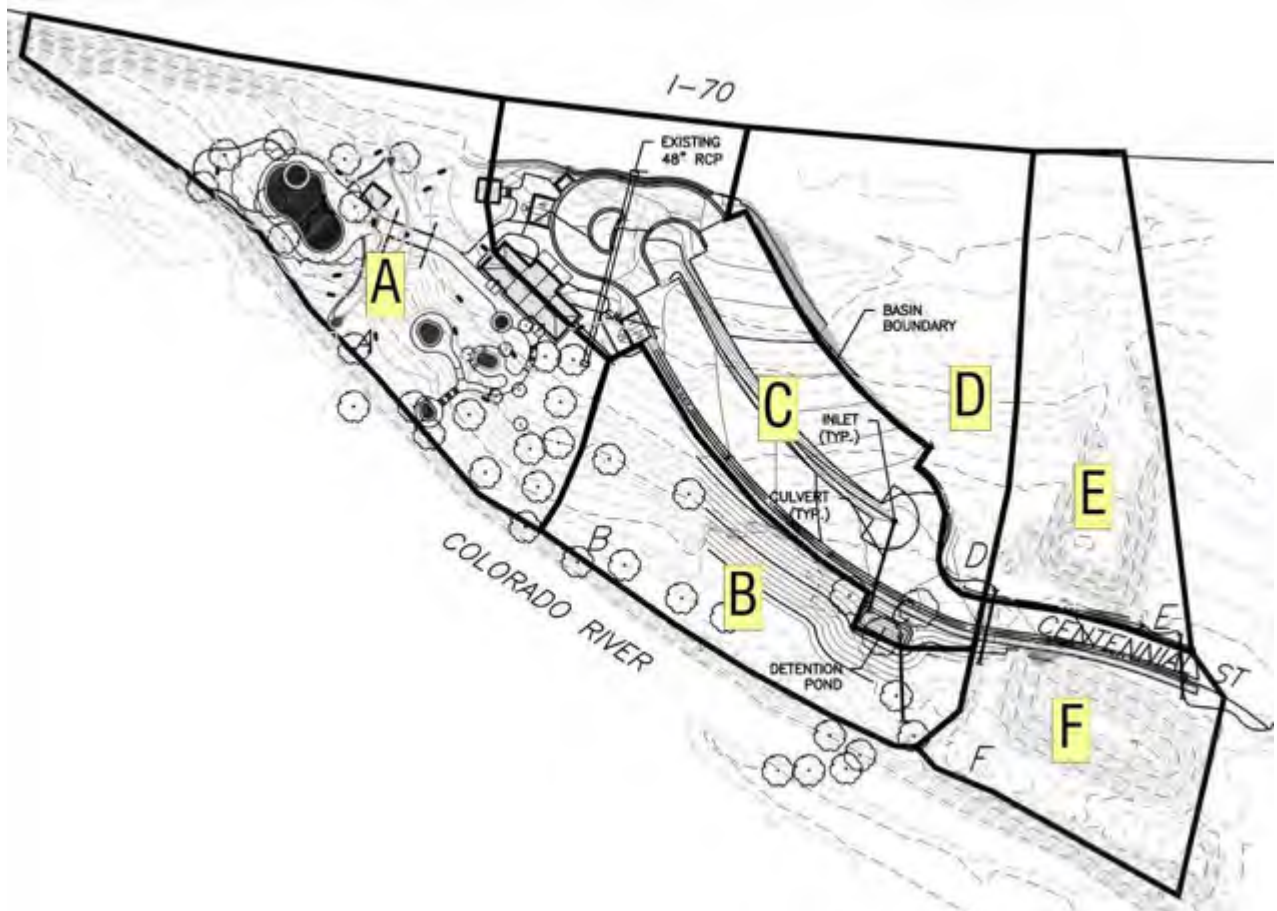


Figure 1: SGM-Inc. Drainage Basins

SGM, Inc. study Section 6 - *Stormwater Quality Plan* defined the parameters for stormwater improvements in the developed area as “The main goal of the site design is to provide detention and sedimentation for the project. Basin C, where the development of the access road, parking and drop off areas occurs is the only basin with a significant increase in runoff coefficient and possible sedimentation. In order to treat that increased flow, the design provides a detention pond that will provide a location for sedimentation and release of the historic 25-year flowrate.

Basin A contains the only other significant development, consisting of the pool area. This will be heavily landscaped throughout the basin, serving to provide improved vegetative cover and improved water quality from the basin area.”

The improvements proposed for this Master Study expand the IMHS pools and support buildings into Basin-B. This area will also be heavily landscaped throughout the basin, serving to provide improved vegetative cover and improve water quality from the basin area. The banks of the Colorado River will also be restored as permitted with the U.S. Army Corps of Engineers (see the Appendix). Therefore, no additional stormwater systems are anticipated for this area.

The previously constructed detention and sediment structures constructed for Basin-C will be replaced with a new storm sewer and water quality (WQ) best management plan (BMP) structure to screen trash/debris and promote sediment dropout. The new system is further defined in this report.

C. Existing Conditions

The construction of I-70 and storm sewer extending from I-70 to the west side of Centennial Street effectively protect the parcel from the majority of potential offsite runoff that might have existed historically. A second culvert also exists under Centennial Street for the conveyance of runoff from the adjacent CDOT Maintenance Facility and parking lot under the roadway and through a swale south of the proposed hotel parking lot. Therefore, the subject area consists primarily of what was previously referred to as Basins B thru F in the SGM-Inc. study and minor surrounding areas which flow into the site. These minor areas include the vegetated buffer between the site and Interstate 70, a portion of Centennial Street and a portion of the existing CDOT Maintenance Facility property. In general, the topography slopes from northeast to southwest at approximately 3%-7% towards the Colorado River. Site vegetation consists of native grasses, weeds and a few deciduous trees in poor to very poor condition. The USDA NRCS Websoil Survey indicates that soils are hydrologic Group B which typically have a moderate infiltration rate when thoroughly wetted. Stormwater runoff is primarily sheet flow in the upper reaches and shallow flow thereafter. A geotechnical investigation was performed by CTL-Thompson (Job No. GS05875-145, dated June 29, 2014). In general, the borings on or near the project site consist of 12”-18” of organic clays over clayey to silty gravel with cobbles over a layer of travertine cemented sandy gravels encountered at a depth range of 4’-6’ below the native finish grade. There are two large mounds of imported soil that will be removed with the construction of Caverns Village and the hotel.

The property is bordered by the regional CDOT Offices/Maintenance Facility to the southeast, I-70 to the northeast, the Colorado River to the west and a multistory office building to the southwest. Existing runoff is captured by poorly shaped swales and storm inlets/culverts traversing under the IMHS access/Centennial Street leading to the Colorado River. No stormwater treatment systems exist.

D. Floodplain

Areas along the banks of the Colorado River are located in the floodplain per the Flood Insurance Rate Map (FIRM) prepared by the Federal Emergency Management Agency (FEMA) for Garfield County, Colorado, Community Panel Number 080071 1432 C, effective date: October 15, 1985. The construction of specific structures area not included in this study and will need to be addressed at the time of building permit application.



Figure 2: FEMA FIRM Map

E. Proposed Conditions

As discussed, the proposed completed development will include additional pools/buildings at Iron Mountain Hot Springs (IMHS), a lift station, a hotel, the Caverns Village multi-family/office complex and parking improvements.



Figure 3: Master Plan

Based on pre-design conversations with the City Engineer, the focus of design is to improve stormwater quality for the minor (25-year, 1-hour storm) event and to convey the major (100-year, 1-hour storm) event as the property is located directly adjacent to the Colorado River. In planning out the design, several stormwater management alternatives were considered, including: bioretention; WQCV Drywells; Extended Detention Basins (EDB); engineered treatment devices; vegetated swales and storm sewers. It was determined that the underlying travertine cemented layer was not suitable for infiltrative or underground BMPs (drywells, bioretention or underground detention). The proposed uses require a significant number of vehicle parking spaces reducing the availability for EDBs. Thus, the remaining available BMP solutions for stormwater quality improvements became engineered treatment devices.

In general, runoff from Caverns Village, the hotel and parking areas/drives will be captured by surface swales and storm sewers leading to two (2) hydrodynamic separators located just above the Colorado River. The buildings will also need to be equipped with roof gutters and downspouts connected to the storm sewer systems. Runoff from landscaped areas around the buildings should be captured by area inlets also connected to the storm sewer.

The configuration of the buildings and the relatively flat nature of the site required attention to detail and the use of a significant number of area inlets. Downspouts on the shadowed sides of the buildings will need to be equipped with heat tape. The vegetated base of the swales will function as an *impervious disconnect* between most of the parking areas and the storm sewers. A detailed storm sewer analysis of this system has been performed using Autodesk Storm and Sanitary Sewer Analysis v2020 software for capture and conveyance of the major event (see the Appendix). The hydrodynamic separators are sized to treat the minor event runoff while safely bypassing the major event runoff (also in the Appendix).

F. Proposed Drainage Basins

The proposed development divides the treatment areas (2) primary basins, Basin-North (similar to SGM, Inc. Basin-C) and Basin-South (similar to SGM-Inc. Basins D-F plus additional outlying areas). This analysis further divides Basin-North into three (3) minor basins and Basin-South into eight (8) minor basins for hydraulic sizing of storm sewer structures. Please see Figure 4 for basin boundaries and Table 1 for basin hydrology (areas, runoff coefficients, times of concentration, runoff rates, etc).

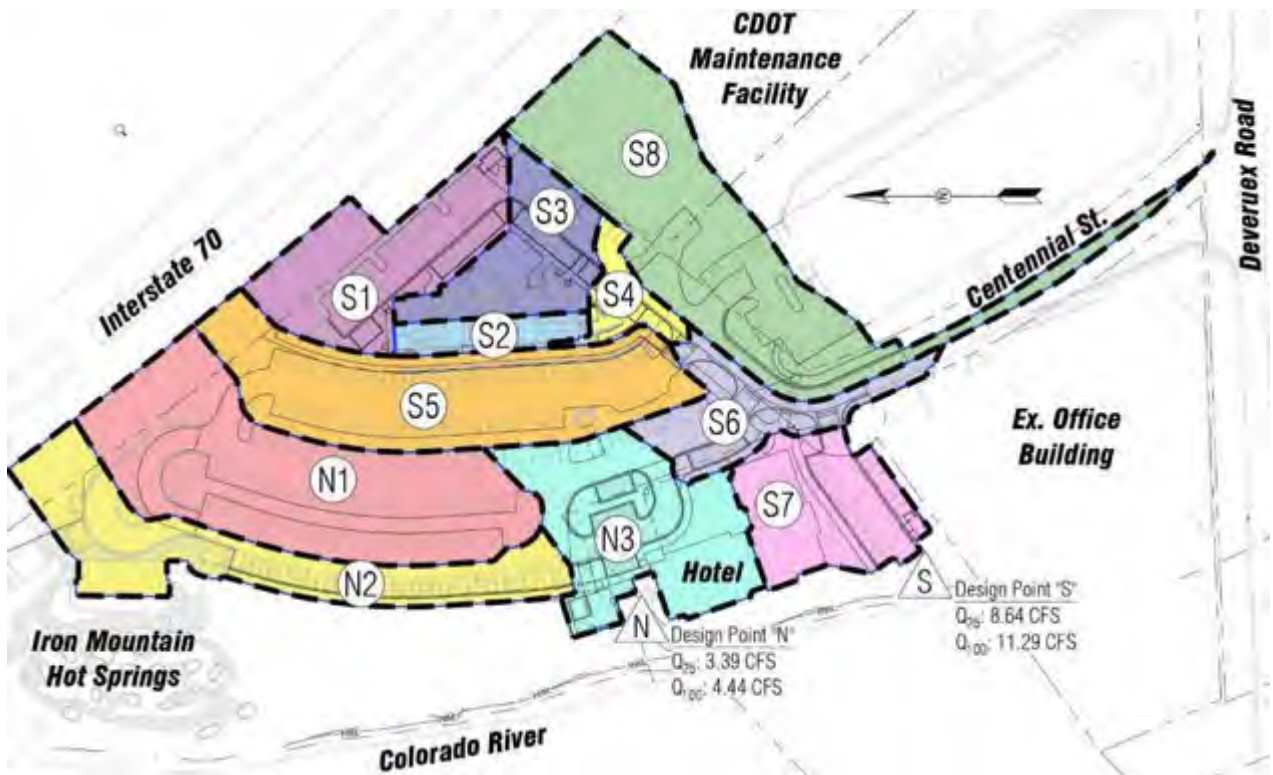


Figure 4: Proposed Drainage Basins

Drainage Basin Summary										
Basin	Area (sf)	Area (Ac)	Impervious		Soil Type B		Runoff C	Tc (min.)	Q ₂₅ (cfs)	Q ₁₀₀ (cfs)
					Open Space/LS					
			Area (sf)	%	Area (sf)	%				
N1	50460	1.158	32970	65.3%	17490	34.7%	0.69	23.66	1.76	2.26
N2	30450	0.699	24750	81.3%	5700	18.7%	0.82	17.66	1.54	2.00
N3	29890	0.686	26840	89.8%	3050	10.2%	0.88	5.00	3.05	3.96
Design Point "N":									3.39	4.44
S1	24660	0.566	12410	50.3%	12250	49.7%	0.58	15.28	0.98	1.27
S2	5800	0.133	5080	87.6%	720	12.4%	0.86	5.00	0.58	0.75
S3	16390	0.376	12140	74.1%	4250	25.9%	0.76	5.00	1.44	1.88
S4	5970	0.137	4620	77.4%	1350	22.6%	0.79	5.00	0.55	0.71
S5	41450	0.952	27290	65.8%	14160	34.2%	0.70	20.83	1.60	2.06
S6	14610	0.335	12560	86.0%	2050	14.0%	0.85	5.00	1.44	1.87
S7	19570	0.449	17000	86.9%	2570	13.1%	0.86	5.00	1.95	2.54
S8	52210	1.199	38860	74.4%	13350	25.6%	0.76	7.24	3.90	5.06
Design Point "S":									8.64	11.29

Table 1: Proposed Drainage Basin Summary

1. Basin N1

Basin N1 (1.158 acres; $C=0.69$; $T_c=23.66$ min.; $Q_{25}=1.76$ cfs; $Q_{100}=2.26$ cfs) consists of the central portion of the existing IMHS parking lot, median landscaping and a portion of vegetated buffer between the site and Interstate 70. Impervious areas include the parking and pervious areas include the median, buffer and landscape islands. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed

percent effective imperviousness is 65.3%. In general, runoff will sheet flow across the I-70 vegetated buffer (and upper asphalt), shallow flow through the swale in the median landscaping, be collected by a 24"Ø dome grate area inlet, piped to the north hydrodynamic separator for treatment, and released to a short swale leading to the Colorado River.

2. Basin N2

Basin N2 (0.699 acres; $C=0.82$; $T_c=17.66$ min.; $Q_{25}=1.54$ cfs; $Q_{100}=2.00$ cfs) consists of the lower reaches of the existing IMHS parking lot, a portion of the existing entry building, walkways, and a portion of the vegetated buffer between the site and Interstate 70. Impervious areas include the parking/walks and pervious areas include the buffer and landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 81.3%. In general, runoff will sheet flow across the I-70 vegetated buffer (and upper asphalt), shallow flow through the proposed curb, be collected by a double Type-16 combination curb inlet, and be piped to the same north hydrodynamic separator previously discussed

3. Basin N3

Basin N3 (0.686 acres; $C=0.88$; $T_c=5.0$ min.; $Q_{25}=3.05$ cfs; $Q_{100}=3.96$ cfs) consists of the north wing of the hotel, the lift station site, a portion of the IMHS access drive and adjacent parking north of the hotel. Impervious areas include the building/parking/walks and pervious areas include the landscape islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 89.8%. In general, runoff will sheet flow across the building roof (and upper asphalt), shallow flow through the proposed curb, be collected by a Type-13 valley pan inlet, and be piped to the same north hydrodynamic separator previously discussed.

4. Basin S1

Basin S1 (0.566 acres; $C=0.58$; $T_c=15.28$ min.; $Q_{25}=0.98$ cfs; $Q_{100}=1.27$ cfs) consists of the north portion of the proposed Caverns Village parking lot, landscaping and a portion of vegetated buffer between the site and Interstate 70. Impervious areas include the parking/walks and pervious areas include the buffer, parking islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 50.3%. In general, runoff will sheet flow across the I-70 vegetated buffer (and upper asphalt), shallow flow through the curb/valley pan, be collected by Type-13 valley pan inlet, be piped to the south hydrodynamic separator for treatment, and released to a short swale leading to the Colorado River.

5. Basin S2

Basin S2 (0.133 acres; $C=0.86$; $T_c=5$ min.; $Q_{25}=0.58$ cfs; $Q_{100}=0.75$ cfs) consists of the west half of the proposed Caverns Village building and adjacent landscaping. Impervious areas include the building and pervious areas include the adjacent landscaping. Site vegetation consists of landscape shrubs and vegetative ground cover. The net watershed percent effective imperviousness is 87.6%. In general, runoff will sheet flow across the building roof (and adjacent landscaping) be collected by roof drains or several 12"Ø area inlets, and be piped to the same south hydrodynamic separator previously discussed.

6. Basin S3

Basin S3 (0.376 acres; $C=0.76$; $T_c=5$ min.; $Q_{25}=1.44$ cfs; $Q_{100}=1.88$ cfs) consists of the east half of the proposed Caverns Village building, the adjacent southwest parking lot, and adjacent landscaping. Impervious areas include the building/parking/walks and pervious areas include parking islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent

effective imperviousness is 74.1%. In general, runoff will sheet flow across the building roof (and upper asphalt), shallow flow through the proposed curb or landscape areas, be collected by several 12"Ø area inlets or a Type-13 valley pan inlet, and be piped to the same south hydrodynamic separator previously discussed.

7. Basin S4

Basin S4 (0.137 acres; $C=0.79$; $T_c=5.0$ min.; $Q_{25}=0.55$ cfs; $Q_{100}=0.71$ cfs) consists of a portion of the parking on the south side of the Caverns Village building and adjacent walks/landscaping. Impervious areas include the parking/walks and pervious areas include the landscape islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 77.4%. In general, runoff will sheet flow across the asphalt (and adjacent landscaping), shallow flow through the proposed valley pan, be collected by a Type-13 valley pan inlet, and be piped to the same south hydrodynamic separator previously discussed.

8. Basin S5

Basin S5 (0.952 acres; $C=0.70$, $T_c=20.83$ min.; $Q_{25}=1.60$ cfs; $Q_{100}=2.06$ cfs) consists of the upper portion of the existing IMHS parking lot, a proposed expansion of that parking lot to the south, a portion of a proposed regional public trail, median landscaping and a portion of vegetated buffer between the site and Interstate 70. Impervious areas include the parking/trail and pervious areas include the buffer, median and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 65.8%. In general, runoff will sheet flow across the Interstate 70 vegetated buffer (and upper asphalt), shallow flow through the swale in the median landscaping, be collected by a 24"Ø dome grate area inlet, and be piped to the same south hydrodynamic separator previously discussed.

9. Basin S6

Basin S6 (0.335 acres; $C=0.85$; $T_c=5.0$ min.; $Q_{25}=1.44$ cfs; $Q_{100}=1.87$ cfs) consists of the north half of the proposed extension of Centennial Street, a portion of the proposed regional public trail, the beginning of the IMHS access drive, adjacent parking and adjacent landscaping. Impervious areas include the street/parking/walks/trail and pervious areas include the landscape islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 86.0%. In general, runoff will sheet flow across the asphalt (and adjacent landscaping), shallow flow through the proposed curb/pans, be collected by two (2) Type-13 valley pan inlets, and be piped to the same south hydrodynamic separator previously discussed.

10. Basin S7

Basin S7 (0.449 acres; $C=0.86$; $T_c=5.0$ min.; $Q_{25}=1.95$ cfs; $Q_{100}=2.54$ cfs) consists of the south wing of the hotel, the hotel parking lot, and adjacent landscaping. Impervious areas include the building/walks/parking and pervious areas include the landscape islands and adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 86.9%. In general, runoff will sheet flow across the building roof (and upper asphalt), shallow flow through the proposed curb, be collected by a Type-16 combination curb inlet, and be piped to the same south hydrodynamic separator previously discussed.

11. Basin S8

Basin S8 (1.199 acres; $C=0.76$, $T_c=7.24$ min.; $Q_{25}=3.90$ cfs; $Q_{100}=5.06$ cfs) consists of a portion of the existing CDOT Maintenance Facility, existing parking, the south half of the proposed extension of Centennial Street, portions of the proposed regional public trail,

walks, and adjacent landscaping. Impervious areas include the street/parking/walks/trail and pervious areas include the adjacent landscaping. Site vegetation consists of landscape trees, shrubs, vegetative ground cover, and lawn. The net watershed percent effective imperviousness is 74.4%. In general, runoff will sheet flow across the upper reaches of the Maintenance Facility, shallow flow through swales in the landscaping, be collected by a Type-C area inlet, and be piped to the same south hydrodynamic separator previously discussed.

G. Low Impact Site Design

The landscaping strategy is to reduce impervious areas where possible, locally reduce slopes and vegetate swale for initial pollutant screening. Low Impact Site Design (LID) principals have been considered for implementation as follows:

Principle 1: Consider stormwater quality needs early in the design process

The need to manage and treat stormwater runoff was discussed during the design process affording the design team and owner an opportunity to discuss the options and evaluate their feasibility. Recognizing that the building footprints and parking would utilize the majority of this property and that the travertine layer prevented adequate infiltration, it was decided that engineered treatment devises were the best solution. Plans were made to install vegetative planting areas in parking islands and between the buildings and the parking. The use of grated inlets will skim larger floating trash and debris. The vegetated swales and hydrodynamic separators will allow for sediment dropout and hydrocarbon reduction.

Principle 2: Use the entire site when planning for stormwater quality treatment

The landscape and grading designs were prepared with the intent of providing an impervious disconnect, initial hydrocarbon removal through vegetation, removing floating trash (grated inlets) and increasing sediment dropout.

Principle 3: Avoid unnecessary impervious area

The access drives parking areas were all reworked during the design process to maximize the length of the landscape medians and extend impervious disconnects.

Principle 4: Reduce runoff rates and volumes to more closely match natural conditions

Based on pre-design conversations with the City Engineer, the focus of design is to improve stormwater quality for the minor (25-year, 1-hour storm) event and to convey the major (100-year, 1-hour storm) event as the property is located directly adjacent to the Colorado River. This will enable the local site runoff to exit the region before additional peak flows from upstream areas enter this reach of the Colorado River.

Principle 5: Integrate stormwater quality management and flood control

The purposed use of vegetated planting areas around the buildings and vegetated medians between the parking lots provides for minor pretreatment while the hydrodynamic separators will provide more extensive treatment. The proposed inlets and storm sewers are located for rapid removal of runoff and sized to convey the major storm event

Principle 6: Develop stormwater quality facilities that enhance the site, the community and the environment

Grading and area inlet placement have been designed to protect the building structures/parking and capture large quantities of runoff. Treating the runoff will bolster the community and environment.

Principle 7: Use a treatment train approach

A train approach has been applied by flowing parking areas over landscaped medians before it enters the inlets. The hydrodynamic separators will also provide a treatment train before discharged runoff enters the swales leading to the Colorado River.

Principle 8: Design sustainable facilities that can be safely maintained

The surface has been designed with relatively flat or gently sloping landscape areas and stepped as needed with curbs or walls.

Principle 9: Design and maintain facilities with public safety in mind

All inlets are grated and the hydrodynamic separators will be equipped with heavy iron access lids similar to what is used on sanitary sewer manholes. These will deter and protect the public.

H. Hydrologic Criteria

The stormwater management system has been designed to capture and convey runoff from the 25-year and 100-year, 1-hour storm events and reduce pollutant discharge from the site. Rainfall intensities are based on rainfall data obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 point precipitation database. Runoff rates and volumes were estimated using Autodesk Storm and Sanitary Sewer Analysis v2020 software. Hydrodynamic separators were sized by Forterra. Copies of this information and data sheets have been included in the Appendix.

I. Hydraulic Criteria

Storm sewer inlets and pipes were sized using Autodesk Storm and Sanitary Sewer Analysis v2020 software. Copies of these data sheets have been included in the Appendix.

J. Proposed Facilities

Erosion control measures and Best Management Practices (BMPs) should be followed during construction to ensure that the property and the adjacent streets are not impacted from sedimentation. Temporary BMPs should include a combination of temporary sediment control fence, erosion control logs, rock socks, certified weed free hay bales, inlet silt sacks and erosion fabrics to minimize erosion and control runoff. A detailed Stormwater Management and Erosion Control Plan may need to be designed and approved by the City of Glenwood Springs Engineering Department prior to construction. Preparation of stormwater management plans, permitting and compliance are the responsibility of the Contractor. This project will also require a permit for stormwater discharge associated with construction activity from the State of Colorado.

Runoff from the 25-year storm event will be captured and treated as discussed. Excess runoff beyond the 25-year storm event will be conveyed through the storm sewer system and bypassed through the hydrodynamic separators to the Colorado River.

As required by Code, the vegetated swales and hydrodynamic separators will reduce the pollutant rates to acceptable levels. Therefore, the proposed improvements should improve the water quality for downstream properties and the environment.

K. Operation and Maintenance

During construction activities and until vegetation can be established, temporary rock socks should be installed in the roadside curb and gutter and around curb inlets. Temporary silt sacks should be installed in roadway drop inlets. Temporary certified weed free hay bales, check dams, silt dikes or erosion control logs should be installed around area drains and culverts in accordance with Colorado Department of Transportation M-Standards. The Contractor shall be responsible for removing debris from the sediment structures after each storm event. The

Contractor shall also repair any damaged erosion controls, remove accumulated sediment from the hydrodynamic separators and from behind hay bales/check dams/silt dikes/erosion control logs after each storm event. After construction, the property owner shall be responsible for removing debris and sediment on an as-needed basis.

The hydrodynamic separators must be inspected and maintained annually to remove sediment and debris that has collected. Removed waters, sediment and debris must be hauled to the landfill for lawful disposal.

The Contractor shall be responsible for complying with all local and state guidelines and permitting issues. Sediment and mud should be prevented from leaving the construction site by the immediate placement of a stabilized construction entrance. The Contractor should be responsible for cleaning and general upkeep of the site and adjacent roadways. As an effective dust control measure, water and weed-free straw may be utilized to minimize uprising of dust.

Good housekeeping BMPs should be followed throughout the entire life of the project. These include but are not limited to:

- refuge receptacles should be regularly emptied and equipped with lids;
- keeping machinery in good operating condition to prevent leakage;
- apply appropriate (not excessive) amounts of fertilizer to the landscaping;
- scheduled maintenance cleaning of any downstream curb and gutter, storm sewer inlets, culverts and ditches;
- general site cleanliness and proper training of employees;
- Grading must be performed (but not in excess) to ensure that drainage is directed away from all structures in all directions.

Once construction activities are completed, final stabilization of the disturbed areas should begin. Within one growing season of the project completion, uniform vegetative site coverage should be equal to or greater than 70% of the pre-disturbance levels, or equivalent permanent, physical erosion reduction methods should be employed. When the site is determined to have reached the final stabilization stage, erosion control structures can be removed.

L. Works Cited

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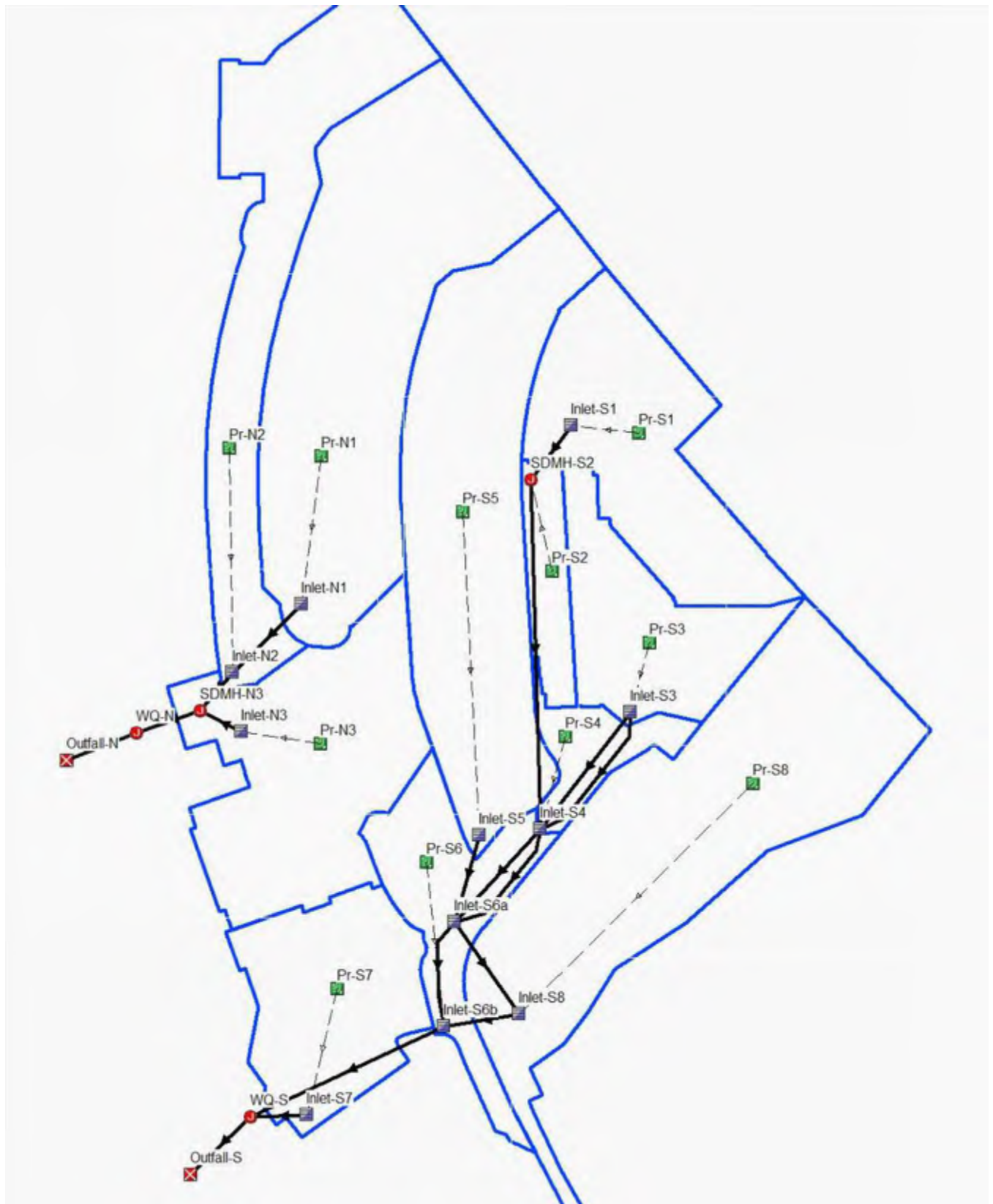
Appendix

- *Drainage Basin Summary Spreadsheet*
- *Autodesk Storm and Sanitary Sewer Analysis (SSA) datasheets*
- *NRCS Websoil Survey*
- *NOAA Atlas 14-IDF Data*
- *Letter-Dept. of the Army, U.S. Army Corps of Engineers (USACE)*
- *USACE Regional General Permit #37 Application*
- *BioClean/Forterra Deflective Screening Device Details*
- *Design Plan & Details (24"x36")*

Drainage Basin Summary

Iron Mountain Hot Springs-Master Plan

Basin	Area (sf)	Area (Ac)	Impervious		Soil Type B		Runoff C	Tc (min.)	Q ₂₅ (cfs)	Q ₁₀₀ (cfs)
					Open Space/LS					
			Area (sf)	%	Area (sf)	%				
N1	50460	1.158	32970	65.3%	17490	34.7%	0.69	23.66	1.76	2.26
N2	30450	0.699	24750	81.3%	5700	18.7%	0.82	17.66	1.54	2.00
N3	29890	0.686	26840	89.8%	3050	10.2%	0.88	5.00	3.05	3.96
Design Point "N":									3.39	4.44
S1	24660	0.566	12410	50.3%	12250	49.7%	0.58	15.28	0.98	1.27
S2	5800	0.133	5080	87.6%	720	12.4%	0.86	5.00	0.58	0.75
S3	16390	0.376	12140	74.1%	4250	25.9%	0.76	5.00	1.44	1.88
S4	5970	0.137	4620	77.4%	1350	22.6%	0.79	5.00	0.55	0.71
S5	41450	0.952	27290	65.8%	14160	34.2%	0.70	20.83	1.60	2.06
S6	14610	0.335	12560	86.0%	2050	14.0%	0.85	5.00	1.44	1.87
S7	19570	0.449	17000	86.9%	2570	13.1%	0.86	5.00	1.95	2.54
S8	52210	1.199	38860	74.4%	13350	25.6%	0.76	7.24	3.90	5.06
Design Point "S":									8.64	11.29



Project Description

File Name 17015-SSA.SPF

Project Options

Flow Units CFS
Elevation Type Elevation
Hydrology Method Rational
Time of Concentration (TOC) Method SCS TR-55
Link Routing Method Kinematic Wave
Enable Overflow Ponding at Nodes YES
Skip Steady State Analysis Time Periods YES

Rainfall Details

Return Period..... 25 year(s)

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Runoff Coefficient	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ft ³)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	Pr-N1	1	0.690	0.87	0.60	2518.8	1.76	0 00:23:39
2	Pr-N2	1	0.820	0.79	0.65	1649.4	1.54	0 00:17:39
3	Pr-N3	1	0.880	0.42	0.37	921.6	3.05	0 00:05:00
4	Pr-S1	1	0.580	0.76	0.44	906.3	0.98	0 00:15:16
5	Pr-S2	0	0.860	0.42	0.36	175.0	0.58	0 00:05:00
6	Pr-S3	0	0.760	0.42	0.32	437.1	1.44	0 00:05:00
7	Pr-S4	0	0.790	0.42	0.33	165.2	0.55	0 00:05:00
8	Pr-S5	1	0.700	0.83	0.58	2017.2	1.60	0 00:20:49
9	Pr-S6	0	0.850	0.42	0.36	435.9	1.44	0 00:05:00
10	Pr-S7	0	0.860	0.42	0.36	590.4	1.95	0 00:05:00
11	Pr-S8	1	0.760	0.51	0.39	1688.1	3.90	0 00:07:14

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ft³)	(min)
1	SDMH-N3	Junction	5719.86	5725.45	5719.86	5721.00	0.00	3.390	5720.66	0.00	4.79	0 00:00	0.00	0.00
2	SDMH-S2	Junction	5729.50	5731.40	5729.50	0.00	0.00	0.860	5730.03	0.00	1.37	0 00:00	0.00	0.00
3	WQ-N	Junction	5719.73	5725.57	5719.73	5721.00	0.00	3.390	5720.34	0.00	5.23	0 00:00	0.00	0.00
4	WQ-S	Junction	5716.00	5721.50	5716.00	0.00	0.00	8.640	5718.48	0.00	3.02	0 00:00	0.00	0.00
5	Outfall-N	Outfall	5717.50					3.390	5717.86					
6	Outfall-S	Outfall	5715.76					8.640	5716.69					

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1	Link-47	Pipe	Inlet-S7	WQ-S	32.09	5718.50	5718.00	1.5600	12.000	0.0150	1.770	3.85	0.46	4.80	0.48
2	Link-57	Pipe	SDMH-N3	WQ-N	8.00	5719.86	5719.73	1.6300	15.000	0.0150	3.390	7.14	0.47	5.74	0.61
3	Pipe-6b	Pipe	Inlet-S6b	WQ-S	163.00	5717.80	5716.50	0.8000	24.000	0.0150	7.150	17.51	0.41	5.34	0.89
4	Pipe-N1	Pipe	Inlet-N1	Inlet-N2	73.00	5721.75	5721.03	0.9900	12.000	0.0150	1.710	3.07	0.56	4.02	0.53
5	Pipe-N2	Pipe	Inlet-N2	SDMH-N3	41.00	5720.53	5720.06	1.1500	15.000	0.0150	2.790	5.99	0.47	4.80	0.60
6	Pipe-N3	Pipe	Inlet-N3	SDMH-N3	21.00	5720.90	5720.06	4.0000	12.000	0.0150	2.800	6.18	0.45	7.67	0.47
7	Pipe-Out-N	Pipe	WQ-N	Outfall-N	20.00	5719.73	5717.50	11.1500	15.000	0.0150	3.390	18.69	0.18	11.54	0.36
8	Pipe-Out-S	Pipe	WQ-S	Outfall-S	24.00	5716.00	5715.00	4.1700	24.000	0.0150	8.640	19.61	0.44	6.04	0.93
9	Pipe-S1	Pipe	Inlet-S1	SDMH-S2	50.00	5730.70	5729.70	2.0000	10.000	0.0150	0.860	2.69	0.32	4.39	0.33
10	Pipe-S2	Pipe	SDMH-S2	Inlet-S4	290.00	5729.50	5723.75	1.9800	10.000	0.0150	0.860	2.67	0.32	4.39	0.33
11	Pipe-S3	Pipe	Inlet-S3	Inlet-S4	106.00	5729.59	5723.95	5.3200	10.000	0.0150	0.970	4.38	0.22	9.37	0.27
12	Pipe-S4	Pipe	Inlet-S4	Inlet-S6a	67.00	5723.75	5720.58	4.7300	12.000	0.0150	2.480	6.72	0.37	7.91	0.42
13	Pipe-S5	Pipe	Inlet-S5	Inlet-S6a	41.00	5721.56	5720.58	2.3900	10.000	0.0150	1.590	3.14	0.51	5.77	0.42
14	Pipe-S6a	Pipe	Inlet-S6a	Inlet-S8	65.00	5720.08	5718.80	1.9700	15.000	0.0150	2.900	7.86	0.37	5.93	0.53
15	Pipe-S8	Pipe	Inlet-S8	Inlet-S6b	43.00	5718.60	5718.20	0.9300	19.000	0.0130	6.170	10.17	0.61	5.96	0.65
16	Road-S3	Channel	Inlet-S3	Inlet-S4	109.00	5732.10	5726.40	5.2300	7.200	0.0320	0.440	114.31	0.00	4.38	0.10
17	Road-S4	Channel	Inlet-S4	Inlet-S6a	69.00	5726.40	5723.50	4.2000	7.200	0.0320	0.060	100.69	0.00	2.94	0.05
18	Road-S6a	Channel	Inlet-S6a	Inlet-S6b	84.00	5723.50	5721.50	2.3800	7.200	0.0320	0.000	79.17	0.00	0.00	0.00

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)
0.48	0.00
0.49	0.00
0.44	0.00
0.53	0.00
0.48	0.00
0.47	0.00
0.29	0.00
0.46	0.00
0.39	0.00
0.39	0.00
0.32	0.00
0.42	0.00
0.50	0.00
0.42	0.00
0.55	0.00
0.16	0.00
0.08	0.00
0.01	0.00

Inlet Summary

SN	Element ID	Inlet Manufacturer	Manufacturer Part Number	Inlet Location	Number of Inlets	Catchbasin Invert Elevation	Max (Rim) Elevation	Initial Water Elevation	Ponded Area	Peak Flow	Peak Flow Intercepted by Inlet	Peak Flow Bypassing Inlet	Inlet Efficiency during Peak Flow	Allowable Spread
						(ft)	(ft)	(ft)	(ft²)	(cfs)	(cfs)	(cfs)	(%)	(ft)
1	Inlet-N1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.75	5724.50	5721.75	400.00	1.760	N/A	N/A	N/A	7.00
2	Inlet-N2	FHWA HEC-22 GENERIC	N/A	On Sag	2	5720.53	5725.35	5722.30	400.00	1.540	N/A	N/A	N/A	7.00
3	Inlet-N3	FHWA HEC-22 GENERIC	N/A	On Sag	1	5720.90	5724.90	5722.00	400.00	3.050	N/A	N/A	N/A	7.00
4	Inlet-S1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5730.70	5733.20	5730.70	400.00	0.980	N/A	N/A	N/A	7.00
5	Inlet-S3	FHWA HEC-22 GENERIC	N/A	On Grade	1	5729.59	5732.09	5729.59	N/A	1.440	0.98	0.46	68.05	7.00
6	Inlet-S4	FHWA HEC-22 GENERIC	N/A	On Grade	1	5723.75	5726.41	5723.42	N/A	0.950	0.89	0.07	93.06	7.00
7	Inlet-S5	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.70	5724.70	5721.70	100.00	1.600	N/A	N/A	N/A	7.00
8	Inlet-S6a	FHWA HEC-22 GENERIC	N/A	On Grade	1	5720.08	5723.62	5720.08	N/A	0.060	0.06	0.00	99.66	7.00
9	Inlet-S6b	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.57	5721.52	5717.50	400.00	1.440	N/A	N/A	N/A	7.00
10	Inlet-S7	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.50	5721.00	5717.50	400.00	1.950	N/A	N/A	N/A	7.00
11	Inlet-S8	FHWA HEC-22 GENERIC	N/A	On Sag	1	5718.30	5721.38	5718.30	400.00	3.900	N/A	N/A	N/A	7.00

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Max Gutter Spread during Peak Flow (ft)	Max Gutter Water Elev. during Peak Flow (ft)
4.24	5724.92
7.34	5725.58
15.14	5725.43
6.37	5733.53
7.38	5732.32
3.68	5726.57
4.03	5725.10
0.29	5723.64
8.54	5721.76
10.89	5721.28
5.21	5721.81

Subbasin Hydrology

Subbasin : Pr-N1

Input Data

Area (ft²) 50460.00
Weighted Runoff Coefficient 0.6900

Runoff Coefficient

Soil/Surface Description	Area (ft ²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	32950.40	B (2-6%)	0.96
Open Space, 25 years or greater	17509.64	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	50460.04		0.69

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
n = Manning's roughness
L_f = Flow Length (ft)
P = 2 yr, 24 hr Rainfall (inches)
S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
V = 20.3282 * (S_f^{0.5}) (paved surface)
V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
V = 5.0 * (S_f^{0.5}) (woodland surface)
V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
R = A_q / W_p
T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
L_f = Flow Length (ft)
R = Hydraulic Radius (ft)
A_q = Flow Area (ft²)
W_p = Wetted Perimeter (ft)
V = Velocity (ft/sec)
S_f = Slope (ft/ft)
n = Manning's roughness

Storm Event

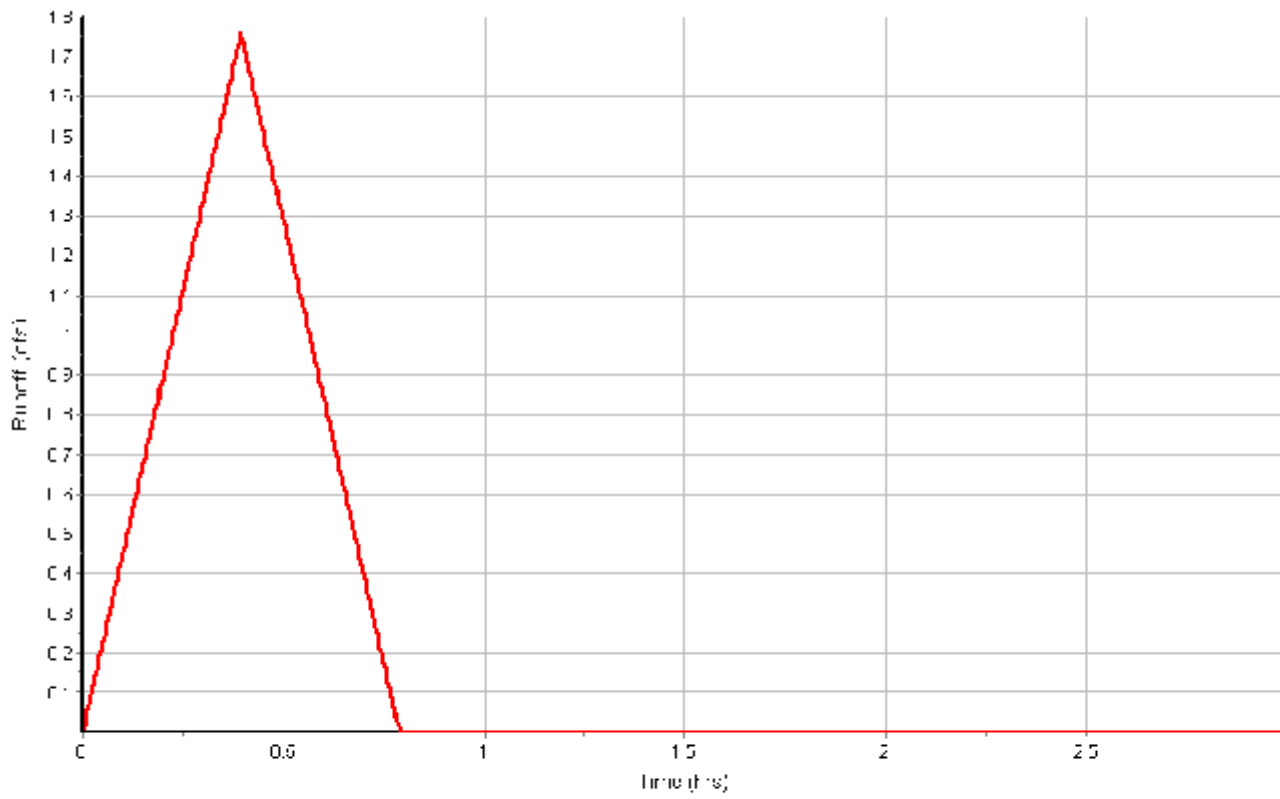
	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	80	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	21.36	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	70	280	0.00
Slope (%) :	3	2.5	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.52	2.37	0.00
Computed Flow Time (min) :	0.33	1.97	0.00
Total TOC (min)	23.66		

Subbasin Runoff Results

Total Rainfall (in)	0.87
Total Runoff (in)	0.60
Peak Runoff (cfs)	1.76
Rainfall Intensity	2.201
Weighted Runoff Coefficient	0.6900
Time of Concentration (days hh:mm:ss)	0 00:23:40

Subbasin : Pr-N1

Runoff Hydrograph



Input Data

Area (ft²) 30450.00
Weighted Runoff Coefficient 0.8200

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	24755.84	B (2-6%)	0.96
Open Space, 25 years or greater	5694.16	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	30450.00		0.82

Time of Concentration

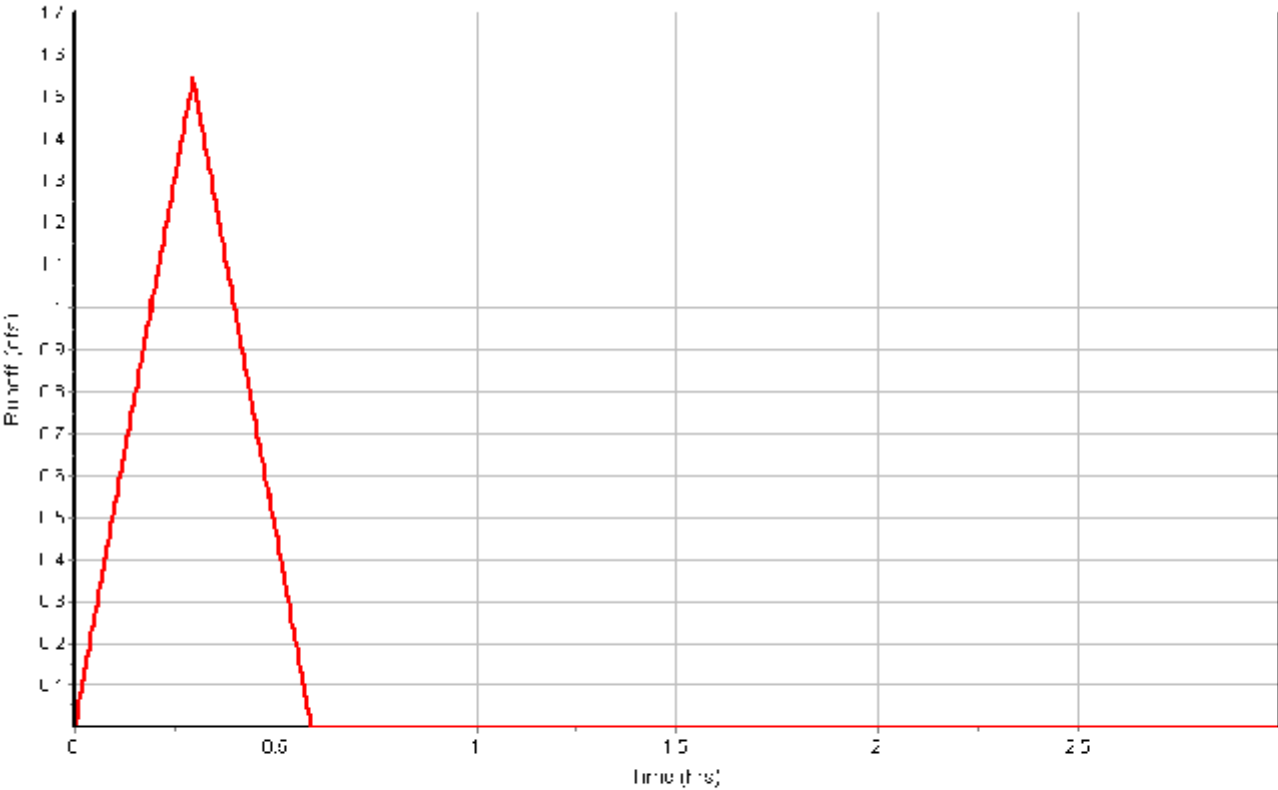
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	14.66	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	170	350	0.00
Slope (%) :	2.1	2	0.00
Surface Type :	Paved	Paved	Paved
Velocity (ft/sec) :	2.95	2.87	0.00
Computed Flow Time (min) :	0.96	2.03	0.00
Total TOC (min)17.66			

Subbasin Runoff Results

Total Rainfall (in) 0.79
Total Runoff (in) 0.65
Peak Runoff (cfs) 1.54
Rainfall Intensity 2.691
Weighted Runoff Coefficient 0.8200
Time of Concentration (days hh:mm:ss) 0 00:17:40

Subbasin : Pr-N2

Runoff Hydrograph



Input Data

Area (ft²) 29890.00
Weighted Runoff Coefficient 0.8800

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	26841.24	B (2-6%)	0.96
Open Space, 25 years or greater	3048.76	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	29890.00		0.88

Time of Concentration

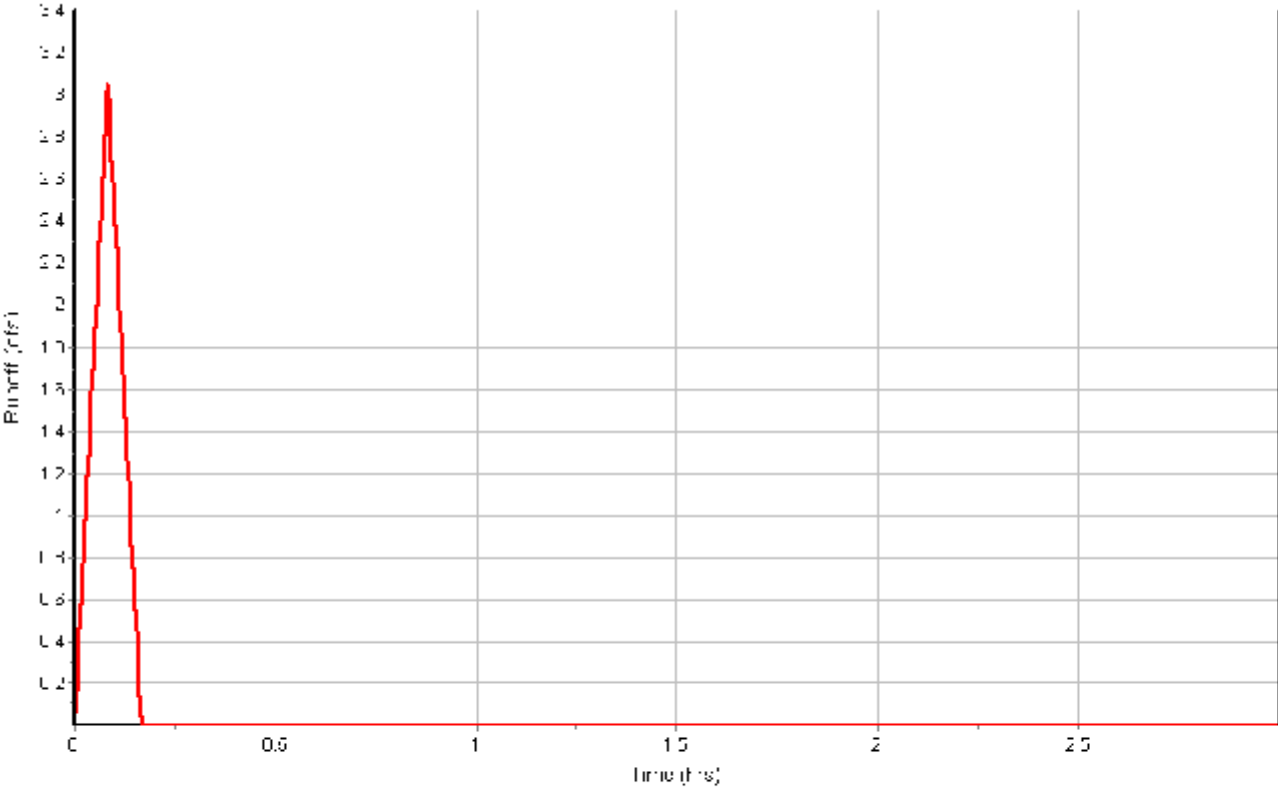
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.37
Peak Runoff (cfs) 3.05
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.8800
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-N3

Runoff Hydrograph



Input Data

Area (ft²) 24660.00
 Weighted Runoff Coefficient 0.5800

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12403.97	B (2-6%)	0.96
Open Space, 25 years or greater	12256.04	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	24660.01		0.58

Time of Concentration

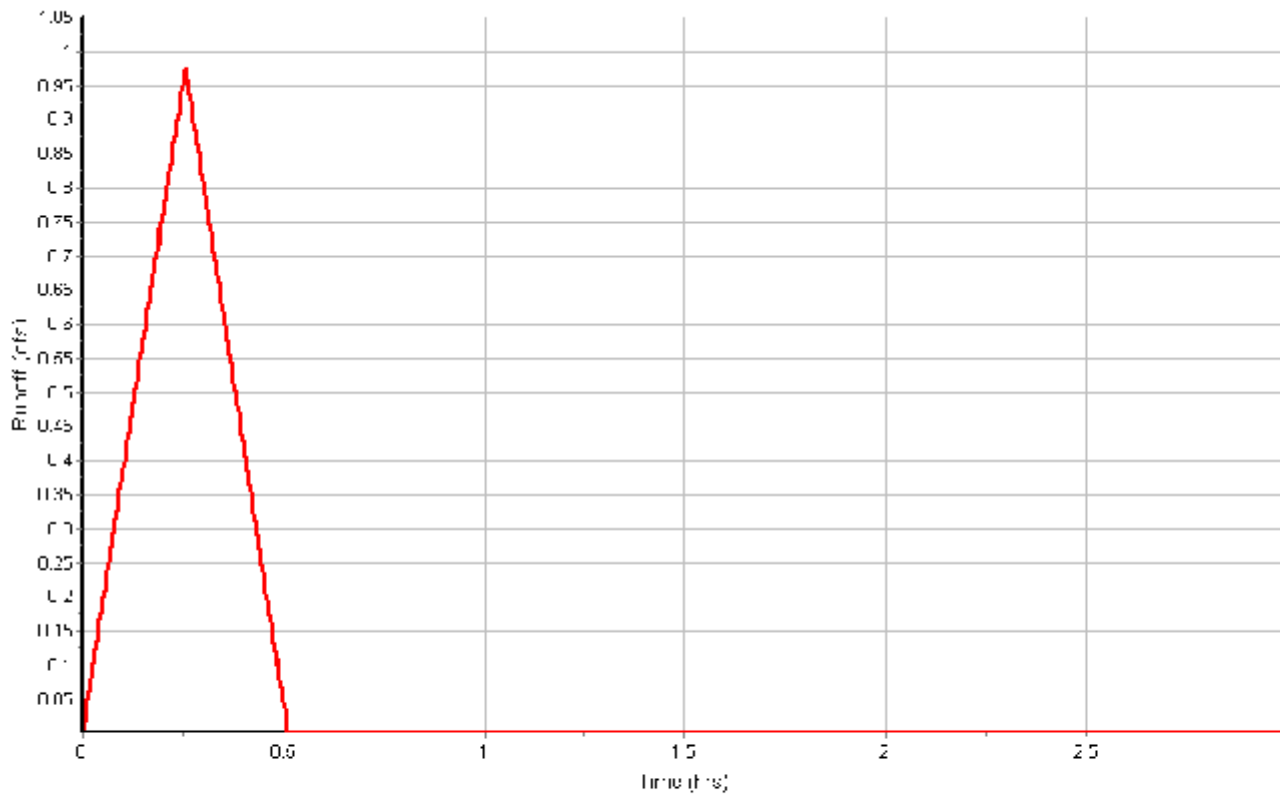
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	14.66	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	50	40	0.00
Slope (%) :	2	1	0.00
Surface Type :	Paved	Paved	Paved
Velocity (ft/sec) :	2.87	2.03	0.00
Computed Flow Time (min) :	0.29	0.33	0.00
Total TOC (min)15.28			

Subbasin Runoff Results

Total Rainfall (in) 0.76
 Total Runoff (in) 0.44
 Peak Runoff (cfs) 0.98
 Rainfall Intensity 2.972
 Weighted Runoff Coefficient 0.5800
 Time of Concentration (days hh:mm:ss) 0 00:15:17

Subbasin : Pr-S1

Runoff Hydrograph



Input Data

Area (ft²) 5800.00
Weighted Runoff Coefficient 0.8600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	5080.79	B (2-6%)	0.96
Open Space, 25 years or greater	719.22	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	5800.01		0.86

Time of Concentration

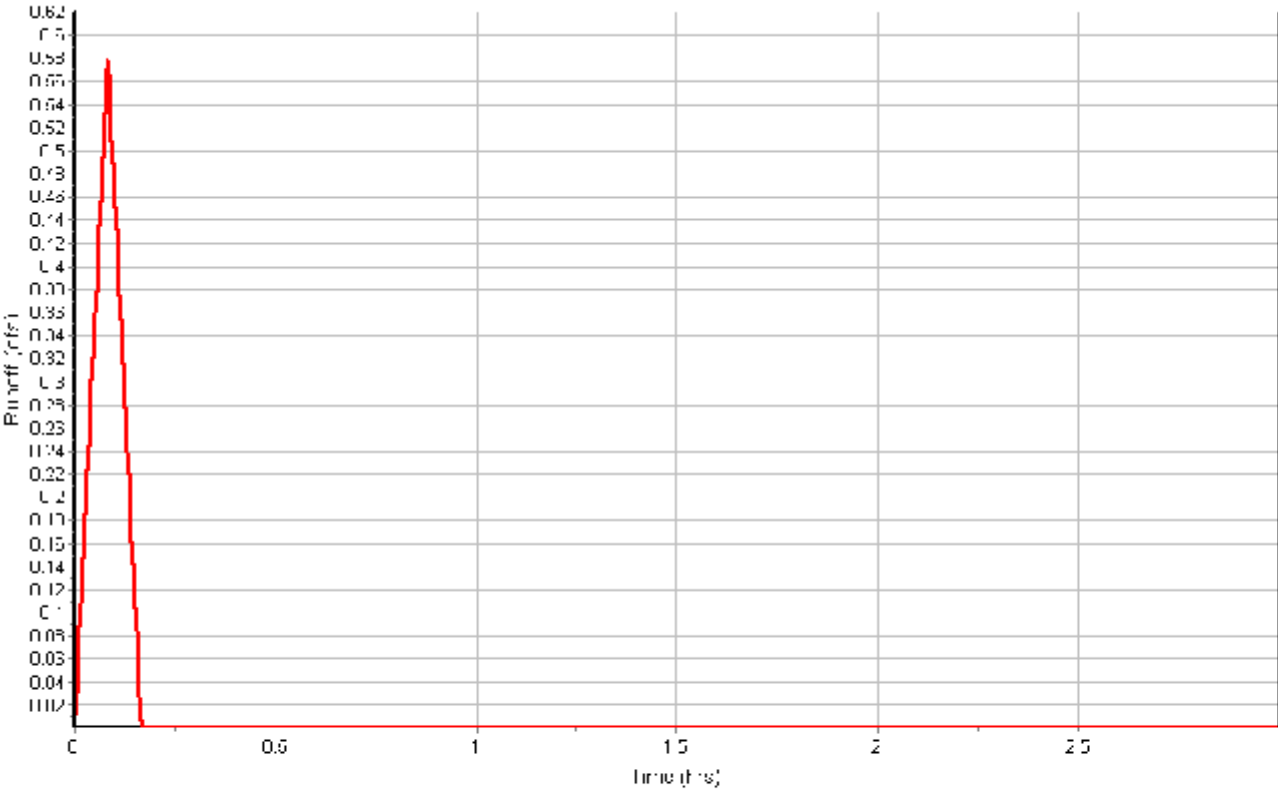
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.36
Peak Runoff (cfs) 0.58
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.8600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S2

Runoff Hydrograph



Input Data

Area (ft²) 16390.00
Weighted Runoff Coefficient 0.7600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12145.01	B (2-6%)	0.96
Open Space, 25 years or greater	4245.01	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	16390.02		0.76

Time of Concentration

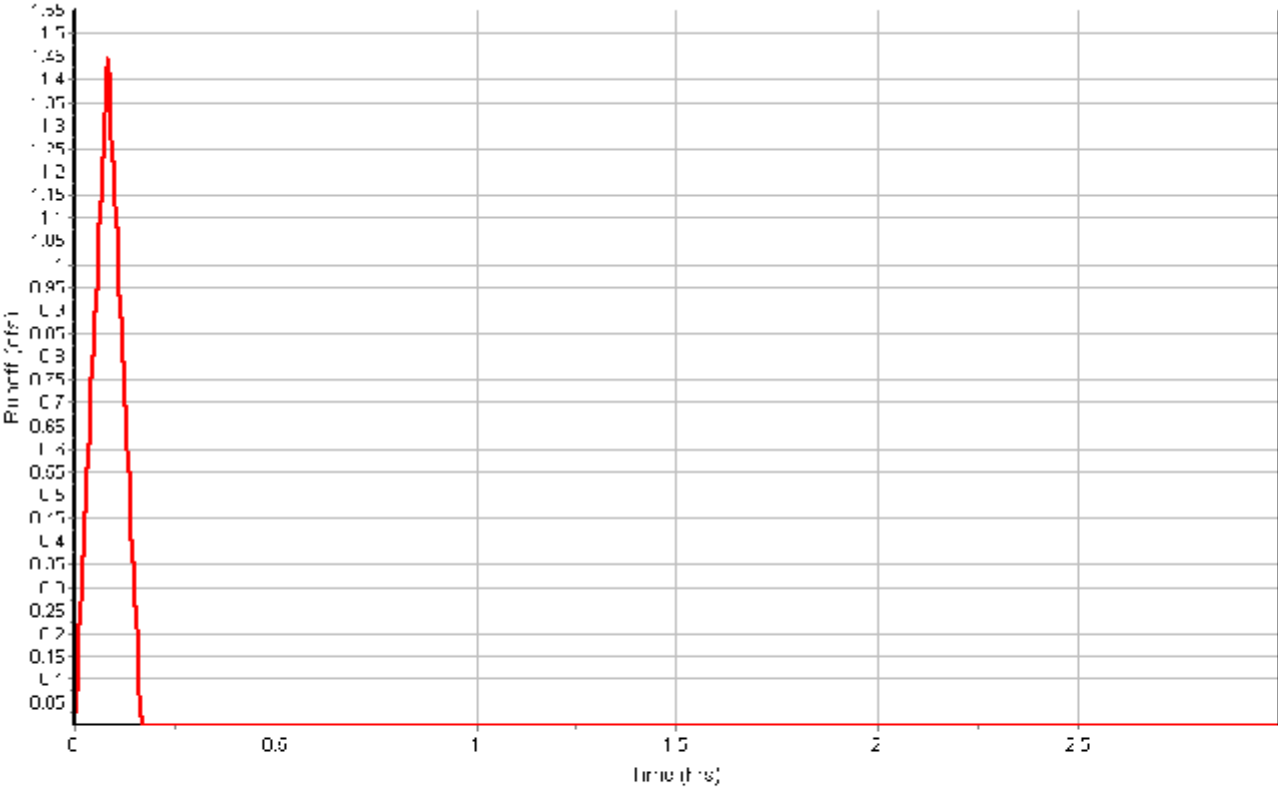
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.32
Peak Runoff (cfs) 1.44
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.7600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S3

Runoff Hydrograph



Input Data

Area (ft²) 5969.99
Weighted Runoff Coefficient 0.7900

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	4620.80	B (2-6%)	0.96
Open Space, 25 years or greater	1349.23	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	5970.03		0.79

Time of Concentration

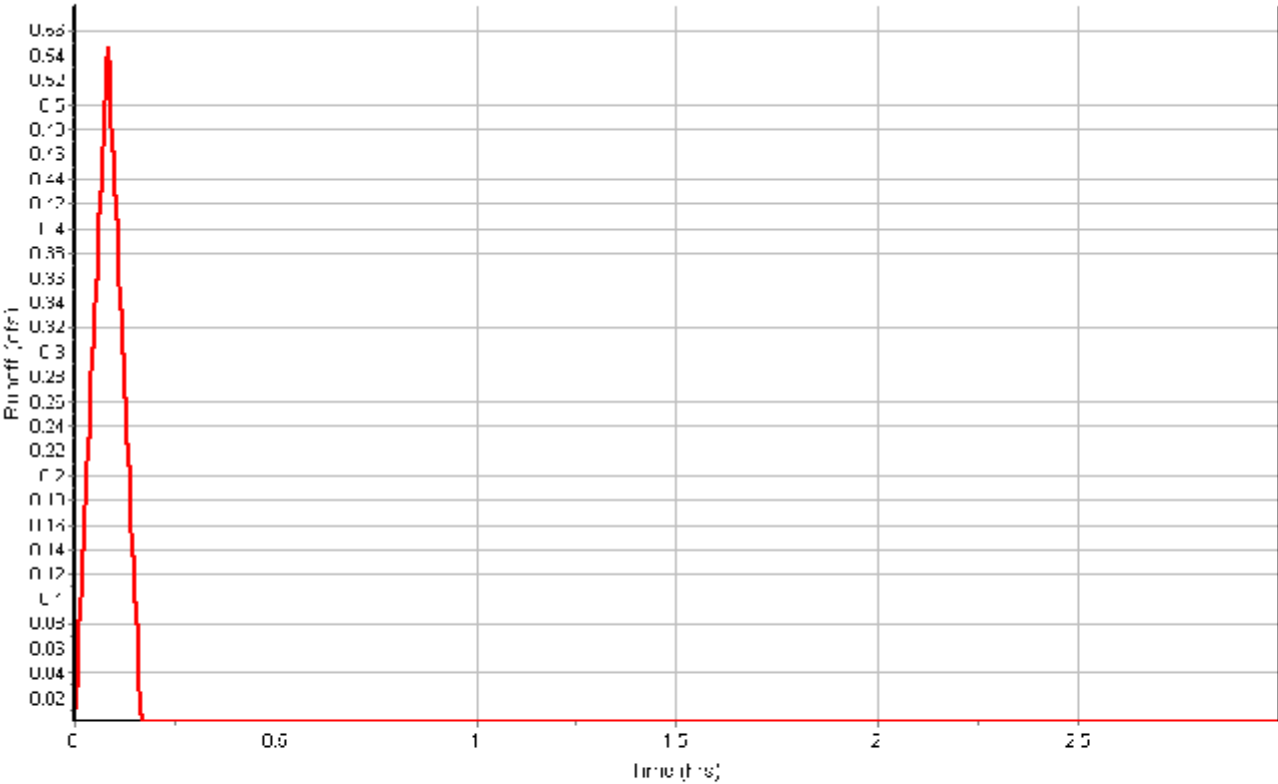
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.33
Peak Runoff (cfs) 0.55
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.7900
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S4

Runoff Hydrograph



Input Data

Area (ft²) 41450.00
Weighted Runoff Coefficient 0.7000

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	27274.09	B (2-6%)	0.96
Open Space, 25 years or greater	14175.91	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	41450.00		0.70

Time of Concentration

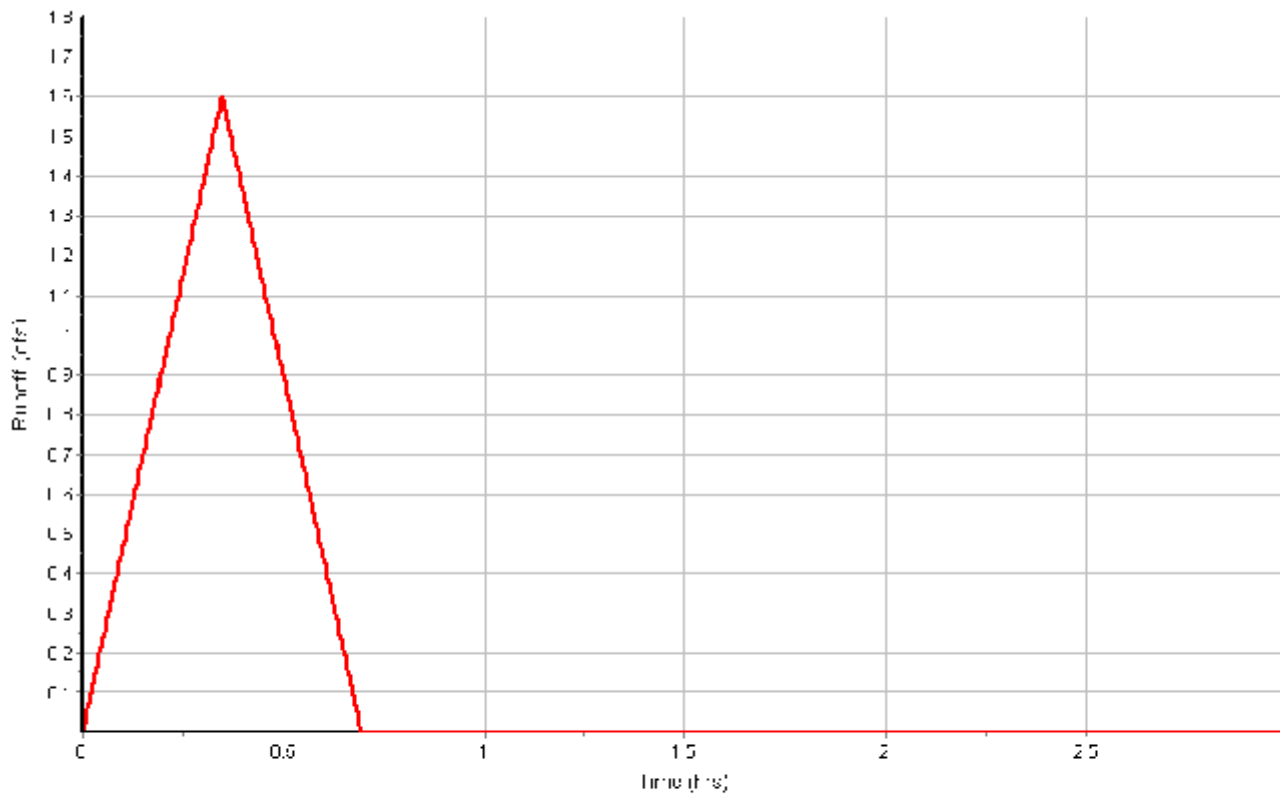
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	65	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	18.09	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	80	390	0.00
Slope (%) :	3.5	3.3	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.80	2.72	0.00
Computed Flow Time (min) :	0.35	2.39	0.00
Total TOC (min)	20.83		

Subbasin Runoff Results

Total Rainfall (in) 0.83
Total Runoff (in) 0.58
Peak Runoff (cfs) 1.60
Rainfall Intensity 2.402
Weighted Runoff Coefficient 0.7000
Time of Concentration (days hh:mm:ss) 0 00:20:50

Subbasin : Pr-S5

Runoff Hydrograph



Input Data

Area (ft²) 14609.98
Weighted Runoff Coefficient 0.8500

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12564.62	B (2-6%)	0.96
Open Space, 25 years or greater	2045.40	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	14610.02		0.85

Time of Concentration

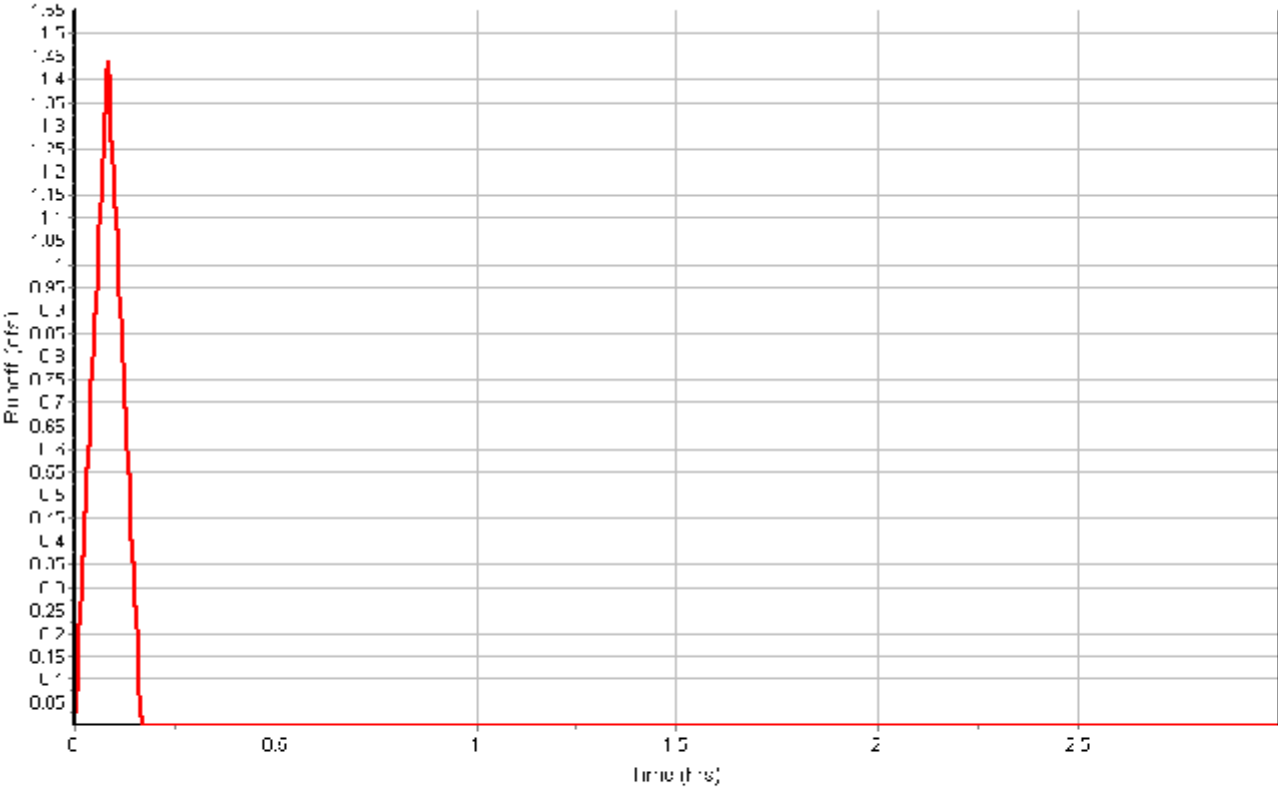
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.36
Peak Runoff (cfs) 1.44
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.8500
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S6

Runoff Hydrograph



Input Data

Area (ft²) 19569.98
Weighted Runoff Coefficient 0.8600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	17006.35	B (2-6%)	0.96
Open Space, 25 years or greater	2563.68	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	19570.03		0.86

Time of Concentration

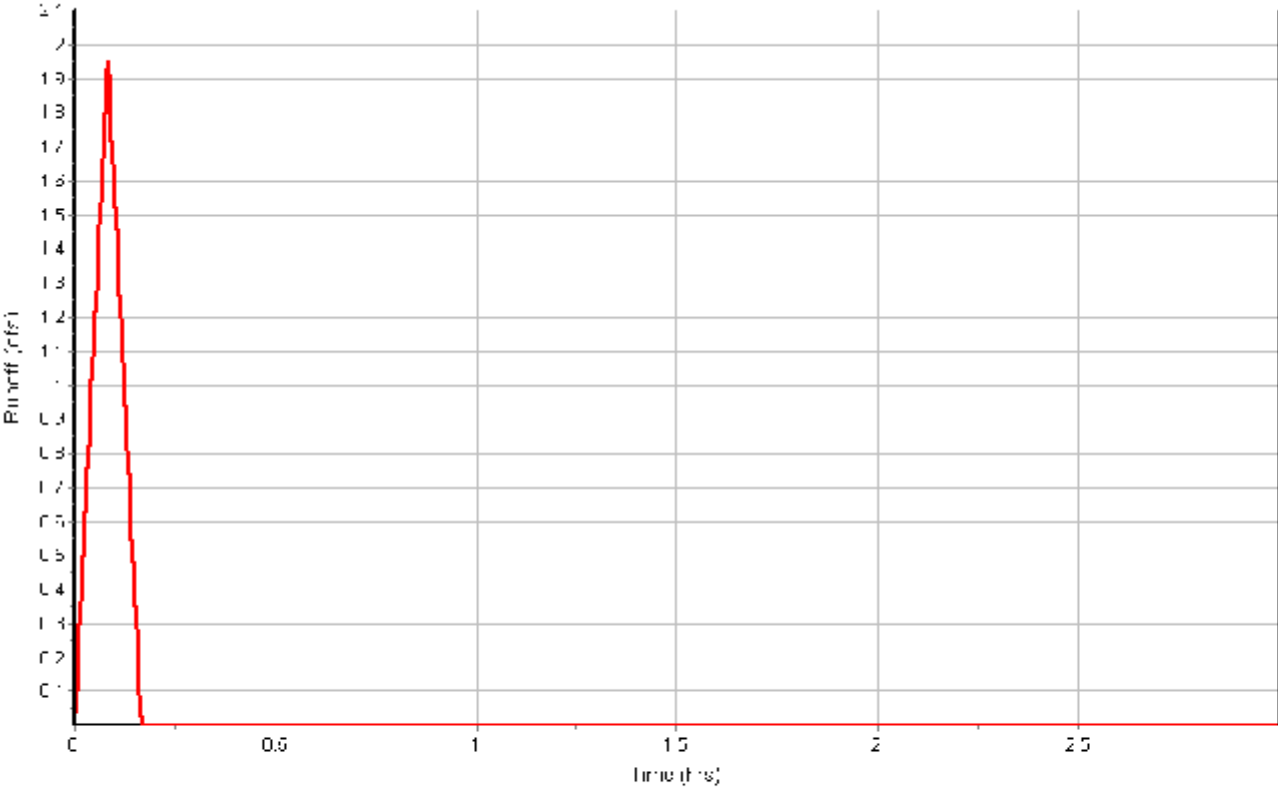
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.42
Total Runoff (in) 0.36
Peak Runoff (cfs) 1.95
Rainfall Intensity 5.050
Weighted Runoff Coefficient 0.8600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S7

Runoff Hydrograph



Input Data

Area (ft²) 52210.01
Weighted Runoff Coefficient 0.7600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	38844.24	B (2-6%)	0.96
Open Space, 25 years or greater	13365.78	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	52210.02		0.76

Time of Concentration

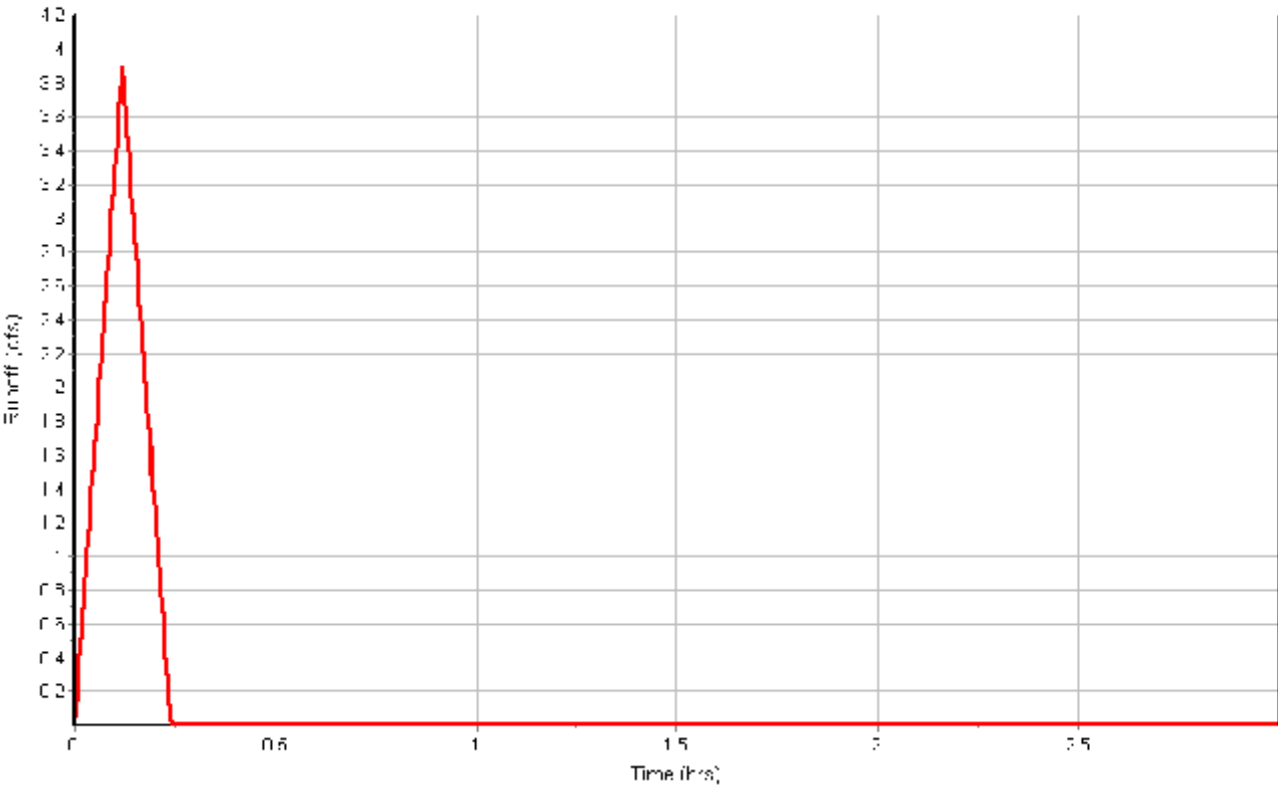
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.40	0.00	0.00
Flow Length (ft) :	20	0.00	0.00
Slope (%) :	10	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	5.43	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	210	150	0.00
Slope (%) :	3.5	3.5	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.80	2.81	0.00
Computed Flow Time (min) :	0.92	0.89	0.00
Total TOC (min)7.24			

Subbasin Runoff Results

Total Rainfall (in) 0.51
Total Runoff (in) 0.39
Peak Runoff (cfs) 3.90
Rainfall Intensity 4.277
Weighted Runoff Coefficient 0.7600
Time of Concentration (days hh:mm:ss) 0 00:07:14

Subbasin : Pr-S8

Runoff Hydrograph



Junction Input

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)
1	SDMH-N3	5719.86	5725.45	5.59	5719.86	0.00
2	SDMH-S2	5729.50	5731.40	1.90	5729.50	0.00
3	WQ-N	5719.73	5725.57	5.84	5719.73	0.00
4	WQ-S	5716.00	5721.50	5.50	5716.00	0.00

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Junction Results

SN	Element ID	Peak Inflow	Max HGL Elevation Attained	Max HGL Depth Attained	Min Freeboard Attained	Average HGL Elevation Attained	Average HGL Depth Attained	Time of Max HGL Occurrence	Total Flooded Volume	Total Time Flooded
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(days hh:mm)	(ft³)	(min)
1	SDMH-N3	3.390	5720.66	0.80	4.79	5720.17	0.31	0 00:19	0.00	0.00
2	SDMH-S2	0.860	5730.03	0.53	1.37	5729.74	0.24	0 00:17	0.00	0.00
3	WQ-N	3.390	5720.34	0.61	5.23	5719.84	0.11	0 00:05	0.00	0.00
4	WQ-S	8.640	5718.48	2.48	3.02	5718.02	2.02	0 00:05	0.00	0.00

Channel Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Shape	Height (ft)	Width (ft)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)
1	Road-S3	109.00	5732.10	2.51	5726.40	2.65	5.70	5.2300	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000
2	Road-S4	69.00	5726.40	2.65	5723.50	3.42	2.90	4.2000	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000
3	Road-S6a	84.00	5723.50	3.42	5721.50	3.93	2.00	2.3800	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Channel Results

SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Reported Surcharged Condition
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)
1 Road-S3	0.440	0 00:05	114.310	0.00	4.38	0.41	0.10	0.16	0.00
2 Road-S4	0.060	0 00:05	100.690	0.00	2.94	0.39	0.05	0.08	0.00
3 Road-S6a	0.000	0 00:07	79.170	0.00	0.00		0.00	0.01	0.00

Pipe Input

SN Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Pipe Shape	Pipe Diameter or Height (in)	Pipe Width (in)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Initial Flow (cfs)	Flap Gate	No. of Barrels
1 Link-47	32.09	5718.50	1.00	5718.00	2.00	0.50	1.5600	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
2 Link-57	8.00	5719.86	0.00	5719.73	0.00	0.13	1.6300	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
3 Pipe-6b	163.00	5717.80	0.23	5716.50	0.50	1.30	0.8000	CIRCULAR	24.000	24.000	0.0150	0.5000	0.5000	0.000	No	1
4 Pipe-N1	73.00	5721.75	0.00	5721.03	0.50	0.72	0.9900	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
5 Pipe-N2	41.00	5720.53	0.00	5720.06	0.20	0.47	1.1500	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
6 Pipe-N3	21.00	5720.90	0.00	5720.06	0.20	0.84	4.0000	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
7 Pipe-Out-N	20.00	5719.73	0.00	5717.50	0.00	2.23	11.1500	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
8 Pipe-Out-S	24.00	5716.00	0.00	5715.00	-0.76	1.00	4.1700	CIRCULAR	24.000	24.000	0.0150	0.5000	0.5000	0.000	No	1
9 Pipe-S1	50.00	5730.70	0.00	5729.70	0.20	1.00	2.0000	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
10 Pipe-S2	290.00	5729.50	0.00	5723.75	0.00	5.75	1.9800	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
11 Pipe-S3	106.00	5729.59	0.00	5723.95	0.20	5.64	5.3200	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
12 Pipe-S4	67.00	5723.75	0.00	5720.58	0.50	3.17	4.7300	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
13 Pipe-S5	41.00	5721.56	-0.14	5720.58	0.50	0.98	2.3900	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
14 Pipe-S6a	65.00	5720.08	0.00	5718.80	0.50	1.28	1.9700	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
15 Pipe-S8	43.00	5718.60	0.30	5718.20	0.63	0.40	0.9300	Horizontal Ellipse	14.040	23.040	0.0130	0.5000	0.5000	0.000	No	1

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Pipe Results

SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)
1 Link-47	1.770	0 00:05	3.85	0.46	4.80	0.11	0.48	0.48	0.00
2 Link-57	3.390	0 00:05	7.14	0.47	5.74	0.02	0.61	0.49	0.00
3 Pipe-6b	7.150	0 00:07	17.51	0.41	5.34	0.51	0.89	0.44	0.00
4 Pipe-N1	1.710	0 00:24	3.07	0.56	4.02	0.30	0.53	0.53	0.00
5 Pipe-N2	2.790	0 00:19	5.99	0.47	4.80	0.14	0.60	0.48	0.00
6 Pipe-N3	2.800	0 00:05	6.18	0.45	7.67	0.05	0.47	0.47	0.00
7 Pipe-Out-N	3.390	0 00:05	18.69	0.18	11.54	0.03	0.36	0.29	0.00
8 Pipe-Out-S	8.640	0 00:06	19.61	0.44	6.04	0.07	0.93	0.46	0.00
9 Pipe-S1	0.860	0 00:17	2.69	0.32	4.39	0.19	0.33	0.39	0.00
10 Pipe-S2	0.860	0 00:18	2.67	0.32	4.39	1.10	0.33	0.39	0.00
11 Pipe-S3	0.970	0 00:05	4.38	0.22	9.37	0.19	0.27	0.32	0.00
12 Pipe-S4	2.480	0 00:05	6.72	0.37	7.91	0.14	0.42	0.42	0.00
13 Pipe-S5	1.590	0 00:21	3.14	0.51	5.77	0.12	0.42	0.50	0.00
14 Pipe-S6a	2.900	0 00:05	7.86	0.37	5.93	0.18	0.53	0.42	0.00
15 Pipe-S8	6.170	0 00:07	10.17	0.61	5.96	0.12	0.65	0.55	0.00

Inlet Input

SN	Element ID	Inlet Manufacturer	Manufacturer Part Number	Inlet Location	Number of Inlets	Catchbasin Invert Elevation (ft)	Max (Rim) Elevation (ft)	Inlet Depth (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Ponded Area (ft ²)	Grate Clogging Factor (%)
1	Inlet-N1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.75	5724.50	2.75	5721.75	0.00	400.00	50.00
2	Inlet-N2	FHWA HEC-22 GENERIC	N/A	On Sag	2	5720.53	5725.35	4.82	5722.30	1.77	400.00	50.00
3	Inlet-N3	FHWA HEC-22 GENERIC	N/A	On Sag	1	5720.90	5724.90	4.00	5722.00	1.10	400.00	50.00
4	Inlet-S1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5730.70	5733.20	2.50	5730.70	0.00	400.00	50.00
5	Inlet-S3	FHWA HEC-22 GENERIC	N/A	On Grade	1	5729.59	5732.09	2.50	5729.59	0.00	N/A	50.00
6	Inlet-S4	FHWA HEC-22 GENERIC	N/A	On Grade	1	5723.75	5726.41	2.66	5723.42	-0.33	N/A	50.00
7	Inlet-S5	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.70	5724.70	3.00	5721.70	0.00	100.00	50.00
8	Inlet-S6a	FHWA HEC-22 GENERIC	N/A	On Grade	1	5720.08	5723.62	3.54	5720.08	0.00	N/A	50.00
9	Inlet-S6b	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.57	5721.52	3.95	5717.50	-0.07	400.00	50.00
10	Inlet-S7	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.50	5721.00	3.50	5717.50	0.00	400.00	50.00
11	Inlet-S8	FHWA HEC-22 GENERIC	N/A	On Sag	1	5718.30	5721.38	3.08	5718.30	0.00	400.00	50.00

Roadway & Gutter Input

SN Element ID	Roadway Longitudinal Slope (ft/ft)	Roadway Cross Slope (ft/ft)	Roadway Manning's Roughness	Gutter Cross Slope (ft/ft)	Gutter Width (ft)	Gutter Depression (in)	Allowable Spread (ft)
1 Inlet-N1	N/A	0.1000	0.0220	0.1000	2.00	0.0000	7.00
2 Inlet-N2	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
3 Inlet-N3	N/A	0.0200	0.0160	0.0620	2.00	0.0656	7.00
4 Inlet-S1	N/A	0.0200	0.0160	0.0620	2.00	0.0656	7.00
5 Inlet-S3	0.0100	0.0200	0.0160	0.0620	2.00	0.0656	7.00
6 Inlet-S4	0.0500	0.0200	0.0160	0.0620	2.00	0.0656	7.00
7 Inlet-S5	N/A	0.1000	0.0220	0.1000	2.00	0.0000	7.00
8 Inlet-S6a	0.0400	0.0200	0.0160	0.0620	2.00	0.0656	7.00
9 Inlet-S6b	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
10 Inlet-S7	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
11 Inlet-S8	N/A	0.1000	0.0160	0.0620	2.00	0.0000	7.00

XXXXXX 25-year 1-hour XXXXXX
Storm Event

BUI#17015

Inlet Results

SN Element ID	Peak Flow	Peak Lateral Inflow	Peak Flow Intercepted by Inlet	Peak Flow Bypassing Inlet	Inlet Efficiency during Peak Flow (%)	Max Gutter Spread during Peak Flow (ft)	Max Gutter Water Elev. during Peak Flow (ft)	Max Gutter Water Depth during Peak Flow (ft)	Time of Max Depth Occurrence	Total Flooded Volume	Total Time Flooded
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days hh:mm)	(ft³)	(hr)
1 Inlet-N1	1.760	1.76	N/A	N/A	N/A	4.24	5724.92	0.42	0 00:24	0.00	0.00
2 Inlet-N2	1.540	1.54	N/A	N/A	N/A	7.34	5725.58	0.23	0 00:00	0.00	0.00
3 Inlet-N3	3.050	3.05	N/A	N/A	N/A	15.14	5725.43	0.53	0 00:00	0.00	0.00
4 Inlet-S1	0.980	0.98	N/A	N/A	N/A	6.37	5733.53	0.33	0 00:17	0.00	0.00
5 Inlet-S3	1.440	1.44	0.98	0.46	68.05	7.38	5732.32	0.23	0 00:05	0.00	0.00
6 Inlet-S4	0.950	0.55	0.89	0.07	93.06	3.68	5726.57	0.16	0 00:05	0.00	0.00
7 Inlet-S5	1.600	1.60	N/A	N/A	N/A	4.03	5725.10	0.40	0 00:21	0.00	0.00
8 Inlet-S6a	0.060	0.00	0.06	0.00	99.66	0.29	5723.64	0.02	0 00:05	0.00	0.00
9 Inlet-S6b	1.440	1.44	N/A	N/A	N/A	8.54	5721.76	0.24	0 00:07	0.00	0.00
10 Inlet-S7	1.950	1.95	N/A	N/A	N/A	10.89	5721.28	0.28	0 00:05	0.00	0.00
11 Inlet-S8	3.900	3.90	N/A	N/A	N/A	5.21	5721.81	0.43	0 00:05	0.00	0.00

Project Description

Storm Event

File Name 17015-SSA.SPF

Project Options

Flow Units CFS
 Elevation Type Elevation
 Hydrology Method Rational
 Time of Concentration (TOC) Method SCS TR-55
 Link Routing Method Kinematic Wave
 Enable Overflow Ponding at Nodes YES
 Skip Steady State Analysis Time Periods YES

Rainfall Details

Return Period..... 100 year(s)

Subbasin Summary

SN	Subbasin ID	Area (ac)	Weighted Runoff Coefficient	Total Rainfall (in)	Total Runoff (in)	Total Runoff Volume (ft ³)	Peak Runoff (cfs)	Time of Concentration (days hh:mm:ss)
1	Pr-N1	1	0.690	1.12	0.77	3237.9	2.26	0 00:23:39
2	Pr-N2	1	0.820	1.03	0.84	2134.0	2.00	0 00:17:39
3	Pr-N3	1	0.880	0.55	0.48	1198.1	3.96	0 00:05:00
4	Pr-S1	1	0.580	0.99	0.57	1175.5	1.27	0 00:15:16
5	Pr-S2	0	0.860	0.55	0.47	227.2	0.75	0 00:05:00
6	Pr-S3	0	0.760	0.55	0.42	566.8	1.88	0 00:05:00
7	Pr-S4	0	0.790	0.55	0.43	214.9	0.71	0 00:05:00
8	Pr-S5	1	0.700	1.08	0.75	2601.0	2.06	0 00:20:49
9	Pr-S6	0	0.850	0.55	0.47	566.1	1.87	0 00:05:00
10	Pr-S7	0	0.860	0.55	0.47	766.5	2.54	0 00:05:00
11	Pr-S8	1	0.760	0.66	0.51	2197.2	5.06	0 00:07:14

Storm Event

Node Summary

SN	Element ID	Element Type	Invert Elevation	Ground/Rim (Max) Elevation	Initial Water Elevation	Surcharge Elevation	Ponded Area	Peak Inflow	Max HGL Elevation Attained	Max Surcharge Depth Attained	Min Freeboard Attained	Time of Peak Flooding Occurrence	Total Flooded Volume	Total Time Flooded
			(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	(ft)	(days hh:mm)	(ft³)	(min)
1	SDMH-N3	Junction	5719.86	5725.45	5719.86	5721.00	0.00	4.440	5720.76	0.00	4.69	0 00:00	0.00	0.00
2	SDMH-S2	Junction	5729.50	5731.40	5729.50	0.00	0.00	1.210	5730.09	0.00	1.31	0 00:00	0.00	0.00
3	WQ-N	Junction	5719.73	5725.57	5719.73	5721.00	0.00	4.440	5720.44	0.00	5.13	0 00:00	0.00	0.00
4	WQ-S	Junction	5716.00	5721.50	5716.00	0.00	0.00	11.290	5718.56	0.00	2.94	0 00:00	0.00	0.00
5	Outfall-N	Outfall	5717.50					4.440	5717.91					
6	Outfall-S	Outfall	5715.76					11.290	5716.85					

Link Summary

SN	Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Average Slope	Diameter or Height	Manning's Roughness	Peak Flow	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1	Link-47	Pipe	Inlet-S7	WQ-S	32.09	5718.50	5718.00	1.5600	12.000	0.0150	2.340	3.85	0.61	5.14	0.56
2	Link-57	Pipe	SDMH-N3	WQ-N	8.00	5719.86	5719.73	1.6300	15.000	0.0150	4.440	7.14	0.62	6.13	0.71
3	Pipe-6b	Pipe	Inlet-S6b	WQ-S	163.00	5717.80	5716.50	0.8000	24.000	0.0150	9.320	17.51	0.53	5.71	1.04
4	Pipe-N1	Pipe	Inlet-N1	Inlet-N2	73.00	5721.75	5721.03	0.9900	12.000	0.0150	2.210	3.07	0.72	4.26	0.63
5	Pipe-N2	Pipe	Inlet-N2	SDMH-N3	41.00	5720.53	5720.06	1.1500	15.000	0.0150	3.610	5.99	0.60	5.10	0.70
6	Pipe-N3	Pipe	Inlet-N3	SDMH-N3	21.00	5720.90	5720.06	4.0000	12.000	0.0150	3.700	6.18	0.60	8.21	0.56
7	Pipe-Out-N	Pipe	WQ-N	Outfall-N	20.00	5719.73	5717.50	11.1500	15.000	0.0150	4.440	18.69	0.24	12.48	0.41
8	Pipe-Out-S	Pipe	WQ-S	Outfall-S	24.00	5716.00	5715.00	4.1700	24.000	0.0150	11.290	19.61	0.58	6.46	1.09
9	Pipe-S1	Pipe	Inlet-S1	SDMH-S2	50.00	5730.70	5729.70	2.0000	10.000	0.0150	1.210	2.69	0.45	4.80	0.39
10	Pipe-S2	Pipe	SDMH-S2	Inlet-S4	290.00	5729.50	5723.75	1.9800	10.000	0.0150	1.200	2.67	0.45	4.82	0.39
11	Pipe-S3	Pipe	Inlet-S3	Inlet-S4	106.00	5729.59	5723.95	5.3200	10.000	0.0150	1.160	4.38	0.27	10.07	0.29
12	Pipe-S4	Pipe	Inlet-S4	Inlet-S6a	67.00	5723.75	5720.58	4.7300	12.000	0.0150	3.150	6.72	0.47	8.42	0.48
13	Pipe-S5	Pipe	Inlet-S5	Inlet-S6a	41.00	5721.56	5720.58	2.3900	10.000	0.0150	2.050	3.14	0.65	6.13	0.49
14	Pipe-S6a	Pipe	Inlet-S6a	Inlet-S8	65.00	5720.08	5718.80	1.9700	15.000	0.0150	3.800	7.86	0.48	6.36	0.61
15	Pipe-S8	Pipe	Inlet-S8	Inlet-S6b	43.00	5718.60	5718.20	0.9300	19.000	0.0130	8.020	10.17	0.79	6.37	0.75
16	Road-S3	Channel	Inlet-S3	Inlet-S4	109.00	5732.10	5726.40	5.2300	7.200	0.0320	0.670	114.31	0.01	6.07	0.12
17	Road-S4	Channel	Inlet-S4	Inlet-S6a	69.00	5726.40	5723.50	4.2000	7.200	0.0320	0.180	100.69	0.00	3.94	0.07
18	Road-S6a	Channel	Inlet-S6a	Inlet-S6b	84.00	5723.50	5721.50	2.3800	7.200	0.0320	0.000	79.17	0.00	0.00	0.01

Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged (min)
0.56	0.00
0.57	0.00
0.52	0.00
0.63	0.00
0.56	0.00
0.56	0.00
0.33	0.00
0.54	0.00
0.47	0.00
0.47	0.00
0.35	0.00
0.48	0.00
0.59	0.00
0.49	0.00
0.64	0.00
0.19	0.00
0.12	0.00
0.01	0.00

Storm Event

Inlet Summary

SN	Element ID	Inlet Manufacturer	Manufacturer Part Number	Inlet Location	Number of Inlets	Catchbasin Invert Elevation	Max (Rim) Elevation	Initial Water Elevation	Ponded Area	Peak Flow	Peak Flow Intercepted by Inlet	Peak Flow Bypassing Inlet	Inlet Efficiency during Peak Flow	Allowable Spread
						(ft)	(ft)	(ft)	(ft²)	(cfs)	(cfs)	(cfs)	(%)	(ft)
1	Inlet-N1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.75	5724.50	5721.75	400.00	2.260	N/A	N/A	N/A	7.00
2	Inlet-N2	FHWA HEC-22 GENERIC	N/A	On Sag	2	5720.53	5725.35	5722.30	400.00	2.000	N/A	N/A	N/A	7.00
3	Inlet-N3	FHWA HEC-22 GENERIC	N/A	On Sag	1	5720.90	5724.90	5722.00	400.00	3.960	N/A	N/A	N/A	7.00
4	Inlet-S1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5730.70	5733.20	5730.70	400.00	1.270	N/A	N/A	N/A	7.00
5	Inlet-S3	FHWA HEC-22 GENERIC	N/A	On Grade	1	5729.59	5732.09	5729.59	N/A	1.880	1.17	0.70	62.48	7.00
6	Inlet-S4	FHWA HEC-22 GENERIC	N/A	On Grade	1	5723.75	5726.41	5723.42	N/A	1.340	1.16	0.19	86.09	7.00
7	Inlet-S5	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.70	5724.70	5721.70	100.00	2.060	N/A	N/A	N/A	7.00
8	Inlet-S6a	FHWA HEC-22 GENERIC	N/A	On Grade	1	5720.08	5723.62	5720.08	N/A	0.180	0.18	0.00	99.66	7.00
9	Inlet-S6b	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.57	5721.52	5717.50	400.00	1.870	N/A	N/A	N/A	7.00
10	Inlet-S7	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.50	5721.00	5717.50	400.00	2.530	N/A	N/A	N/A	7.00
11	Inlet-S8	FHWA HEC-22 GENERIC	N/A	On Sag	1	5718.30	5721.38	5718.30	400.00	5.060	N/A	N/A	N/A	7.00

XXXXXXX 100-year 1-hour Storm Event XXXXXXXX

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Max Gutter Spread during Peak Flow (ft)	Max Gutter Water Elev. during Peak Flow (ft)
4.84	5724.98
9.09	5725.61
18.28	5725.50
7.75	5733.60
8.33	5732.34
4.62	5726.59
4.63	5725.16
0.86	5723.67
10.52	5721.80
13.18	5721.33
5.95	5721.89

Subbasin Hydrology

Subbasin : Pr-N1

Input Data

Area (ft²) 50460.00
 Weighted Runoff Coefficient 0.6900

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	32950.40	B (2-6%)	0.96
Open Space, 25 years or greater	17509.64	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	50460.04		0.69

Time of Concentration

TOC Method : SCS TR-55

Sheet Flow Equation :

$$T_c = (0.007 * ((n * L_f)^{0.8})) / ((P^{0.5}) * (S_f^{0.4}))$$

Where :

T_c = Time of Concentration (hr)
 n = Manning's roughness
 L_f = Flow Length (ft)
 P = 2 yr, 24 hr Rainfall (inches)
 S_f = Slope (ft/ft)

Shallow Concentrated Flow Equation :

V = 16.1345 * (S_f^{0.5}) (unpaved surface)
 V = 20.3282 * (S_f^{0.5}) (paved surface)
 V = 15.0 * (S_f^{0.5}) (grassed waterway surface)
 V = 10.0 * (S_f^{0.5}) (nearly bare & untilled surface)
 V = 9.0 * (S_f^{0.5}) (cultivated straight rows surface)
 V = 7.0 * (S_f^{0.5}) (short grass pasture surface)
 V = 5.0 * (S_f^{0.5}) (woodland surface)
 V = 2.5 * (S_f^{0.5}) (forest w/heavy litter surface)
 T_c = (L_f / V) / (3600 sec/hr)

Where:

T_c = Time of Concentration (hr)
 L_f = Flow Length (ft)
 V = Velocity (ft/sec)
 S_f = Slope (ft/ft)

Channel Flow Equation :

V = (1.49 * (R^{2/3}) * (S_f^{0.5})) / n
 R = A_q / W_p
 T_c = (L_f / V) / (3600 sec/hr)

Where :

T_c = Time of Concentration (hr)
 L_f = Flow Length (ft)
 R = Hydraulic Radius (ft)
 A_q = Flow Area (ft²)
 W_p = Wetted Perimeter (ft)
 V = Velocity (ft/sec)
 S_f = Slope (ft/ft)
 n = Manning's roughness

XXXXXXX 100-year 1-hour XXXXXXXX
Storm Event

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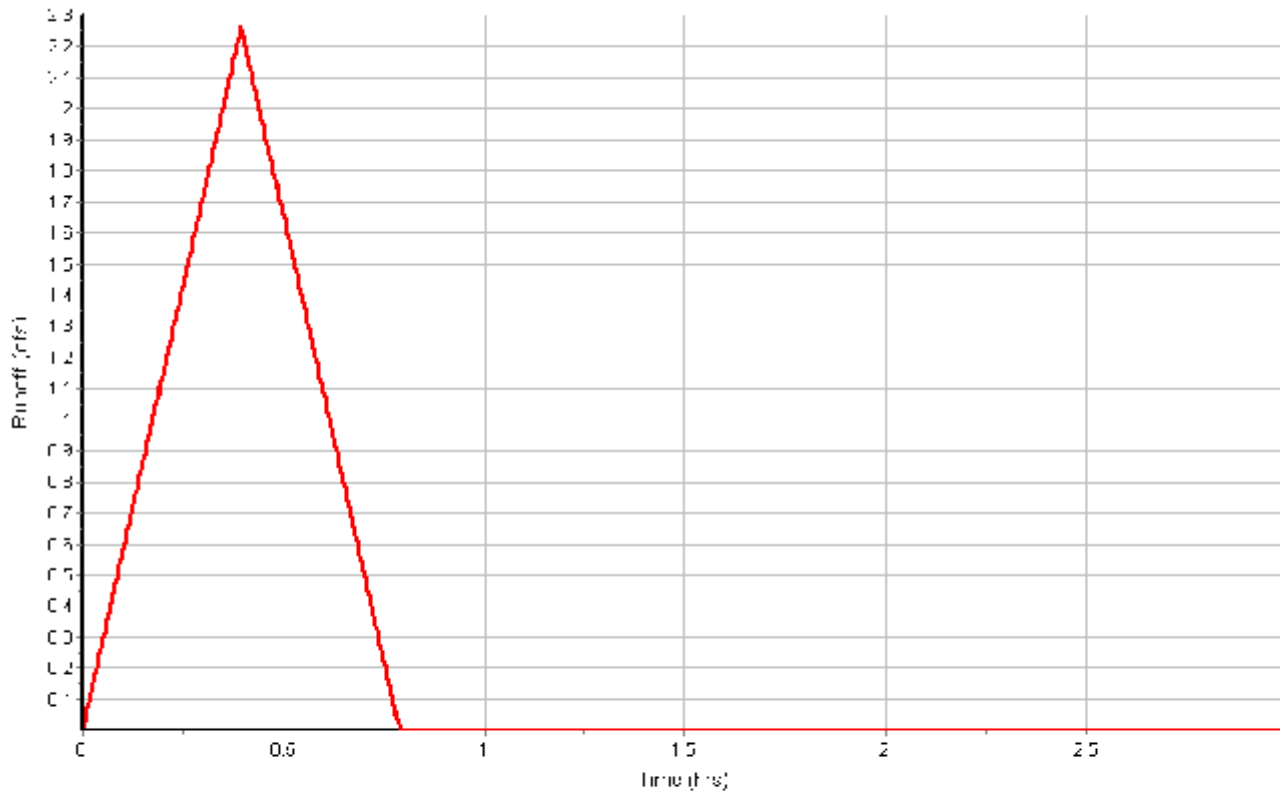
	Subarea	Subarea	Subarea
	A	B	C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	80	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	21.36	0.00	0.00
	Subarea	Subarea	Subarea
	A	B	C
Shallow Concentrated Flow Computations			
Flow Length (ft) :	70	280	0.00
Slope (%) :	3	2.5	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.52	2.37	0.00
Computed Flow Time (min) :	0.33	1.97	0.00
Total TOC (min)	23.66		

Subbasin Runoff Results

Total Rainfall (in)	1.12
Total Runoff (in)	0.77
Peak Runoff (cfs)	2.26
Rainfall Intensity	2.829
Weighted Runoff Coefficient	0.6900
Time of Concentration (days hh:mm:ss)	0 00:23:40

Subbasin : Pr-N1

Runoff Hydrograph



Input Data

Area (ft²) 30450.00
 Weighted Runoff Coefficient 0.8200

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	24755.84	B (2-6%)	0.96
Open Space, 25 years or greater	5694.16	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	30450.00		0.82

Time of Concentration

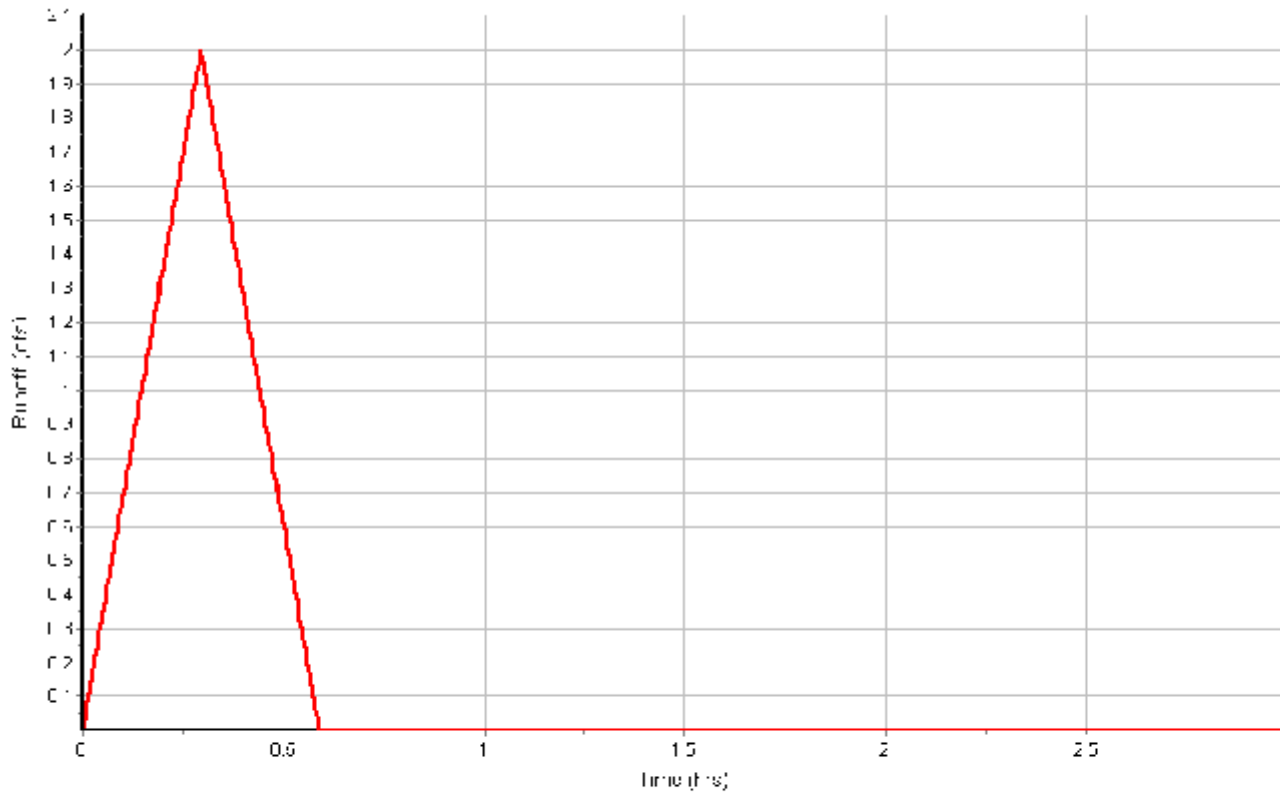
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	14.66	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	170	350	0.00
Slope (%) :	2.1	2	0.00
Surface Type :	Paved	Paved	Paved
Velocity (ft/sec) :	2.95	2.87	0.00
Computed Flow Time (min) :	0.96	2.03	0.00
Total TOC (min)	17.66		

Subbasin Runoff Results

Total Rainfall (in) 1.03
 Total Runoff (in) 0.84
 Peak Runoff (cfs) 2.00
 Rainfall Intensity 3.482
 Weighted Runoff Coefficient 0.8200
 Time of Concentration (days hh:mm:ss) 0 00:17:40

Subbasin : Pr-N2

Runoff Hydrograph



Input Data

Area (ft²) 29890.00
Weighted Runoff Coefficient 0.8800

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	26841.24	B (2-6%)	0.96
Open Space, 25 years or greater	3048.76	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	29890.00		0.88

Time of Concentration

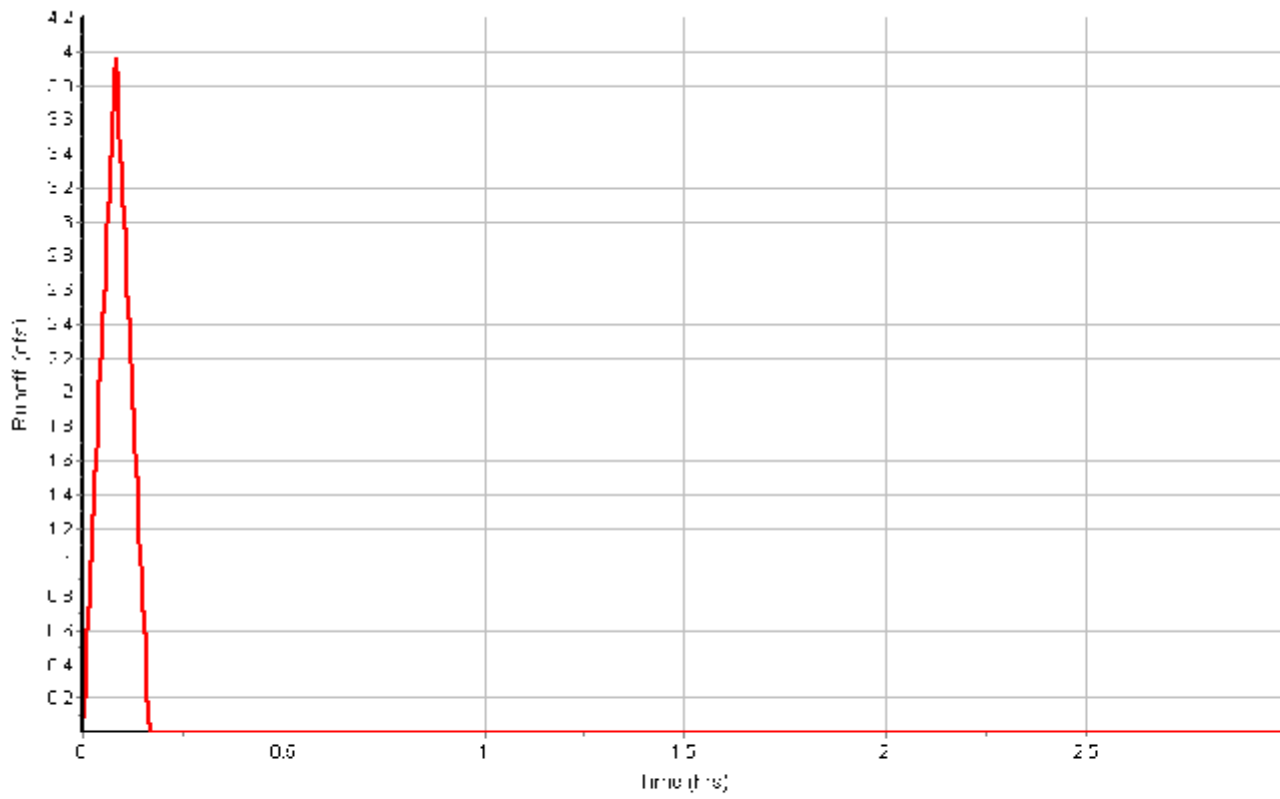
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.48
Peak Runoff (cfs) 3.96
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.8800
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-N3

Runoff Hydrograph



Input Data

Area (ft²) 24660.00
Weighted Runoff Coefficient 0.5800

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12403.97	B (2-6%)	0.96
Open Space, 25 years or greater	12256.04	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	24660.01		0.58

Time of Concentration

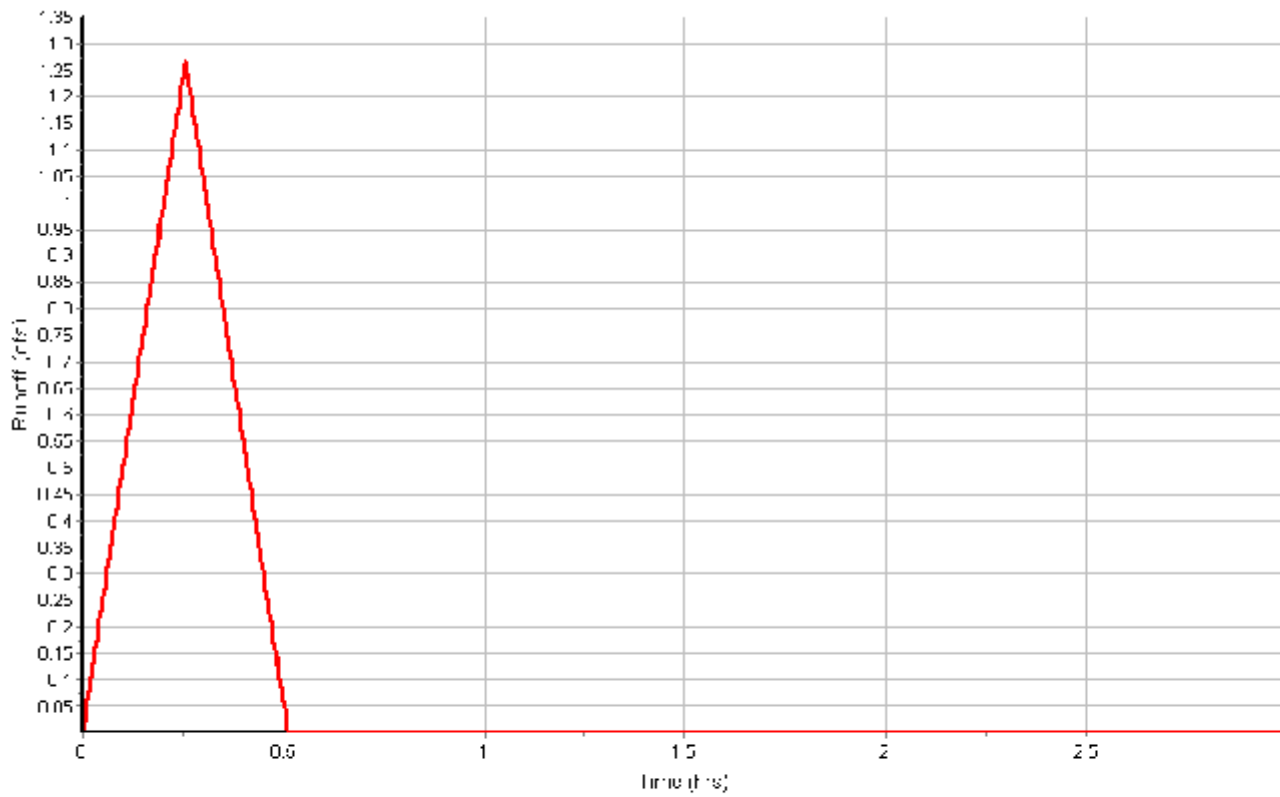
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	50	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	14.66	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	50	40	0.00
Slope (%) :	2	1	0.00
Surface Type :	Paved	Paved	Paved
Velocity (ft/sec) :	2.87	2.03	0.00
Computed Flow Time (min) :	0.29	0.33	0.00
Total TOC (min)15.28			

Subbasin Runoff Results

Total Rainfall (in) 0.99
Total Runoff (in) 0.57
Peak Runoff (cfs) 1.27
Rainfall Intensity 3.859
Weighted Runoff Coefficient 0.5800
Time of Concentration (days hh:mm:ss) 0 00:15:17

Subbasin : Pr-S1

Runoff Hydrograph



Input Data

Area (ft²) 5800.00
Weighted Runoff Coefficient 0.8600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	5080.79	B (2-6%)	0.96
Open Space, 25 years or greater	719.22	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	5800.01		0.86

Time of Concentration

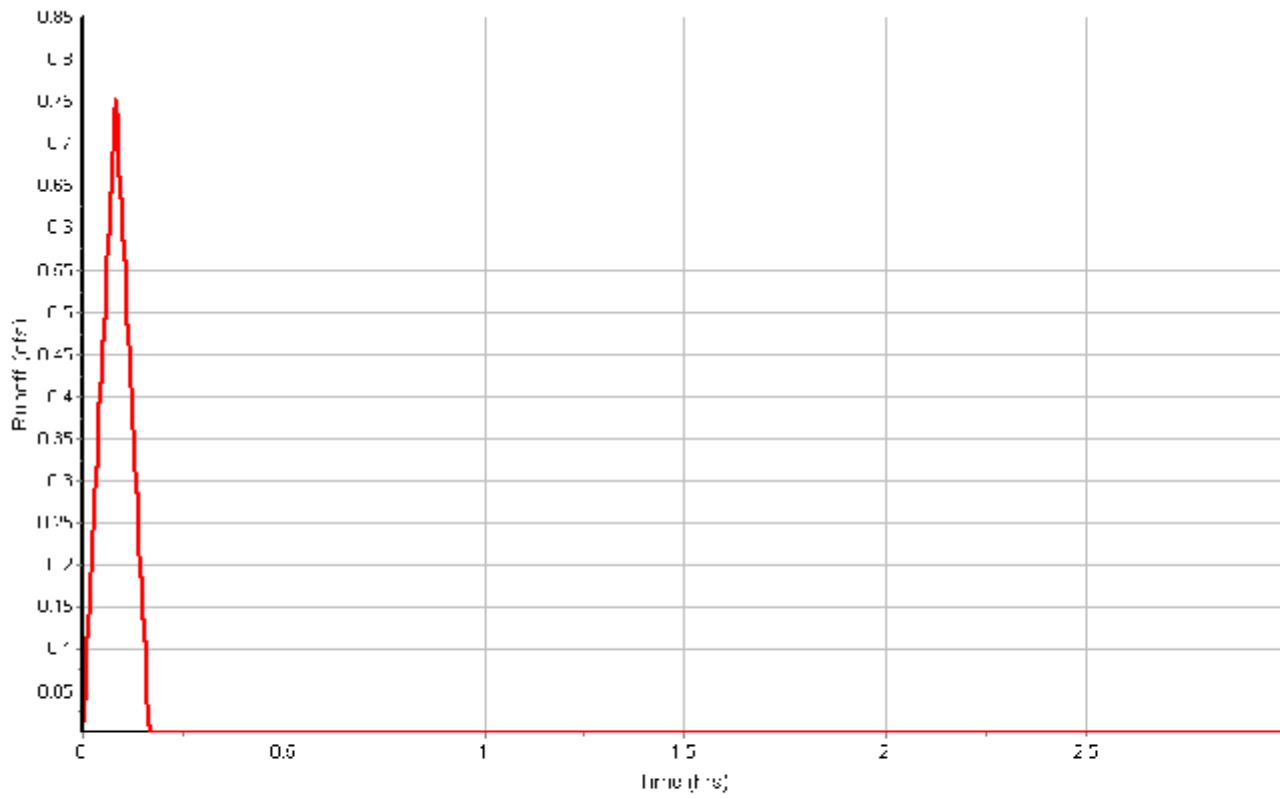
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.47
Peak Runoff (cfs) 0.75
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.8600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S2

Runoff Hydrograph



Input Data

Area (ft²) 16390.00
Weighted Runoff Coefficient 0.7600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12145.01	B (2-6%)	0.96
Open Space, 25 years or greater	4245.01	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	16390.02		0.76

Time of Concentration

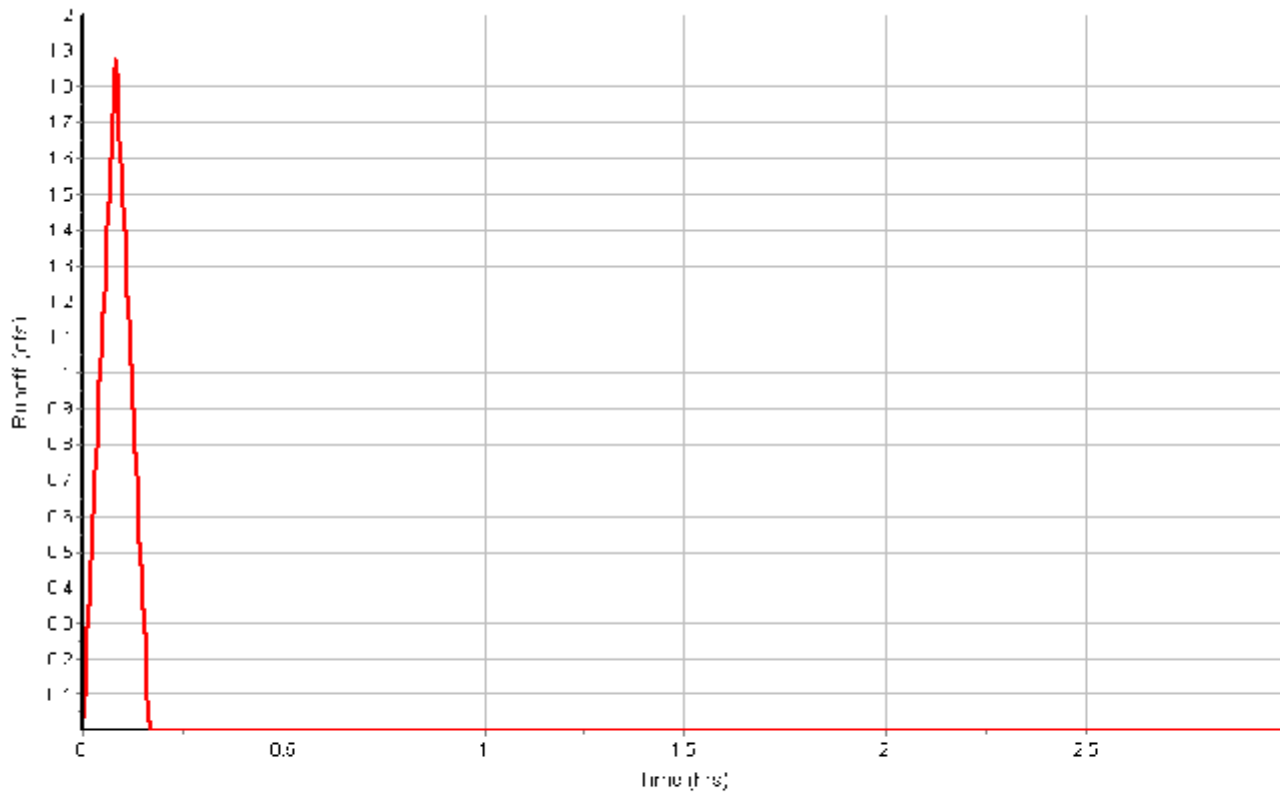
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.42
Peak Runoff (cfs) 1.88
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.7600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S3

Runoff Hydrograph



Input Data

Area (ft²) 5969.99
Weighted Runoff Coefficient 0.7900

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	4620.80	B (2-6%)	0.96
Open Space, 25 years or greater	1349.23	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	5970.03		0.79

Time of Concentration

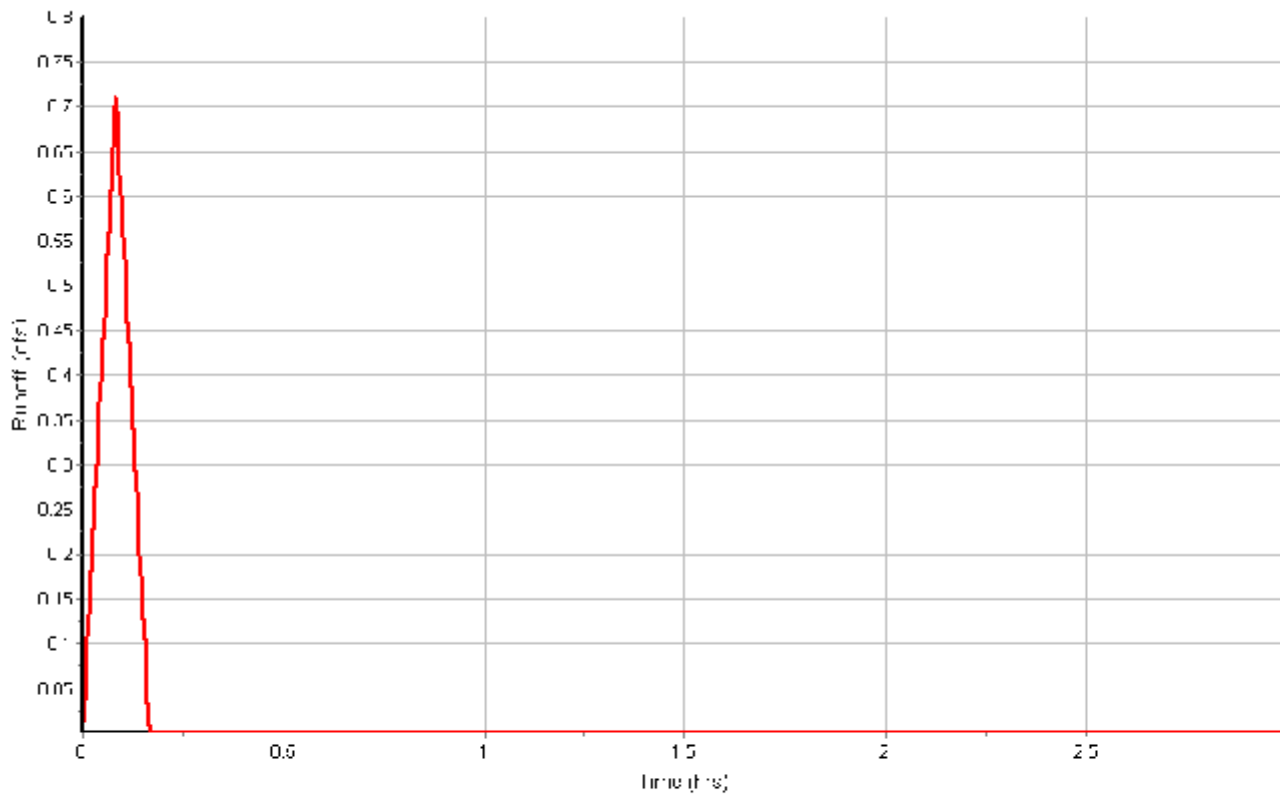
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.43
Peak Runoff (cfs) 0.71
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.7900
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S4

Runoff Hydrograph



Input Data

Area (ft²) 41450.00
Weighted Runoff Coefficient 0.7000

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	27274.09	B (2-6%)	0.96
Open Space, 25 years or greater	14175.91	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	41450.00		0.70

Time of Concentration

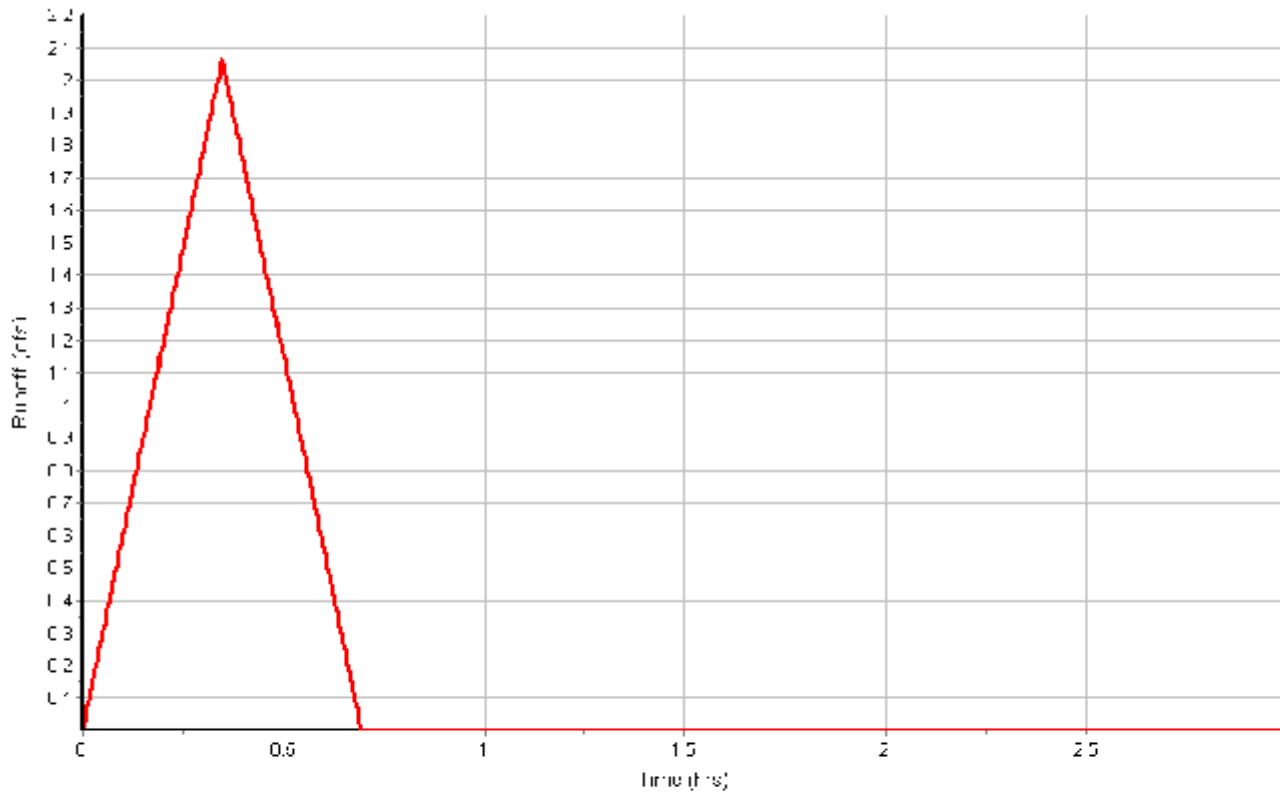
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.35	0.00	0.00
Flow Length (ft) :	65	0.00	0.00
Slope (%) :	4	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	18.09	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	80	390	0.00
Slope (%) :	3.5	3.3	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.80	2.72	0.00
Computed Flow Time (min) :	0.35	2.39	0.00
Total TOC (min)	20.83		

Subbasin Runoff Results

Total Rainfall (in) 1.08
Total Runoff (in) 0.75
Peak Runoff (cfs) 2.06
Rainfall Intensity 3.097
Weighted Runoff Coefficient 0.7000
Time of Concentration (days hh:mm:ss) 0 00:20:50

Subbasin : Pr-S5

Runoff Hydrograph



Input Data

Area (ft²) 14609.98
Weighted Runoff Coefficient 0.8500

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	12564.62	B (2-6%)	0.96
Open Space, 25 years or greater	2045.40	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	14610.02		0.85

Time of Concentration

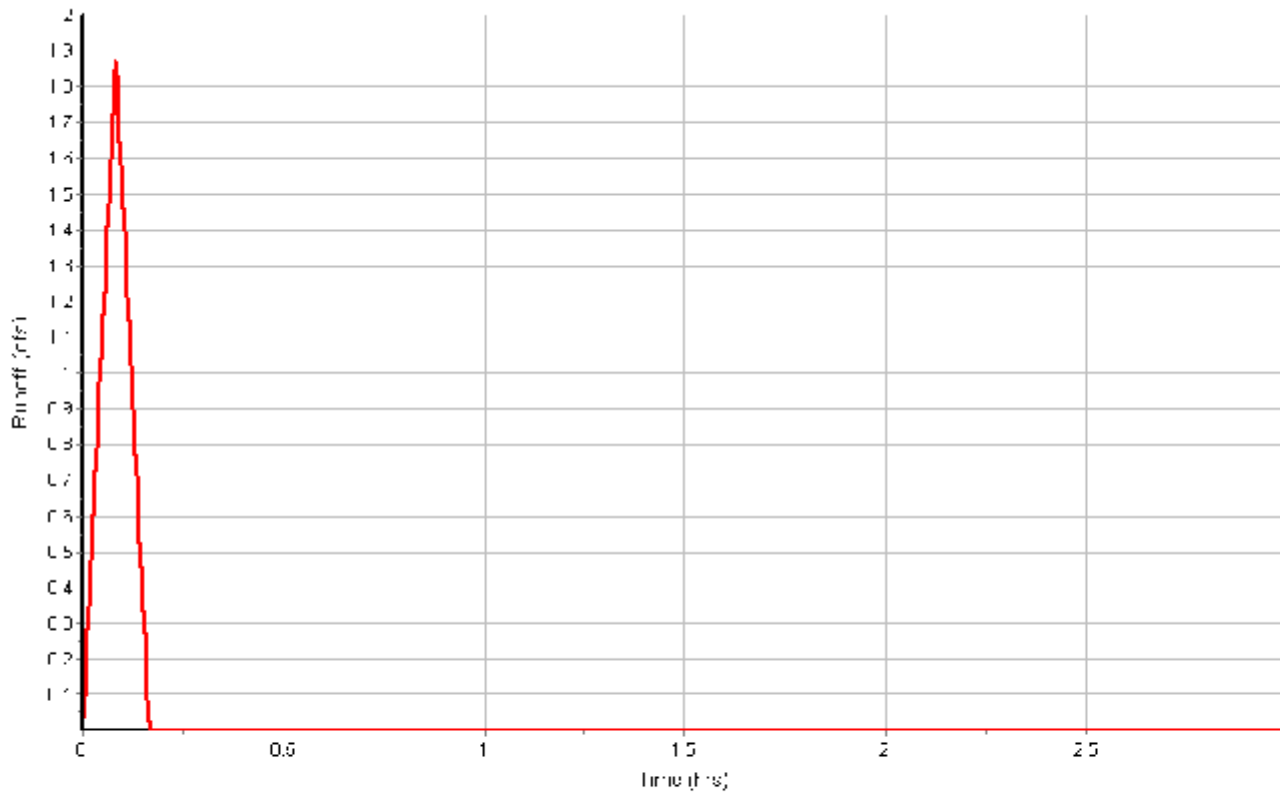
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.47
Peak Runoff (cfs) 1.87
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.8500
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S6

Runoff Hydrograph



Input Data

Area (ft²) 19569.98
Weighted Runoff Coefficient 0.8600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	17006.35	B (2-6%)	0.96
Open Space, 25 years or greater	2563.68	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	19570.03		0.86

Time of Concentration

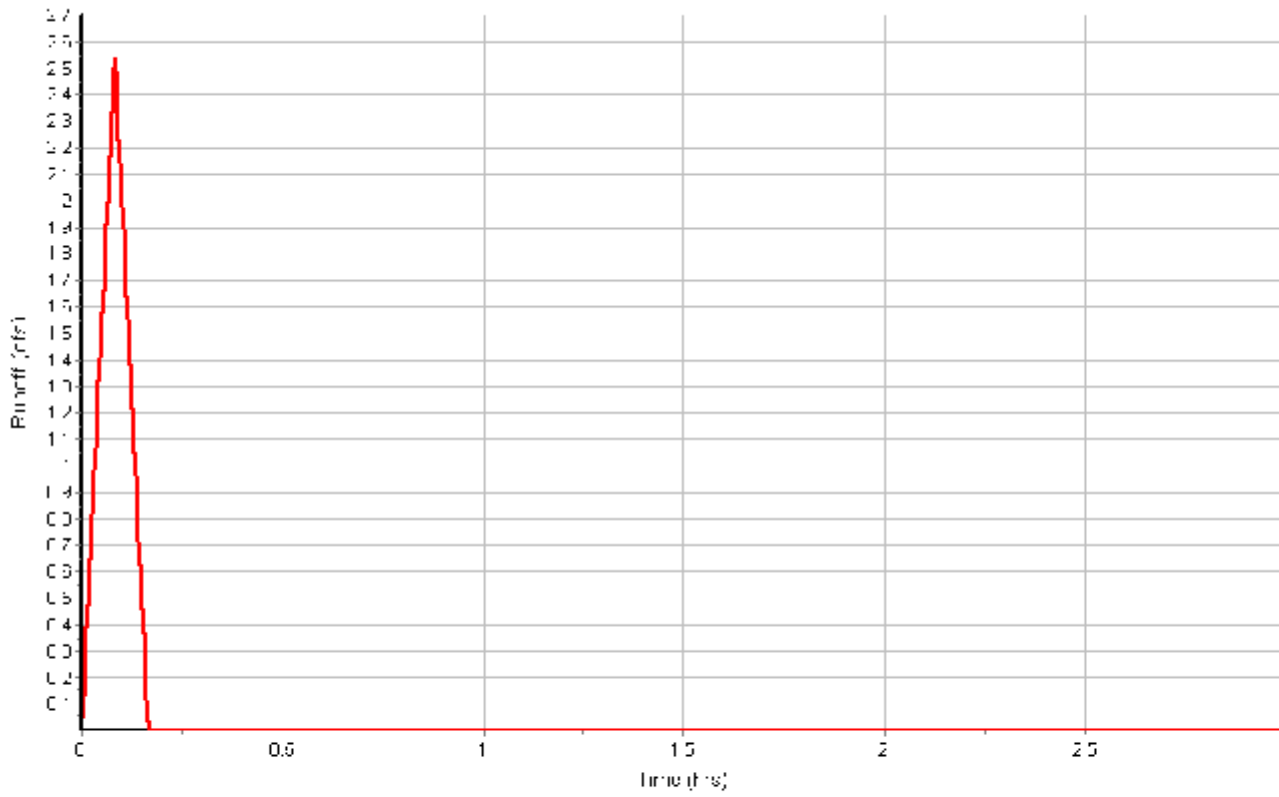
User-Defined TOC override (minutes): 5

Subbasin Runoff Results

Total Rainfall (in) 0.55
Total Runoff (in) 0.47
Peak Runoff (cfs) 2.54
Rainfall Intensity 6.560
Weighted Runoff Coefficient 0.8600
Time of Concentration (days hh:mm:ss) 0 00:05:00

Subbasin : Pr-S7

Runoff Hydrograph



Input Data

Area (ft²) 52210.01
 Weighted Runoff Coefficient 0.7600

Runoff Coefficient

Soil/Surface Description	Area (ft²)	Soil Group	Runoff Coeff.
Parking, 25 years or greater	38844.24	B (2-6%)	0.96
Open Space, 25 years or greater	13365.78	B (2-6%)	0.19
Composite Area & Weighted Runoff Coeff.	52210.02		0.76

Time of Concentration

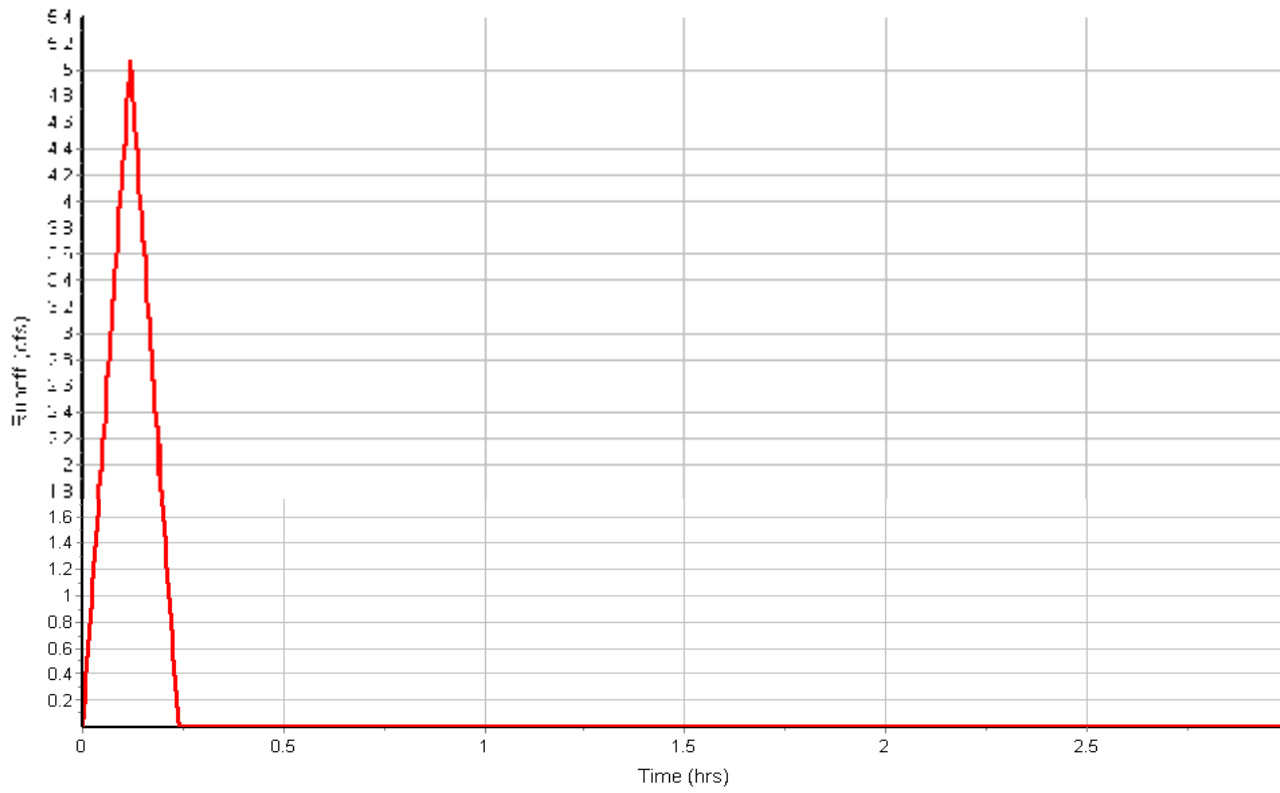
	Subarea A	Subarea B	Subarea C
Sheet Flow Computations			
Manning's Roughness :	0.40	0.00	0.00
Flow Length (ft) :	20	0.00	0.00
Slope (%) :	10	0.00	0.00
2 yr, 24 hr Rainfall (in) :	1.05	0.00	0.00
Velocity (ft/sec) :	0.06	0.00	0.00
Computed Flow Time (min) :	5.43	0.00	0.00
Shallow Concentrated Flow Computations			
Flow Length (ft) :	210	150	0.00
Slope (%) :	3.5	3.5	0.00
Surface Type :	Paved	Grassed waterway	Paved
Velocity (ft/sec) :	3.80	2.81	0.00
Computed Flow Time (min) :	0.92	0.89	0.00
Total TOC (min)7.24			

Subbasin Runoff Results

Total Rainfall (in) 0.66
 Total Runoff (in) 0.51
 Peak Runoff (cfs) 5.06
 Rainfall Intensity 5.558
 Weighted Runoff Coefficient 0.7600
 Time of Concentration (days hh:mm:ss) 0 00:07:14

Subbasin : Pr-S8

Runoff Hydrograph



Junction Input

Storm Event

SN	Element ID	Invert Elevation (ft)	Ground/Rim (Max) Elevation (ft)	Ground/Rim (Max) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)
1	SDMH-N3	5719.86	5725.45	5.59	5719.86	0.00
2	SDMH-S2	5729.50	5731.40	1.90	5729.50	0.00
3	WQ-N	5719.73	5725.57	5.84	5719.73	0.00
4	WQ-S	5716.00	5721.50	5.50	5716.00	0.00

Junction Results

SN	Element ID	Peak Inflow	Max HGL Elevation Attained	Max HGL Depth Attained	Min Freeboard Attained	Average HGL Elevation Attained	Average HGL Depth Attained	Time of Max HGL Occurrence	Total Flooded Volume	Total Time Flooded
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(days hh:mm)	(ft³)	(min)
1	SDMH-N3	4.440	5720.76	0.90	4.69	5720.19	0.33	0 00:19	0.00	0.00
2	SDMH-S2	1.210	5730.09	0.59	1.31	5729.75	0.25	0 00:16	0.00	0.00
3	WQ-N	4.440	5720.44	0.71	5.13	5719.86	0.13	0 00:05	0.00	0.00
4	WQ-S	11.290	5718.56	2.56	2.94	5718.02	2.02	0 00:05	0.00	0.00

Storm Event

Channel Input

SN	Element ID	Length (ft)	Inlet Invert Elevation (ft)	Inlet Invert Offset (ft)	Outlet Invert Elevation (ft)	Outlet Invert Offset (ft)	Total Drop (ft)	Average Slope (%)	Shape	Height (ft)	Width (ft)	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Additional Losses	Initial Flow (cfs)
1	Road-S3	109.00	5732.10	2.51	5726.40	2.65	5.70	5.2300	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000
2	Road-S4	69.00	5726.40	2.65	5723.50	3.42	2.90	4.2000	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000
3	Road-S6a	84.00	5723.50	3.42	5721.50	3.93	2.00	2.3800	User-Defined	0.600	33.000	0.0320	0.5000	0.5000	0.0000	0.000

Channel Results

SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Reported Surcharged Condition
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)
1 Road-S3	0.670	0 00:05	114.310	0.01	6.07	0.30	0.12	0.19	0.00
2 Road-S4	0.180	0 00:05	100.690	0.00	3.94	0.29	0.07	0.12	0.00
3 Road-S6a	0.000	0 00:07	79.170	0.00	0.00		0.01	0.01	0.00

Pipe Input

SN Element ID	Length	Inlet Invert Elevation	Inlet Invert Offset	Outlet Invert Elevation	Outlet Invert Offset	Total Drop	Average Slope	Pipe Shape	Pipe Diameter or Height	Pipe Width	Manning's Roughness	Entrance Losses	Exit/Bend Losses	Initial Flow	Flap Gate	No. of Barrels
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)				(cfs)		
1 Link-47	32.09	5718.50	1.00	5718.00	2.00	0.50	1.5600	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
2 Link-57	8.00	5719.86	0.00	5719.73	0.00	0.13	1.6300	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
3 Pipe-6b	163.00	5717.80	0.23	5716.50	0.50	1.30	0.8000	CIRCULAR	24.000	24.000	0.0150	0.5000	0.5000	0.000	No	1
4 Pipe-N1	73.00	5721.75	0.00	5721.03	0.50	0.72	0.9900	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
5 Pipe-N2	41.00	5720.53	0.00	5720.06	0.20	0.47	1.1500	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
6 Pipe-N3	21.00	5720.90	0.00	5720.06	0.20	0.84	4.0000	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
7 Pipe-Out-N	20.00	5719.73	0.00	5717.50	0.00	2.23	11.1500	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
8 Pipe-Out-S	24.00	5716.00	0.00	5715.00	-0.76	1.00	4.1700	CIRCULAR	24.000	24.000	0.0150	0.5000	0.5000	0.000	No	1
9 Pipe-S1	50.00	5730.70	0.00	5729.70	0.20	1.00	2.0000	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
10 Pipe-S2	290.00	5729.50	0.00	5723.75	0.00	5.75	1.9800	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
11 Pipe-S3	106.00	5729.59	0.00	5723.95	0.20	5.64	5.3200	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
12 Pipe-S4	67.00	5723.75	0.00	5720.58	0.50	3.17	4.7300	CIRCULAR	12.000	12.000	0.0150	0.5000	0.5000	0.000	No	1
13 Pipe-S5	41.00	5721.56	-0.14	5720.58	0.50	0.98	2.3900	CIRCULAR	9.960	9.960	0.0150	0.5000	0.5000	0.000	No	1
14 Pipe-S6a	65.00	5720.08	0.00	5718.80	0.50	1.28	1.9700	CIRCULAR	15.000	15.000	0.0150	0.5000	0.5000	0.000	No	1
15 Pipe-S8	43.00	5718.60	0.30	5718.20	0.63	0.40	0.9300	Horizontal Ellipse	14.040	23.040	0.0130	0.5000	0.5000	0.000	No	1

Pipe Results

SN Element ID	Peak Flow	Time of Peak Flow Occurrence	Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Travel Time	Peak Flow Depth	Peak Flow Depth/ Total Depth Ratio	Total Time Surcharged
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		(min)
1 Link-47	2.340	0 00:05	3.85	0.61	5.14	0.10	0.56	0.56	0.00
2 Link-57	4.440	0 00:05	7.14	0.62	6.13	0.02	0.71	0.57	0.00
3 Pipe-6b	9.320	0 00:07	17.51	0.53	5.71	0.48	1.04	0.52	0.00
4 Pipe-N1	2.210	0 00:24	3.07	0.72	4.26	0.29	0.63	0.63	0.00
5 Pipe-N2	3.610	0 00:19	5.99	0.60	5.10	0.13	0.70	0.56	0.00
6 Pipe-N3	3.700	0 00:05	6.18	0.60	8.21	0.04	0.56	0.56	0.00
7 Pipe-Out-N	4.440	0 00:05	18.69	0.24	12.48	0.03	0.41	0.33	0.00
8 Pipe-Out-S	11.290	0 00:06	19.61	0.58	6.46	0.06	1.09	0.54	0.00
9 Pipe-S1	1.210	0 00:16	2.69	0.45	4.80	0.17	0.39	0.47	0.00
10 Pipe-S2	1.200	0 00:17	2.67	0.45	4.82	1.00	0.39	0.47	0.00
11 Pipe-S3	1.160	0 00:05	4.38	0.27	10.07	0.18	0.29	0.35	0.00
12 Pipe-S4	3.150	0 00:05	6.72	0.47	8.42	0.13	0.48	0.48	0.00
13 Pipe-S5	2.050	0 00:21	3.14	0.65	6.13	0.11	0.49	0.59	0.00
14 Pipe-S6a	3.800	0 00:05	7.86	0.48	6.36	0.17	0.61	0.49	0.00
15 Pipe-S8	8.020	0 00:07	10.17	0.79	6.37	0.11	0.75	0.64	0.00

Inlet Input

SN	Element ID	Inlet Manufacturer	Manufacturer Part Number	Inlet Location	Number of Inlets	Catchbasin Invert Elevation (ft)	Max (Rim) Elevation (ft)	Inlet Depth (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Ponded Area (ft ²)	Grate Clogging Factor (%)
1	Inlet-N1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.75	5724.50	2.75	5721.75	0.00	400.00	50.00
2	Inlet-N2	FHWA HEC-22 GENERIC	N/A	On Sag	2	5720.53	5725.35	4.82	5722.30	1.77	400.00	50.00
3	Inlet-N3	FHWA HEC-22 GENERIC	N/A	On Sag	1	5720.90	5724.90	4.00	5722.00	1.10	400.00	50.00
4	Inlet-S1	FHWA HEC-22 GENERIC	N/A	On Sag	1	5730.70	5733.20	2.50	5730.70	0.00	400.00	50.00
5	Inlet-S3	FHWA HEC-22 GENERIC	N/A	On Grade	1	5729.59	5732.09	2.50	5729.59	0.00	N/A	50.00
6	Inlet-S4	FHWA HEC-22 GENERIC	N/A	On Grade	1	5723.75	5726.41	2.66	5723.42	-0.33	N/A	50.00
7	Inlet-S5	FHWA HEC-22 GENERIC	N/A	On Sag	1	5721.70	5724.70	3.00	5721.70	0.00	100.00	50.00
8	Inlet-S6a	FHWA HEC-22 GENERIC	N/A	On Grade	1	5720.08	5723.62	3.54	5720.08	0.00	N/A	50.00
9	Inlet-S6b	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.57	5721.52	3.95	5717.50	-0.07	400.00	50.00
10	Inlet-S7	FHWA HEC-22 GENERIC	N/A	On Sag	1	5717.50	5721.00	3.50	5717.50	0.00	400.00	50.00
11	Inlet-S8	FHWA HEC-22 GENERIC	N/A	On Sag	1	5718.30	5721.38	3.08	5718.30	0.00	400.00	50.00

Storm Event

Roadway & Gutter Input

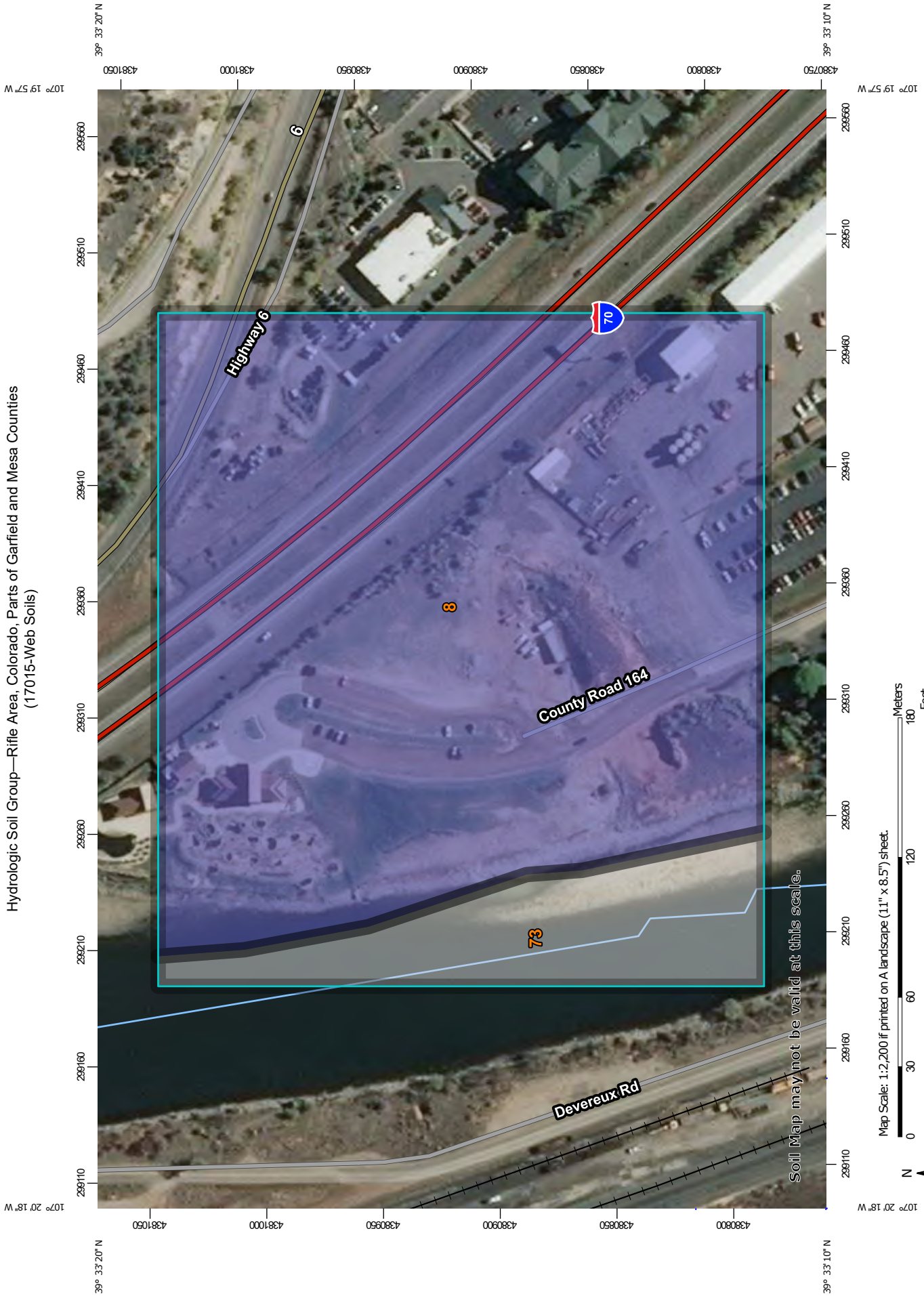
SN Element ID	Roadway Longitudinal Slope (ft/ft)	Roadway Cross Slope (ft/ft)	Roadway Manning's Roughness	Gutter Cross Slope (ft/ft)	Gutter Width (ft)	Gutter Depression (in)	Allowable Spread (ft)
1 Inlet-N1	N/A	0.1000	0.0220	0.1000	2.00	0.0000	7.00
2 Inlet-N2	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
3 Inlet-N3	N/A	0.0200	0.0160	0.0620	2.00	0.0656	7.00
4 Inlet-S1	N/A	0.0200	0.0160	0.0620	2.00	0.0656	7.00
5 Inlet-S3	0.0100	0.0200	0.0160	0.0620	2.00	0.0656	7.00
6 Inlet-S4	0.0500	0.0200	0.0160	0.0620	2.00	0.0656	7.00
7 Inlet-S5	N/A	0.1000	0.0220	0.1000	2.00	0.0000	7.00
8 Inlet-S6a	0.0400	0.0200	0.0160	0.0620	2.00	0.0656	7.00
9 Inlet-S6b	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
10 Inlet-S7	N/A	0.0200	0.0160	0.0620	2.00	0.0000	7.00
11 Inlet-S8	N/A	0.1000	0.0160	0.0620	2.00	0.0000	7.00

Storm Event

Inlet Results

SN	Element ID	Peak Flow	Peak Lateral Inflow	Peak Flow Intercepted	Peak Flow Bypassing Inlet	Inlet Efficiency during Peak Flow (%)	Max Gutter Spread during Peak Flow (ft)	Max Gutter Water Elev. during Peak Flow (ft)	Max Gutter Water Depth during Peak Flow (ft)	Time of Max Depth Occurrence	Total Flooded Volume	Total Time Flooded
		(cfs)	(cfs)	(cfs)	(cfs)					(days hh:mm)	(ft ³)	(hr)
1	Inlet-N1	2.260	2.26	N/A	N/A	N/A	4.84	5724.98	0.48	0 00:24	0.00	0.00
2	Inlet-N2	2.000	2.00	N/A	N/A	N/A	9.09	5725.61	0.26	0 00:00	0.00	0.00
3	Inlet-N3	3.960	3.96	N/A	N/A	N/A	18.28	5725.50	0.60	0 00:00	0.00	0.00
4	Inlet-S1	1.270	1.27	N/A	N/A	N/A	7.75	5733.60	0.40	0 00:16	0.00	0.00
5	Inlet-S3	1.880	1.88	1.17	0.70	62.48	8.33	5732.34	0.25	0 00:05	0.00	0.00
6	Inlet-S4	1.340	0.71	1.16	0.19	86.09	4.62	5726.59	0.18	0 00:05	0.00	0.00
7	Inlet-S5	2.060	2.06	N/A	N/A	N/A	4.63	5725.16	0.46	0 00:21	0.00	0.00
8	Inlet-S6a	0.180	0.00	0.18	0.00	99.66	0.86	5723.67	0.05	0 00:21	0.00	0.00
9	Inlet-S6b	1.870	1.87	N/A	N/A	N/A	10.52	5721.80	0.28	0 00:07	0.00	0.00
10	Inlet-S7	2.530	2.53	N/A	N/A	N/A	13.18	5721.33	0.33	0 00:05	0.00	0.00
11	Inlet-S8	5.060	5.06	N/A	N/A	N/A	5.95	5721.89	0.51	0 00:05	0.00	0.00

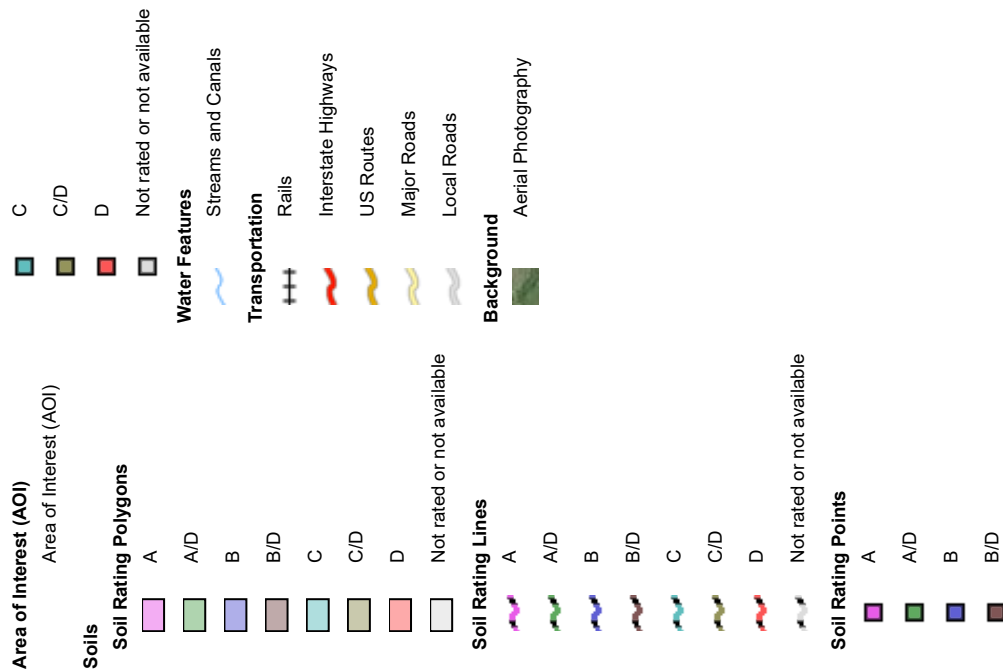
Hydrologic Soil Group—Rifle Area, Colorado, Parts of Garfield and Mesa Counties (17015-Web Soils)



Map Scale: 1:2,200 if printed on A landscape (11" x 8.5") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND



MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rifle Area, Colorado, Parts of Garfield and Mesa Counties

Survey Area Data: Version 11, Sep 10, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 24, 2015—Nov 2, 2015

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Atencio-Azeltine complex, 1 to 3 percent slopes	B	16.2	86.9%
73	Water		2.4	13.1%
Totals for Area of Interest			18.6	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition



Component Percent Cutoff: None Specified

Tie-break Rule: Higher



NOAA Atlas 14, Volume 8, Version 2
Location name: Glenwood Springs, Colorado,
USA*



Latitude: 39.546°, Longitude: -107.3237°
Elevation: 5795.68 ft**



* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffrey Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aeriels](#)

PF tabular

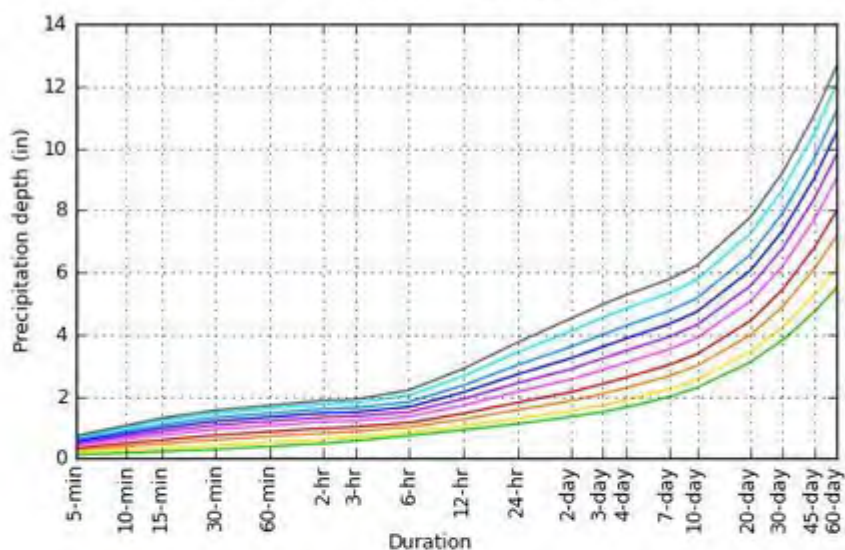
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.121 (0.096-0.157)	0.177 (0.140-0.229)	0.264 (0.207-0.343)	0.332 (0.260-0.434)	0.421 (0.313-0.565)	0.486 (0.354-0.664)	0.547 (0.385-0.769)	0.605 (0.407-0.878)	0.677 (0.437-1.01)	0.726 (0.460-1.12)
10-min	0.178 (0.140-0.230)	0.259 (0.204-0.335)	0.386 (0.304-0.502)	0.486 (0.380-0.635)	0.617 (0.459-0.827)	0.712 (0.518-0.972)	0.801 (0.563-1.13)	0.886 (0.596-1.29)	0.991 (0.640-1.49)	1.06 (0.673-1.64)
15-min	0.217 (0.171-0.280)	0.316 (0.249-0.409)	0.471 (0.370-0.612)	0.593 (0.464-0.775)	0.752 (0.560-1.01)	0.868 (0.632-1.19)	0.977 (0.687-1.37)	1.08 (0.727-1.57)	1.21 (0.781-1.81)	1.30 (0.821-2.00)
30-min	0.291 (0.230-0.376)	0.412 (0.325-0.534)	0.601 (0.472-0.780)	0.748 (0.585-0.977)	0.937 (0.696-1.25)	1.07 (0.780-1.46)	1.20 (0.841-1.68)	1.32 (0.884-1.90)	1.46 (0.940-2.18)	1.55 (0.982-2.39)
60-min	0.387 (0.305-0.500)	0.512 (0.404-0.663)	0.707 (0.556-0.918)	0.860 (0.672-1.12)	1.06 (0.786-1.41)	1.20 (0.873-1.63)	1.33 (0.935-1.87)	1.45 (0.978-2.11)	1.60 (1.04-2.40)	1.71 (1.08-2.62)
2-hr	0.482 (0.386-0.615)	0.611 (0.488-0.781)	0.813 (0.647-1.04)	0.971 (0.768-1.25)	1.18 (0.887-1.55)	1.32 (0.977-1.78)	1.46 (1.04-2.02)	1.59 (1.08-2.28)	1.75 (1.14-2.59)	1.86 (1.19-2.82)
3-hr	0.565 (0.456-0.715)	0.683 (0.550-0.865)	0.870 (0.697-1.11)	1.02 (0.811-1.30)	1.21 (0.924-1.59)	1.36 (1.01-1.81)	1.49 (1.07-2.05)	1.62 (1.11-2.30)	1.79 (1.18-2.62)	1.90 (1.22-2.86)
6-hr	0.738 (0.602-0.920)	0.840 (0.684-1.05)	1.01 (0.818-1.26)	1.15 (0.927-1.45)	1.35 (1.05-1.76)	1.50 (1.14-1.99)	1.66 (1.21-2.26)	1.82 (1.27-2.56)	2.04 (1.36-2.96)	2.20 (1.43-3.26)
12-hr	0.917 (0.757-1.13)	1.05 (0.862-1.29)	1.26 (1.04-1.56)	1.45 (1.18-1.79)	1.71 (1.35-2.20)	1.92 (1.47-2.51)	2.13 (1.57-2.87)	2.35 (1.66-3.27)	2.65 (1.80-3.80)	2.89 (1.90-4.21)
24-hr	1.11 (0.930-1.35)	1.28 (1.07-1.55)	1.56 (1.30-1.89)	1.80 (1.49-2.20)	2.15 (1.71-2.73)	2.42 (1.88-3.13)	2.71 (2.03-3.59)	3.01 (2.15-4.12)	3.41 (2.34-4.82)	3.73 (2.48-5.36)
2-day	1.34 (1.14-1.60)	1.53 (1.30-1.83)	1.86 (1.56-2.22)	2.14 (1.79-2.57)	2.55 (2.07-3.20)	2.88 (2.27-3.67)	3.23 (2.45-4.23)	3.60 (2.60-4.86)	4.11 (2.85-5.72)	4.51 (3.03-6.38)
3-day	1.50 (1.28-1.77)	1.72 (1.46-2.03)	2.08 (1.77-2.47)	2.40 (2.03-2.86)	2.86 (2.33-3.54)	3.22 (2.55-4.06)	3.60 (2.74-4.66)	3.99 (2.91-5.34)	4.54 (3.16-6.26)	4.97 (3.35-6.95)
4-day	1.64 (1.41-1.92)	1.87 (1.60-2.20)	2.27 (1.93-2.67)	2.60 (2.21-3.08)	3.08 (2.52-3.79)	3.46 (2.76-4.33)	3.86 (2.95-4.96)	4.27 (3.12-5.66)	4.82 (3.37-6.60)	5.26 (3.57-7.31)
7-day	1.98 (1.72-2.29)	2.23 (1.93-2.59)	2.65 (2.29-3.09)	3.01 (2.58-3.52)	3.52 (2.91-4.26)	3.92 (3.15-4.83)	4.33 (3.35-5.48)	4.75 (3.50-6.21)	5.33 (3.76-7.18)	5.77 (3.95-7.92)
10-day	2.28 (1.99-2.62)	2.54 (2.22-2.92)	2.98 (2.59-3.44)	3.36 (2.90-3.89)	3.88 (3.23-4.66)	4.30 (3.48-5.25)	4.72 (3.68-5.93)	5.16 (3.83-6.69)	5.76 (4.09-7.70)	6.22 (4.28-8.46)
20-day	3.11 (2.75-3.52)	3.44 (3.04-3.89)	3.99 (3.51-4.53)	4.44 (3.89-5.07)	5.08 (4.27-5.99)	5.57 (4.57-6.68)	6.07 (4.78-7.49)	6.58 (4.94-8.38)	7.27 (5.21-9.54)	7.79 (5.42-10.4)
30-day	3.81 (3.40-4.27)	4.22 (3.76-4.73)	4.89 (4.34-5.50)	5.44 (4.79-6.15)	6.19 (5.24-7.21)	6.76 (5.58-8.01)	7.33 (5.81-8.93)	7.90 (5.96-9.94)	8.65 (6.24-11.2)	9.21 (6.45-12.2)
45-day	4.71 (4.24-5.23)	5.25 (4.71-5.82)	6.10 (5.45-6.79)	6.78 (6.02-7.59)	7.69 (6.55-8.84)	8.36 (6.94-9.79)	9.02 (7.19-10.9)	9.66 (7.33-12.0)	10.5 (7.59-13.4)	11.1 (7.80-14.5)
60-day	5.49 (4.97-6.05)	6.15 (5.55-6.77)	7.17 (6.44-7.92)	7.97 (7.12-8.86)	9.02 (7.71-10.3)	9.79 (8.16-11.4)	10.5 (8.41-12.5)	11.2 (8.53-13.8)	12.0 (8.77-15.3)	12.6 (8.95-16.5)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

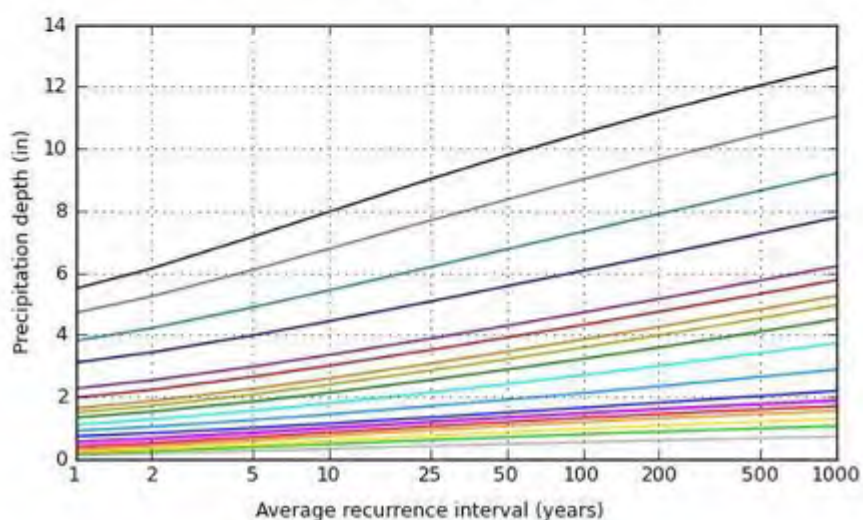
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PF graphical

PDS-based depth-duration-frequency (DDF) curves
Latitude: 39.5460°, Longitude: -107.3237°



Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000



Duration	
5-min	2-day
10-min	3-day
15-min	4-day
30-min	7-day
60-min	10-day
2-hr	20-day
3-hr	30-day
6-hr	45-day
12-hr	60-day
24-hr	

Maps & aerals

Small scale terrain



Large scale terrain



Large scale map



Large scale aerial



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[National Oceanic and Atmospheric Administration](#)
[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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DEPARTMENT OF THE ARMY
U.S. ARMY CORPS OF ENGINEERS, SACRAMENTO DISTRICT
1325 J STREET
SACRAMENTO CA 95814-2922

December 22, 2017

Regulatory Division (SPK-2017-00468)

Recreation Engineering & Planning
Attn: Mr. Gary Lacy
485 Arapahoe Avenue
Boulder, Colorado 80302

Dear Mr. Lacy:

We are responding to your request, on behalf of Iron Mountain Hot Springs, LLC, for a Department of the Army permit for the Iron Mountain Hot Springs Riverbank Improvements project. This project involves activities, including discharges of fill material, in waters of the United States to stabilize 1,000 linear-feet of streambank. The project site is located along the north bank (river right) of the Colorado River, within Government Lot 7 of Section 9 and Government Lot 8 of Section 4, Township 6 South, Range 89 West, 6th Principal Meridian, between Latitude 39.552808°, Longitude -107.336577° and Latitude 39.556465°, Longitude -107.337230°, in the City of Glenwood Springs, Garfield County, Colorado.

Based on the information you provided to this office, the proposed activity, including discharges of dredge and fill material in waters of the United States, involves the removal of 4,760 cubic-yards of river cobble and installation of 1,850 cubic-yards of triangular current deflectors and vegetated stone terracing to stabilize approximately 1,000 linear-feet of streambank, in accordance with the Permit Set drawings, dated May 25, 2017. These activities will result in permanent impacts to approximately 0.1 acre of perennial non-wetland waters.

We have determined activities associated with this project are authorized by Regional General Permit (RGP) 37 - Stream Stabilization Projects in Colorado. Your work must comply with the general terms and conditions listed on the enclosed RGP information sheets. Additionally, it is your responsibility to ensure that all contract personnel are familiar with permit requirements. Please note that you are required to sign the enclosed Compliance Certification and return it to this office within 30 days after completion of the remaining authorized work.

This verification is valid until October 22, 2022. Failure to comply with the General Conditions of this Regional General Permit may result in the suspension or revocation of your authorization.

Please refer to identification number SPK-2017-00468 in any correspondence concerning this project. If you have any questions, please contact me at the Colorado West Regulatory Branch, 400 Rood Avenue, Room 224, Grand Junction, Colorado 81501, by email at w.travis.morse@usace.army.mil, or telephone at (970) 243-1199 ext. 1014.

For more information regarding our program, please visit our website at www.spk.usace.army.mil/Missions/Regulatory.aspx. We would appreciate your feedback. At your earliest convenience, please tell us how we are doing by completing the customer survey from the link on our website, listed above.

Sincerely,

Travis Morse
Senior Project Manager
Colorado West Section
Regulatory Division

Enclosures

1. RGP 37
2. Compliance Certification

cc:

Mr. Gary Lacy, Recreation Engineering & Planning, gary@boaterparks.com
Mr. Steve Beckley, Iron Mountain Hot Springs, LLC, sbeckley@glenwoodcaverns.com
Mr. Tom Barnes, City of Glenwood Springs, tom.barnes@cogs.us
Ms. Sheryl Bower, Garfield County Building & Planning Department, sbower@garfield-county.com
Ms. Kendall Backich, Colorado Parks and Wildlife, kendall.backich@state.co.us

SITE SPECIFIC DATA*				
PROJECT NUMBER		11276		
PROJECT NAME		IRON MOUNTAIN HOT SPRINGS		
PROJECT LOCATION		GLENWOOD SPRINGS, CO		
STRUCTURE ID		WQ-BMP-N		
WATER QUALITY FLOW RATE (CFS)		5.05		
PEAK FLOW RATE (CFS)		4.5		
PEAK STORM DURATION (YEARS)		100		
PIPE DATA		I.E.	MATERIAL	DIAMETER
INLET PIPE 1		5719.73	ADS	24"
OUTLET PIPE 1		5719.73	ADS	24"
RIM ELEVATION		5725.57		
SUMP ELEVATION		5710.73		
SURFACE LOADING REQUIREMENT				HS-20
FRAME AND COVER				2 EA ø30"
PRECAST CONCRETE SKIMMER WALL HEIGHT				60"
KNOWN GROUNDWATER ELEVATION				
NOTES:				
*PER ENGINEER OF RECORD				

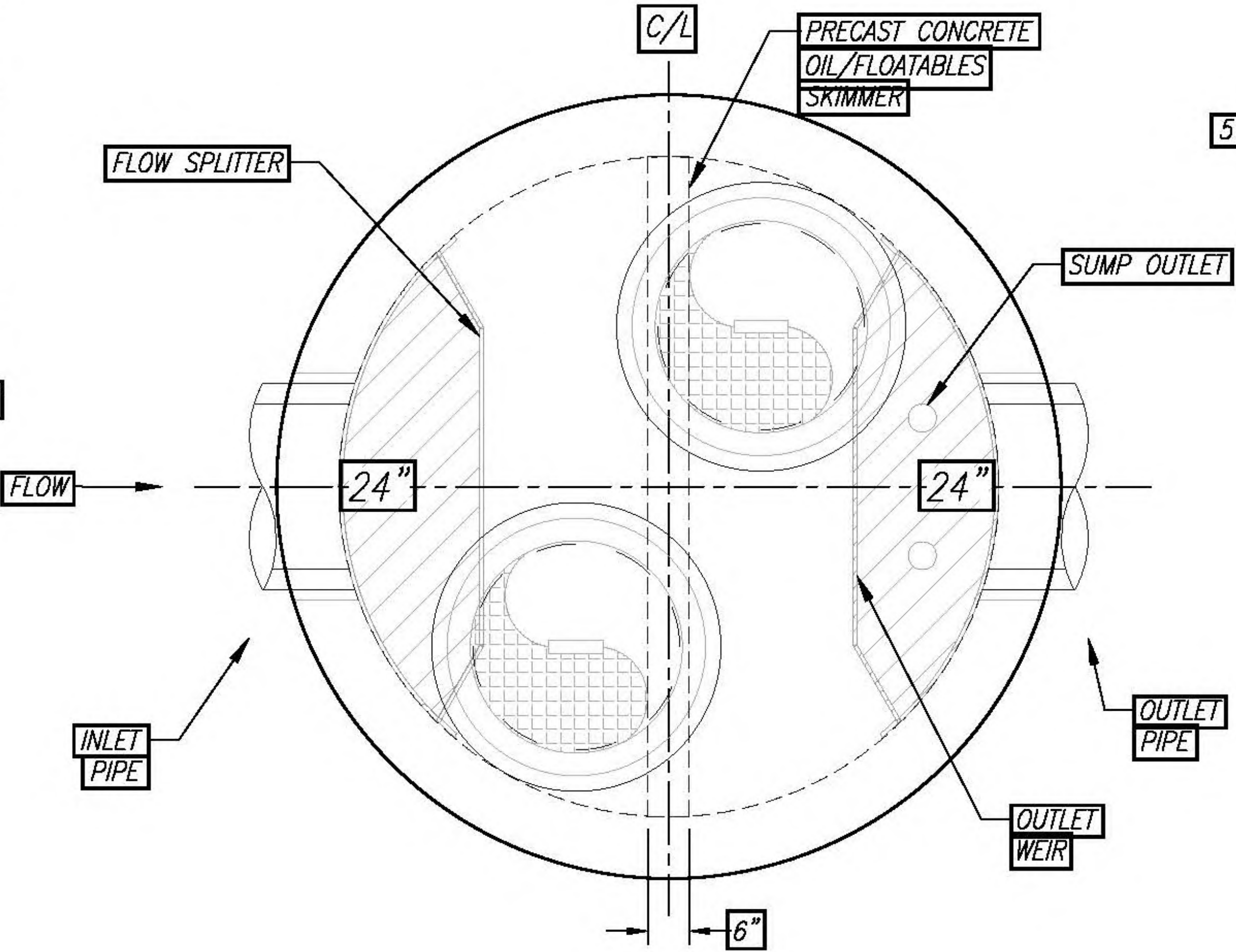
GENERAL NOTES

- BIO CLEAN TO PROVIDE ALL MATERIALS UNLESS OTHERWISE NOTED.
- ALL DIMENSIONS, ELEVATIONS, SPECIFICATIONS, AND CAPACITIES ARE SUBJECT TO CHANGE. FOR PROJECT SPECIFIC DRAWINGS DETAILING EXACT DIMENSIONS, WEIGHTS, AND ACCESSORIES PLEASE CONTACT BIO CLEAN.
- ALTERNATIVE HATCHES OR MANHOLES AVAILABLE UPON REQUEST.

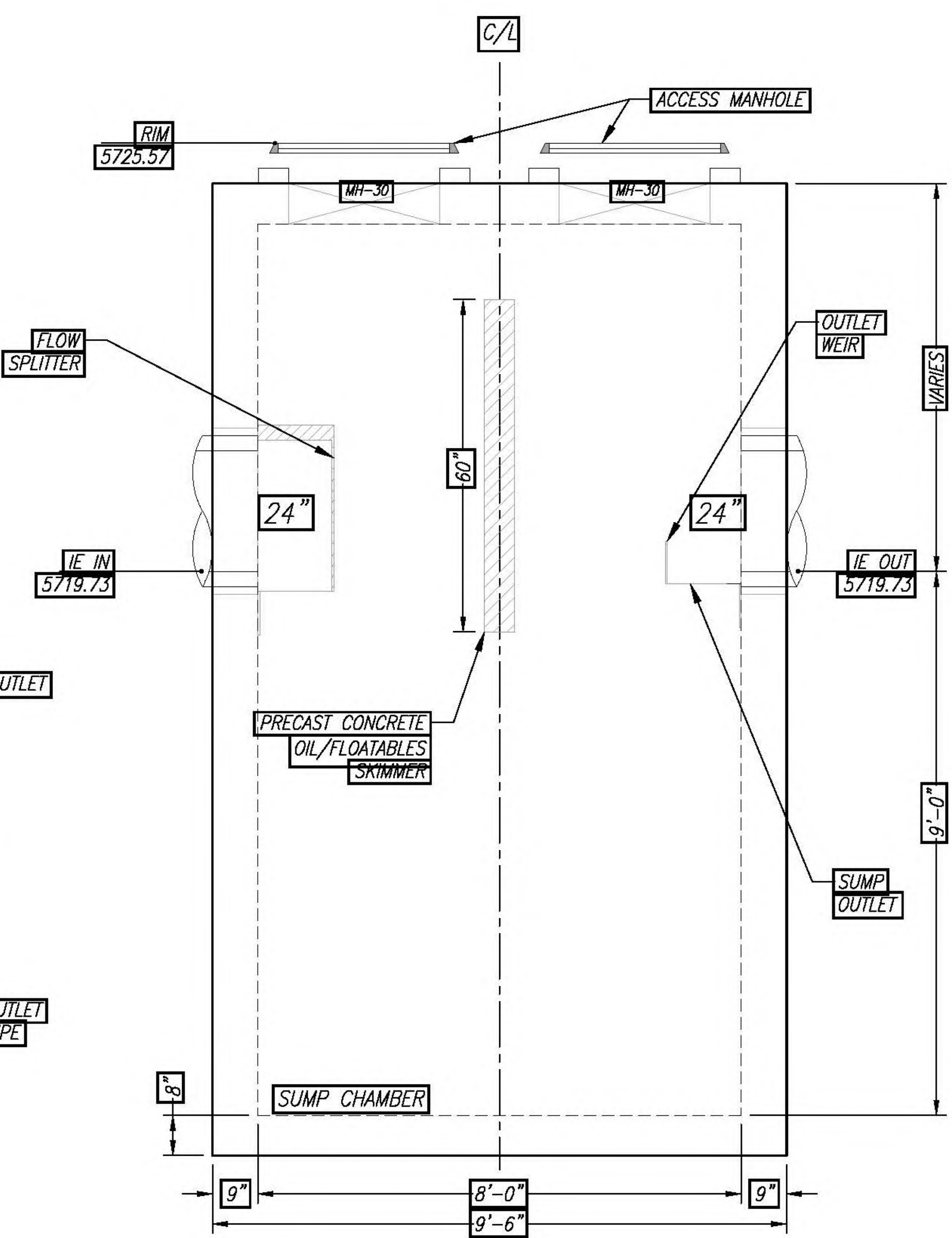
INSTALLATION NOTES

- CONTRACTOR TO PROVIDE ALL LABOR, EQUIPMENT, MATERIALS, AND INCIDENTALS REQUIRED TO OFFLOAD AND INSTALL THE UNIT AND APPURTENANCES IN ACCORDANCE WITH THIS DRAWING AND THE MANUFACTURER'S SPECIFICATIONS, UNLESS OTHERWISE STATED IN MANUFACTURER'S CONTRACT.
- MANUFACTURER RECOMMENDS A 6"-12" LEVEL ROCK BASE UNLESS SPECIFIED BY THE PROJECT ENGINEER. CONTRACTOR IS RESPONSIBLE FOR VERIFYING PROJECT ENGINEER'S RECOMMENDED BASE SPECIFICATIONS.
- ALL PIPES MUST BE FLUSH WITH INSIDE SURFACE OF CONCRETE. (PIPES CANNOT INTRUDE BEYOND FLUSH).
- ALL GAPS AROUND PIPES SHALL BE SEALED WATERTIGHT WITH A NON-SHRINK GROUT PER MANUFACTURER'S STANDARD CONNECTION DETAIL AND SHALL MEET OR EXCEED REGIONAL PIPE CONNECTION STANDARDS.
- CONTRACTOR RESPONSIBLE FOR INSTALLATION OF ALL RISERS, MANHOLES, AND HATCHES. ALL COVERS SHALL BE SHIPPED LOOSE. CONTRACTOR TO GROUT ALL MANHOLES AND HATCHES TO MATCH FINISHED SURFACE UNLESS SPECIFIED OTHERWISE.

PERFORMANCE DATA				
TREATMENT FLOW RATE (CFS)	5.05			
TSS REMOVAL EFFICIENCY	80%			
SUMP CHAMBER CAPACITY				
	DIAMETER (FT)	AREA (SF)	HEIGHT (FT)	TOTAL (CF)
SUMP	8.00	50.24	9.00	452.16



PLAN VIEW
NTS



ELEVATION VIEW
NTS

THE PRODUCT DESCRIBED
MAY BE PROTECTED BY
THE FOLLOWING PATENTS:
BIO CLEAN PATENTS
OR OTHER PATENTS
PENDING

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THE INFORMATION CONTAINED IN THIS DRAWING IS THE SOLE PROPERTY OF
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Bio Clean
A Forterra Company

SC-8-V
HYDRODYNAMIC SEPARATOR
STANDARD DETAIL

7/21/2024 WALTER

<u>Sheet No.</u>	<u>Index of Sheets</u>
C.1	Cover Sheet
C.2	Demolition Plan
C.3	Overall Site Plan
C.4	Sanitary Sewer Plan
C.5	Potable Water Plan
C.6	Storm Sewer & Shallow Utility Plan
C.7	Details-Sanitary Sewer
C.8	Details-Potable Water
C.9	Details-Site Electric
C.10	Details-Storm Sewer
C.11	Grading - Centennial Street
C.12	Grading - Access Drive & Site
C.13	Grading - Access Drive & Site
C.14	Enlarged Grading-ADA Ramps
C.15	Enlarged Grading-ADA Ramps
C.16	Driveway Point Table
C.17	Details-Civil Site
1-5	Subsurface Utility Engineering Maps (SGM, Inc.)

Scale: 1"=400'

EXISTING 1' CONDUIT
EXISTING 5' CONDUIT
EXISTING DISCHARGE FLOW LINE
EXISTING STORM SEWER/VALENT EASEMENT
BUILDING ENVELOPE
PROPERTY RIGHT-OF-WAY LINE
CONCRETE LINE
EXISTING FENCE
EXISTING UNDERGROUND ELECTRIC
EXISTING TELEPHONE
EXISTING GAS
EXISTING WATER MAIN/VALVE
EXISTING FIRE HYDRANT ASSEMBLY
EXISTING SEWER MAIN/VALVE
EXISTING SEWER SERVICE & CLEANOUT
EXISTING FIRE HYDRANT ASSEMBLY
EXISTING PROPOSED STREET SIGN
EXISTING/PROPOSED STREET LAMP-CARRIAGE
EXISTING/PROPOSED STREET LIGHT
EXISTING/PROPOSED STREET LAMP BOX
EXISTING/PROPOSED TRANSFORMER
EXISTING/PROPOSED ELEC. FILE/CABLE PEDESTAL
EXISTING/PROPOSED SWITCH CABINET
EXISTING/PROPOSED ELECTRIC VAULT
EXISTING/PROPOSED ELECTRIC METER (PED. WALL)

PROPOSED 1' CONDUIT
PROPOSED 5' CONDUIT
PROPOSED DISCHARGE FLOW LINE
PROPOSED STORM SEWER/ROCKS
PROPOSED 10" STORM DRAIN
PROPOSED 12" STORM DRAIN
PROPOSED 18" STORM DRAIN
PROPOSED 24" STORM DRAIN
PROPOSED 30" STORM DRAIN
PROPOSED SEGMENT CONTROL FENCING
PROPOSED TELEPHONE
PROPOSED ELECTRIC
PROPOSED GAS
PROPOSED FIBER OPTICS
PROPOSED WATER MAIN/VALVE
PROPOSED WATER SERVICE/SHUT OFF VALE
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PROJECT NO.
17015.04
Designer:
DJW
Drafter:
RIA

CONTRACTOR SHALL PROTECT AND PRESERVE ALL SURVEY CONTROL AND SURVEY MONUMENTATION ANY AND ALL MONUMENTS SHALL BE REPAIRED BY A PROFESSIONAL LAND SURVEYOR LICENSED IN THE PROJECT'S STATE OF THE CONTRACTORS EXPENSE.

FOR MOVING OFF THE JOB SITE THE CONTRACTOR SHALL NOTIFY THE ENGINEER OR THE OWNERS OF THE PROJECT PROJECT MANAGER AT LEAST 72 HOURS THROUGH THE CONSTRUCTION SITE.

EMPLOYER PHONE, TELEPHONE, WAX, SATISFACTION AND WASTE DISPOSAL SERVICE FOR THE SITE IS THE CONTRACTORS RESPONSIBILITY UNLESS OTHERWISE SPECIFIED.

CONTRACTOR SHALL REFER TO OTHER DRAWINGS ISSUED BY ARCHITECT, STRUCTURAL, ELECTRICAL, AND MECHANICAL ENGINEERS. ENSURE COORDINATION OF EXACT LOCATION AND DIMENSIONS OF BUILDINGS, EXITS, MANHOLES, UTILITY ENTRANCE LOCATIONS AND GRADES AROUND THE BUILDING. IMMEDIATELY NOTIFY THE PROJECT MANAGER IF THERE ARE ANY DISCREPANCIES.

BELOW GRADE WORK SHALL BE BACKFILLED INCLUDING BEDDING MATERIAL ABOVE THE SPRING LINE OF THE POLE (UNTIL THE CONSTRUCTION HAS BEEN INSPECTED AND APPROVED) FOR BACKFILLING BY THE APPOINTED INSPECTION AGENCY, PROJECT ENGINEER AND OWNER OWNERS CONSTRUCTION PROJECT MANAGER. AFTER WORK SHALL BE SCHEDULED AT LEAST 48 HOURS PRIOR.

THE CONTRACTOR SHALL PROVIDE A TRAFFIC CONTROL PLAN, NECESSARY TO COMPLETE THE WORK, ALL TRAFFIC CONTROL PLAN SHALL BE SUBJECT TO CONFORMANCE WITH LOCAL AND CONSTRUCTION ZONES SHALL BE IN CONFORMANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, AND ALL DISCREPANCIES HEREIN INCLUDING LOCAL AND STATE SUPPLEMENTATIONS. ADDITIONAL WORK IN THE RIGHT-OF-WAY OR OUTSIDE THE RIGHT-OF-WAY, PERMITS MAY BE NECESSARY AND SHALL BE THE CONTRACTORS RESPONSIBILITY.

ANY CONSTRUCTION ACTIVITY WITHIN A ROAD RIGHT-OF-WAY, CONTRACTOR SHALL PROVIDE ALL WARNING SIGNS, SIGNS, BARRICADES, FLAG MEN OR OTHER DEVICES NECESSARY TO PROVIDE FOR PUBLIC SAFETY IN CONFORMANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION ZONE TRAFFIC CONTROL. A CONSTRUCTION TRAFFIC CONTROL PLAN MUST BE SUBMITTED FOR REVIEW AND APPROVAL TO THE TOWNSHIP AND PROJECT ENGINEER, 7 DAYS PRIOR TO COMMENCING CONSTRUCTION WITH A PUBLIC RIGHT-OF-WAY.

UNIDENTIFIED HAZARDOUS MATERIALS OF ANY KIND ARE ENCOUNTERED IN THE WORK, THE CONTRACTOR SHALL IMMEDIATELY CEASE ALL CONSTRUCTION OPERATIONS AND NOTIFY THE PROJECT ENGINEER AND TOWNSHIP CONSTRUCTION PROJECT MANAGER IMMEDIATELY.

ALL DEBRIS RESULTING FROM CONSTRUCTION AND DEMOLITION SHALL BE HALLED OFF SITE AND DISPOSED OF PROPERLY AND LEGALLY AND SHALL BE CONSIDERED PART OF THE WORK.

CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE AWAY FROM ALL STRUCTURES IN ALL DIRECTIONS. DRAINAGE FACILITIES MAY NEED TO BE CONSTRUCTED & MAINTAINED AS NECESSARY TO PROTECT PUBLIC & WASTE PROPERTY.

CONSTRUCTION SHALL BE SUBJECT TO DISSEMINATION AND REVIEW BY THE TOWNSHIP AND THE PROJECT MANAGER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF OBSERVATIONS AND REVIEW OF INSTALLATIONS, OBSERVATIONS AND REVIEW OF CONSTRUCTION WORK IN PROGRESS ARE NOT TO BE CONSIDERED AS A GUARANTEE OR WARRANTY BY THE OBSERVER OF THE COMPLETED WORK OR GUARANTEES OR WARRANTIES REMAIN WITHIN THE CONTRACT.

THE REMOVED AREAS MUST BE RECLAIMED WITH VEGETATION AS DIRECTED BY THE PROJECT LANDSCAPE ARCHITECT OR OWNERS CONSTRUCTION PROJECT MANAGER.

CONTRACTOR SHALL TAKE APPROPRIATE PRECAUTIONS TO SIGNIFICANTLY REDUCE POTENTIAL POLLUTION FROM CONSTRUCTION ACTIVITIES, INCLUDING VEHICLE FUELING & MAINTENANCE, AND STORAGE AND USE OF CHEMICALS & FERTILIZERS. THE CONTRACTOR SHALL IDENTIFY BEST MANAGEMENT PRACTICES FOR HANDLING OF FUEL, OIL, FLUIDS AND LUBRICANTS, PREVENTION AND RESPONSE PROCEDURES TO THE TOWNSHIP AND PROJECT ENGINEER PRIOR TO ANY ACTIVITY AT THE PROJECT SITE.

CONTRACTOR SHALL VERIFY EXISTING PAVT OR MULTIPLE INTERSECTIONS PRIOR TO CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER.

THE CITY WILL PROVIDE THE CONTRACTOR WITH A TAX EXEMPT FORM FOR USE IN PURCHASING OF MATERIALS AND SUPPLIES.

PROJECT INFORMATION SHOWN AS PROVIDED BY THE MANUFACTURER, THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE INFORMATION FOR ANY FACTORS, RECOMMENDATIONS AND SPECIFICATIONS FOR THE INSTALLATION OF ALL MATERIALS WITH SPECIAL ATTENTION TO PROPER SELECTION OF MATING SURFACES. THE CONTRACTOR SHALL IMMEDIATELY CONTACT THE ENGINEER IF ANY DISCREPANCIES ARE FOUND BETWEEN THE CONTRACT DOCUMENTS AND THOSE SHOWN ON THE PLAN.

ALL SIGNS REMOVED FROM THE RIGHT-OF-WAY WILL BE RETURNED TO THE CITY'S STORAGE DEPOSIT. CONTRACTOR TO PROVIDE PHOTOS OF THE AREAS AND REMOVED SIGNS THAT ARE REMOVED DURING CONSTRUCTION.

ANY AND ALL OTHER MATERIAL TRACKED OR OTHERWISE DEPOSITED ON THE ROADWAY SHALL BE REMOVED IMMEDIATELY AS ORDERED BY THE DEPARTMENT INSPECTOR.

CONTRACTOR SHALL PROVIDE ON SITE SANITARY FACILITIES UNLESS FACILITIES ARE ALREADY PROVIDED THAT ARE REASONABLY CLOSE TO THE CONSTRUCTION SITE.

CONTRACTOR SHALL MAINTAIN ACCESS TO AFFECTED PROPERTY OWNERS AT ALL TIMES DURING CONSTRUCTION AT NO ADDITIONAL COST.

PERMITTEE SHALL DESIGNATE A CERTIFIED TRAFFIC CONTROL SUPERVISOR (TCS) TO MANAGE CONSTRUCTION AND ENSURE SAFETY OF OPERATIONS DURING ACTIVITIES. THE TCS SHALL BE A TOWNSHIP TRAFFIC CONTROL SUPERVISOR THAT IS AVAILABLE WHENEVER WORK IS IN PROGRESS.

ADDITIONAL NOTES ARE LOCATED THROUGHOUT THE CONSTRUCTION DOCUMENTS.

NOTES

PRIOR TO BIDDING, CONTRACTOR SHALL VISIT THE SITE TO BECOME FAMILIAR WITH THE EXISTING CONDITIONS OF THE PROJECT AREA AND ANTICIPATED DEMOLITION REQUIREMENTS.

CONTRACTOR TO PROTECT ALL UTILITY, PAVING, BUILDINGS, ETC. OUTSIDE OF LIMITS OF PROPOSED CONSTRUCTION DURING DEMOLITION ACTIVITIES.

EXISTING PAVEMENT AND CONCRETE WITHIN THE CONSTRUCTION LIMITS SHALL BE REMOVED UNLESS OTHERWISE NOTED.

EXISTING HERBACEOUS VEGETATION AND TOPSOIL SHALL BE STRIPPED TO A MINIMUM DEPTH OF 12" AND REMOVED FROM SITE OR STORED FOR FUTURE USE IN LANDSCAPING AREAS. TREES DESIGNATED FOR REMOVAL SHALL BE COMPLETELY REMOVED, INCLUDING STUMPS AND ROOT SYSTEMS.

CONTRACTOR SHALL REFER TO DEMOLITION PLANS FOR DETAILS ON LIMITS OF DEMOLITION AND THE SITE'S DRAINAGE GRADING PLANS FOR PERMETER DRAINAGE CONSTRUCTION.

PRIOR TO DEMOLITION WORK, EROSION CONTROL DEVICES ARE TO BE INSTALLED. EROSION CONTROL MEASURES SHALL BE ADJUSTED AS DEMOLITION AND CONSTRUCTION SEQUENCING WARRANTS.

DEMOLITION WORK ON THE CONSTRUCTION SITE SHALL BE IN CONFORMANCE WITH LOCAL STANDARDS AND ORDINANCES.

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17.	CONTRACTOR TO NOTIFY UTILITY USERS AT LEAST SEVENTY TWO (72) HOURS IN ADVANCE OF ANY UTILITY SHUT DOWN. MORE NOTICE MAY BE REQUIRED FOR MAJOR CONNECTIONS.	
18.	CONTRACTOR SHALL ADJUST ALL MANHOLE RIMS, GRATES, VALVE/CURB/IRRIGATION BOXES, METER PITS, ETC. TO 1/4" BELOW FINISH GRADE ELEVATION. FIRE HYDRANTS SHALL BE ADJUSTED SUCH THAT THE BOTTOM OF FLANGE IS 2" ABOVE THE FINISH GRADE WHILE MAINTAINING MINIMUM COVER REQUIREMENTS.	
	<u>STANDARD SPECIFICATIONS:</u>	
1.	STORM SEWER PIPE MATERIALS SHALL BE WATER TIGHT HIPS DOUBLE WALL, SMOOTH INTERIOR PIPE (ACS N-12/10) OR ENGINEER APPROVED (DUAL) OR RUBBER GASKET REINFORCED CONCRETE PIPE (RCP) UNLESS OTHERWISE SPECIFIED IN THE PLANS.	
2.	STORM SEWER PIPE SHALL BE BEHIND, INSTALLED, AND BACKFILLED IN ACCORDANCE WITH THE DETAILS INCLUDED IN THE PLANS, MANUFACTURERS INSTALLATION REQUIREMENTS, AND/OR STANDARD DETAILS INCLUDED BY REFERENCE.	
3.	ALL CAST-IN PLACE CONCRETE DRAINAGE STRUCTURES SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4,000 PSI, TYPE OR (EQUIVALENT) SUSTAIN RESISTANT CONCRETE (UNLESS OTHERWISE SPECIFIED) WITH KRYDEX CRYSTALLINE ADDITIVE AND SHALL CONFORM TO THE LOCAL TOWNSHIP'S SPECIFICATIONS.	
	SITE BENCH-MARK:	
	CONTROLS ARE SHOWN AT ONE FOOT INTERVALS. HORIZONTAL AND VERTICAL CONTROL IS BASED CITY DATUM.	
	THE THREE (3) UTILIZED CONTROL POINTS ARE:	
	• #1 N+2571.880 E+1688.479	ELEV:573.962 1.5' AC+ILEG
	• #10 N+42813.563 E+16760.363	ELEV:571.810 1.25' US+LS#15710
	• #14 N+42813.534 E+15985.320	ELEV:573.960 1.5' AC+ILEG
	<u>REQUIRED SHOP DRAWINGS:</u>	
	• AGGREGATES	
	• CONCRETE	
	• STEEL	
	• ASPHALT MIX DESIGN	
	• FLOWABLE FILL	
	• ALL UTILITY STRUCTURES, PIPE & FITTINGS	
	• ANY OTHERS INDICATED WITHIN THE PLANS OR AS REQUIRED BY THE TOWNSHIP OR THE ENGINEER.	
	<u>UTILITY REFERENCES/OWNERS:</u>	
	ELECTRIC: DOUG HAZARD	CITY ELECTRIC SYSTEM 970.384.6553
	FIBER OPTIC: ERIC ARMETTE	CITY BROADBAND 970.384.6166
	WATER/GAS, SEWER: MIKE HOFFMAN	CITY WWW 970.384.6388
	STORM/IRRIGATION: TERRI PARTCH	CITY ENGINEERING 970.384.6425
	STREETS: JANE VELASQUEZ	CITY STREETS 970.384.6379
	NATURAL GAS: JASON COX	BLACKHILLS ENERGY 970.456.2117
	TELECOMMUNICATIONS: MICHAEL JOHNSON	COMCAST 970.930.4113
	TELECOMMUNICATIONS: JASON SHARPE	CENTURY LINK 970.384.0228
	<u>MATERIAL TESTING/QUALITY ASSURANCE NOTES:</u>	
	1. ALL TESTS AND INSPECTIONS AS REQUIRED BY CODES, ORDINANCES OR FOR MATERIAL CERTIFICATION AND CONNECTIONS AS NOTED IN THE PLANS AND SPECIFICATIONS SHALL BE PAID BY THE CONTRACTOR, WHEN NOT SPECIFIED AS AN INDIVIDUAL LINE ITEM IN THE BID. HOWEVER, THE COSTS OF TESTING PAID BY THE CONTRACTOR AND SHALL BE INCLUDED IN THE COST OF THE ASSOCIATED ITEM OF WORK.	
	2. TESTS SHALL BE MADE BY AN INDEPENDENT TESTING LABORATORY AND BE ACCEPTABLE TO THE CITY, PROJECT MANAGER AND PROJECT ENGINEER, EXCEPT AS OTHERWISE PROVIDED. SAMPLING OF MATERIALS, TESTING METHODS AND TESTING EQUIPMENT SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS OR THE TENTATIVE METHOD OF THE REFERENCE(S) SPECIFICATIONS.	
	3. CONTRACTOR TO NOTIFY ENGINEER OF RECORD WHEN TOSPOILS/ORGANICS HAVE BEEN STRIPPED TO CONFIRM REMOVAL IS ADEQUATE.	
	4. ENGINEER OF RECORD AND TESTING AGENT TO FIELD VERIFY SUBGRADE BY WITNESSING A LOADED TANDDEM WHEEL ROLL TEST OF SUBGRADE FOR CURB/ASPHALT.	
	5. TESTING FREQUENCY SHALL CONFORM TO THE CDOT FIELD MATERIALS MANUAL, LATEST EDITION, OR THE FOLLOWING, WHO-EVER IS MORE FREQUENT, AT A MINIMUM, TESTING AGENT TO PERFORM NUCLEA DENSITY TESTING OF THE FOLLOWING:	
5.a.	TRENCHES:	EVERY 100 FEET FOR THE LOWER 12-INCHES OF FILL AND EACH 24-INCHES OF FILL THEREAFTER.
5.b.	STRUCTURAL FILL:	EVERY 100 FEET, OR 3 TESTS PER BLOCK MIN, FOR EACH 2- FEET OF FILL.
5.c.	SUBGRADE:	EVERY 100 FEET, OR 3 TESTS PER BLOCK MIN.
5.d.	BASE COURSE:	EVERY 100 FEET, OR 3 TESTS PER BLOCK MIN.
5.e.	ASPHALT:	EVERY 100 FEET, OR 3 TESTS PER BLOCK MIN.
	6. TESTING AGENT SHALL OBTAIN CONTACT COUNTEERS FOR TESTING EACH DAY CONCRETE IS POURED.	
	7. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE MATERIAL AND COMPACTION TESTING WITH THE PROJECT ENGINEER AND TESTING AGENT AND ADHERING TO THE TESTING SCHEDULE DEFINED.	
	8. CONTRACTOR SHALL PROVIDE THE CITY AND ENGINEER OF RECORD WITH COPIES OF ALL TEST RESULTS AND REPORTS CONFIRMING COMPLIANCE.	
	<u>CONTACT INFORMATION:</u>	
3.a.	<u>QUALITY ASSURANCE - PROJECT ENGINEER/ENGINEER OF RECORD:</u>	
	DERIC J. WALTER, PE	
	BOUNCAIRES UNLIMITED, INC.	
	970.945.6252	
3.b.	<u>MATERIALS TESTING - TESTING AGENT:</u>	
	BY THE CONTRACTOR AND AS APPROVED BY THE TOWNSHIP.	

LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

1

By	
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[illegible]

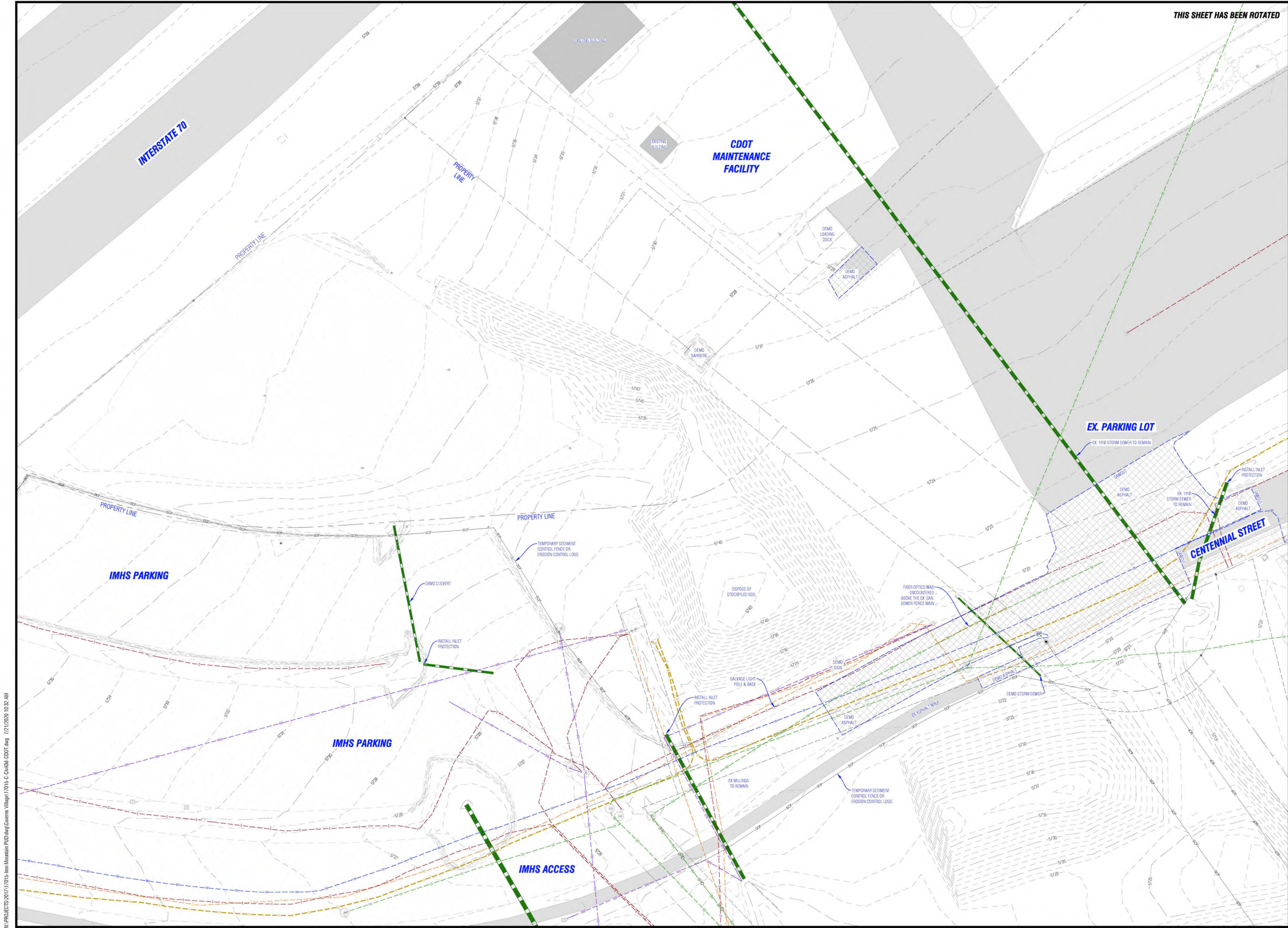
Revision

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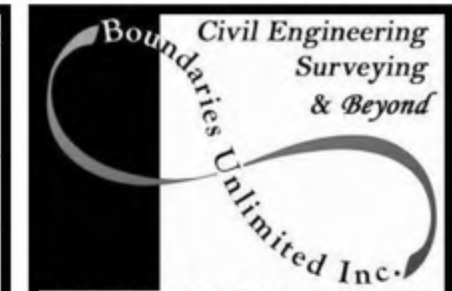
No.	Date
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PROJECT
17015
Design
DJW
Drafting

BUI
Date:
7/21/2
File Na



THIS SHEET HAS BEEN ROTATED



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Glenwood Springs, CO 81601
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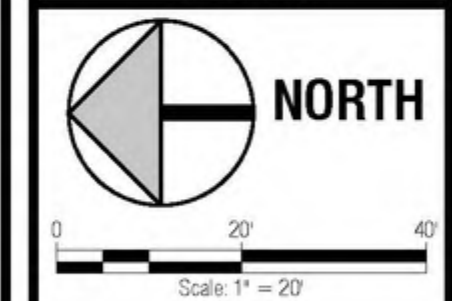
Subsurface Utility Engineering (SUE)
Required Project: **Yes**

If "Yes", SUE Certification by:
Firm: **2018-208.004**
Job #: **SGM, Inc.**
Date: **July 2020**

Engineer or Surveyor Seal



Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766



LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

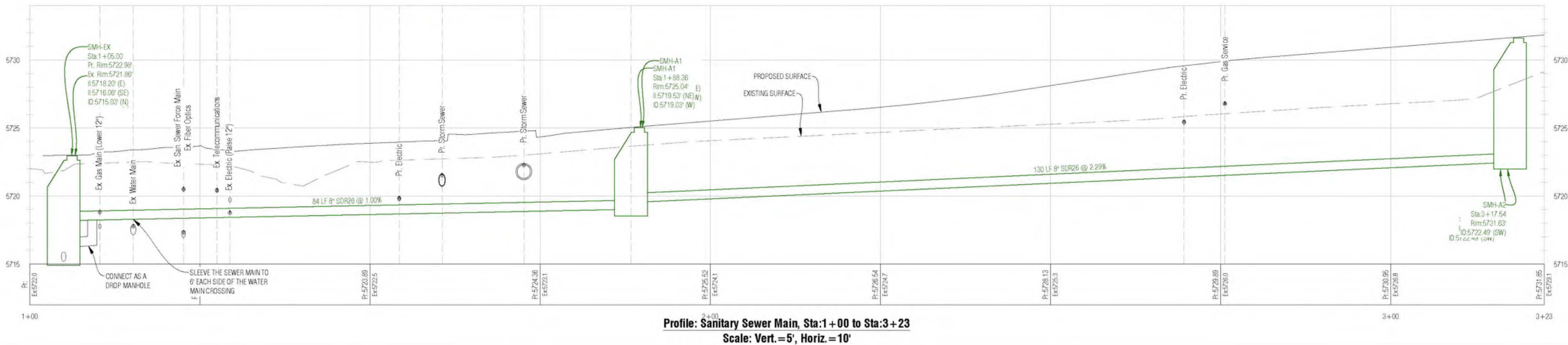
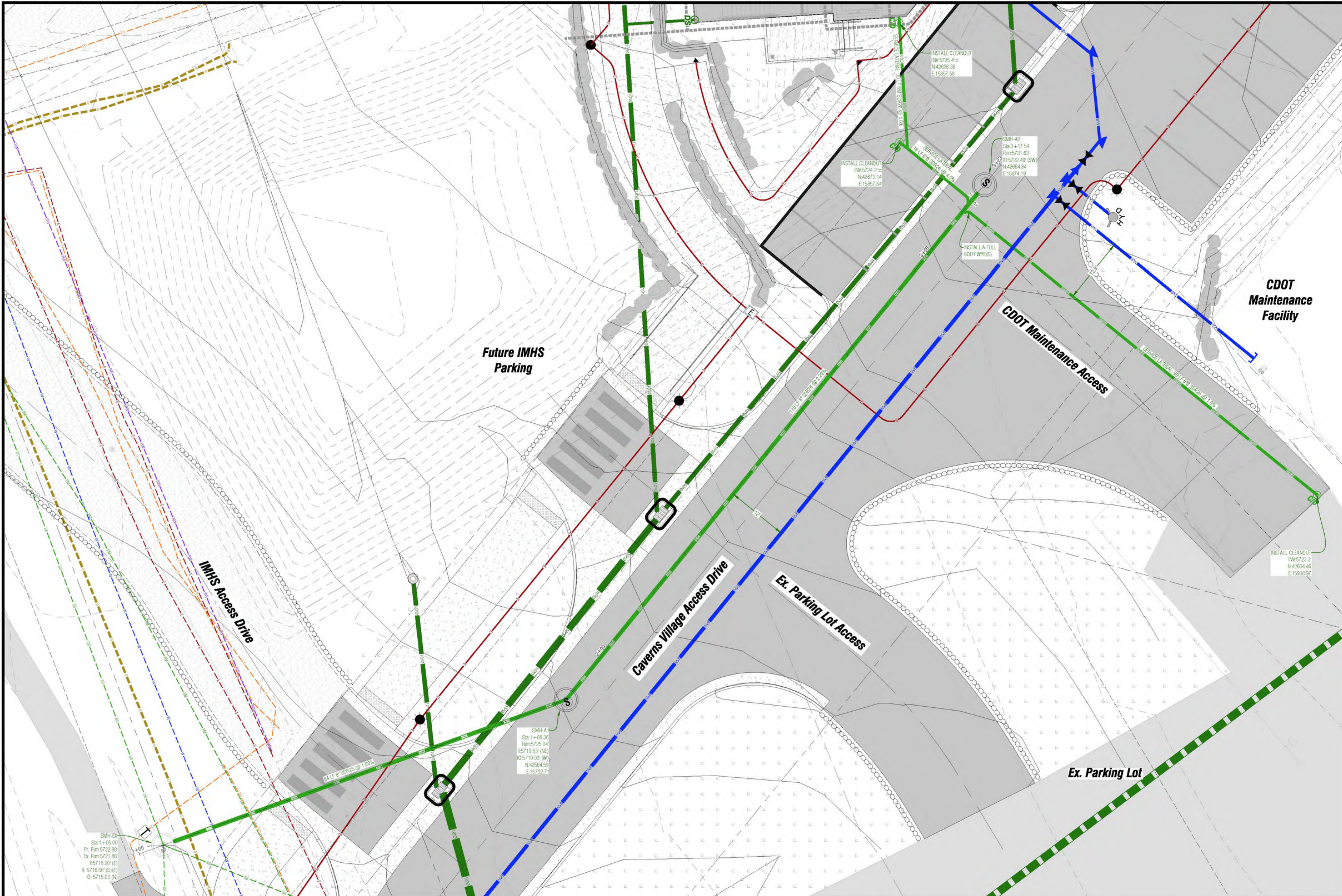
Caverns Village
Demolition Plan
Construction Plan Set

Revisions	By	Date
1		
2		
3		
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8		
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PROJECT NO. 17016.04 Design: DJW Check: BU Date: 7/21/20	Sheet C.2
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File Name: 17016-c-chris3-cdot.dwg

N:\PROJECTS\2017\17015 Iron Mountain PUD\Avail Caverns Village\17015-C Sewer CDOT.dwg 7/21/2020 10:32 AM



Civil Engineering
Surveying
& Beyond

Boundaries Unlimited Inc.

923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
(T) 970.945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: **Yes**

If "Yes", SUE Certification by:
Firm: **2018-208.004**
Job #: **SGM, Inc.**
Date: **July 2020**

Engineer or Surveyor Seal

COLORADO LICENSED
1/21/20
37110
PROFESSIONAL ENGINEER

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

NORTH

0 10' 20'
Scale: 1" = 10'

811

Know what's below.
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LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village
Sanitary Sewer Plan
Construction Plan Set

By

Revision

Date

No.

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PROJECT NO.
17015-04

DATE
07/21/20

BY
BU

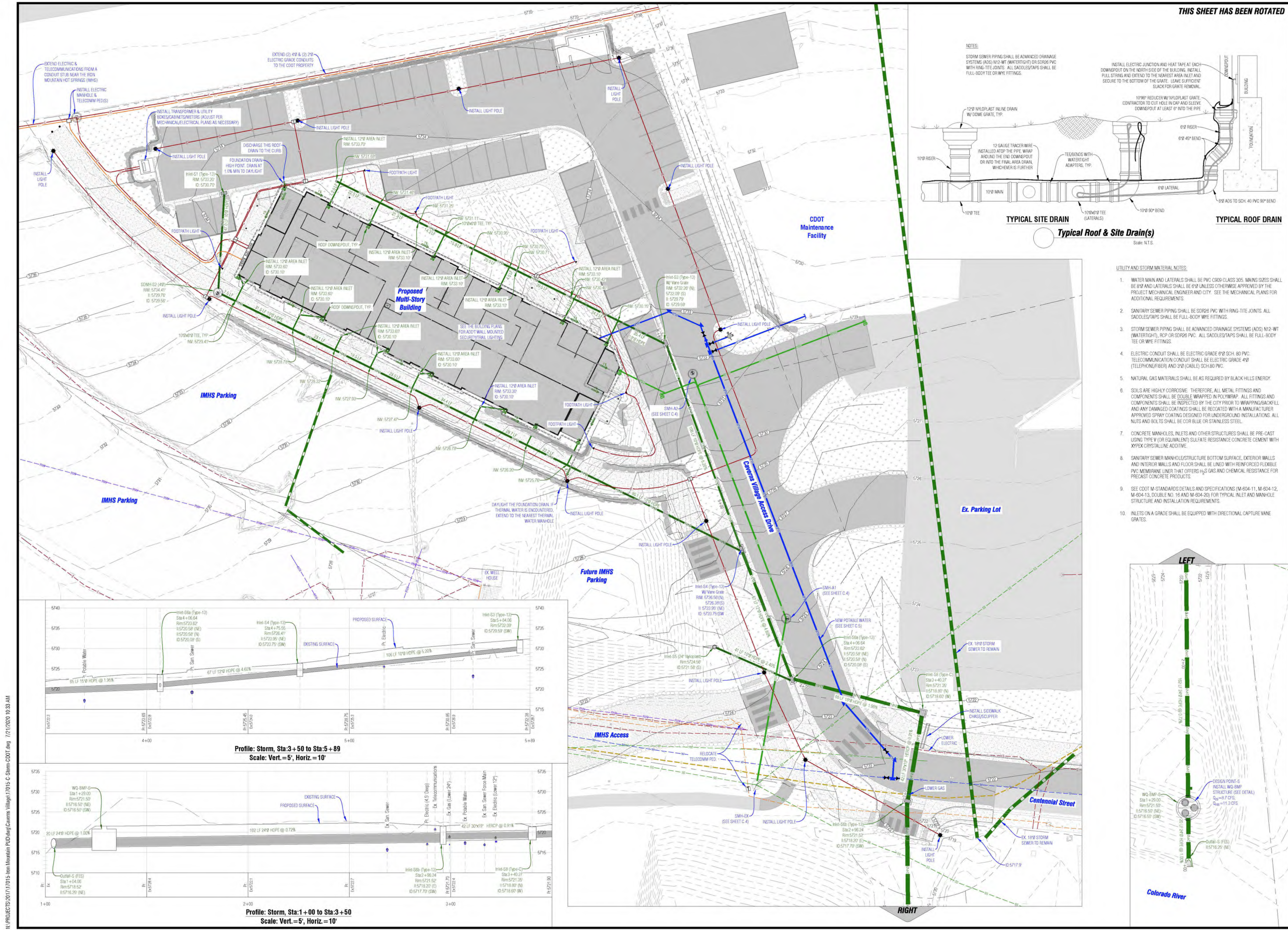
DATE
7/21/20

Sheet

C.4

File Name: **17015-c-sewer-cdot.dwg**

Packet Page 412

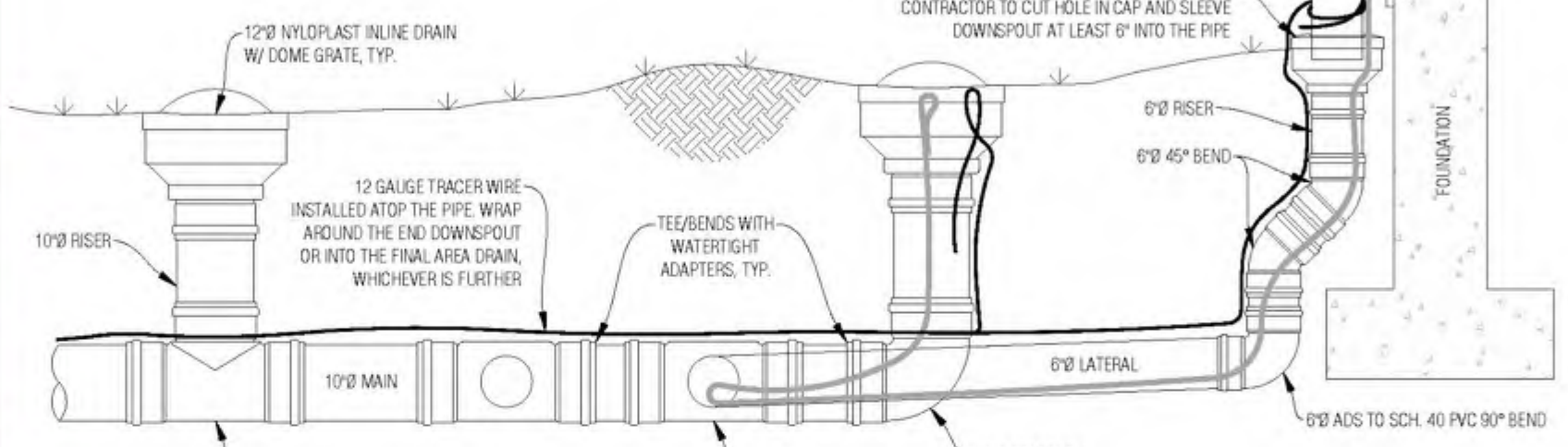


THIS SHEET HAS BEEN ROTATED

NOTES

STORM SEWER PIPING SHALL BE ADVANCED DRAINAGE SYSTEMS (ADS) N12-WT WATER TIGHT OR SD268 PVC WITH RING-TITE JOINTS. ALL SADDLES/TAPS SHALL BE FULL-BODY TEE OR WYE FITTINGS.

INSTALL ELECTRIC JUNCTION AND HEAT TAPE AT EACH DOWNSPOUT ON THE NORTH SIDE OF THE BUILDING. INSTALL PULL STRING AND EXTEND TO THE NEAREST AREA INLET AND SECURE TO THE BOTTOM OF THE GRATE. LEAVE SUFFICIENT SLACK FOR GRATE REMOVAL.



Typical Roof & Site Drain(s)

Scale: N.T.S.

UTILITY AND STORM MATERIAL NOTES:

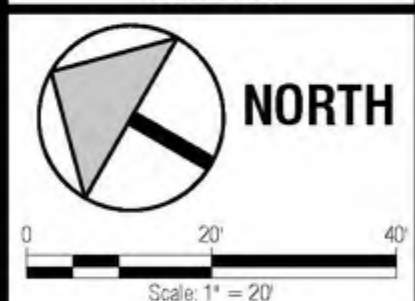
1. WATER MAIN AND LATERALS SHALL BE PVC C509 CLASS 305. MAINS SIZES SHALL BE 6" AND LATERALS SHALL BE 4" UNLESS OTHERWISE APPROVED BY THE PROJECT MECHANICAL ENGINEER AND CITY. SEE THE MECHANICAL PLANS FOR ADDITIONAL REQUIREMENTS.
2. SANITARY SEWER PIPING SHALL BE SD268 PVC WITH RING-TITE JOINTS. ALL SADDLES/TAPS SHALL BE FULL-BODY WYE FITTINGS.
3. STORM SEWER PIPING SHALL BE ADVANCED DRAINAGE SYSTEMS (ADS) N12-WT (WATER TIGHT), RCP OR SD268 PVC. ALL SADDLES/TAPS SHALL BE FULL-BODY TEE OR WYE FITTINGS.
4. ELECTRIC CONDUIT SHALL BE ELECTRIC GRADE 4" SCH. 40 PVC. TELECOMMUNICATION CONDUIT SHALL BE ELECTRIC GRADE 4" (TELEPHONE/FIBER) AND 3" (CABLE) SCH. 40 PVC.
5. NATURAL GAS MATERIALS SHALL BE AS REQUIRED BY BLACK HILLS ENERGY.
6. SOILS ARE HIGHLY CORROSIVE. THEREFORE, ALL METAL FITTINGS AND COMPONENTS SHALL BE DOUBLE WRAPPED IN POLYWRAP. ALL FITTINGS AND COMPONENTS SHALL BE INSPECTED BY THE CITY PRIOR TO WRAPPING/BACKFILL AND ANY DAMAGED COATINGS SHALL BE RECOATED WITH A MANUFACTURER APPROVED SPRAY COATING DESIGNED FOR UNDERGROUND INSTALLATIONS. ALL NUTS AND BOLTS SHALL BE COR-BLUE OR STAINLESS STEEL.
7. CONCRETE MANHOLES, INLETS AND OTHER STRUCTURES SHALL BE PRE-CAST USING TYPE V (OR EQUIVALENT) SULFATE RESISTANCE CONCRETE CEMENT WITH XYPEX CRYSTALLINE ADDITIVE.
8. SANITARY SEWER MANHOLE/STRUCTURE BOTTOM SURFACE, EXTERIOR WALLS AND INTERIOR WALLS AND FLOOR SHALL BE LINED WITH REINFORCED FLEXIBLE PVC MEMBRANE LINER THAT OFFERS H₂S GAS AND CHEMICAL RESISTANCE FOR PRECAST CONCRETE PRODUCTS.
9. SEE CDOT M-STANDARDS DETAILS AND SPECIFICATIONS (M-604-11, M-604-12, M-604-13, DOUBLE NO. 16 AND M-604-20) FOR TYPICAL INLET AND MANHOLE STRUCTURE AND INSTALLATION REQUIREMENTS.
10. INLETS ON A GRADE SHALL BE EQUIPPED WITH DIRECTIONAL CAPTURE VANE GRATES.

Boulevard Engineering & Surveying, Inc.
923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
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Subsurface Utility Engineering (SUE)
Required Project: Yes
If "Yes", SUE Certification by:
Firm: 2018-208.004
Job #: SUE, Inc.
Date: July 2020



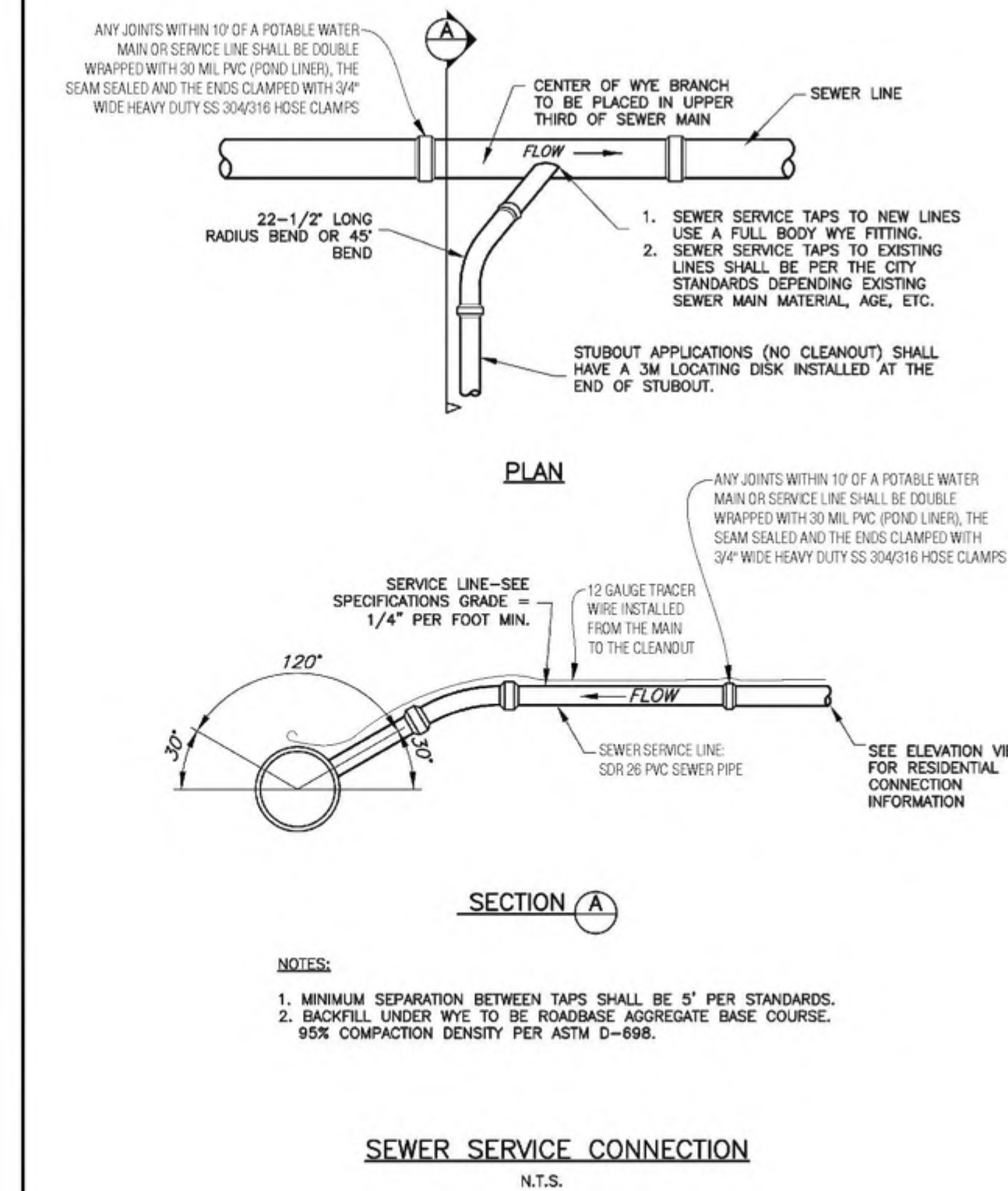
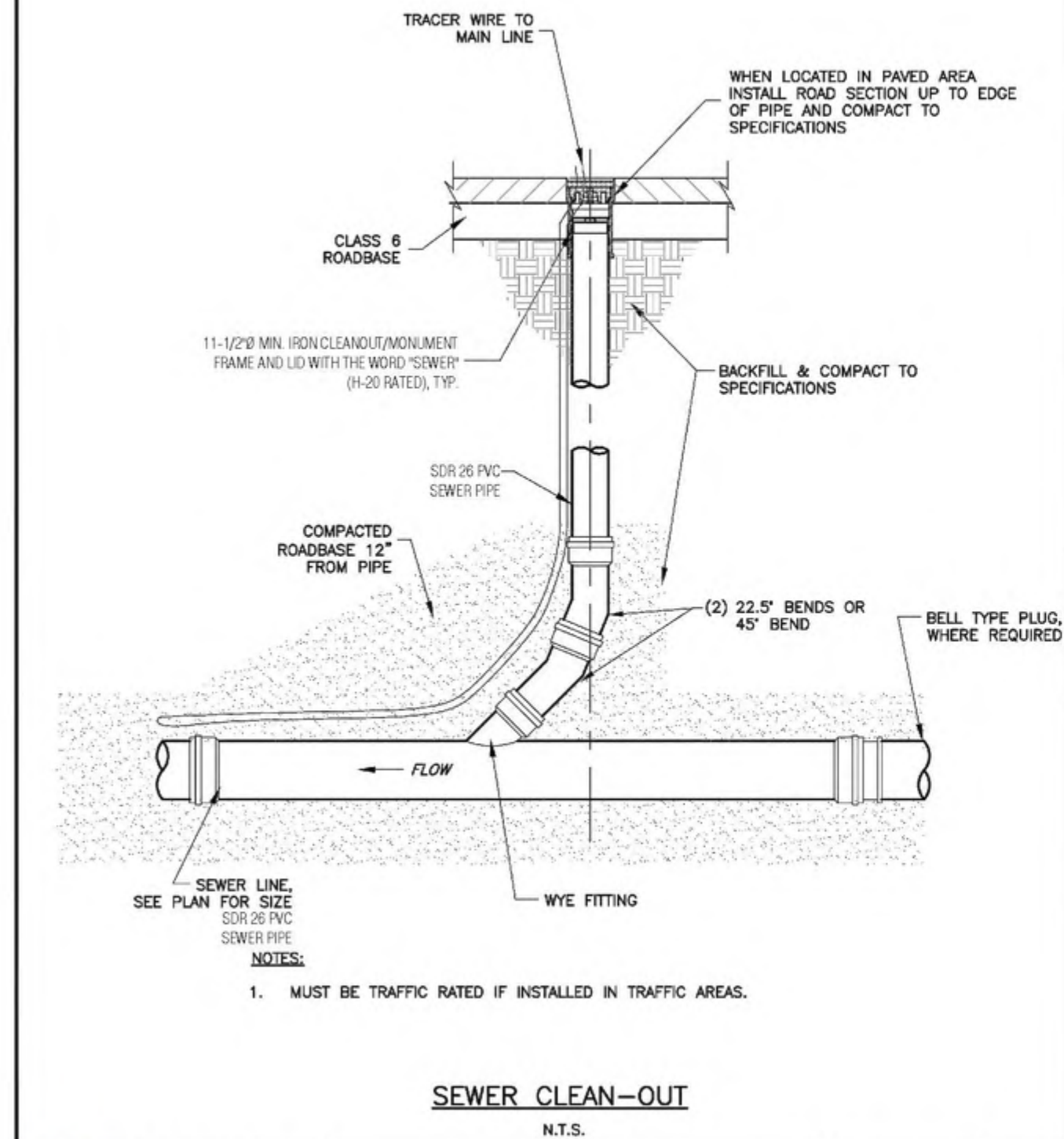
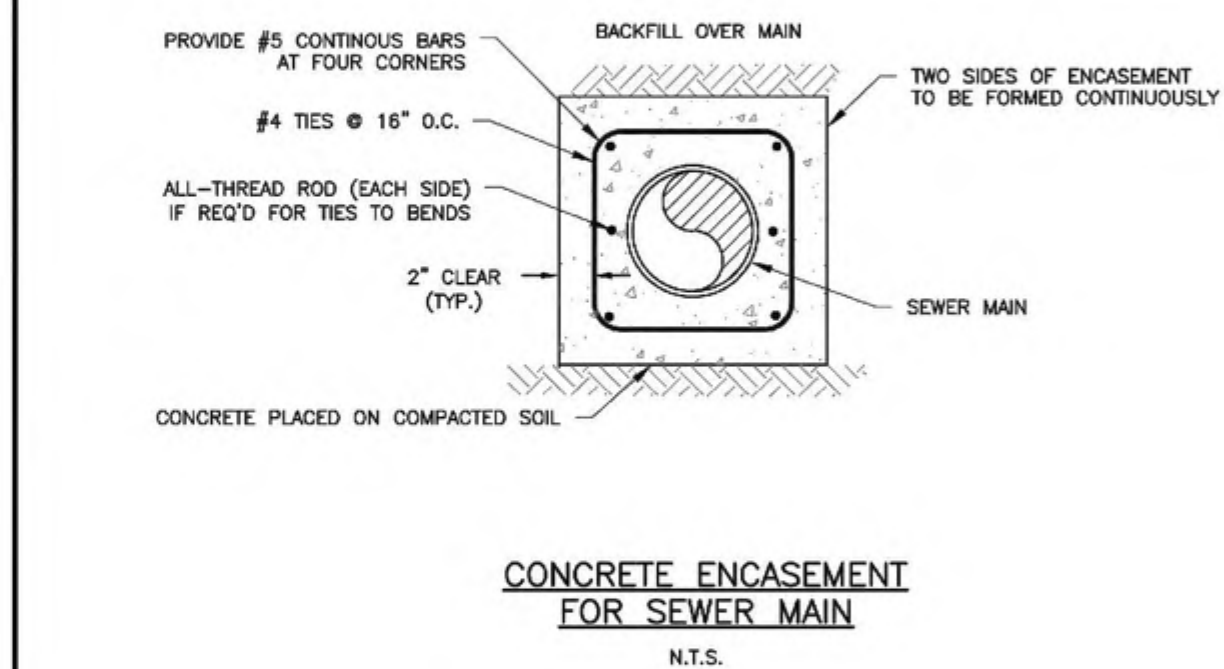
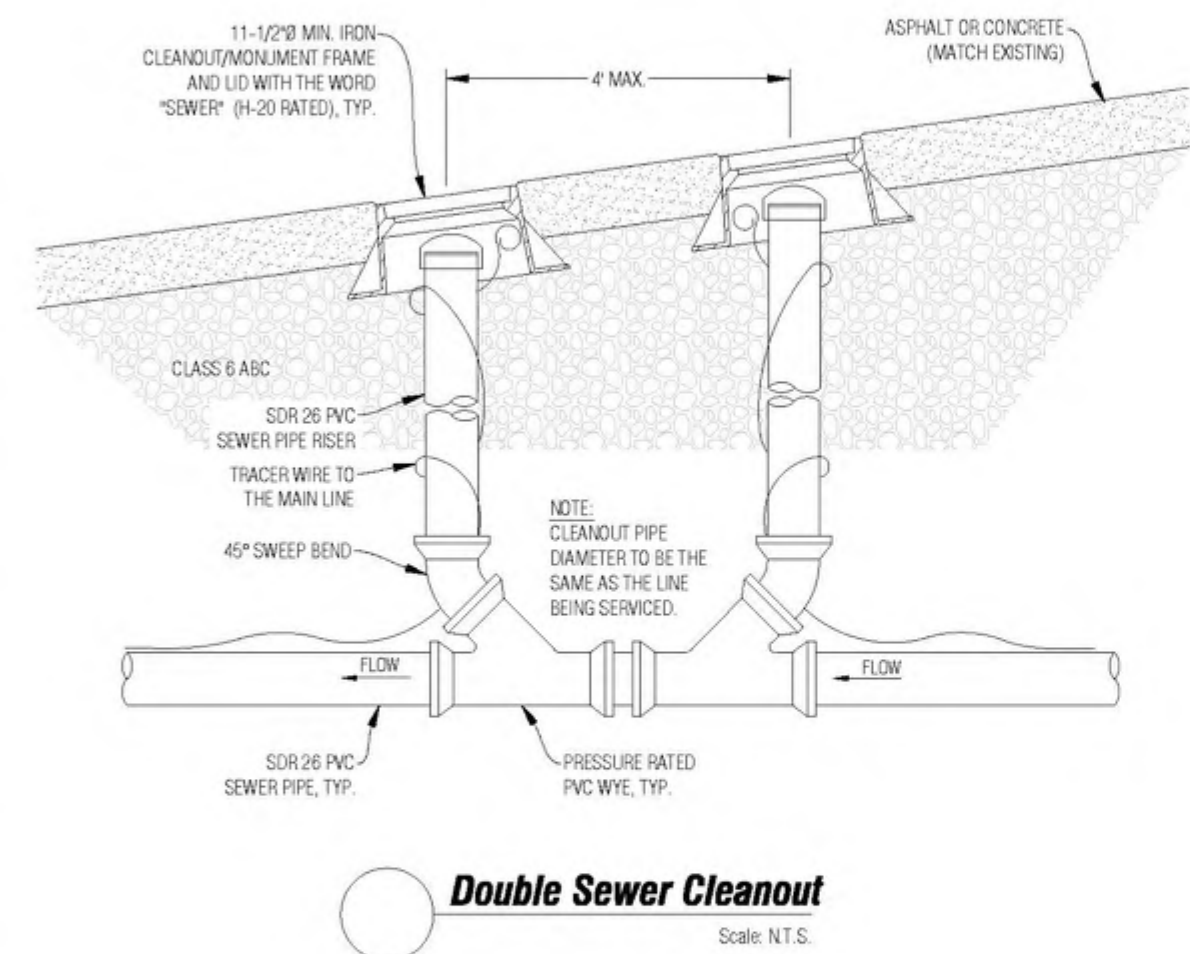
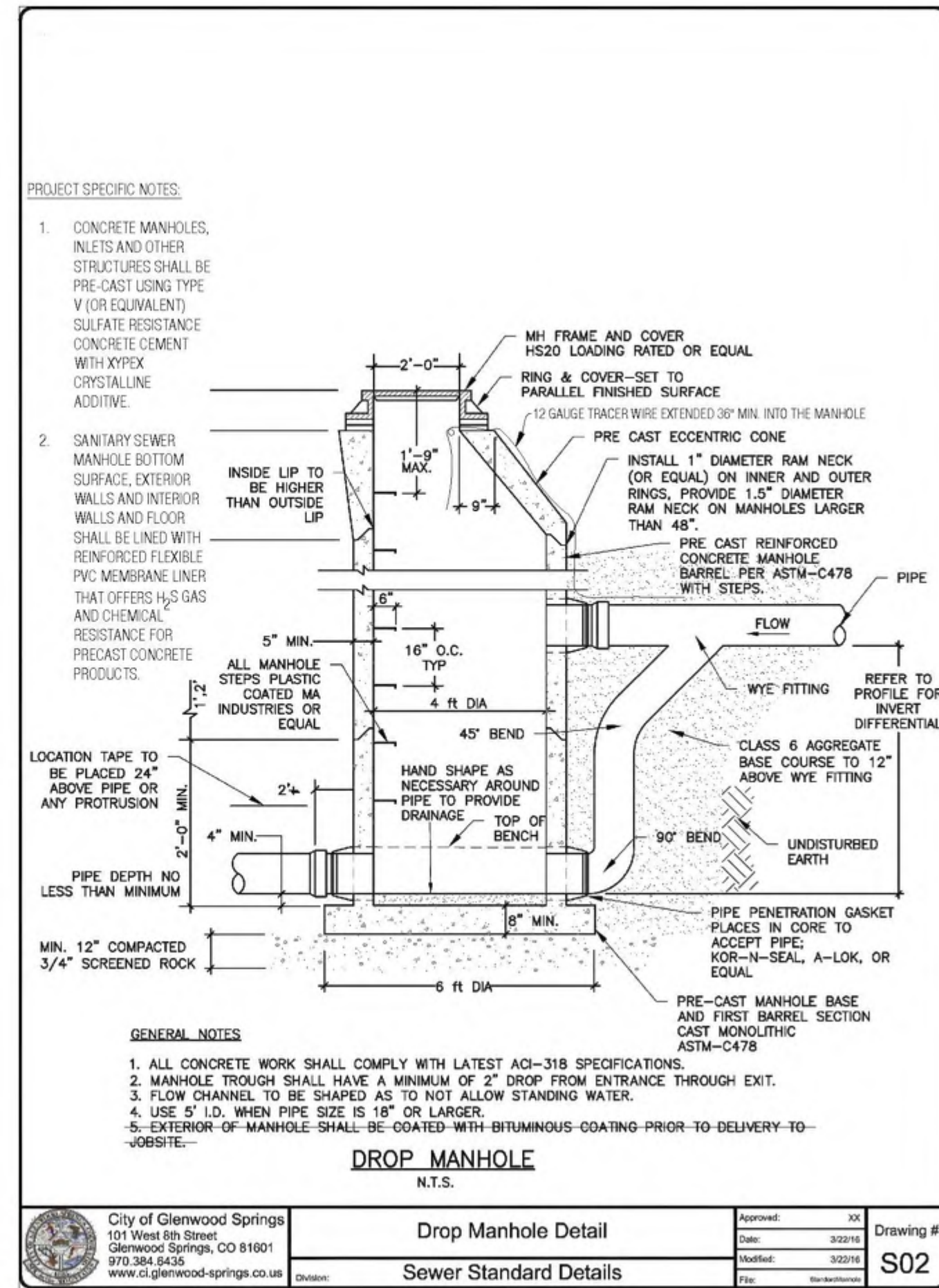
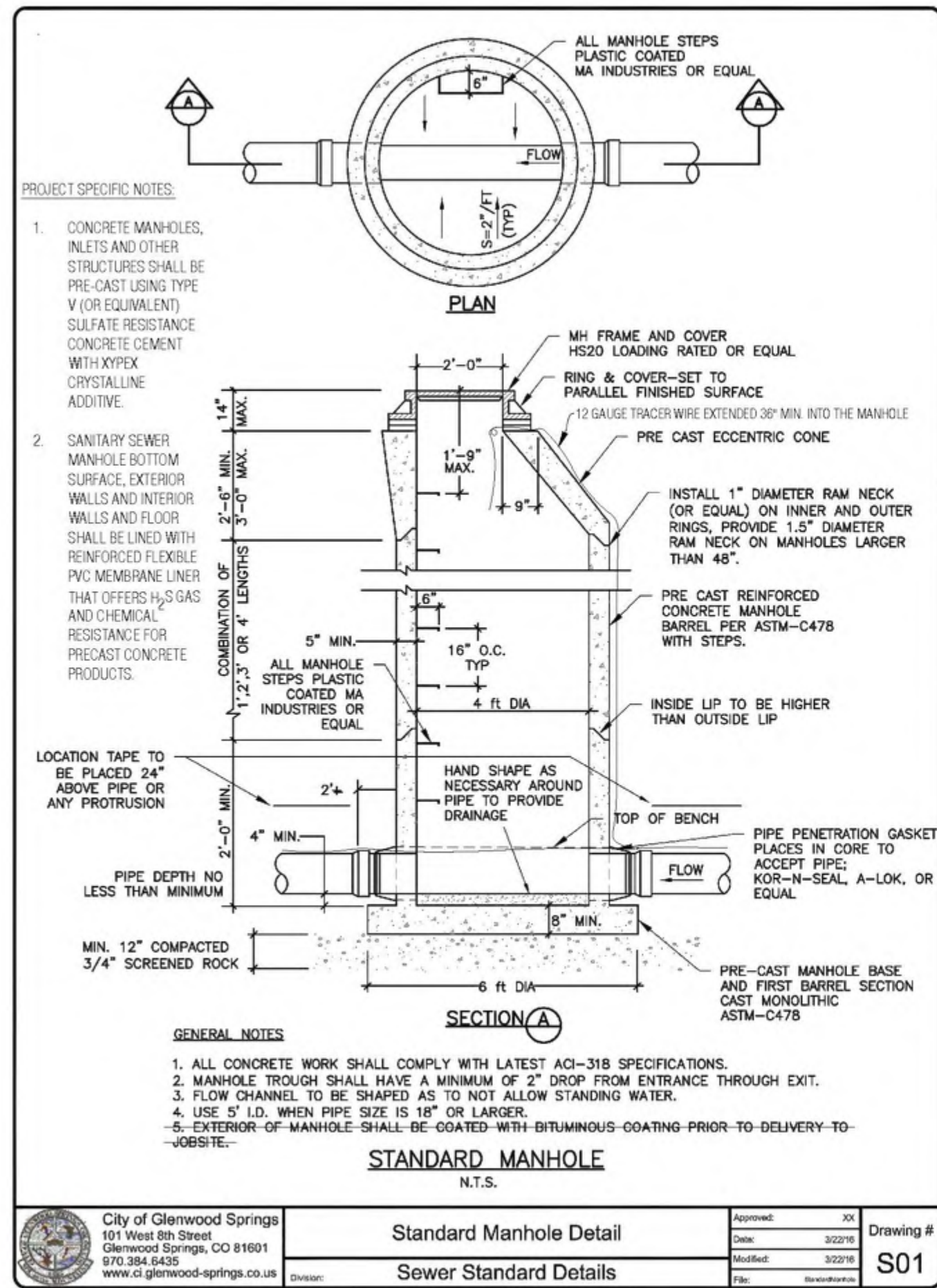
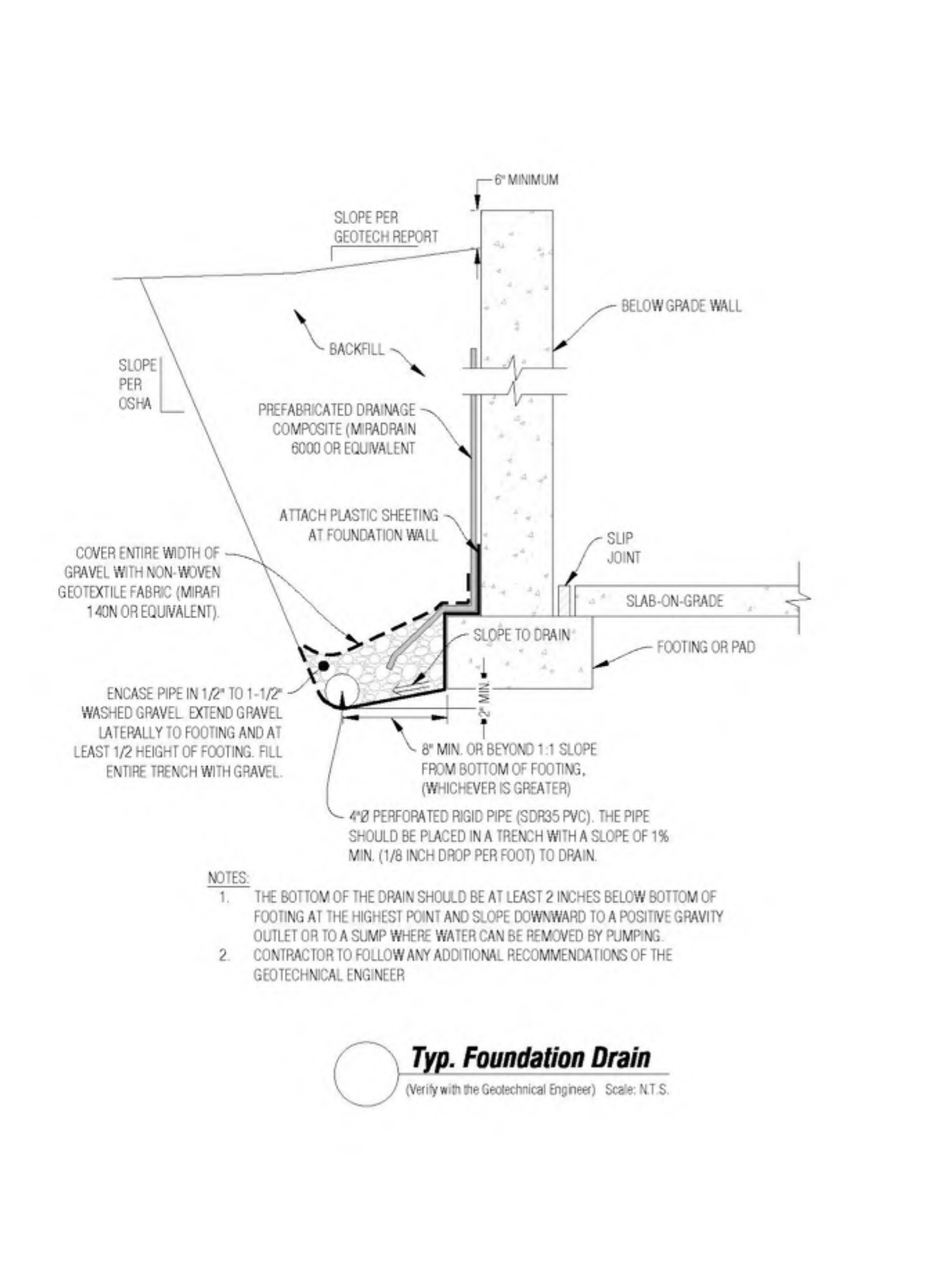
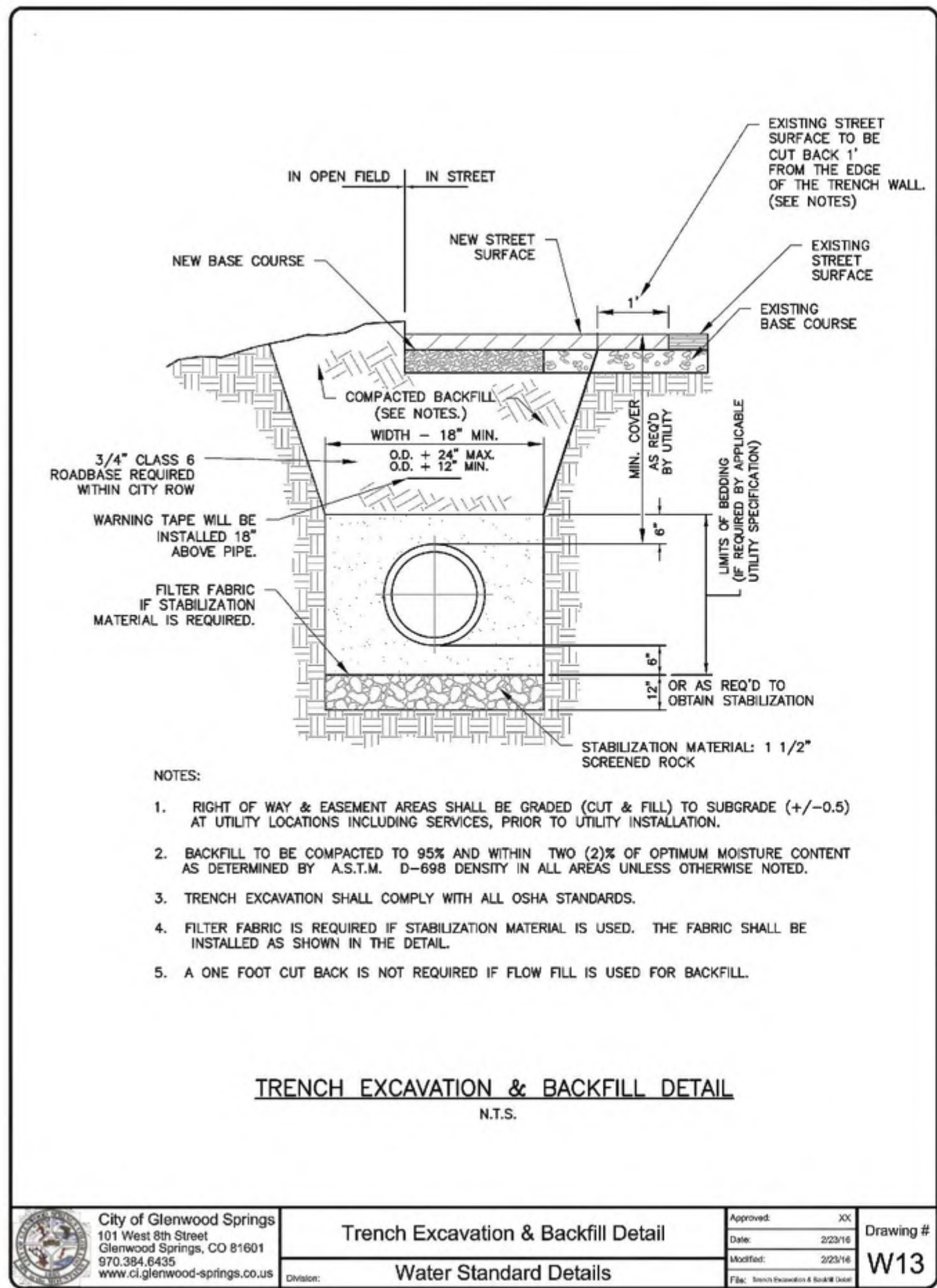
Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
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LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION
Caverns Village
Storm Sewer & Shallow Utility Plan
Construction Plan Set

Revision	By	Date	Sheet
1			C.6

PROJECT NO. 17015-04
DWG NO. C.6
BUI
7/21/20
File Name: 17015-c-storm-cdot.dwg



Civil Engineering
Surveying
& Beyond

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Glenwood Springs, CO 81601
(T) 970.945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: Yes

If "Yes", SUE Certification by:
Firm: 2018-208.004
Job #: SSM, Inc.
Date: July 2020

Engineer or Surveyor Seal

COLORADO LICENSE
17015
37110
PROFESSIONAL ENGINEER

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

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LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village
Details - Sanitary Sewer
Construction Plan Set

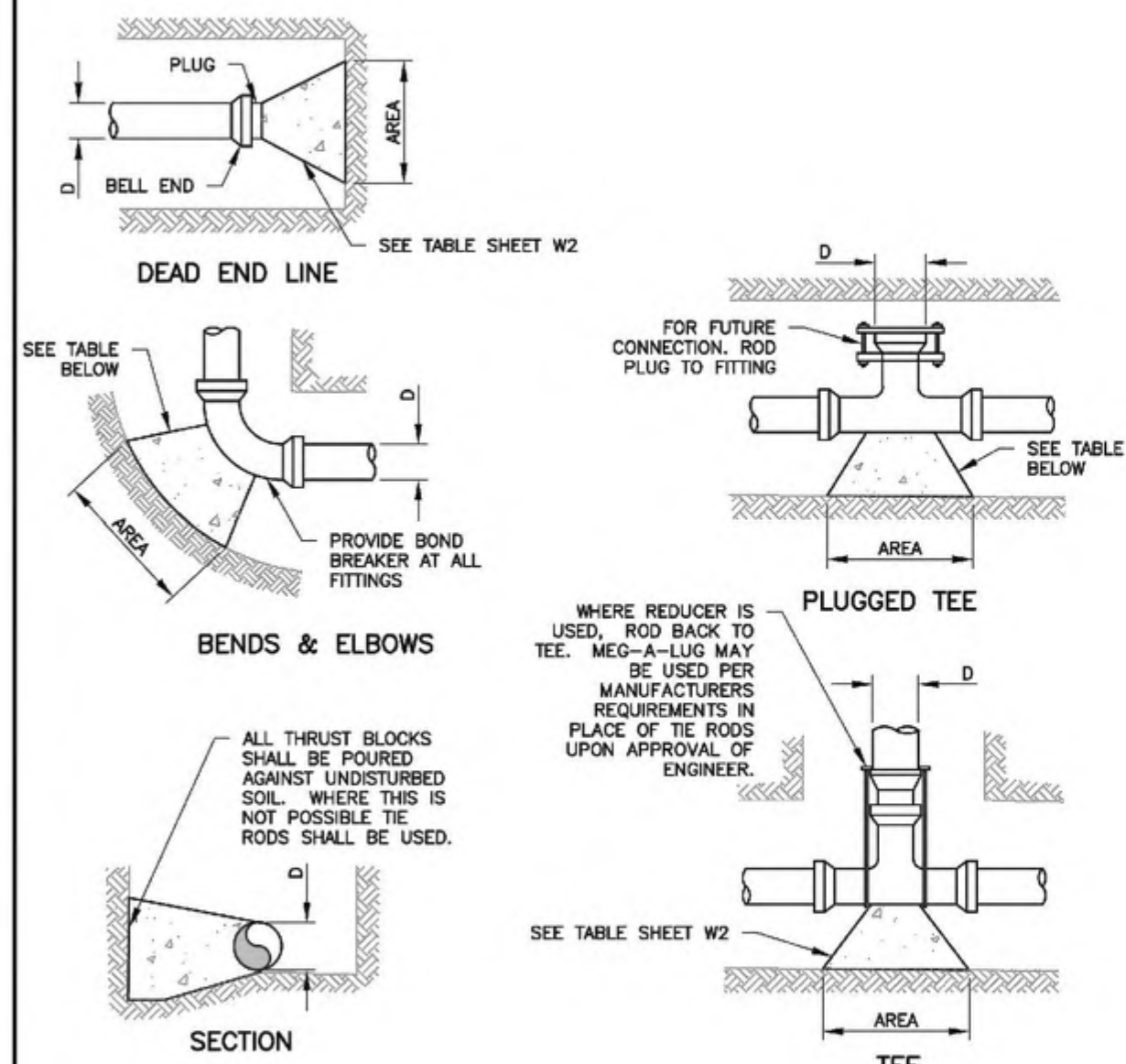
By	Revision	Date	1	2	3	4	5	6	7	8	9	10

PROJECT NO.
17015-04
Language
DWG
BUI
7/21/20

Sheet
C.7

File Name: 17015-C-Storm-CDD.dwg

Packet Page 415



City of Glenwood Springs
101 West 8th Street
Glenwood Springs, CO 81601
970.384.6435
www.ci.glenwood-springs.co.us

Concrete Thrust Block Details
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W01**

SIZE	BEARING AREAS (IN SQ. FT.)				TEES, DEAD ENDS, AND CROSS w/ DEAD END BRANCHES
	90°	45°	22 1/2°	11 1/2°	
3	1.0	0.6	0.3	0	0.7
4	1.8	1.0	0.5	0	1.3
6	4.0	2.2	1.1	0	2.8
8	7.1	3.8	2.0	1.0	5.0
10	11.1	6.0	3.0	1.5	7.8
12	16.0	8.6	4.4	2.2	11.3
14	21.7	11.8	6.0	3.0	15.4
15	25.0	13.5	7.0	3.5	17.6
16	28.4	15.3	8.0	4.0	20.0
18	36.0	19.4	10.0	5.0	25.4
20	44.2	24.0	12.2	6.1	31.4
21	49.0	26.5	13.5	6.8	34.6
22	54.0	29.0	14.8	7.4	38.0
24	64.0	34.5	17.7	8.8	45.0
30	100.0	54.0	27.6	13.8	71.0
36	144.0	78.0	40.0	20.0	102.0

NOTE: TEE SIZE IS BRANCH SIZE.

AREAS GIVEN IN TABLE ARE BASED UPON INTERNAL STATIC PRESSURE OF 100 P.S.I. AND SOIL BEARING CAPACITY OF 1,000 LBS. PER SQ. FT.

BEARING AREAS FOR ANY PRESSURE AND SOIL BEARING CAPACITY MAY BE OBTAINED BY MULTIPLYING TABULATED VALUES BY A CORRECTION FACTOR "F"

F = ACTUAL SPECIFIED TEST PRESSURE IN HUNDREDS OF LBS. / ACTUAL SOIL BEARING CAPACITY IN THOUSANDS OF LBS.

SOIL BEARING CAPACITIES SHALL BE DETERMINED BY THE ENGINEER

ALL WATER LINE PLANS SHALL CONTAIN THE FOLLOWING TABLE, WITH THE VALUES FILLED IN BY THE ENGINEER:

SOIL BEARING CAPACITY -	2000	LBS/SQ. FT.
TEST PRESSURE -	195	P.S.I. (1.5" 130 PSI)
BEARING AREA MULTIPLIER (F) -	1.0	

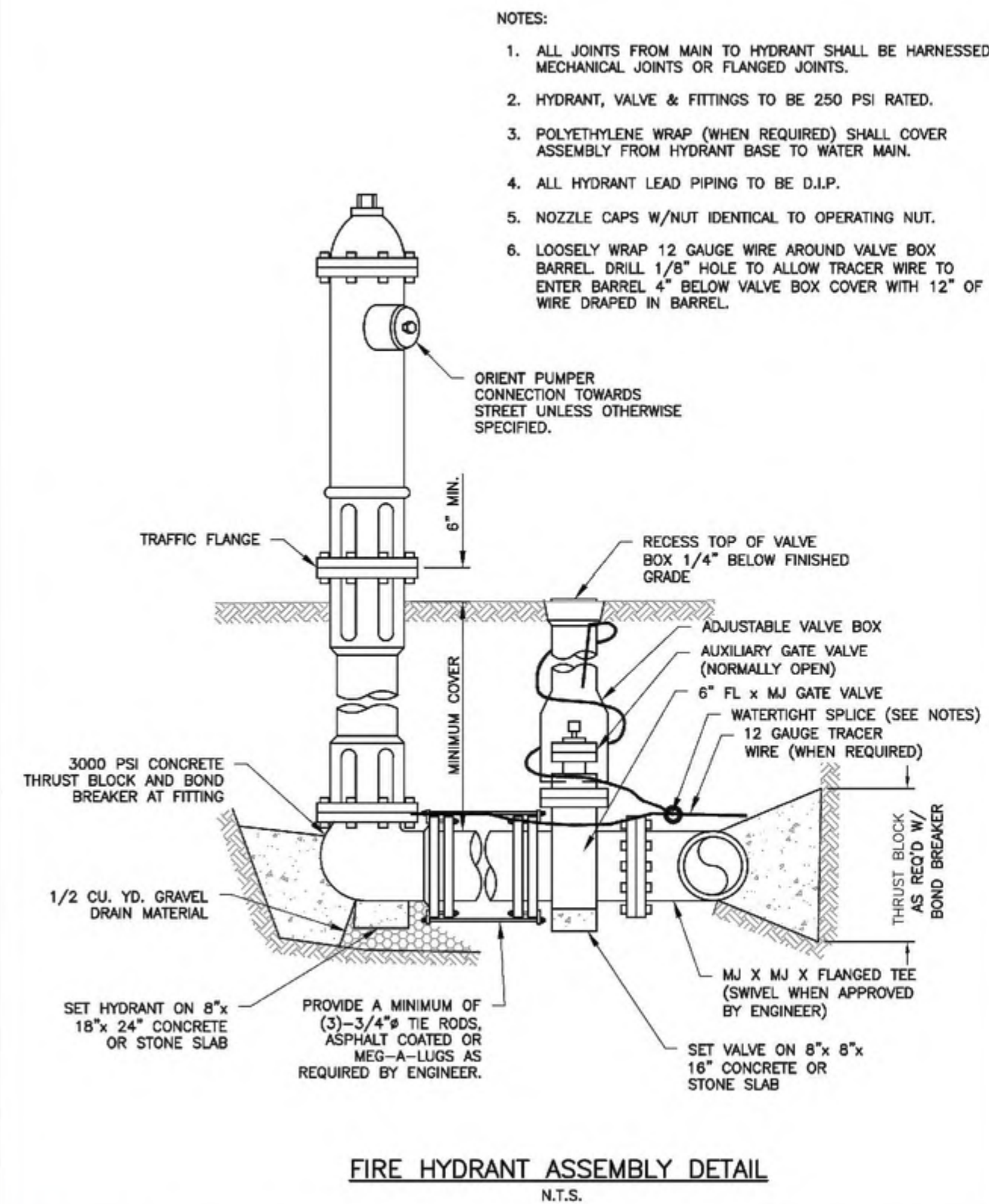
CONCRETE THRUST BLOCK DETAILS
N.T.S.

City of Glenwood Springs
101 West 8th Street
Glenwood Springs, CO 81601
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www.ci.glenwood-springs.co.us

Concrete Thrust Block Details
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W02**

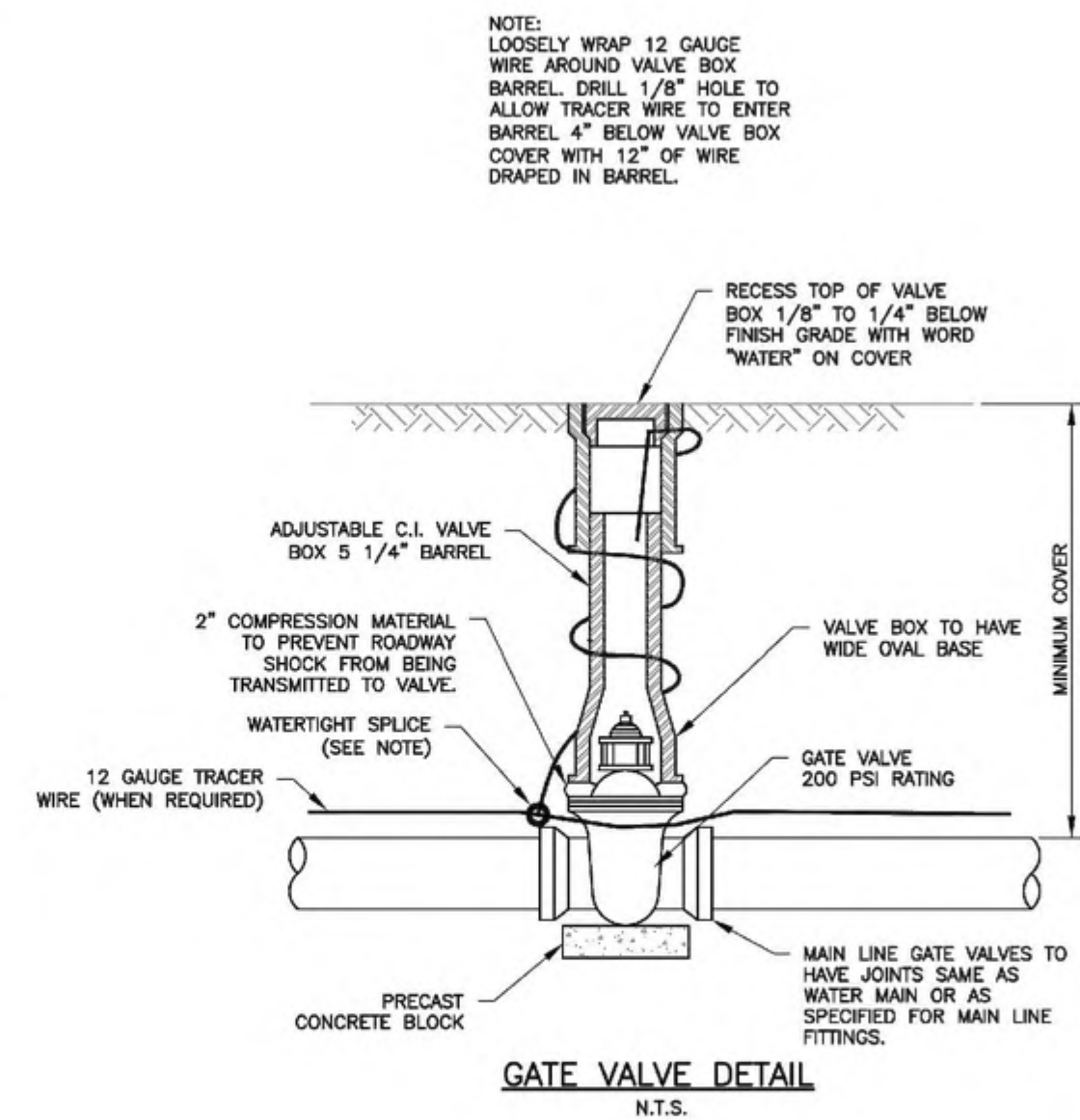


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Fire Hydrant Assembly Detail
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W03**

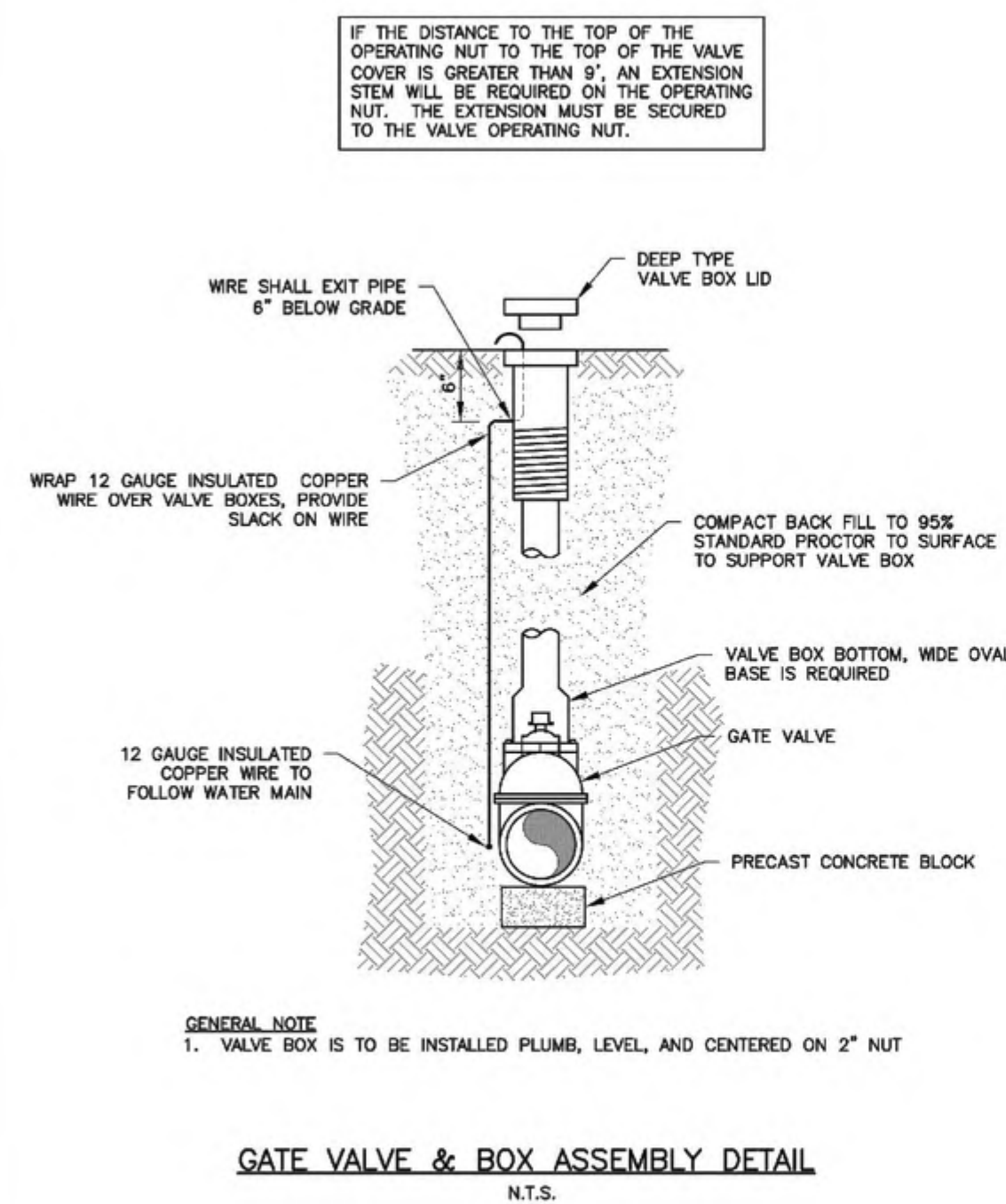


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Gate Valve Detail
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W04**

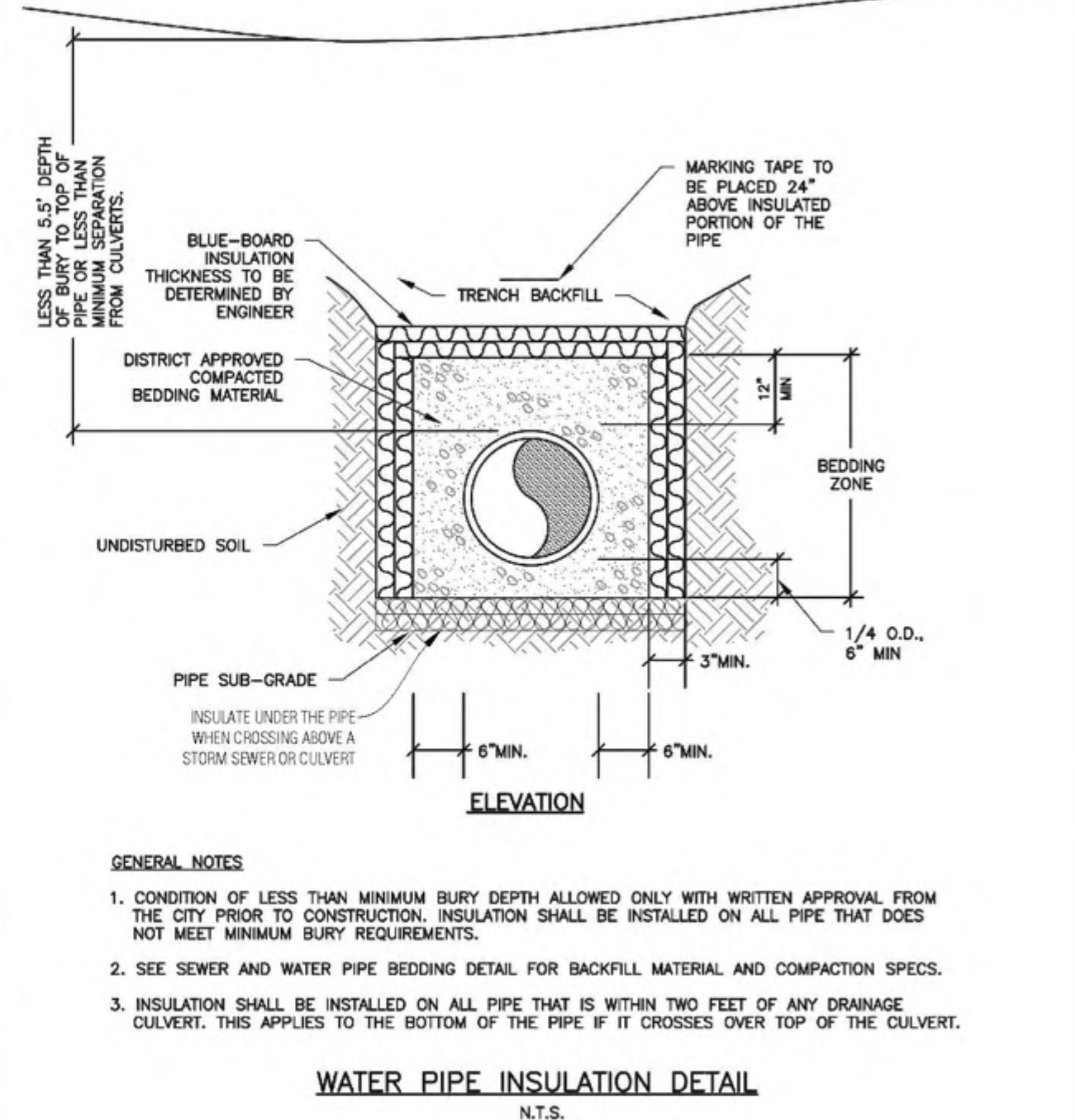


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Gate Valve And Box Assembly
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W05**

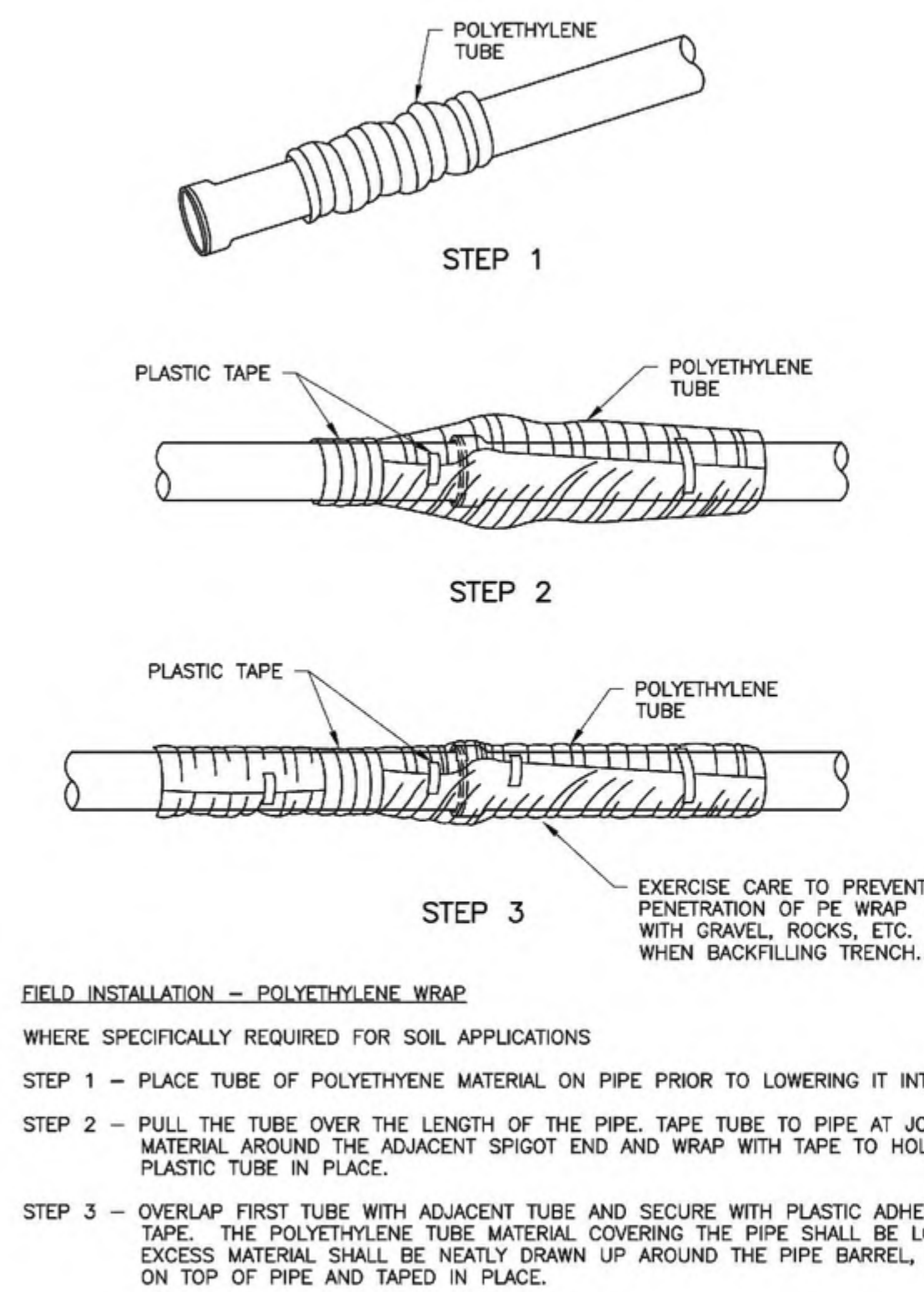


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Water Pipe Insulation Detail
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W06**

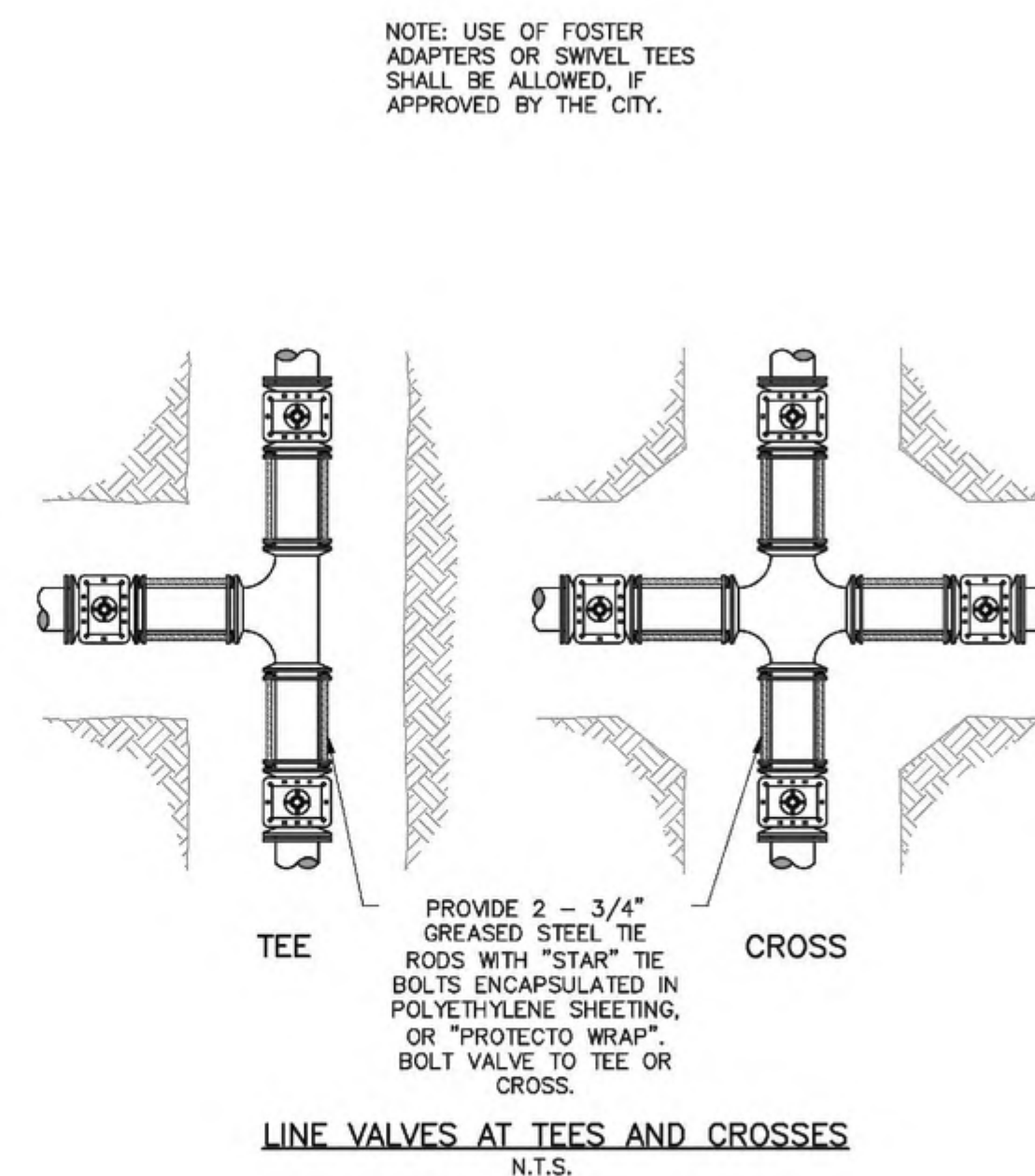


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Glenwood Springs, CO 81601
970.384.6435
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Polywrap Detail
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W07**

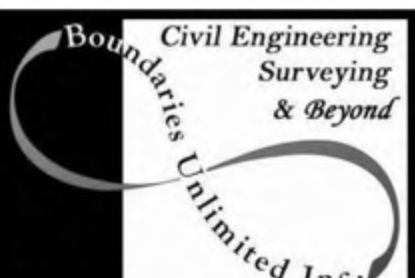


City of Glenwood Springs
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Line Valves at Tees and Crosses
Water Standard Details

Approved: [Signature] XX
Date: 2/23/16
Modified: 2/23/16
File: WaterStandard

Drawing # **W10**



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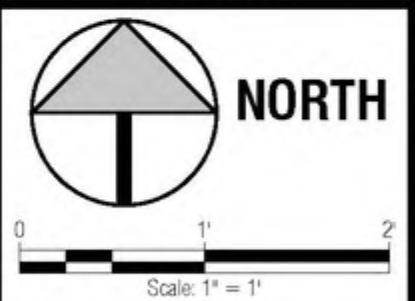
Subsurface Utility Engineering (SUE)
Required Project: Yes

If "Yes", SUE Certification by:
Firm: 2018-208.004
Job #: SSM, Inc.
Date: July 2020

Engineer or Surveyor Seal



Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

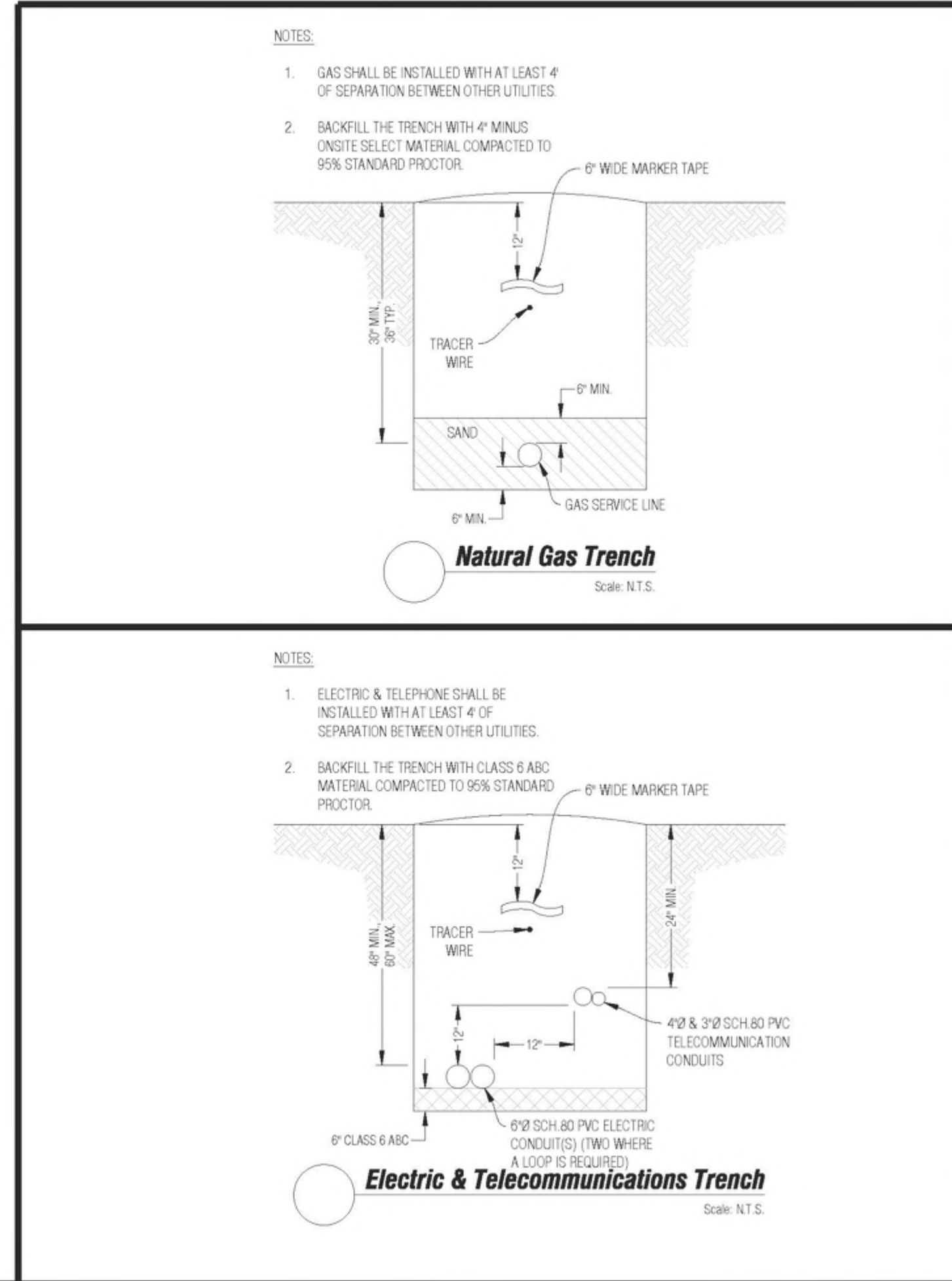
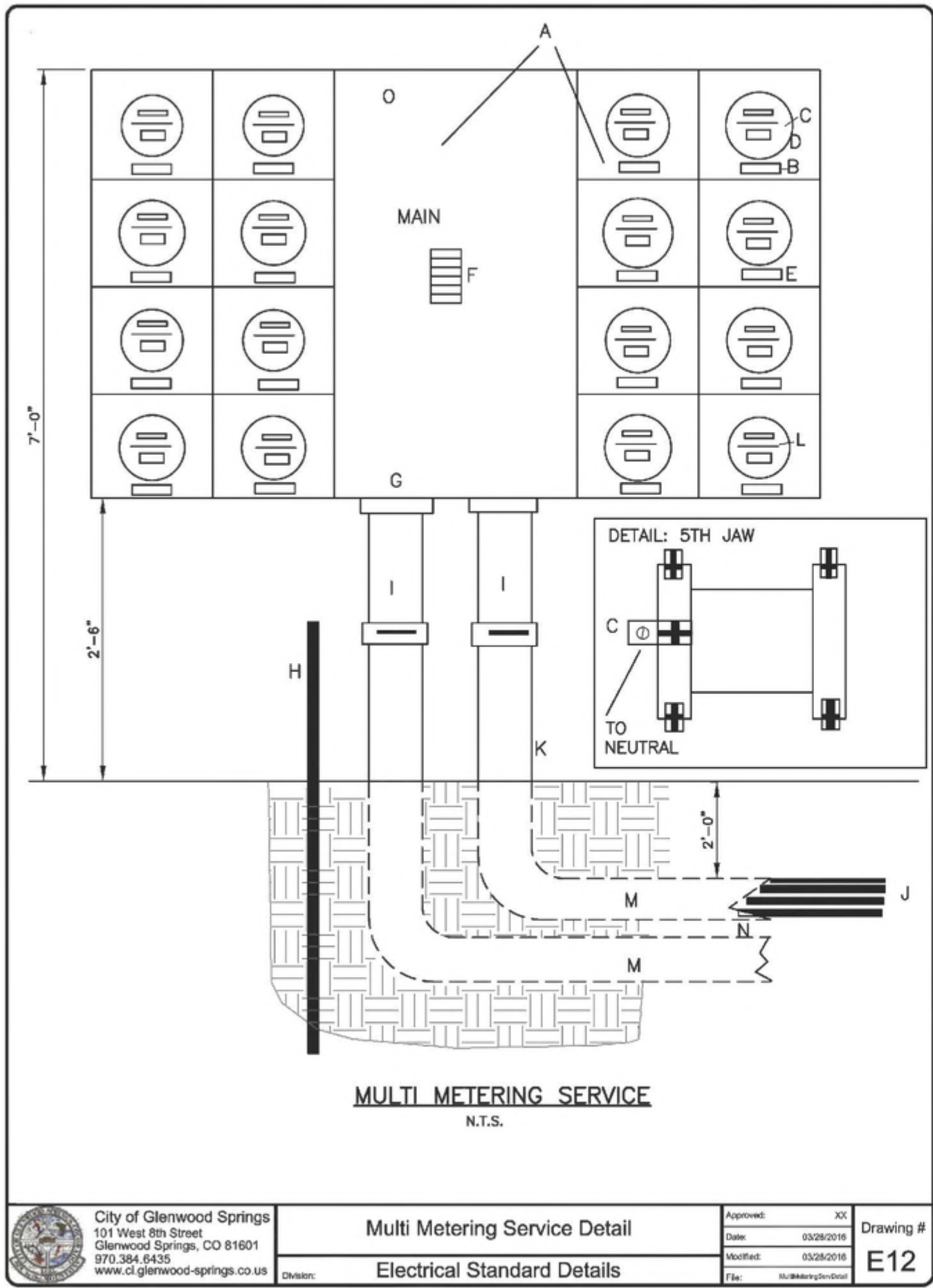
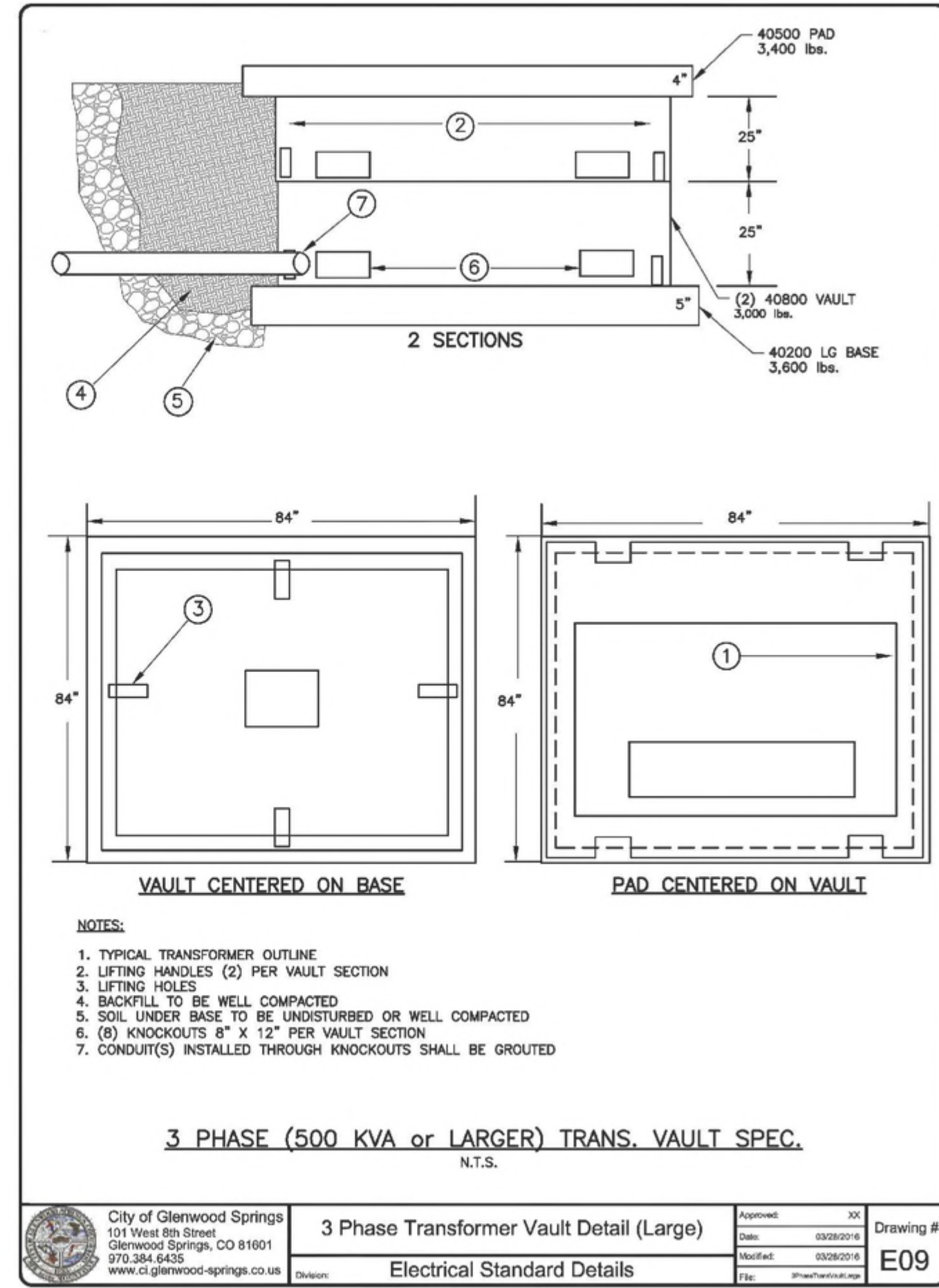
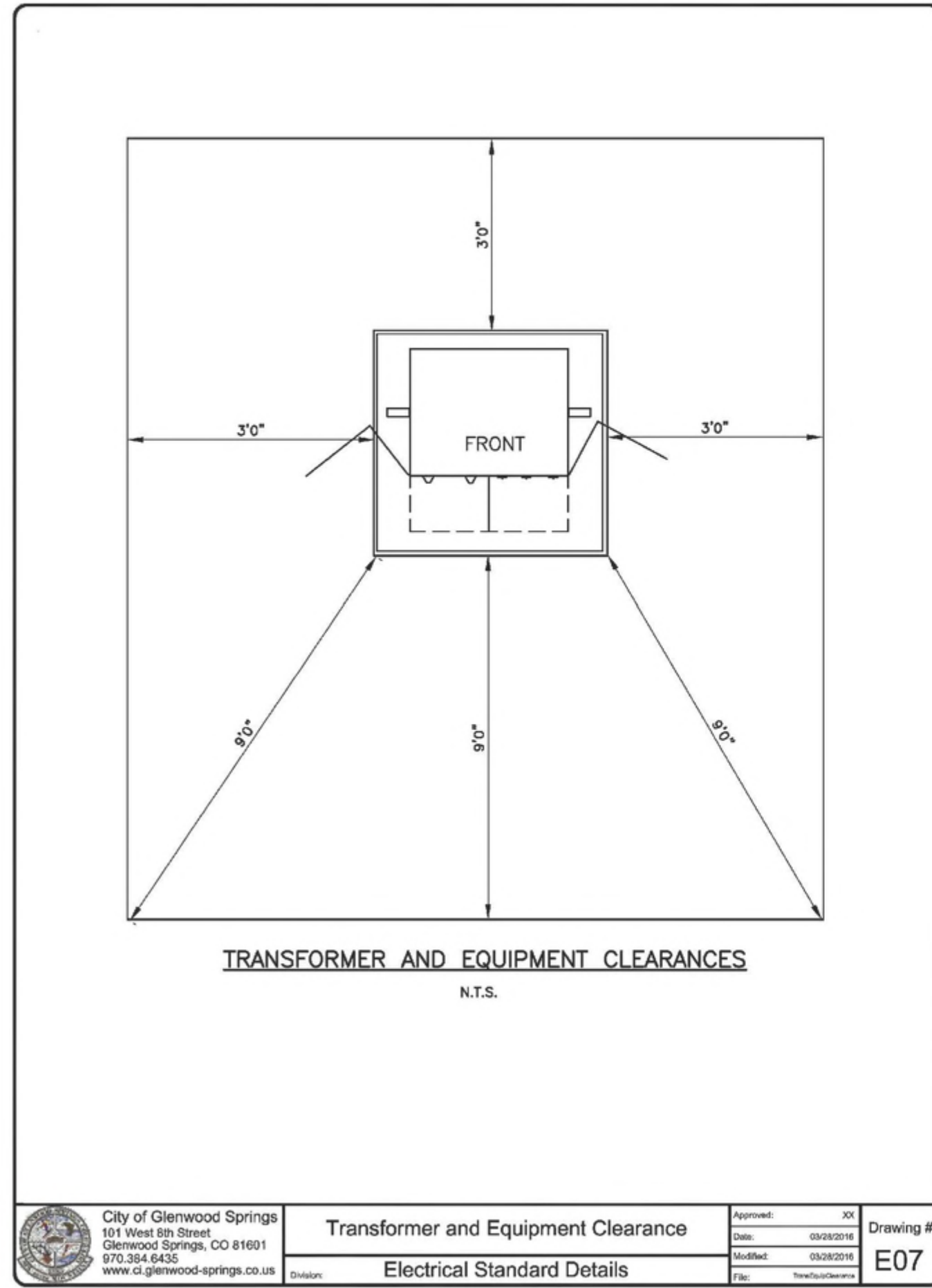
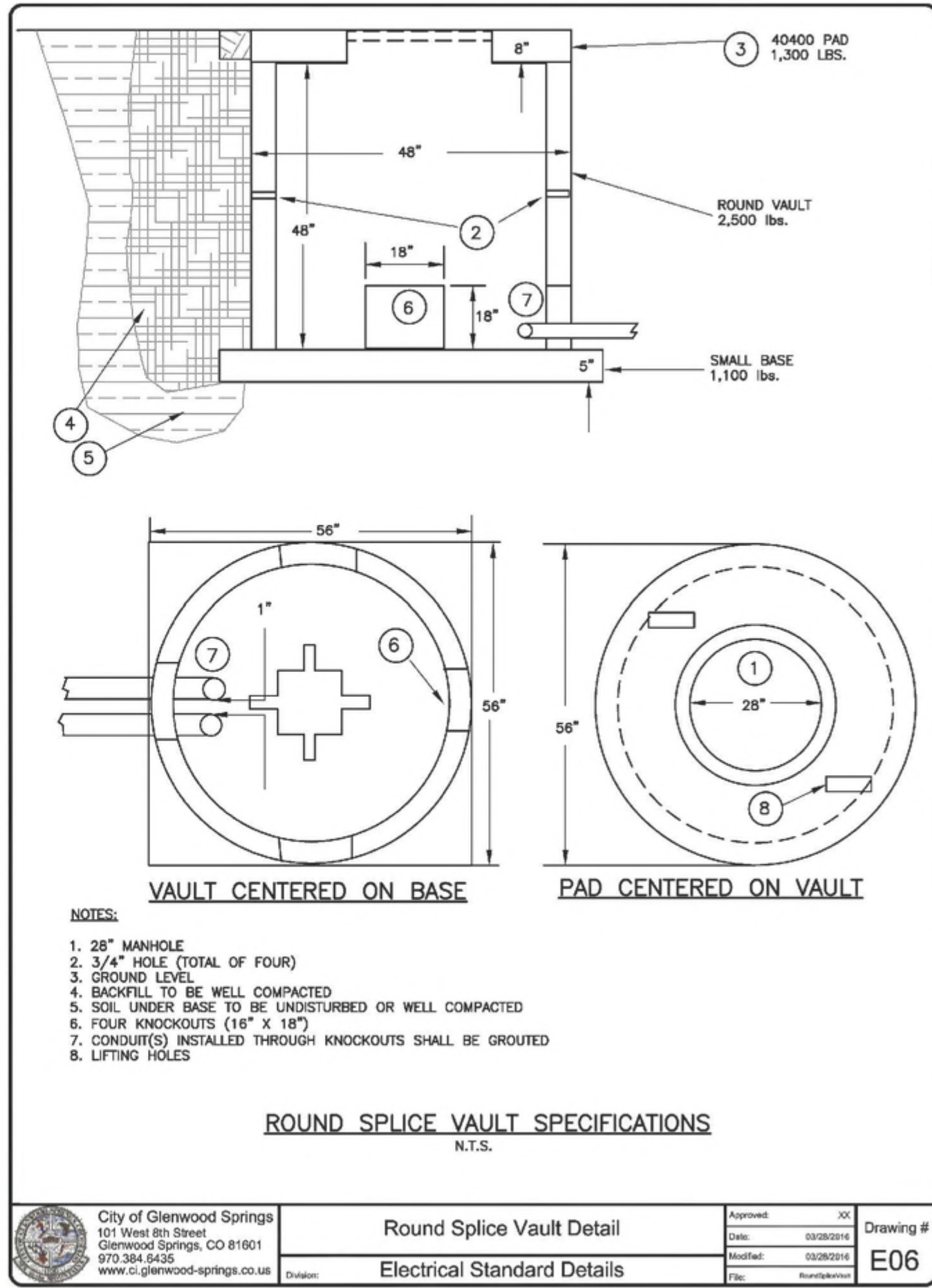
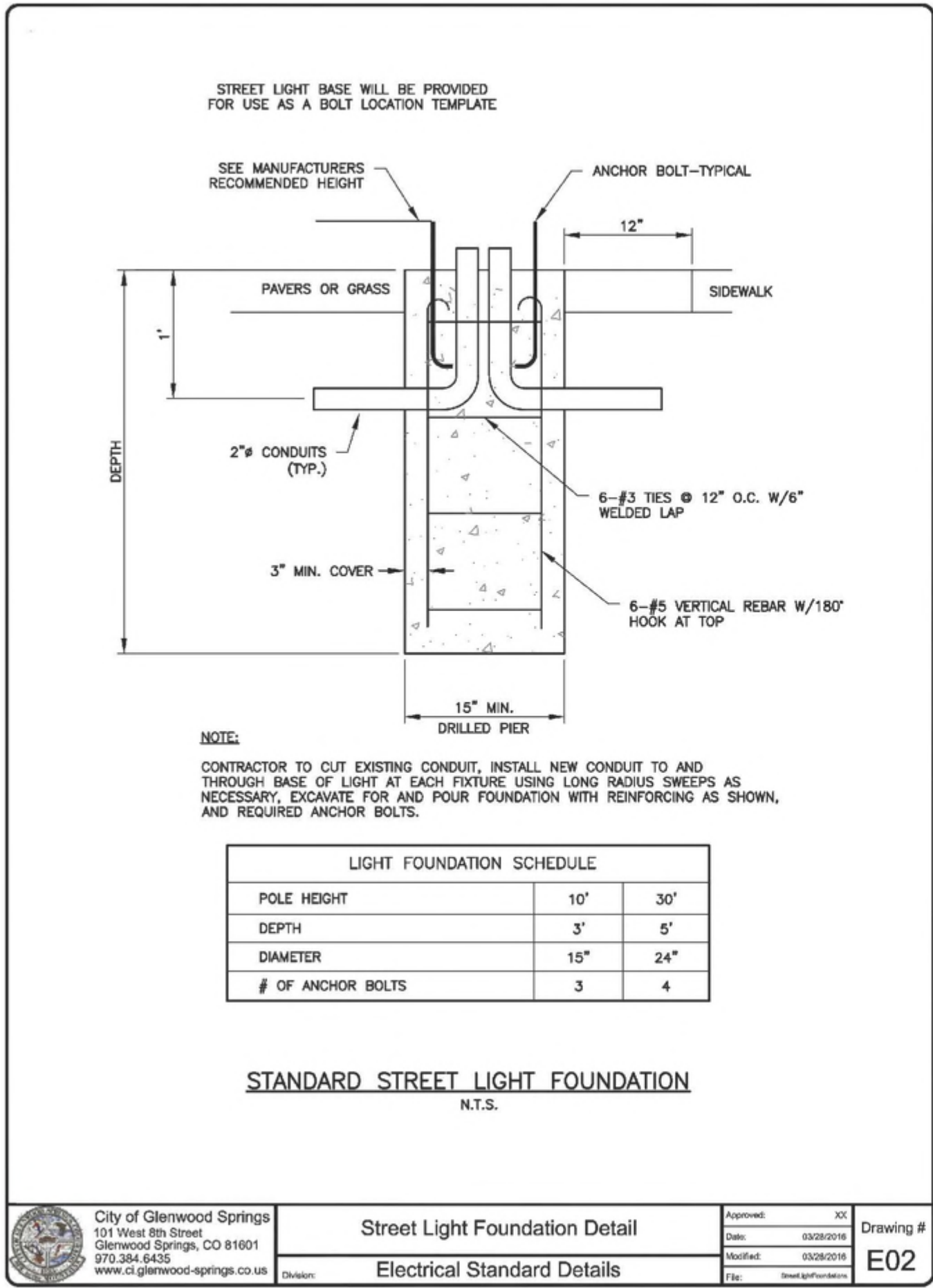


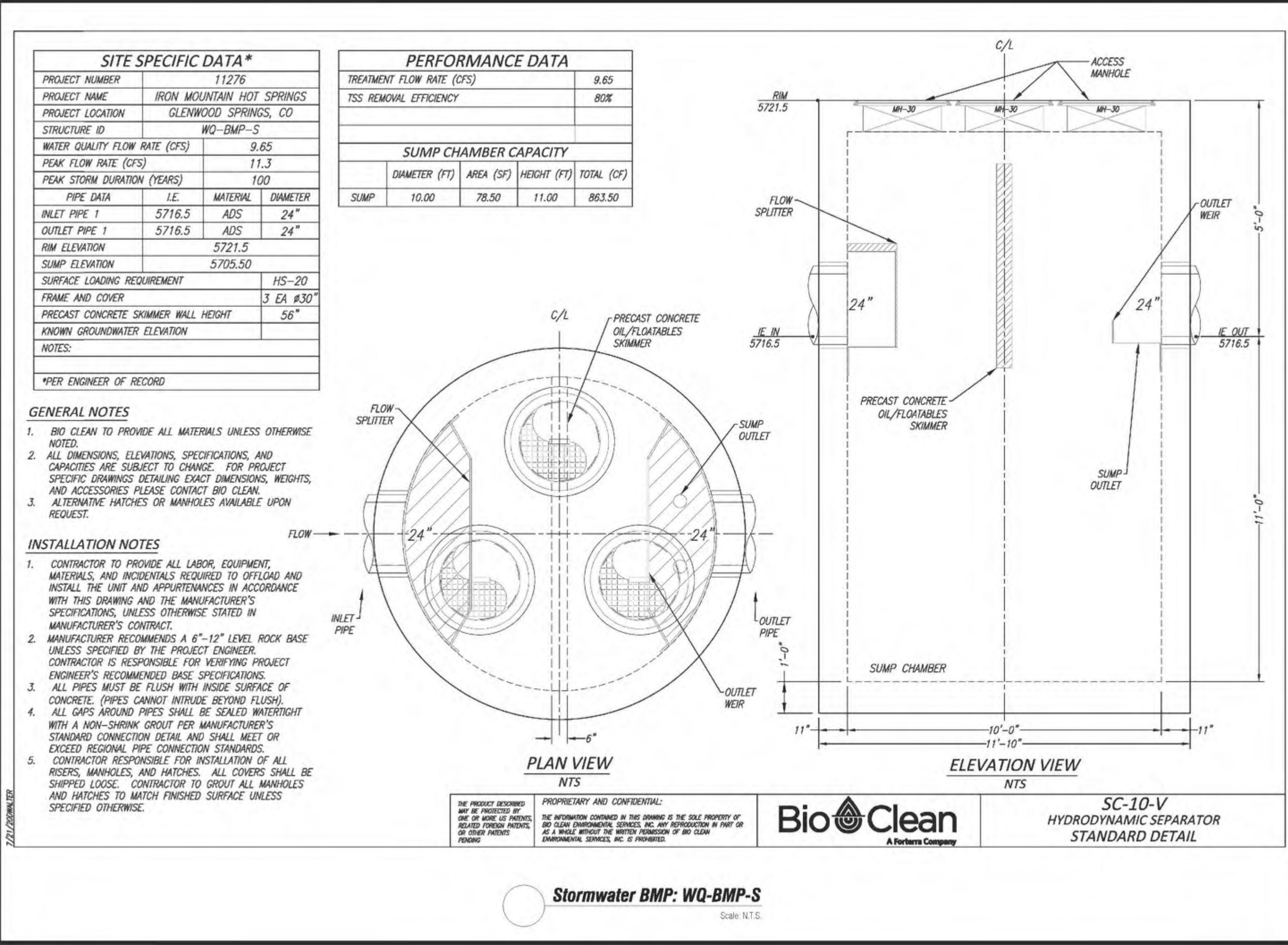
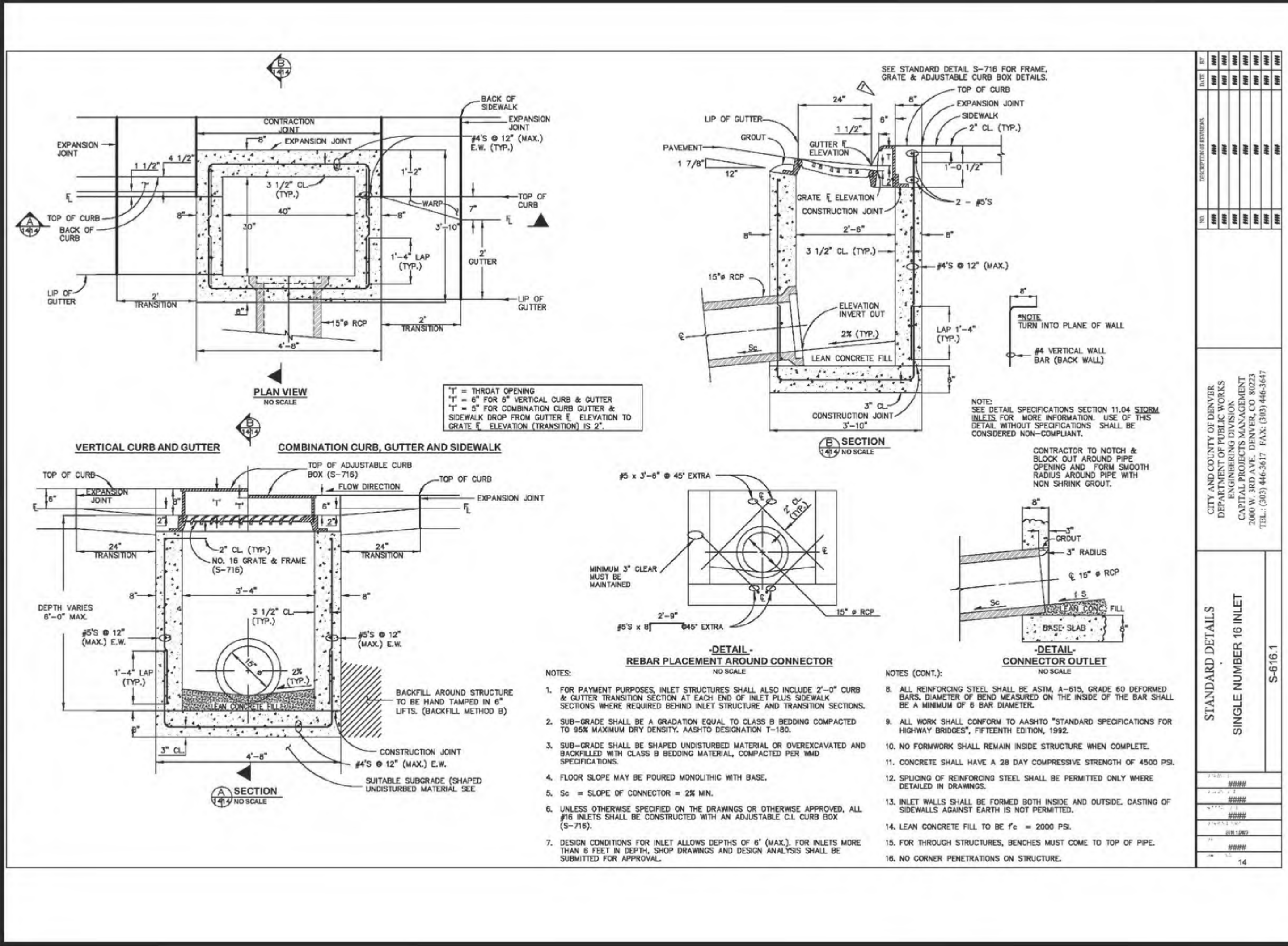
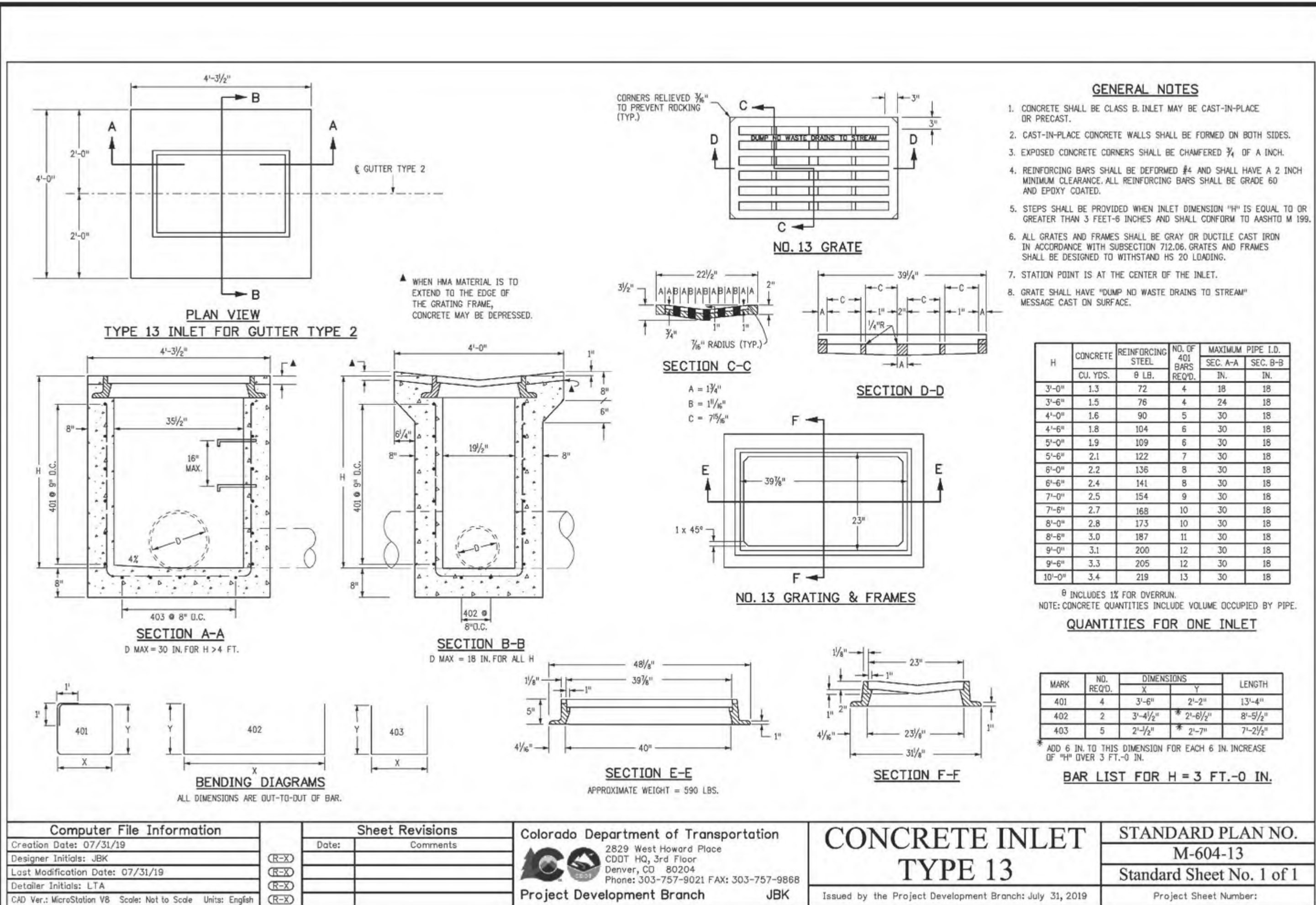
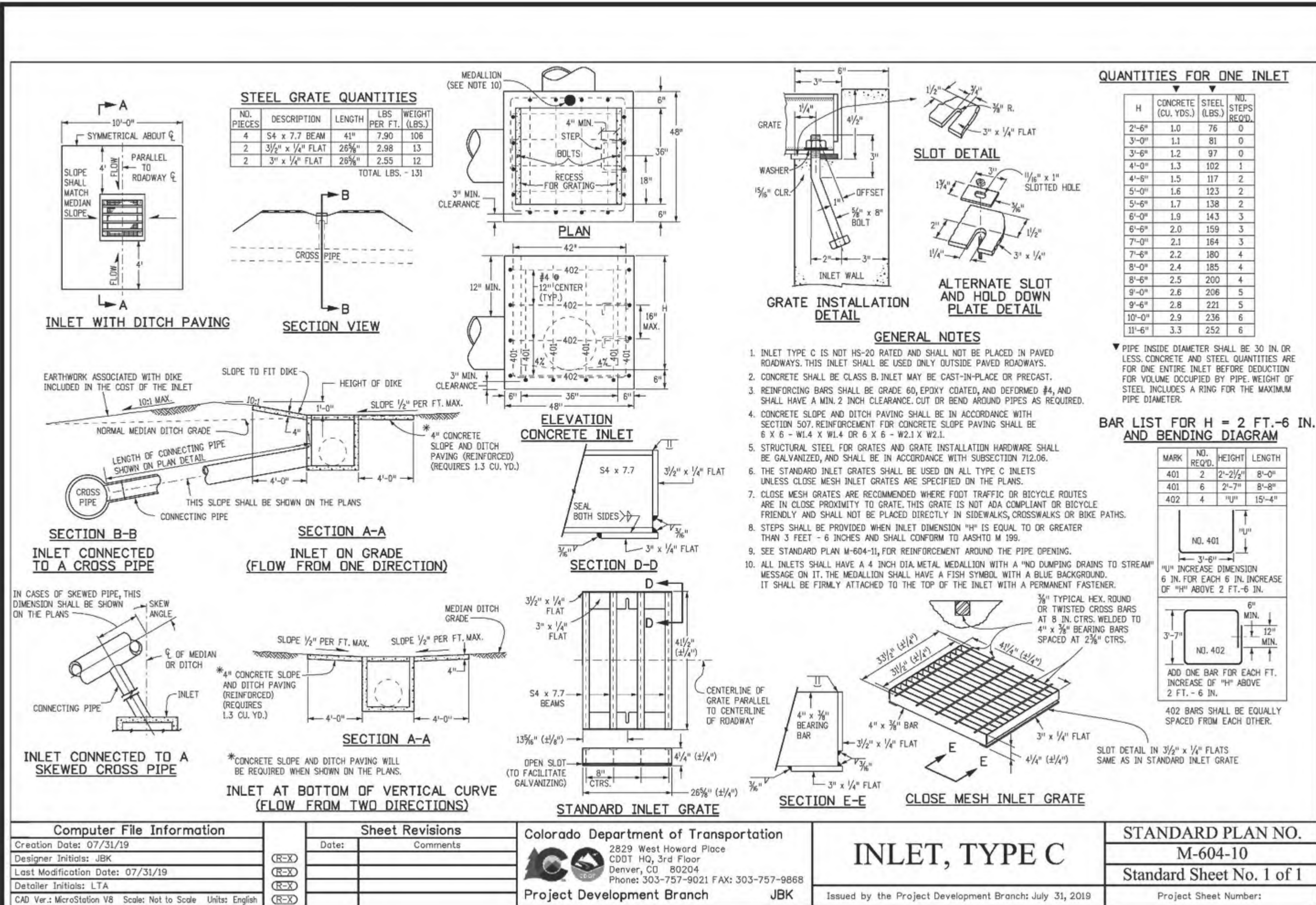
LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village
Details - Potable Water
Construction Plan Set

Revision	By	Date
1		
2		
3		
4		
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PROJECT NO. 17015-04
DWG NO. 17015-04-01
BY: BUI
DATE: 7/21/20
SHEET
C.8
FILE NAME: 17015-0-storm-cdot.dwg





Bourgeois Civil Engineering Surveying & Beyond

923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
(770) 945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: Yes

If "Yes", SUE Certification by:
Firm: 2018-208.004
Job #: 2018-208.004
Date: July 2020

Engineer or Surveyor Seal

COLORADO LICENSED PROFESSIONAL ENGINEER
1/21/20
37110

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

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LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

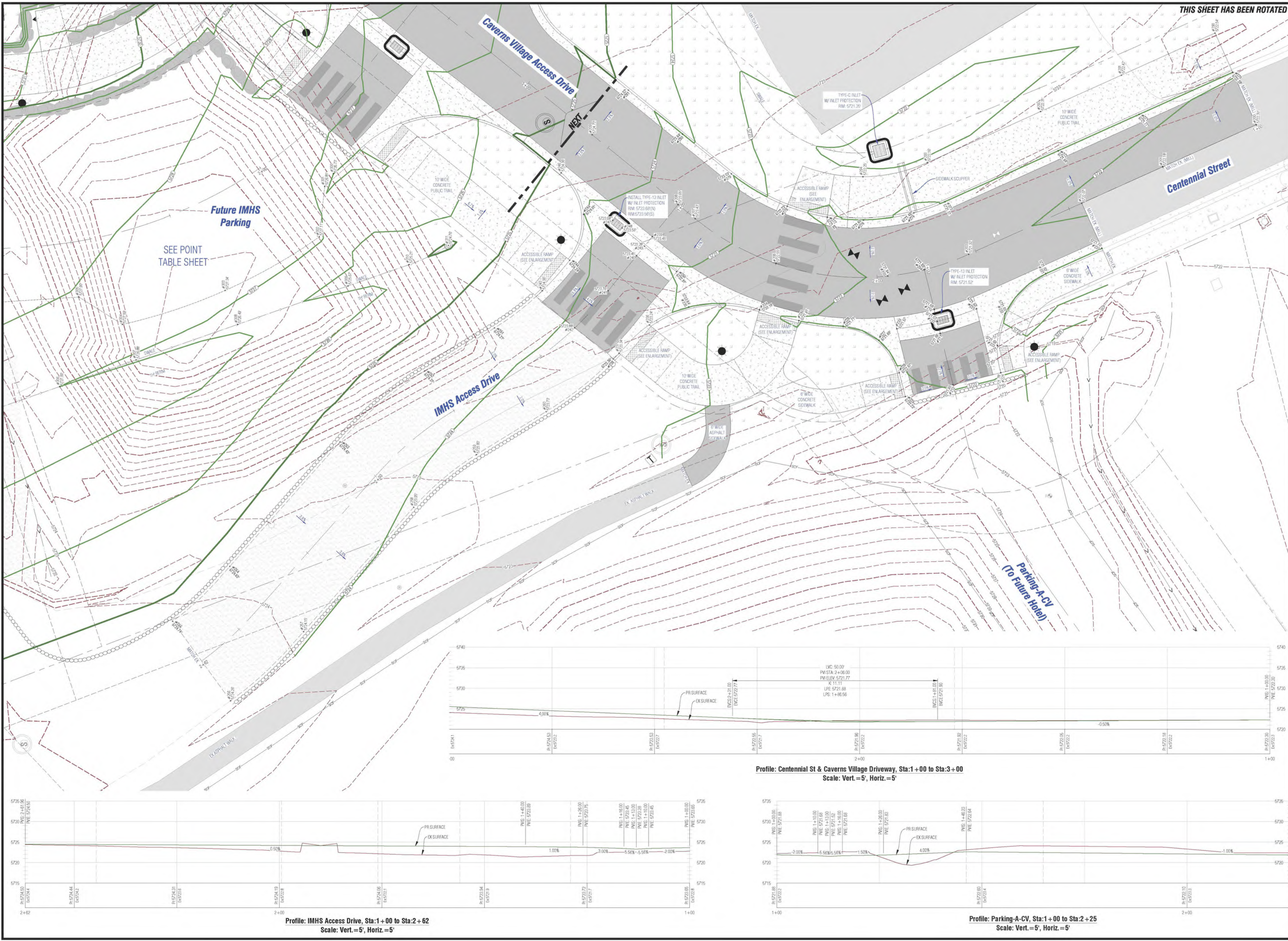
Caverns Village
Details - Storm Sewer #1
Construction Plan Set

By: Revision: Date: Title: 1 2 3 4 5 6 7 8 9 10

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Civil Engineering
Surveying
& Beyond

Boundaries Unlimited Inc.

923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
(T) 970.945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: **Yes**

If "Yes", SUE Certification by:
Firm: **2018-208.004**
Job #: **SGM, Inc.**
Date: **July 2020**

Engineer or Surveyor Seal

COLORADO LICENSED
1/21/20
37110
PROFESSIONAL ENGINEER

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

NORTH

Scale: 1" = 10'

811

Know what's below.
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LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village

Grading - Centennial Street

Construction Plan Set

Rev	Date	By	Reason
1			
2			
3			
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7			
8			

PROJECT NO.
17015-04

Design
DJW

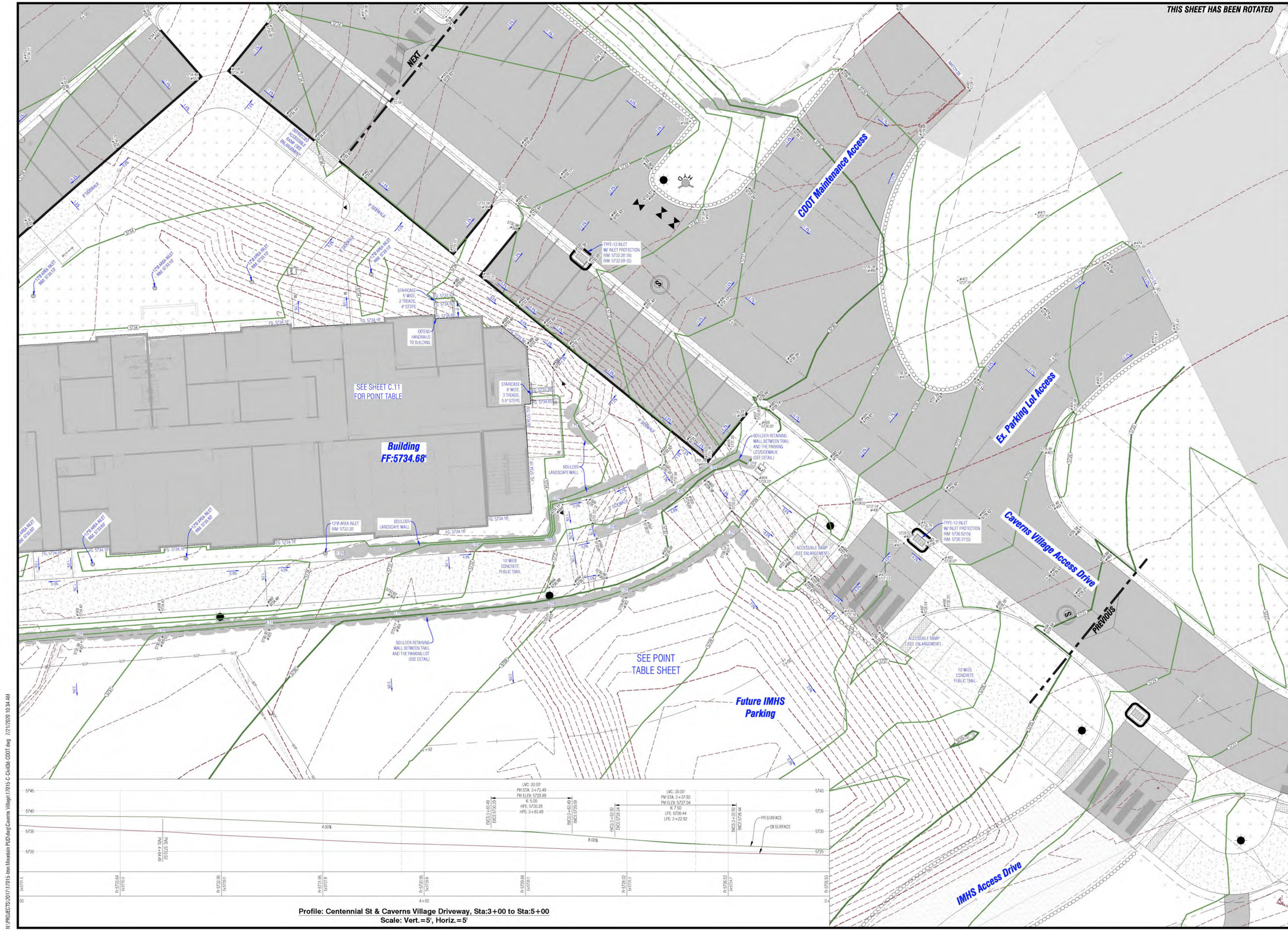
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BU

Date
7/21/20

Sheet
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File Name: **17015-c-chi83-cdot.dwg**

Packet Page 419



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Civil Engineering
Surveying
& Beyond

923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
(T) 970.945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: **Yes**

If "Yes", SUE Certification by:
Firm: **2018-208.004**
Job #: **SGM, Inc.**
Date: **July 2020**

Engineer or Surveyor Seal

COLORADO LICENSED
1/21/20
37110
PROFESSIONAL ENGINEER

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

NORTH

Scale: 1" = 10'

811

Know what's below.
Call before you dig.

LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village

Grading - Access Drive & Site

Construction Plan Set

Rev.	Date	By	Revision
1			
2			
3			
4			
5			
6			
7			
8			

PROJECT NO.
17015.04

DESIGNED BY
DJW

CHECKED BY
BU

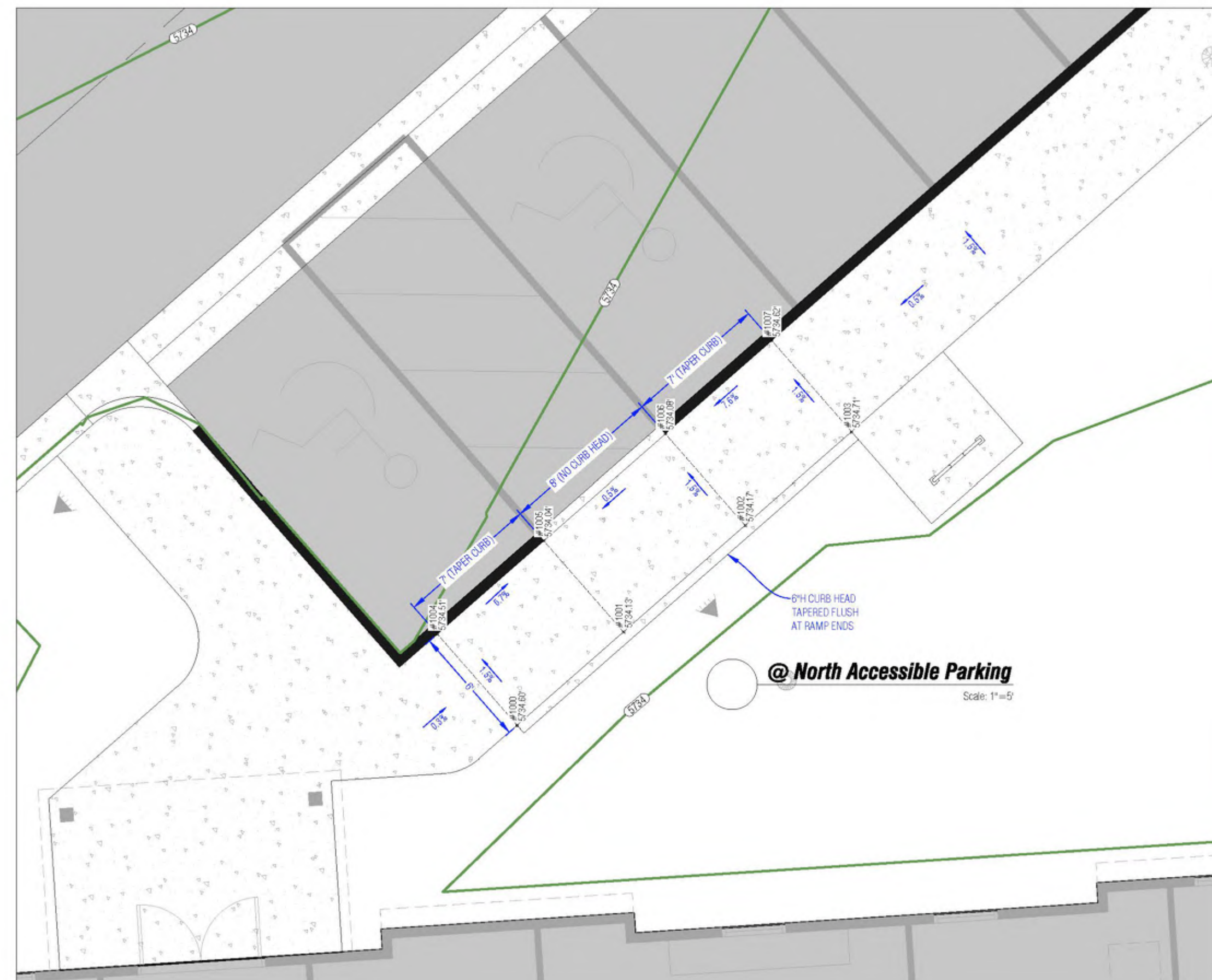
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7/21/20

SHEET
C.12

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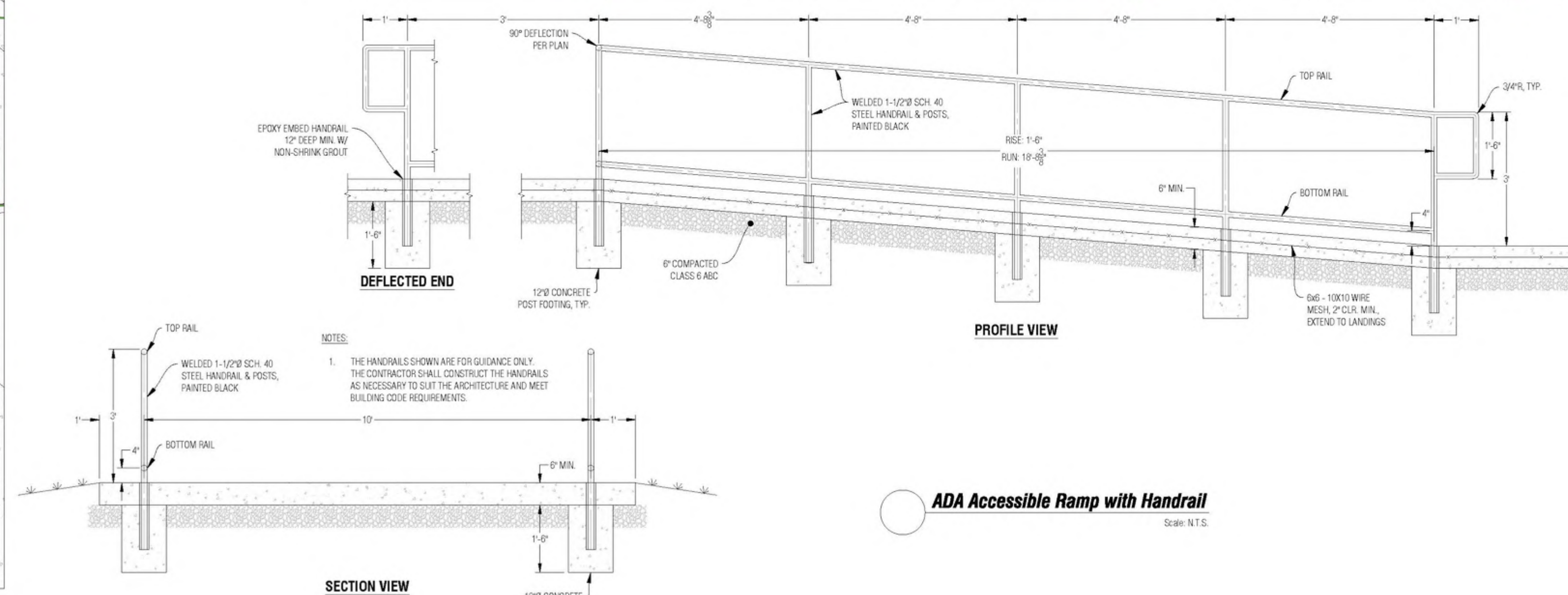
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Packet Page 420

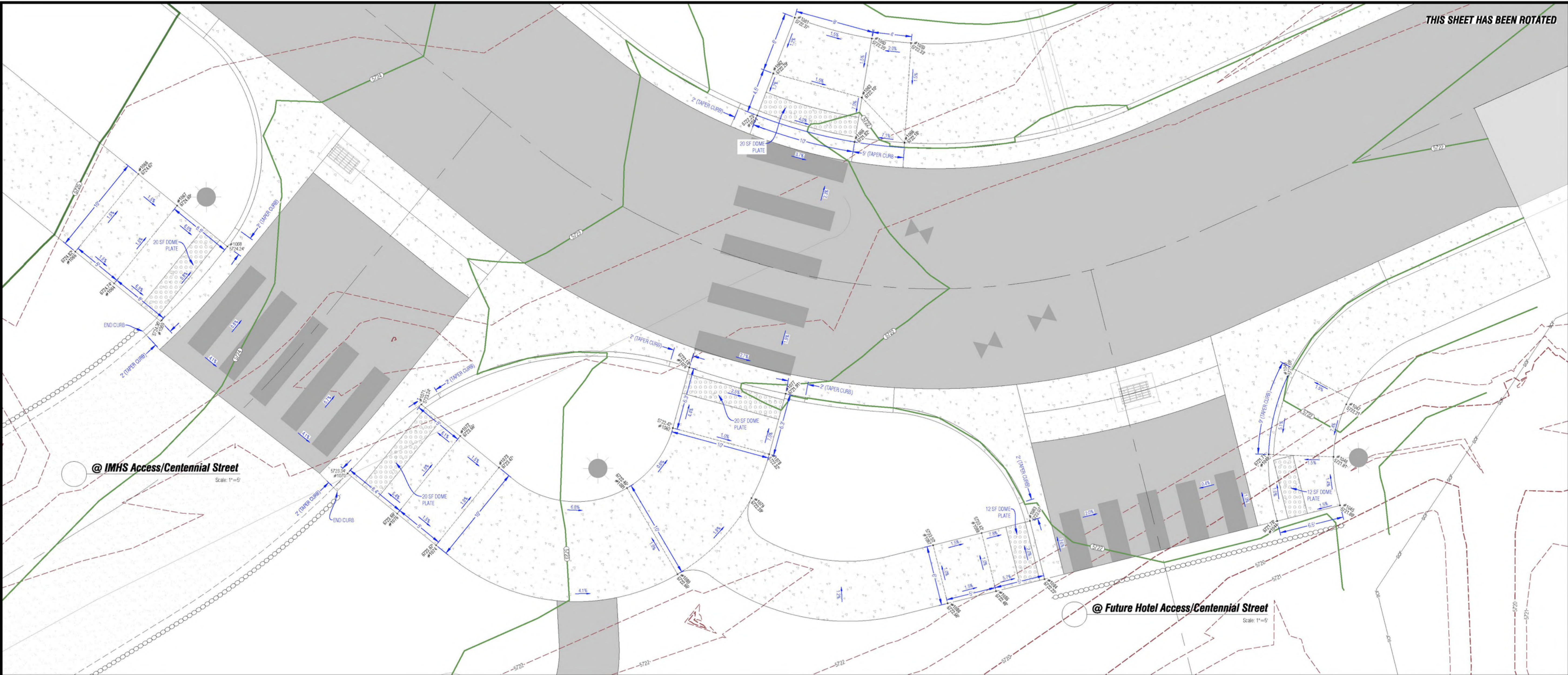


- ### ACCESSIBLE RAMP NOTES

1. ALL ACCESSIBLE CURB RAMPS SHALL CONFORM TO COLDPAID DEPARTMENT OF TRANSPORTATION (CDOT) M-STANDARD M-608-1 "CURB RAMPS". THIS PLAN PROVIDES DETAILED SLOPE ELEVATIONS AND ACCOMPANYING SLOPES TO INDICATE THAT MAXIMUM ALLOWABLE SLOPES HAVE NOT BEEN EXCEEDED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL OTHER CONSTRUCTION REQUIREMENTS OF M-STANDARD M-608-1 (LATEST EDITION) ARE ADHERED TO UNLESS OTHERWISE APPROVED IN WRITING BY THE CITY ENGINEER.
2. ALL CONCRETE SHALL CONTAIN COTD APPROVED FIBER MESH REINFORCEMENT AT 3.5 LBS PER CUBIC YARD.
3. CONCRETE RAMP AND LANDING SURFACES SHALL BE TEXTURED WITH A NON-SLIP COARSE HORIZONTAL BROOMED SURFACE AND SHALL HAVE 3/8" TROWELED SLOPE LINES (AKA GROOVES) TOOLED PERPENDICULAR TO THE RAMP AT 6' O.C.



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Boundaries Unlimited Inc.
Civil Engineering
Surveying
& Beyond
923 Cooper Ave., Ste. 201
Glenwood Springs, CO 81601
(T) 970.945.5252
www.bu-inc.com

Subsurface Utility Engineering (SUE)
Required Project: **Yes**
If "Yes", SUE Certification by:
Firm: **2018-208.004**
Job #: **SGM, Inc.**
Date: **July 2020**

Engineer or Surveyor Seal

PROFESSIONAL ENGINEER

Client Information:
IRON MOUNTAIN HOT SPRINGS
281 CENTENNIAL STREET
GLENWOOD SPRINGS, CO 81601
970.945.4766

NORTH
Scale: 1" = 5'

Know what's below.
Call before you dig.

LOT 1, BLOCK 2 OF THE NORTH GLENWOOD CENTER SUBDIVISION

Caverns Village
Enlarged Grading-ADA Ramps (Cont)
Construction Plan Set

No.	Date	Revision	By
1			
2			
3			
4			
5			
6			
7			
8			

PROJECT NO.
17015.04
Design:
DJW
Check:
BU
7/21/20

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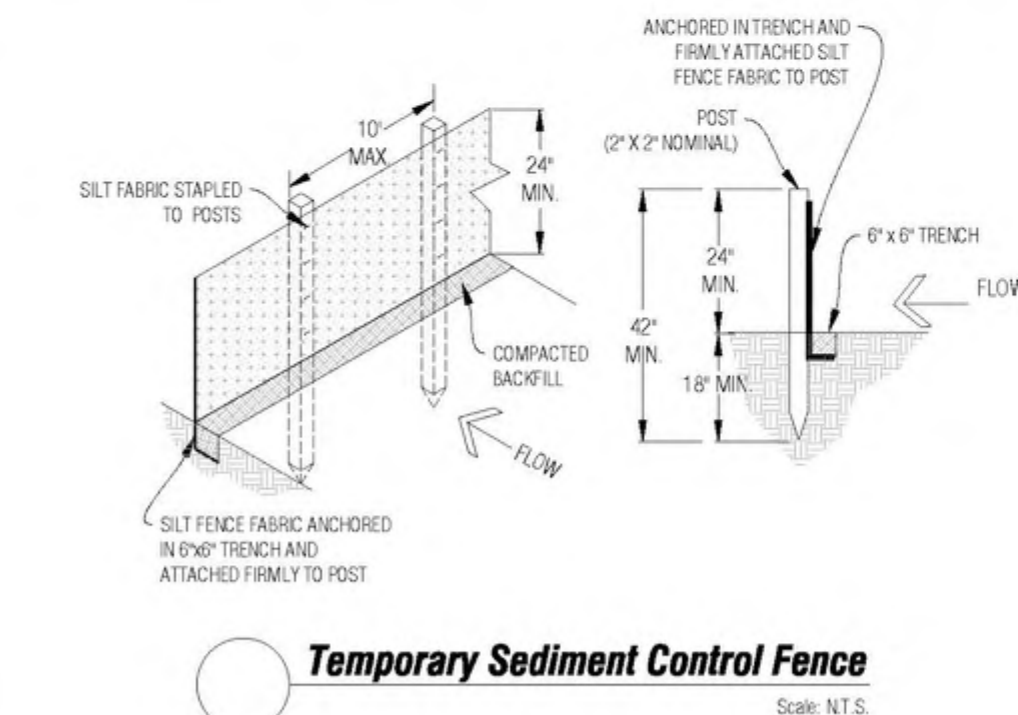
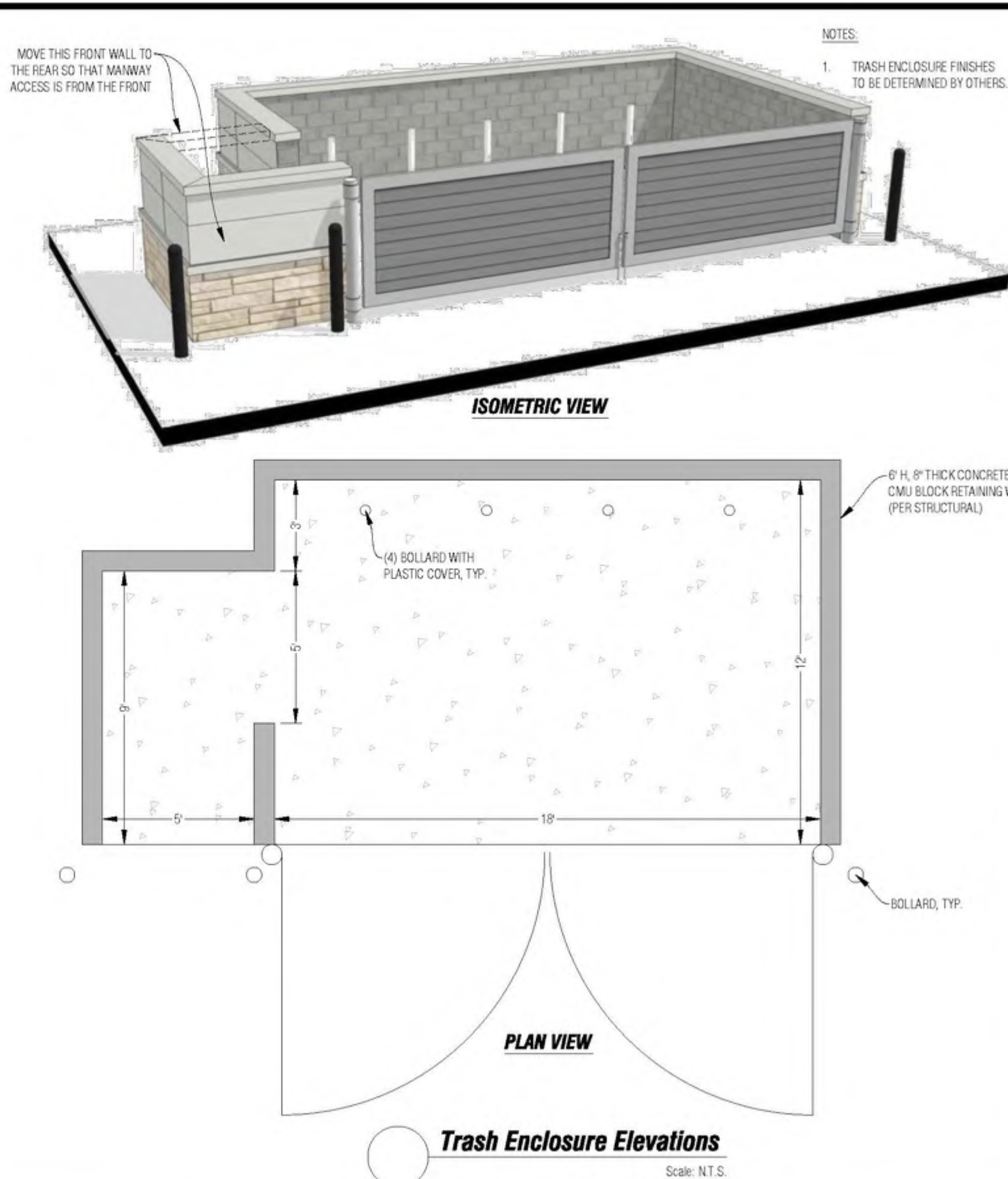
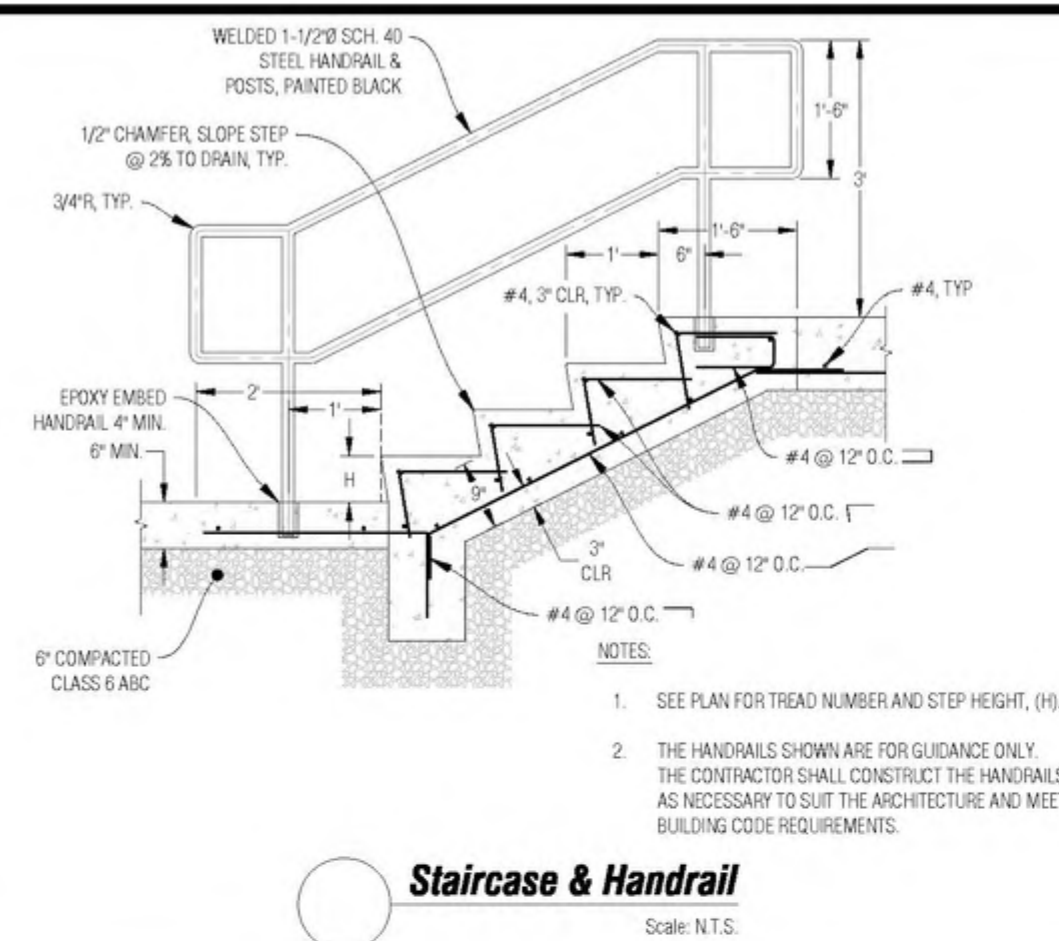
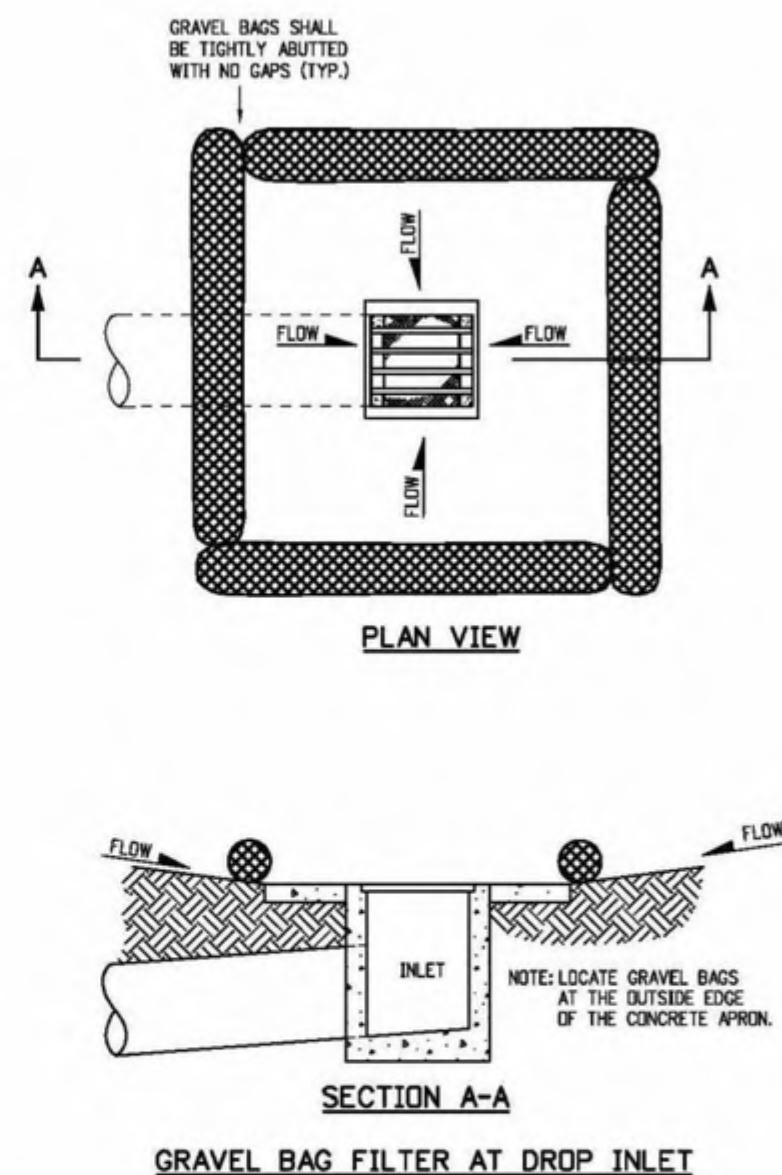
File Name: 17015-C-CHM36-CDOT.dwg

Point Table				
Point #	Raw Description	Elevation	Northing	Eastng
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201	CL-FG	5722.18	42414.05	15782.01
202	CL-FG	5722.07	42433.78	15773.13
203	CL-FG	5721.92	42461.38	15760.72
204	CL-FG	5721.88	42472.15	15756.99
205	CL-FG	5721.94	42481.08	15755.42
206	CL-FG	5722.55	42508.30	15758.51
207	CL-FG	5723.43	42528.00	15769.42
208	CL-FG	5723.65	42532.17	15772.80
209	CL-FG	5724.73	42553.12	15789.83
210	CL-FG	5721.68	42499.73	15747.28
211	CL-FG	5721.68	42468.28	15741.46
212	CL-FG	5721.99	42465.15	15728.94
220	Curb-FL-Match	5721.70	42428.86	15762.19
221	Curb-FL	5721.62	42442.58	15756.02
222	Curb-FL	5721.60	42452.47	15748.29
223	Curb-FL	5721.69	42454.36	15738.51
224	Curb-FL	5721.78	42453.51	15731.85
225	Curb-FL	5722.23	42478.80	15726.04
226	Curb-FL	5722.07	42478.25	15731.86
227	Curb-FL	5721.69	42483.24	15739.78
228	Curb-FL	5721.71	42491.95	15743.20
230	Curb-FL	5721.91	42502.71	15744.59
231	Inlet-X Pan-FL	5721.53	42459.46	15747.42
232	X Pan-FL	5721.52	42469.00	15744.37
233	X Pan-FL	5721.57	42478.86	15742.58
234	Inlet-Curb-FL	5722.18	42512.36	15747.22
235	Inlet-Curb-FL	5722.84	42529.80	15747.90
236	Curb-FL	5723.24	42539.19	15743.52
237	Curb-FL	5723.34	42546.57	15736.83
238	Curb-FL	5723.38	42547.97	15735.21
239	CL-FG	5723.49	42538.48	15765.04
240	CL-FG	5723.49	42542.26	15780.39
241	CL-FG	5723.76	42549.20	15751.85
242	CL-FG	5723.88	42557.39	15742.81
243	Curb-FL	5724.38	42565.29	15751.85
244	Curb-FL	5724.24	42558.61	15759.30
245	Curb-FL	5724.01	42555.23	15770.42
246	Curb-FL	5724.36	42560.89	15760.52
247	Inlet-X Pan-FL	5723.68	42548.13	15769.02
248	Inlet-X Pan-FL	5723.58	42545.80	15767.13
249	X Pan-FL	5723.28	42540.37	15782.71
250	X Pan-FL	5722.91	42532.52	15756.52
251	FG-Millings	5724.21	42576.93	15742.80
252	FG-Millings	5724.31	42593.96	15732.31
253	FG-Millings	5724.43	42614.60	15714.97
254	FG-Millings	5724.63	42641.45	15684.68
255	FG-Millings	5724.74	42655.40	15671.30
256	FG-Millings	5724.28	42641.26	15651.90
257	FG-Millings	5724.19	42623.48	15668.74
258	FG-Millings	5723.99	42596.63	15699.06
259	FG-Millings	5723.83	42581.38	15711.88
260	FG-Millings	5723.73	42584.35	15722.36
270	Curb-FL-Match	5722.39	42397.01	15805.07
271	Curb-FL	5721.77	42419.79	15794.78
272	Curb-FL	5721.66	42439.53	15785.90
273	Curb-FL	5721.51	42487.12	15773.48
275	Inlet-Curb-FL	5721.48	42474.73	15770.78
276	Curb-FL	5721.60	42488.98	15769.09
277	Curb-FL	5721.82	42495.81	15769.75

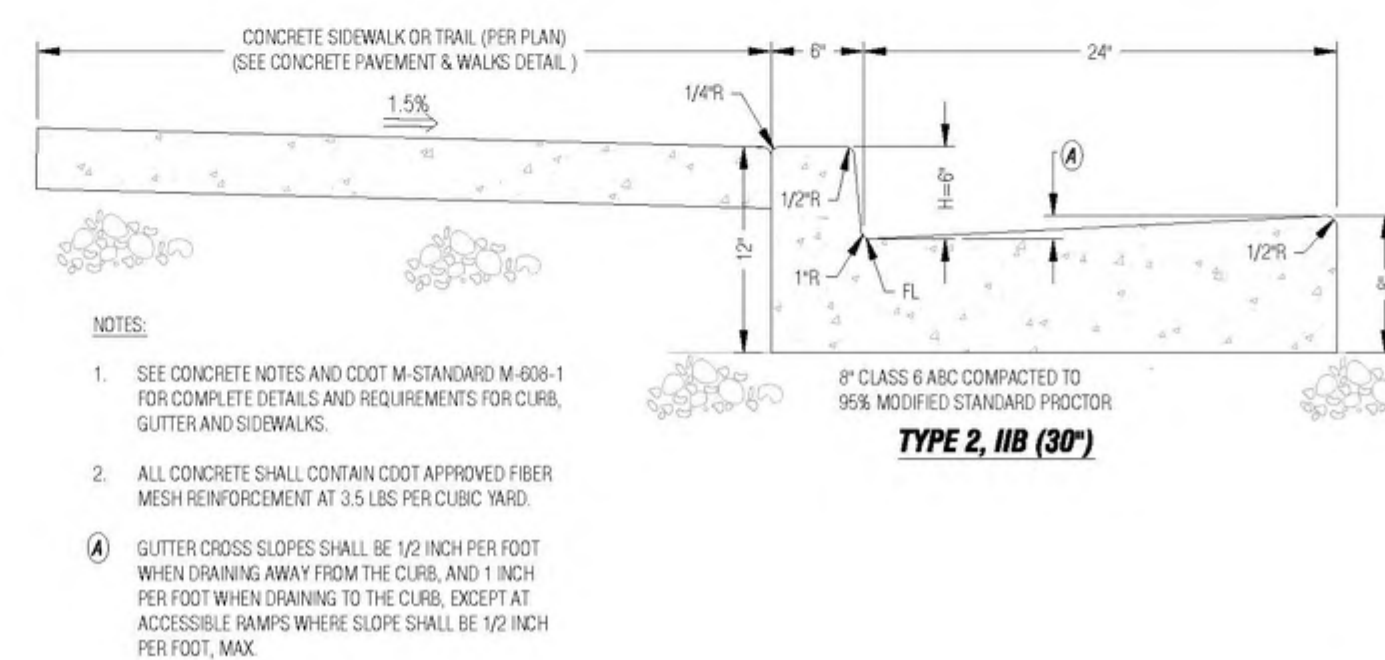
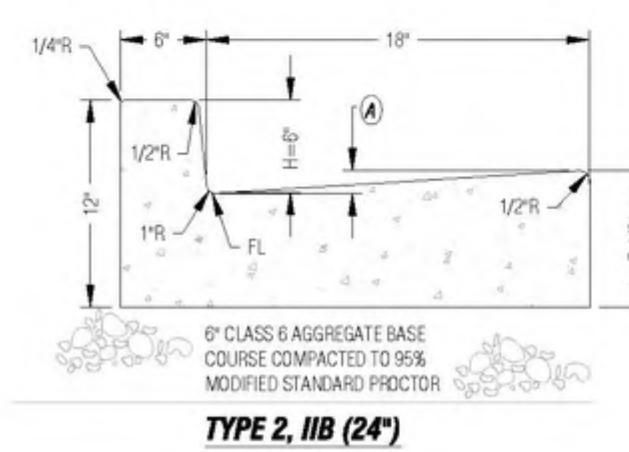
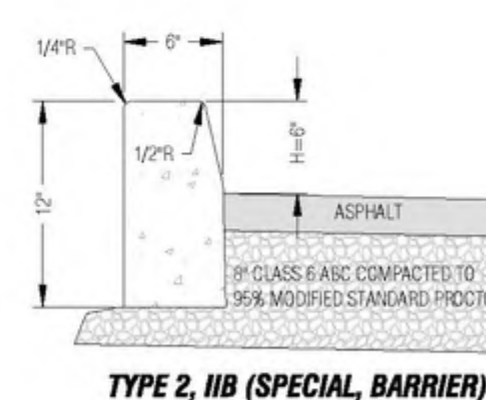
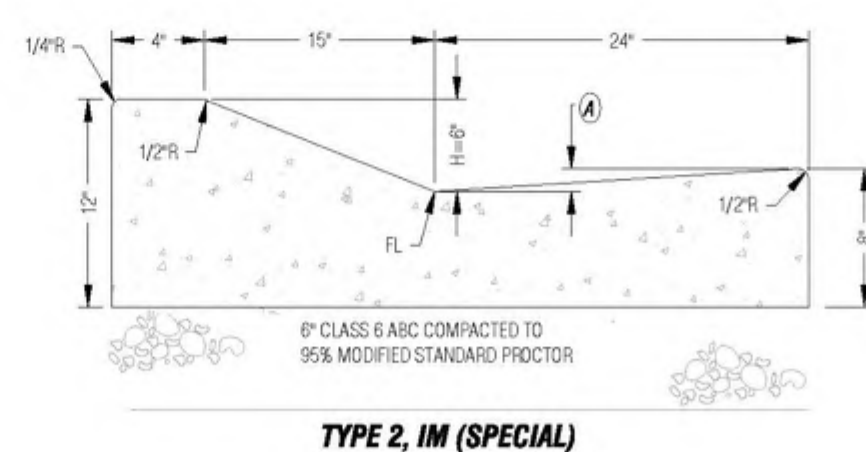
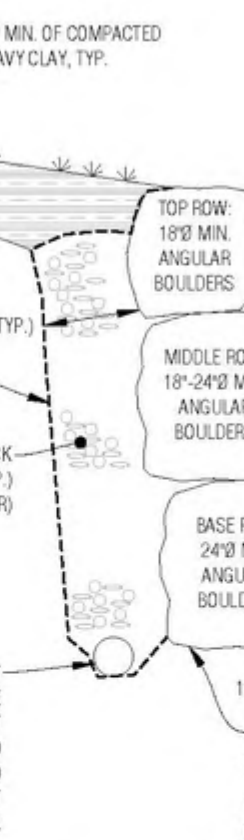
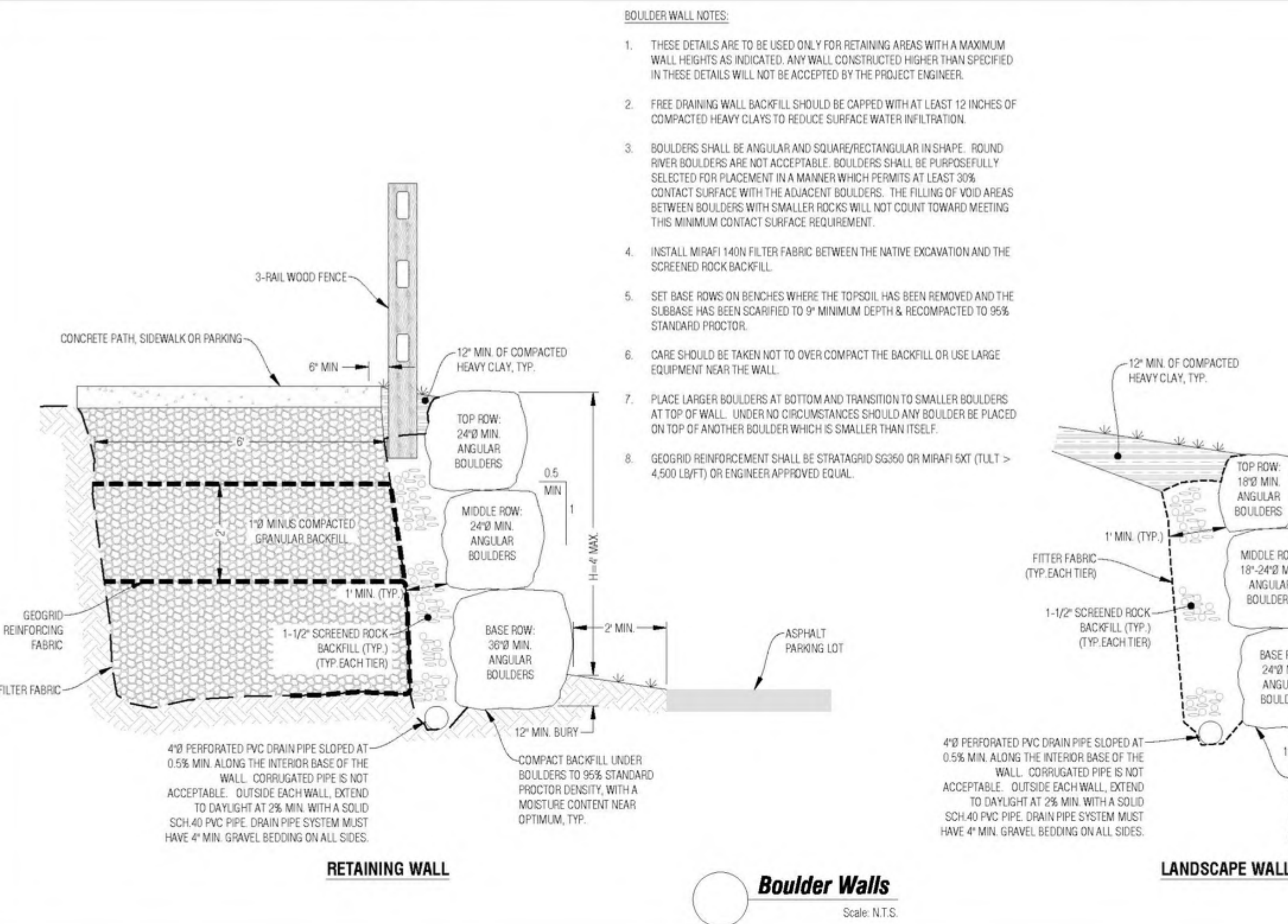
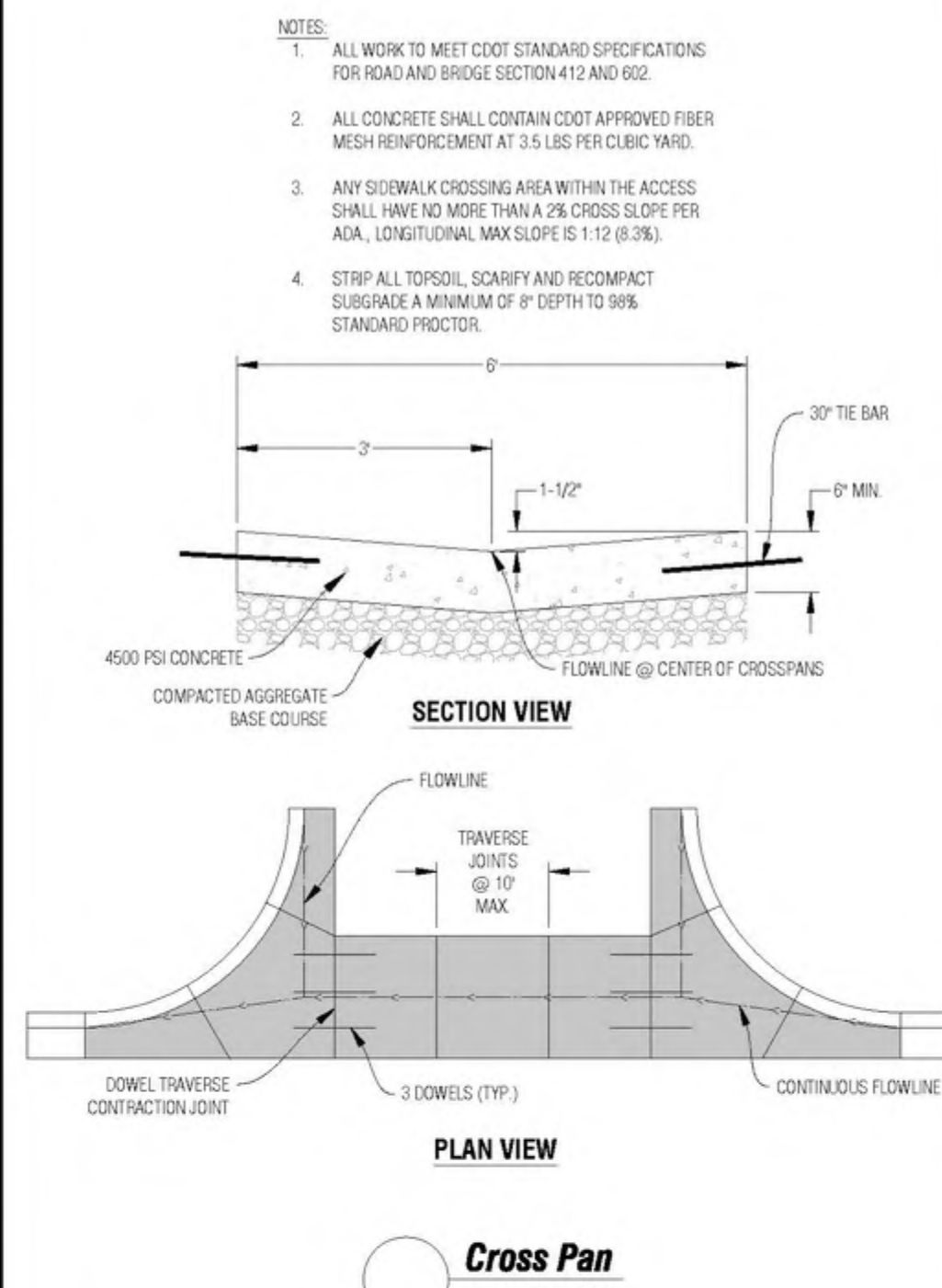
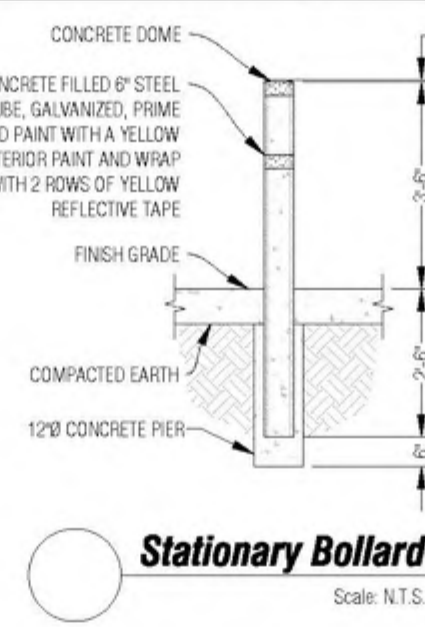
Point Table				
Point #	Raw Description	Elevation	Northing	Eastng
278	Curb-FL	5722.23	42505.57	15772.41
279	Curb-FL	5723.03	42519.17	15780.28
280	Curb-FL	5723.64	42531.10	15789.97
281	Curb-FL	5724.32	42544.29	15800.69
290	BOW	5722.54	42401.30	15814.61
291	BOW	5722.42	42424.10	15804.35
292	BOW	5722.31	42443.83	15795.46
293	BOW	5722.16	42471.43	15783.06
294	BOW-Ramp	5722.25	42487.12	15779.59
300	Curb-FL	5726.73	42618.21	15780.33
301	FG	5726.81	42618.33	15777.26
302	FG	5727.11	42619.66	15764.97
303	FG	5727.34	42642.58	15751.93
304	FG	5727.59	42667.61	15743.64
305	FG	5727.90	42678.25	15750.32
306	Swale-FL	5727.00	42682.92	15728.46
307	Swale-FL	5726.88	42684.28	15734.99
308	Swale-FL	5726.49	42639.54	15743.63
309	Swale-FL	5725.67	42612.65	15753.02
310	Swale-FL	5725.21	42597.75	15758.22
311	Inlet-Rim	5724.76	42588.16	15762.91
400	CL-FG	5725.21	42562.37	15797.35
401	CL-FG	5725.53	42568.64	15802.44
402	CL-FG	5726.42	42586.00	15816.54
403	CL-FG	5726.92	42594.25	15823.26
404	CL-FG	5726.81	42615.22	15840.30
405	CL-FG	5730.59	42633.43	15855.11
406	CL-FG	5731.17	42650.52	15869.00
407	CL-FG	5732.53	42676.89	15880.43
408	CL-FG	5733.17	42689.30	15900.52
409	CL-FG	5733.56	42698.94	15908.36
410	CL-FG	5733.82	42718.79	15924.49
411	CL-FG	5734.08	42737.74	15939.89
412	CL-FG	5734.27	42753.71	15952.87
413	CL-FG	5734.49	42774.48	15960.03
414	CL-FG	5734.38	42794.98	15962.11
415	CL-FG	5734.21	42820.98	15929.34
416	CL-FG	5734.08	42839.79	15912.87
417	CL-FG	5733.94	42861.93	15893.48
418	CL-FG	5733.76	42888.98	15869.80
419	CL-FG	5733.57	42916.53	15845.67
420	CL-FG	5734.81	42939.70	15825.38
421	CL-FG	5726.71	42800.67	15815.11
422	CL-FG	5726.71	42804.66	15810.46
423	CL-FG	5727.13	42811.39	15802.20
424	CL-FG	5727.16	42819.77	15793.29
480	Curb-FL	5724.80	42553.54	15808.22
481	Curb-FL	5725.18	42561.09	15814.37
482	Curb-FL	5725.48	42565.79	15820.45
483	Curb-FL	5725.98	42567.55	15833.21
484	Curb-FL	5725.98	42562.80	15842.76
485	EOA	5725.78	42556.37	15849.23
486	EOA	5725.41	42542.82	15859.32
487	EOA-Match	5725.43	42537.52	15862.52
471	EOA	5727.19	42572.13	15869.53
472	EOA	5727.19	42591.35	15875.19
474	EOA-Match	5725.93	42548.36	15884.02
475	EOA	5725.89	42555.24	15879.86
476	EOA	5726.22	42570.80	15868.51
477	EOA	5727.19	42587.16	15851.00

Point Table				
Point #	Raw Description	Elevation	Northing	Eastng
478	EOA	5727.79	42595.48	15847.33
479	EOA	5728.34	42603.34	15851.91
480	EOA	5730.28	42611.28	15878.11
481	EOA	5728.88	42600.07	15910.47
482	EOA-Match	5728.12	42588.36	15922.05
483	EOA-Match	5728.57	42605.78	15941.41
490	Pan-FL	5727.14	42610.52	15819.09
491	Inlet-Rim	5726.52	42602.44	15812.52
492	Inlet-Rim	5726.31	42599.15	15809.85
493	Pan-FL	5726.07	42595.00	15806.46
494	Curb-FL	5724.84	42569.94	15788.04
495	Curb-FL	5725.59	42581.65	15793.88
496	Curb-FL	5726.29	42594.15	15794.29
497	Curb-FL	5726.54	42599.73	15792.72
498	Curb-FL	5726.62	42608.78	15787.19
499	Curb-FL	5726.79	42610.45	15785.59
500	EOA	5729.00	42619.91	15927.28
501	EOA	5730.12	42631.75	15912.71
502	EOA	5731.24	42643.58	15898.15
503	EOA	5731.94	42653.68	15882.69
504	EOA	5732.61	42666.21	15897.22
505	EOA	5732.89	42669.79	15902.84
506	EOA	5733.19	42657.33	15914.48
507	EOA	5733.21	42678.29	15931.51
508	Curb-FL	5734.56	42708.22	15954.22
509	Curb-FL	5734.17	42717.58	15940.25
510	Curb-FL	5732.73	42722.86	15943.26
511	Curb-FL	5734.23	42723.30	15947.48
512	Curb-FL	5734.59	42713.21	15959.90
513	EOA	5734.89	42734.16	15976.93
514	EOA	5735.25	42762.09	15999.63
515	EOA	5735.04	42771.21	15988.42
516	EOA	5735.08	42777.32	15988.31
517	EOA	5735.10	42779.12	15990.36
518	FG	5734.84	42798.66	15973.25
519	EOA	5735.29	42787.47	15999.89
520	EOA	5735.13	42807.00	15982.78
521	EOA	5734.83	42841.64	15952.46
522	EOA	5734.53	42831.75	15941.17
523	EOA	5734.43	42832.50	15935.20
524	EOA	5734.47	42838.21	15934.92
525	EOA	5734.78	42848.41	15946.53
526	EOA	5734.80	42875.49	15922.81
527	EOA	5734.42	42902.57	15899.10
528	EOA	5734.12	42892.69	15887.81
529	EOA	5734.02	42893.44	15881.84
530	EOA	5734.07	42899.16	15881.48
531	EOA	5734.38	42909.35	15893.17
532	EOA	5734.81	42933.04	15872.42
533	EOA	5735.29	42958.74	15851.68
534	EOA	5734.99	42946.86	15840.38
535	EOA	5735.03	42947.23	15834.74
536	Curb-FL	5735.11	42948.74	15833.42
537	Curb-FL	5734.49	42932.93	15815.36
538	Curb-FL	5734.41	42931.42	15816.68
539	Curb-EOA	5734.49	42925.78	15816.31
540	Curb-EOA	5734.70	42915.90	15805.02
541	Curb-EOA	5733.62	42895.58	15822.81
542	Curb-EOA	5733.41	42905.47	15834.09
543	Curb-FL	5733.23	42905.09	15839.74

Point Table				
Point #	Raw Description	Elevation	Northing	Easting
544	Curb-FL	5733.38	42881.07	15860.77
545	Curb-FL	5733.59	42854.03	15884.45
546	Curb-EOA	5733.77	42848.38	15884.08
547	Curb-EOA	5734.00	42838.50	15872.79
548	Curb-EOA	5734.04	42831.73	15878.72
549	Curb-EOA	5734.08	42825.71	15883.99
550	Curb-EOA	5734.62	42820.11	15888.22
551	Curb-EOA	5734.26	42798.63	15907.71
552	Curb-EOA	5734.40	42778.31	15925.50
553	Curb-EOA	5734.17	42788.20	15936.78
554	Curb-FL	5734.00	42787.82	15942.43
555	Curb-FL	5734.12	42774.26	15948.03
556	Curb-FL	5734.04	42781.99	15944.11
557	Curb-EOA	5734.09	42781.49	15938.54
558	Curb-EOA	5734.28	42770.94	15926.92
559	Curb-EOA	5734.10	42756.97	15915.57
560	Curb-EOA	5734.01	42749.98	15909.89
561	Curb-EOA	5733.93	42743.76	15904.85
562	Curb-EOA	5733.86	42738.34	15900.43
563	Curb-EOA	5733.57	42715.84	15882.14
564	Curb-EOA	5733.34	42706.38	15893.78
565	Curb-FL	5733.02	42700.38	15894.05
566	FG	5732.98	42699.39	15888.10
567	FG	5733.21	42708.85	15876.46
568	Curb-EOA	5732.85	42699.72	15869.04
569	Curb-EOA	5732.89	42693.49	15863.98
570	Curb-EOA	5732.73	42687.18	15858.84
571	Curb-EOA	5731.39	42658.20	15835.29
579	Curb-EOA	5731.28	42652.58	15831.05
580	Curb-EOA	5730.33	42643.52	15842.69
581	Curb-FL	5729.74	42637.89	15843.27
582	Curb-FL	5728.44	42622.79	15830.99
583	Curb-FL	5728.02	42617.31	15820.73
584	Curb-FL	5727.82	42620.70	15809.77
585	Curb-FL	5727.84	42627.55	15802.45
586	Curb-FL	5727.67	42629.02	15801.07
587	FG	5733.89	42715.84	15873.76
588	FG	5733.49	42703.82	15863.99
589	FG	5733.45	42697.59	15858.93
590	FG	5733.32	42691.26	15853.80
591	FG	5732.02	42684.71	15832.20
592	FG	5732.03	42684.74	15829.77
593	FG	5731.11	42683.00	15821.19
594	FG	5731.11	42688.87	15819.97
595	TOWNFG	5731.02	42681.47	15815.39
596	FG	5730.81	42680.49	15811.52
597	FG	5730.21	42686.40	15816.01
598	TOWNFG	5731.52	42670.16	15819.61
599	TOWNFG	5731.94	42651.45	15824.75
800	FG	5729.64	42658.03	15822.37
801	Pan-FL	5733.20	42608.10	15837.10
822	TOWNFG	5731.78	42652.91	15830.35
803	FG	5729.39	42653.79	15825.84
804	FG	5729.23	42640.83	15826.13
805	TOWNFG	5731.39	42647.37	15833.71
806	FG	5730.33	42640.17	15839.79
807	TOWNFG	5730.83	42640.89	15840.48
820	Pan-FL	5734.29	42528.41	15819.31
822	Pan-FL	5733.62	42651.02	15867.09
823	Pan-FL	5733.09	42831.46	15904.28



STANDARD PLAN NO.
M-208-1
Sheet No. 6 of 12



Site Use Calculations

Hatched area #1 (parking area) = 91,379 sq. ft.
Hatched area #2 (building and pool area) = 55,561 sq. ft
Total site area = 146,940 sq. ft.

Common Open Space Area (10% of total site area)

Total site area = 146,940 sq. ft.
129,946 sq. ft x 10% = 14,694 sq. ft required Common Open Space
Actual Common Open Space = 44,927 sq. ft. (Area #2 less buildings and pool)

Minimum Landscaped Site Area (10% of total site area)

Total site area = 146,940 sq. ft
146,940 sq. ft x 10% = 14,694 sq. ft required landscaped site area
Actual Landscaped Site Area = 25,821 sq. ft. (not including parking landscape)

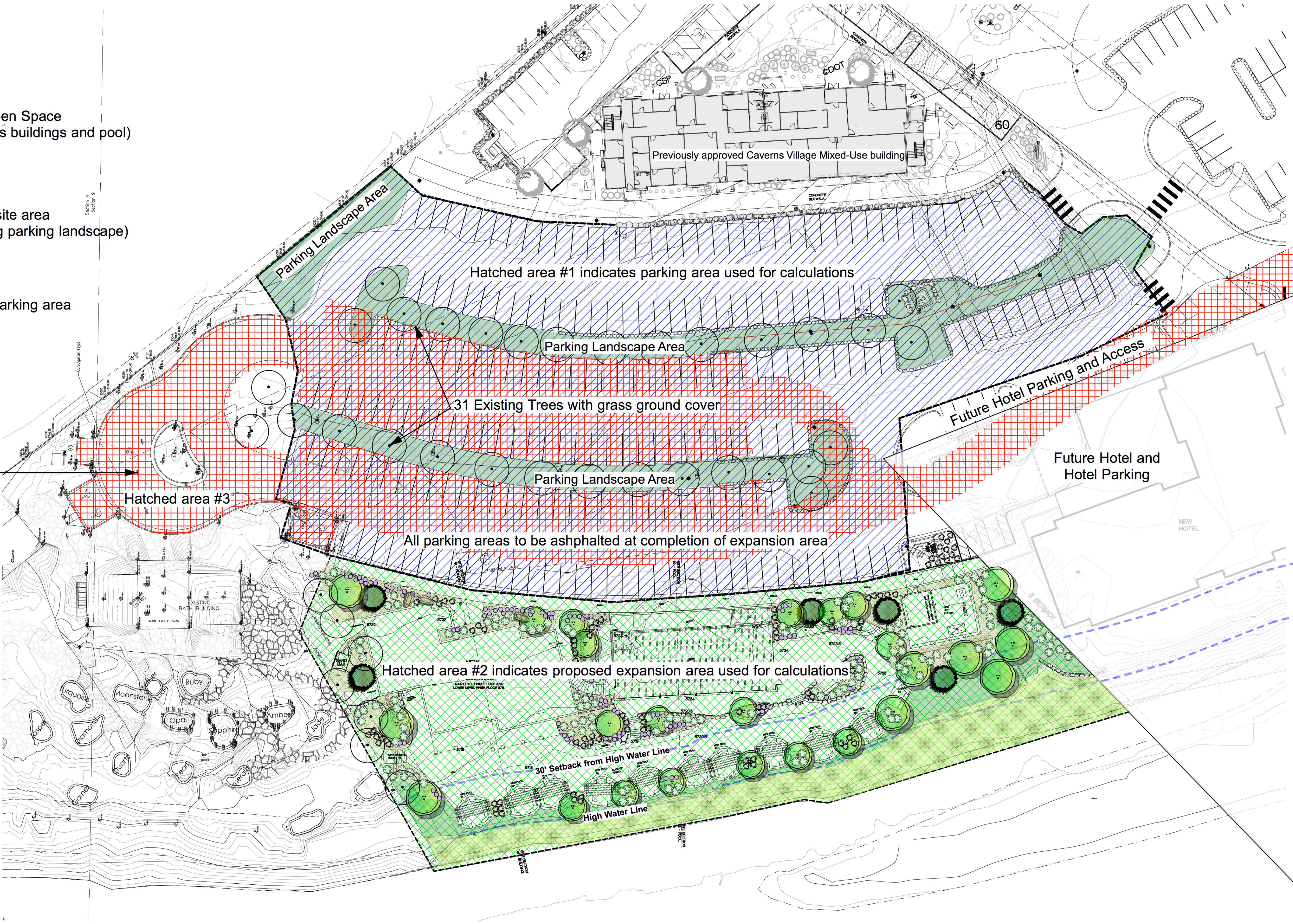
Parking Landscaped Area

Parking surface area = 74,479 sq. ft
74,479 sq. ft x 20% = 14,895 sq. ft required landscaped parking area
Actual Parking Landscaped Area = 16,900 sq. ft.

Landscape Requirements

Perennials: 1 per 50 s.f. of required landscape = 284
Shrubs: 1 per 100 s.f. of required landscape area = 142
Trees: 1 per 400 s.f. of required landscape area = 36

Hatched area #3 designates the approved parking and acces extents as shown on the "Recently Constructed Site Features" plan sheet prepared by SGM, dated 06/30/2015. Hatched area contains 94 parking stalls.



Landscape Plan

Iron Mountain Hot Springs Expansion

Parcel ID #218509200031

Glenwood Springs, Colorado
Updated August 31, 2020 by The Land Studio, Inc.

The **LAND** Studio
365 River Bend Way
Glenwood Springs, CO 81601
970.927.3690
landstudio2@comcast.net

Sheet
2 of 2



Planning and Zoning Commission Report

Date: October 27, 2020
To: Planning and Zoning Commission
From: Trent L. Hyatt, Senior Planner
Subject: #51-20 - Special Use Review for a Retail Marijuana Establishment, Kind Castle

REQUEST	Consideration of a request for a special use permit for a retail marijuana establishment
APPLICANT	Chris Hawkins/Alpine Planning, LLC
OWNER	Glenwood Meadows, LLC
LOCATION	150 Wulfsohn Road, Lot 11B Glenwood Meadows No. 5, A Minor Subdivision of Glenwood Meadows No. 4 Lot Line Adjustment (PIN: 2185-081-41-004)
ZONE	Mixed-Use Core (M2) District
SURROUNDING LAND USES	North: Hotel lodging South: Multi-family residential East: Civic West: Multi-family residential
LOT SIZE	Approximately 0.40 acres/17,816 square feet

ACTION ITEM:

The Planning and Zoning Commission (Commission) is the recommending body to City Council on marijuana related uses requiring a special use permit.

Action 1 - Special Use Permit - for a retail marijuana establishment in the Mixed-Use Core (M2) District.

Staff recommendation: *Staff recommends **approval (with conditions)** of the Special Use Permit with the findings and conditions on page 8.*

BACKGROUND/ANALYSIS

Proposal

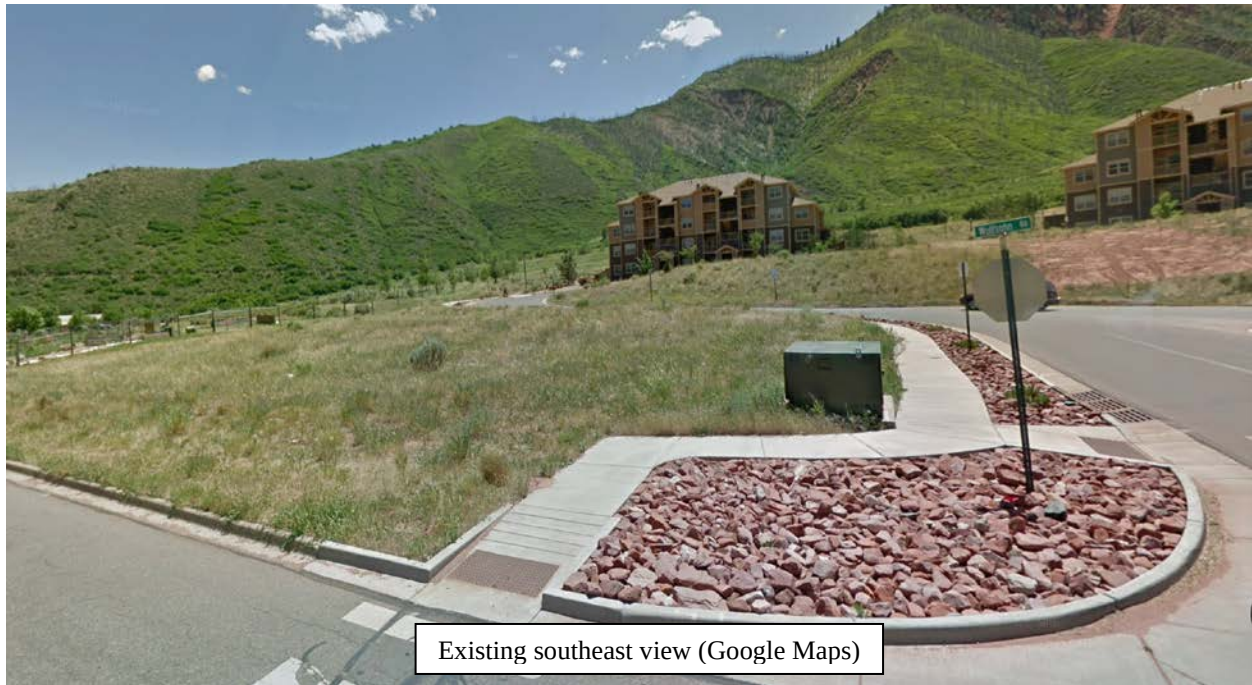
The applicant, Chris Hawkins of Alpine Planning, LLC on behalf of Kind Castle, proposes the construction a new approximately 4,504 square foot, two unit retail building at 150 Wulfsohn Road to accommodate the retail sale of cannabis and related products as well as another retail use to be determined at a later date. The retail marijuana establishment will occupy 3,720 square feet of the proposed building, whereas, the future retail use will occupy 784 square feet.

The proposed use falls under the definition of a *retail marijuana establishment* as defined by Section 070.070.020(c)(7)b of the *Glenwood Springs Municipal Code* (Code) outlined in *italics* below. This use requires additional licensing through the State of Colorado and City of Glenwood Springs.

Retail marijuana establishment means the retail sale, cultivation, storage, product manufacturing or testing of marijuana to the extent permitted by Article XVIII, Section 16 of the Colorado Constitution and in various provisions of the Colorado Retail Marijuana Code, C.R.S. Article 43.4 of Title 12, and any other applicable state or city law or regulation.

Special Review Criteria

There are two sets of criteria in the Municipal Code to consider in the analysis of marijuana related businesses. Section 070.030.030(e)(11) of the Code are use specific standards which apply to retail marijuana businesses. Section 070.060.050(e)(3)e.2 applies generally to special review uses. Staff lists each of the criteria in **bold** below along with comments.



Section 070.030.030(e)(11) – Retail Marijuana Establishment

b. License Required

Retail marijuana establishments shall comply with City Code Article 050.090.

The applicant shall apply for a City business license for the proposed use prior to future operation.

c. Location of Licensed Establishment Retail marijuana establishments shall be located only in areas of the city as allowed in the City Code. No retail marijuana establishment shall be located:

- 1. Within 500 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade. The distance shall be property computed by a straight line measurement from the nearest property line of the school to the nearest property line of the site housing the retail marijuana establishment;**

The retail marijuana establishment is located over 1,700 feet from Yampah Mountain High School to the southeast, the closest in proximity.

- 2. Within 900 feet of another retail marijuana establishment or a medical marijuana business. A retail marijuana establishment may locate on the same licensed premises as a medical marijuana business of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other retail marijuana establishments and medical marijuana businesses. This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities; and**

The proposed retail establishment is located over 1,800 from The Green Dragon along Devereux Road to the northwest.

- 3. In any zoning district not specifically permitted by the City Code.**

The use is proposed in a zone district, M2, which allows retail marijuana establishments. Additionally, the Commission determined the use is permitted in the Glenwood Meadows development at a special meeting on August 5, 2020.

d. Requirements Related to Operation of a Retail Marijuana Establishment

- 1. Retail marijuana establishments shall meet all operational criteria for the procurement, display, dispensing, labeling, sanitation, security, and any other criteria as required by the state pursuant to the CRMC and the City Code Article 050.090.**

The operation shall apply for and receive all applicable permits from the state and City and maintain compliance therewith.

- 2. In addition, all retail marijuana establishments shall comply with the following local restrictions:**
 - i. Hours of operation shall be limited to between 8:00 a.m. and 7:00 p.m. Monday through Sunday; and**

The business shall comply with the restricted hours of operation.

- ii. A sign permit shall be obtained through the Community Development Department. All exterior signage associated with a retail marijuana establishment will meet the standards established in the CRMC and in the City Code.**

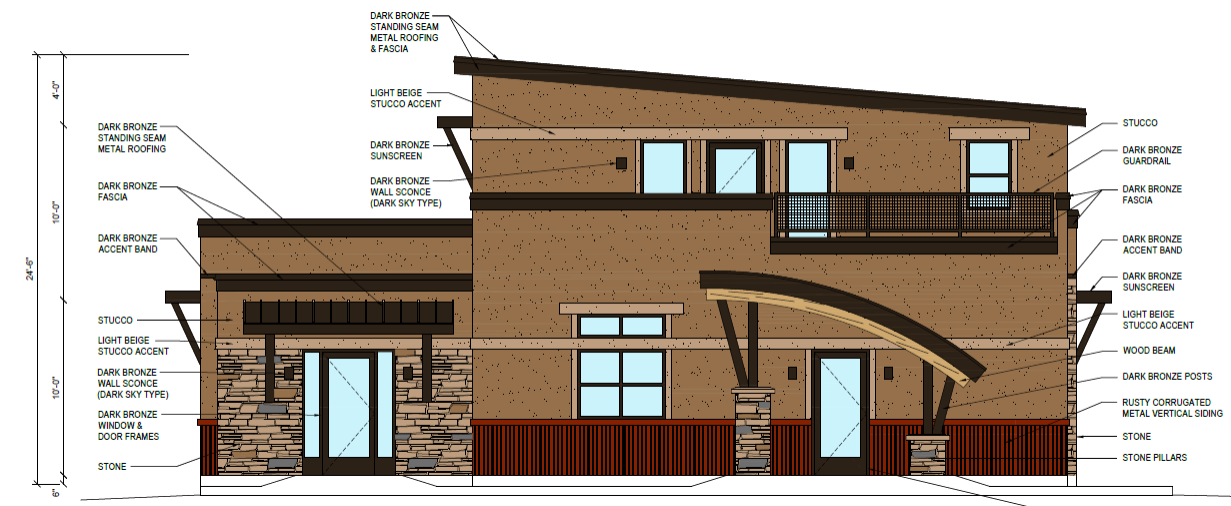
The applicant shall apply for and receive a sign permit prior erecting any signage at the site.

- iii. Storage of retail marijuana shall be considered an accessory use. Not more than 25 percent of the licensed premises of a retail marijuana business shall be used for such purposes.**

All cannabis product stored onsite shall be for retail sale. Significantly less than 25 percent of the floor plan is proposed for storage at approximately 289 square feet, or nine percent.

iv. No discernible odor shall be projected beyond the exterior walls of the licensed premises.

Odor is a concern with any marijuana related business and is an expected nuisance. However, the issue is more closely related to grow facilities as the City has generally not received complaints for sales only businesses. That being said, the applicant proposes a negative air heating and air conditioning system having an activated carbon filter exhaust system. Additionally, four independent activated carbon filtration units will operate inside the retail establishment to combat any potential odors at the site. In addition, the applicant proposes off site prepackaging of flower products. As is the norm with all applications associated with marijuana businesses, staff is recommending a condition of the special use permit that requires compliance with the above criteria at all times. In the event that documented inspections of the premises by City staff, preceded by complaints, indicate that odor is an issue, the applicant will need to employ alternative methods to mitigate odors or be at risk of losing their City license.



Section 070.060.050(e)(3)e.2 – All Land Uses

i. The use will be compatible with the surrounding area;

Similar retail uses, are currently operating in the vicinity of the site in the Glenwood Meadows development. A mix of uses are located in close proximity including lodging, multi-family residential, veterinary services, and banking/office. It appears the use is compatible with the surrounding area.

ii. The impacts of the use on surrounding areas have been adequately minimized;

The Engineering Department has requested additional information related to traffic impacts for the use/site. Various mitigation techniques, such as a right in-right out access or a four way stop at the adjacent intersection, has been discussed to mitigate the issue.

the proposed use is not expected to create any additional impacts than currently exist at the site.

iii. The use will be consistent with the general purpose and intent of this Code;

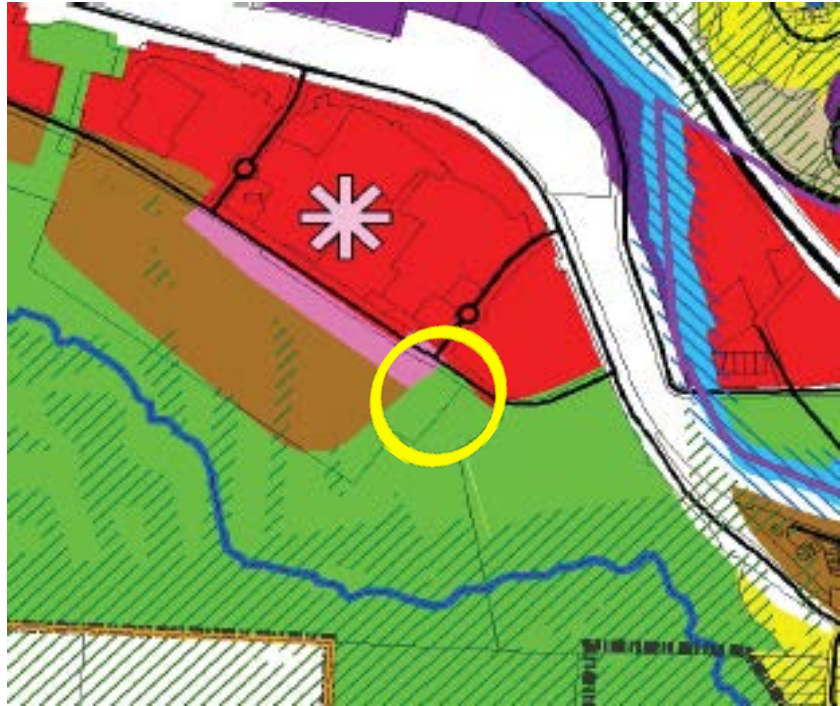
The use/proposal appears to be consistent with the general purpose and intent of the Code.

iv. The use will comply with all applicable standards of this Code;

The use is in compliance with the Code or will be following amendments by the applicant related to staff comments.

v. The use is in conformance with the Comprehensive Plan and other City plans and policies; and

As shown below, Map 2.1 Future Land Use – North Portion of the City of the Glenwood Springs of the Comprehensive Plan (Comp Plan) identifies the property as being split between the “Mixed-use” (in fuschia) and “Parks Open Space” (green) classifications. The parcel was rezoned M2 by City Council in June 2020 so the Mixed-use classsication was intended and is appropriate. According to the Comp Plan, the “Mixed-use” designation “...allows for a variety of uses including commercial, retail, office,restaurant, entertainment and multi-family housing co-existing through design either in a horizontal or vertical fashion” in line with the current proposal



Numerous community goals and related objectives and strategies outlined in the Executive Summary and subsequent chapters of the Comp Plan encourage economic development, preserving our small-town character, convenient transportation, infill development, housing opportunities, social diversity, and preserving natural resources all of which are supported by this proposal. These goals are outlined in the following table.

<u>Comprehensive Plan Goal Supported</u>	<u>Objectives and Strategies</u>
-Promote long-term, sustainable, diverse, economic development	-Direct development into a compact form: infill and redevelopment (Chapter 3, Page 34) -Attract diverse businesses and industries (Chapter 4, Page 50) -Maintain the role as regional center (Chapter 4, Page 54) -Retain role as a regional center (Chapter 7, Page 83)
Staff Comment: The objective/strategy outlined above are supported by the current application	

as it includes infill retail development/redevelopment.	
-Maintain Glenwood Springs as the regional tourism, retail, commercial, and governmental center of Garfield County	-Attract diverse businesses and industries (Chapter 4, Page 50) -Maintain the role as regional center (Chapter 4, Page 54) -Retain role as a regional center (Chapter 7, Page 83)
Staff Comment: Creating additional space for retail, as proposed, helps to maintain/retain the City's role as a regional center.	
-Preserve small town character while maintaining the livability of Glenwood Springs and increasing the vibrancy and commercial success of the downtown	-Develop according to the vision (Chapter 4, Page 34) -Direct development into a compact form: infill and redevelopment (Chapter 3, Page 34) -Enhance pedestrian-friendliness while facilitating traffic (Chapter 3, Page 36) -Maintain the role as regional center (Chapter 4, Page 54) -Retain role as a regional center (Chapter 7, Page 83) -Direct growth inward (Chapter 9, Page 102)
Staff Comment: Infill, as proposed, makes improvements more cost effective. The use is in character with surrounding the site area and aligns with zone district purpose.	
-Address transportation and provide multiple convenient travel choices	-Direct development into a compact form: infill and redevelopment (Chapter 3, Page 34)
Staff Comment: Infill, as proposed, is a land use strategy used to address transportation issues per the Comp Plan.	
-Direct development to locations and building forms that are cost effective to serve	-Direct development into a compact form: infill and redevelopment (Chapter 3, Page 34) -Direct growth to areas where utilities and services can be provided efficiently (Chapter 7, Page 82) -Retain role as a regional center (Chapter 7, Page 83) -Direct growth inward (Chapter 9, Page 102)
Staff Comment: The objective/strategy outlined above are supported by the current application as it includes infill development/redevelopment.	
-Support social diversity	-Direct development into a compact form: infill and redevelopment (Chapter 3, Page 34) -Retain role as a regional center (Chapter 7, Page 83) -Direct growth inward (Chapter 9, Page 102)
Staff Comment: The objective/strategy outlined above are supported by the current application as it includes infill development/redevelopment.	
-Preserve natural resources	-Promote water conservation (Chapter 7, Page 84)

	-Direct growth inward (Chapter 9, Page 102)
Staff Comment: The objective/strategy outlined above are supported by the current application as it includes infill development/redevelopment.	

vi. The use minimizes adverse impacts to the health, safety, and welfare of the inhabitants of the surrounding areas and the city.

The use/proposal does not appear to negatively impact health, safety, and welfare of the inhabitants of the surrounding areas and the city.

Reviewing Agency Comments

An overview of comments received from referral agencies is provided below. Said comments are also attached.

Electric Department-Comments address the need to comply with the City's line extension, potential conflict between proposed bicycle parking and transformer location, and the need for electric load information.

Engineering Department-Comment address the need for a utility plan, updates to the traffic analysis, restricted access movements or a four way stop at the adjacent intersection, and submittals required for the Construction Plans review phase, if applicable.

Public Works-Outline the need for various line extension to the site and related street cuts, restricted access to the site, electric load requirements, pedestrian access, and fire suppression water service needs.

Transportation-Outline the need for bicycle parking at the site.

Adjacent Property Owner Comments

None were received as of the drafting of the staff report.

ACTION ALTERNATIVES AND STAFF RECOMMENDATION:

Section 070.060.050(e)(1)e.1 of the Code explains, "The Planning Commission shall review and approve, approve with conditions, or deny the special use permit application in accordance with 070.060.030(g) and the approval criteria in paragraph 2 below [addressed above in in this instance]. The Planning Commission may also remand the application back to the Director for further consideration." Section 070.060.030(g)(1)d also states, "The body may also continue the hearing or refer the application to another authority as provided in 070.060.050(a)(2)c, *Referral and Call-Up Procedures*. In the instance of marijuana related business, Section 070.060.020(d)/ Table 060.1 Summary of Development Review Procedures outlines the Commission is the review and reminding authority to City Council.

Action 1 - Special Use Permit - for a retail marijuana establishment in the Mixed-Use Core (M2) District.

Staff recommendation: Staff recommends **approval with conditions** of the Special Use Permit application with the **findings** and **conditions** below:

Recommended Findings:

1. The use is compatible with the surrounding area;
2. The impacts of the use on surrounding areas have been adequately minimized;
3. The use is consistent with the general purpose and intent of this Code;
4. The use complies with all applicable standards of this Code;
5. The use is in conformance with the Comprehensive Plan and other City plans and policies; and
6. The use minimizes adverse impacts to the health, safety, and welfare of the inhabitants of the surrounding areas and the city.

Recommended Conditions:

1. The applicant shall provide evidence of approval of licenses from the State of Colorado and the City of Glenwood Springs for a retail marijuana establishment prior to operating the Special Use.
2. Any future physical changes or improvements to the property or buildings on the parcel are subject to the review and approval of the Community Development Director for compliance with applicable requirements of the *Glenwood Springs Municipal Code* and the *Glenwood Meadows Zoning and Development Plan*.
3. The proposed retail marijuana establishment shall comply with all verbal representations made by the applicant during the public hearings and plans/application materials submitted by the applicant or revised per conditions of the Special Use Permit and approved by the Community Development Director prior to issuance of a Special Use Permit.
4. Any changes to signage on the site shall require a sign permit and demonstrated compliance with applicable requirements of the of the *Glenwood Springs Municipal Code*.
5. The use shall comply with the standards and special review criteria for marijuana uses specified in the *Glenwood Springs Municipal Code* at all times. Failure to comply with the applicable standards may result in an enforcement action or the incorporation of additional mitigation methods for Code compliance.
6. If complaints are received from adjacent property owners regarding any odors emitting from the site or any other matter regulated by the Code, City Council shall have the ability to re-evaluate or revoke the special use permit. The special use permit shall be subject to reconsideration and revocation by City Council within sixty (60) days of notification of the violation if the business owners are convicted by the City's Municipal Court of two *Glenwood Springs Municipal Code* nuisance violations pertaining to odors, etc. emitting from the site within a six (6) month period.
7. The applicant shall submit a building permit application as required. All contractors must receive a license to work in the City of Glenwood Springs.
8. The applicant shall address and comply with all comments received from referral agencies and as otherwise required by the *Glenwood Springs Municipal Code*.
9. Prior to submitting a building or grading permit application, the applicant shall:
 - a. Submit a Construction Plans application in accordance with Municipal Code section 070.060.050(c);
 - b. Enter into a development agreement with the City which shall be recorded with the Garfield County Clerk and Recorder;
 - c. Submit an engineer's estimate of all public improvement and submit a financial security for any required improvement not yet completed;
 - d. Schedule a pre-construction meeting with applicable City departments;
 - e. The applicant shall obtain a grading permit from Engineering Department.

PROPOSED NEW DISPENSARY:
THE KIND CASTLE
(GLENWOOD MEADOWS, LOT 11B) WULFSOHN ROAD
GLENWOOD SPRINGS, CO 81601

ABBREVIATIONS

@	-AT		
#	-NUMBER	LBR.	-LUMBER
A.B.	-ANCHOR BOLT	LBS.	-POUNDS
A.F.F.	-ABOVE FINISH FLOOR		
ARCH.	-ARCHITECTURAL	MAX.	-MAXIMUM
		MTL.	-METAL
BLDG.	-BUILDING	MIN.	-MINIMUM
BM.	-BEAM	M.O.	-MASONRY OPENING
BRG.	-BEARING	MECH.	-MECHANICAL
BLK.	-BLOCK	MOIST.	-MOISTURE
C/C	-CENTER TO CENTER	N.I.C.	-NOT IN CONTRACT
CLG.	-CEILING	N/A	-NOT APPLICABLE
C.J.	-CONTROL JOINT	N.T.S.	-NOT TO SCALE
CONC.	-CONCRETE	O.C.E.W.	-ON CENTER EACH WAY
CONT.	-CONTINUOUS	O.C.	-ON CENTER
C/L	-CENTER LINE	O.H. DR.	-OVERHEAD DOOR
DBL.	-DOUBLE		
DIA.	-DIAMETER	PEMB	-PRE ENG. METAL BLDG
DWG.	-DRAWING	PSI	-POUNDS PER SQUARE INCH
		PSF	-POUNDS PER SQUARE FT.
EA.	-EACH	PLMG.	-PLUMBING
ELECT.	-ELECTRICAL	PRE-ENG	-PRE-ENGINEERED
EQ.	-EQUAL	PLYWD.	-PLYWOOD
EXIST.	-EXISTING	PL.	-PLATE
EXT.	-EXTERIOR		
		REQ.	-REQUIRED
F.D.	-FLOOR DRAIN	REINF.	-REINFORCED/REINFORCING
FLR.	-FLOOR	R.O.	-ROUGH OPENING
FOUND.	-FOUNDATION	RESIST.	-RESISTANT
FT.	-FOOT/FEET	RETARD.	-RETARDANT
FBRGLS.	-FIBERGLASS		
FTG.	-FOOTING	S.F.	-SQUARE FOOT/FEET
		STD.	-STANDARD
GYP. BD.	-GYPSUM BOARD	SQ.	-SQUARE
G.F.I.	-GROUND FAULT INTERRUPT	STL.	-STEEL
		STYRO.	-STYROFOAM
IN.	-INCHES/INCHES	SH.	-SHELF
INSUL.	-INSULATION		
INT.	-INTERIOR	T & B	-TOP AND BOTTOM
JT.	-JOINT	TYP.	-TYPICAL
L.	-LONG	WD.	-WOOD
LAV.	-LAVATORY		
		W.W.M.	-WELDED WIRE MESH
		W/	-WITH

PROJECT INFORMATION

PROPERTY ADDRESS:	LOT 11B, GLENWOOD MEADOWS, FILING 5 WULFSOHN ROAD GLENWOOD SPRINGS, COLORADO 81601
PROPERTY OWNER:	GLENWOOD MEADOWS LLC 710 E DURANT AVENUE, SUITE W6 ASPEN, CO 81611
APPLICANT:	KIND WEST GLENWOOD, LLC 1888 SHERMAN STREET, SUITE 200 DENVER, CO 80203
ZONING:	M2 MIXED USE CENTRAL CORE

SHEET INDEX

CS1	COVER SHEET
SP1	PROPOSED SITE CONCEPT PLAN
LS1	PROPOSED LANDSCAPING CONCEPT PLAN
LS2	PROPOSED PLANT SCHEDULE & LANDSCAPE AREA PLANS
C2	CONCEPTUAL GRADING PLAN
A1	PROPOSED EXTERIOR ELEVATIONS
A2	PROPOSED EXTERIOR ELEVATIONS
A3	PROPOSED FLOOR PLAN - MAIN LEVEL
A4	PROPOSED FLOOR PLAN - UPPER LEVEL

PROJECT SUMMARY TABLE

LOT SIZE:	ACRES	SF	LOT COVERAGE:	EXISTING (SF)	PROPOSED (SF)
	0.409	17,816		#	#
LIMITS OF DISTURBANCE:	EXISTING (SF)	PROPOSED (SF)	AVERAGE SLOPE WITHIN LIMITS OF DISTURBANCE:	SLOPE (%)	SLOPE (%)
	0	17,816		#	#
OFF-STREET PARKING:	EXISTING (SPACES)	PROP. (SPACES)	BICYCLE PARKING:	EXISTING (SPACES)	PROP. (SPACES)
	0	16		0	3
RESIDENTIAL UNITS:	EXISTING (UNITS)	PROPOSED (UNITS)	NON-RESIDENTIAL USES:	TYPE(S)	GR. FLR AREA (SF)
	0	0		RETAIL MARIJUANA ESTABLISHMENT	L1: 2,738 L2: 1,766*
OPEN SPACE:	EXISTING (SF)	PROPOSED (SF)	TOTAL LANDSCAPED AREA:	EXISTING (SF)	PROPOSED (SF)
	17,816	#		#	#
BUILDING HEIGHT:	EXISTING (FT)	PROPOSED (FT)	TOTAL IMPERVIOUS AREA:	EXISTING (SF)	PROPOSED (SF)
	NA	< 25		#	#
SNOW STORAGE:	REQUIRED (SF)	PROPOSED (SF)	OTHER:		
	--	1,709			

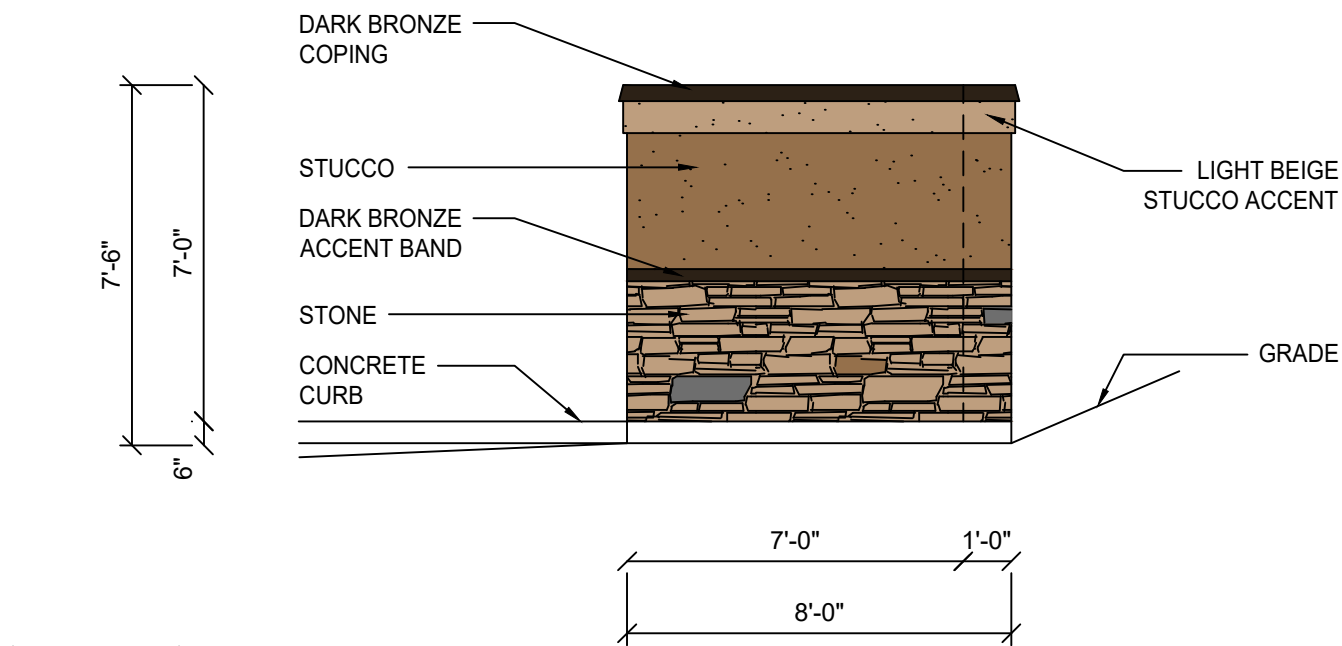
*INCLUDES 423 SF EXTERIOR BALCONY

PROPERTY DESCRIPTION

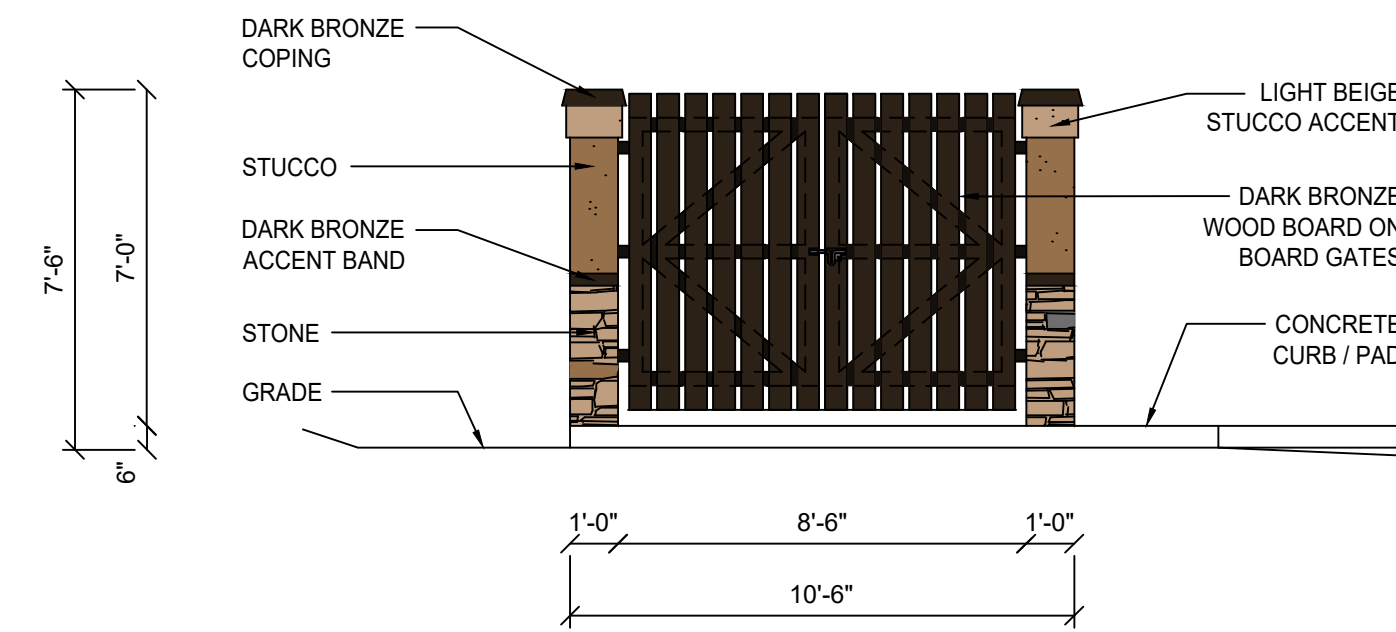
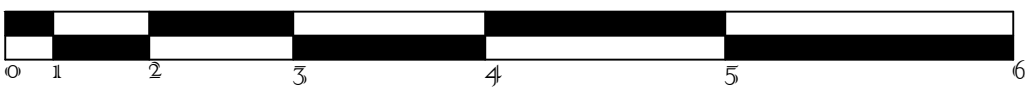
LOT 11B, GLENWOOD MEADOWS, FILING 5; SECTION 8, T. 6 S., R. 89 W., OF THE 6th P.M.; CITY OF GLENWOOD SPRINGS; GARFIELD COUNTY, COLORADO

SYMBOLS LEGEND

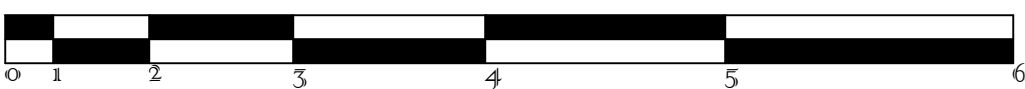
North		NORTH ARROW
1		KEY NOTE
1 A3		ENLARGED DETAIL REFERENCE
		ELEVATION DATUM



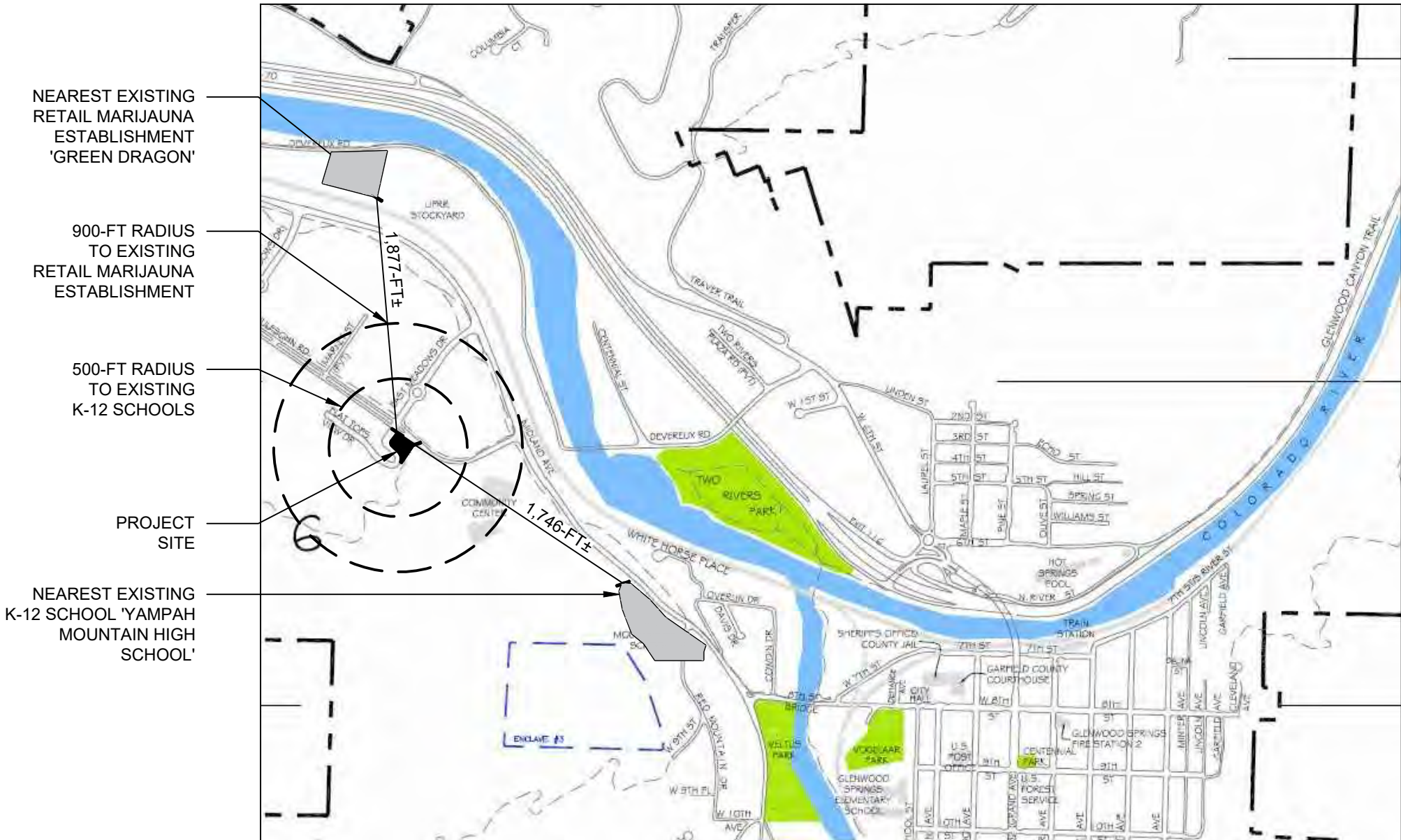
(PROPOSED)
REFUSE ENCLOSURE
(WEST) ELEVATION
SCALE 1/4"=1'-0"



(PROPOSED)
REFUSE ENCLOSURE
(NORTH) ELEVATION
SCALE 1/4"=1'-0"

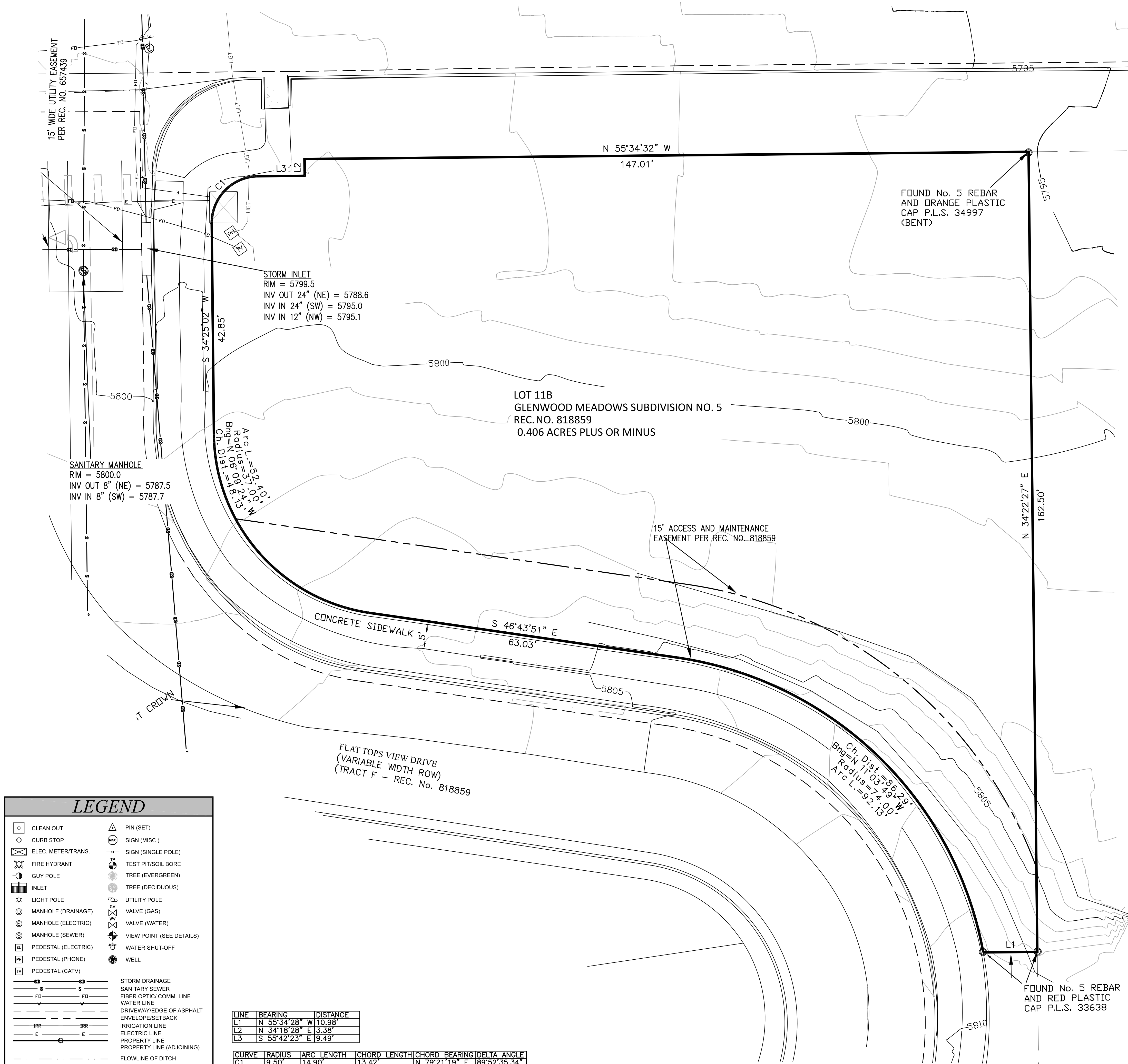


VICINITY MAP



REVISIONS:				

JOB NO. 19-233	DATE: 09/11/2020
CHECKED: PRG	DRAWN: JAS



TOPOGRAPHIC MAP
LOT 11B, GLENWOOD MEADOWS, FILING 5
SECTION 8, T. 6 S., R. 89 W., OF THE 6th P.M.
CITY OF GLENWOOD SPRINGS
GARFIELD COUNTY, COLORADO

TOPOGRAPHIC MAP
LOT 11B, GLENWOOD MEADOWS, FILING 5
CITY OF GLENWOOD SPRINGS
GARFIELD COUNTY, COLORADO

Stars to Stones Surveying
Professional Land Surveying & Mapping
Stars2StonesSurveying@yahoo.com
970.618.2118 Cell

- NOTES:
- 1) DATE OF TOPOGRAPHY: 01/10/2020.
 - 2) BASIS OF ELEVATIONS: NORTHWESTERLY SEWER MANHOLE, ELEV. = 5800.0' RVTN NETWORK NAVD 88 DATUM.
 - 3) NOTICE: ACCORDING TO COLORADO LAW YOU MUST COMMENCE ANY LEGAL ACTION BASED UPON ANY DEFECT IN THIS SURVEY WITHIN THREE YEARS AFTER YOU FIRST DISCOVER SUCH DEFECT. IN NO EVENT, MAY ANY ACTION BASED UPON ANY DEFECT IN THIS SURVEY BE COMMENCED MORE THAN TEN YEARS FROM THE DATE OF CERTIFICATION SHOWN HEREON.
 - 4) THE BOUNDARY DIMENSIONS, EASEMENTS AND RIGHTS OF WAY SHOWN HEREON ARE PER THE RECORD PLAT FOR THE SUBJECT PROPERTY AND TITLE SEARCH PERFORMED BY LAND TITLE GUARANTEE COMPANY, ORDER No. ABS63014393, DATED NOVEMBER 11, 2019.
 - 5) VERIFY CURRENT BUILDING SETBACKS AND RESTRICTIONS WITH THE PROPER GOVERNING AUTHORITY.

SURVEYOR'S CERTIFICATE

I, IVAN MARTIN, A PROFESSIONAL LAND SURVEYOR REGISTERED UNDER THE LAWS OF THE STATE OF COLORADO, DO HEREBY CERTIFY THAT THIS TOPOGRAPHIC MAP WAS MADE BY ME AND UNDER MY SUPERVISION, AND THAT THE MAP IS ACCURATE AND CORRECT TO THE BEST OF MY KNOWLEDGE.

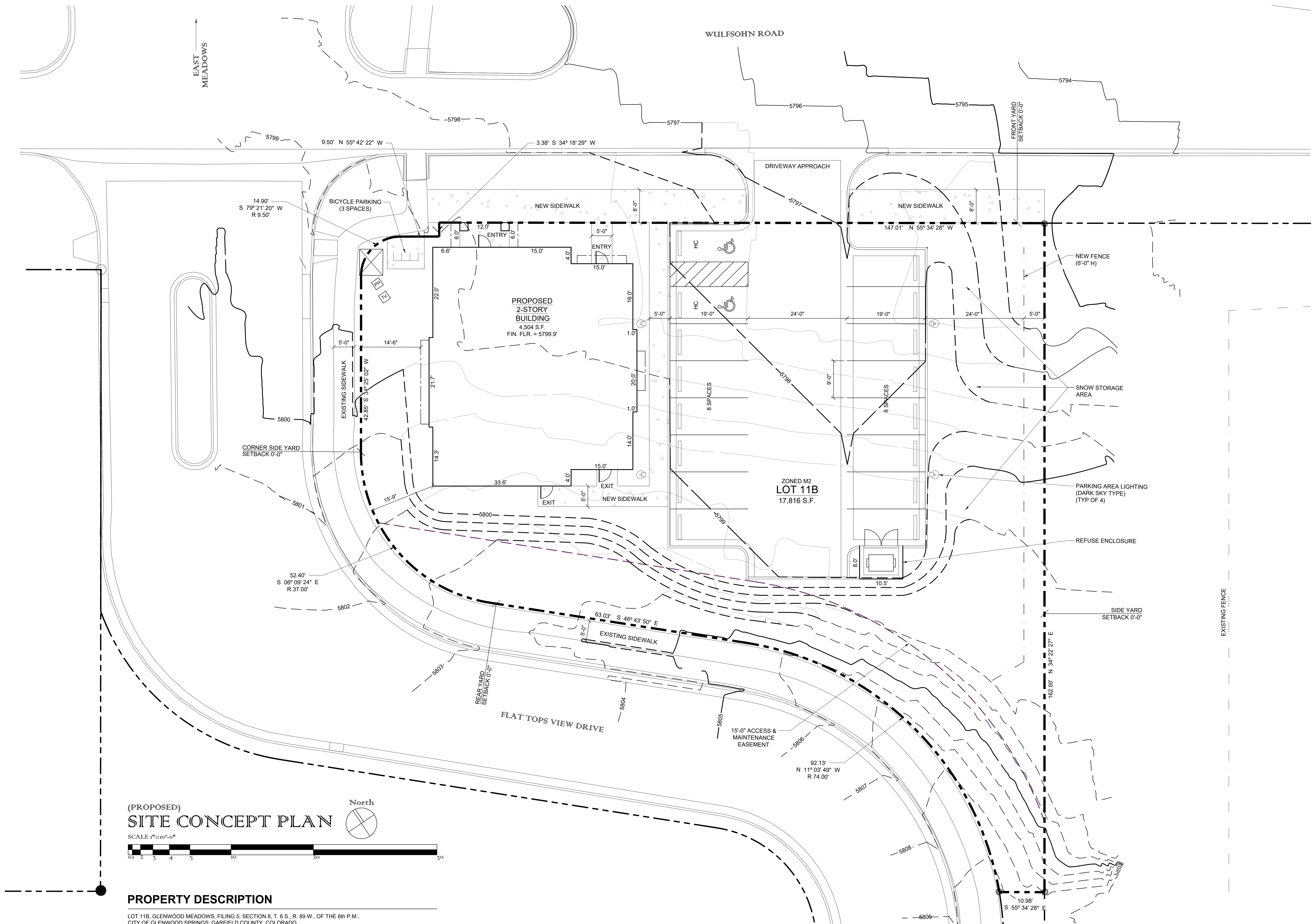


IVAN MARTIN
COLORADO P.L.S. No. 38440
FOR AND ON BEHALF OF
STARS TO STONES SURVEYING

LEGEND	
	CLEAN OUT
	CURB STOP
	ELEC. METER/TRANS.
	FIRE HYDRANT
	GUY POLE
	INLET
	LIGHT POLE
	MANHOLE (DRAINAGE)
	MANHOLE (ELECTRIC)
	MANHOLE (SEWER)
	PEDESTAL (ELECTRIC)
	PEDESTAL (PHONE)
	PEDESTAL (CATV)
	PIN (SET)
	SIGN (MISC.)
	SIGN (SINGLE POLE)
	TEST PIT/SOIL BORE
	TREE (EVERGREEN)
	TREE (DECIDUOUS)
	UTILITY POLE
	VALVE (GAS)
	VALVE (WATER)
	VIEW POINT (SEE DETAILS)
	WATER SHUT-OFF
	WELL
	STORM DRAINAGE
	SANITARY SEWER
	FIBER OPTIC/ COMM. LINE
	WATER LINE
	ENVELOPE/SETBACK
	IRRIGATION LINE
	ELECTRIC LINE
	PROPERTY LINE
	PROPERTY LINE (ADJOINING)
	FLOWLINE OF DITCH

LINE	BEARING	DISTANCE
L1	N 55°34'28" W	10.98'
L2	N 34°18'28" E	3.38'
L3	S 55°42'23" E	9.49'

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1	9.50'	14.90'	13.42'	N 79°21'19" E	189°52'35.34"



(PROPOSED)
SITE CONCEPT PLAN



PROPERTY DESCRIPTION

LOT 11B, GLENWOOD MEADOWS, FILING 5; SECTION 8, T. 6 S., R. 89 W., OF THE 6th P.M.;
CITY OF GLENWOOD SPRINGS; GARFIELD COUNTY, COLORADO

Architecture and Planning Inc
320 Jenkins Ranch road Durango, Colorado 81301
870.602.9237
info@psmarchitect.com

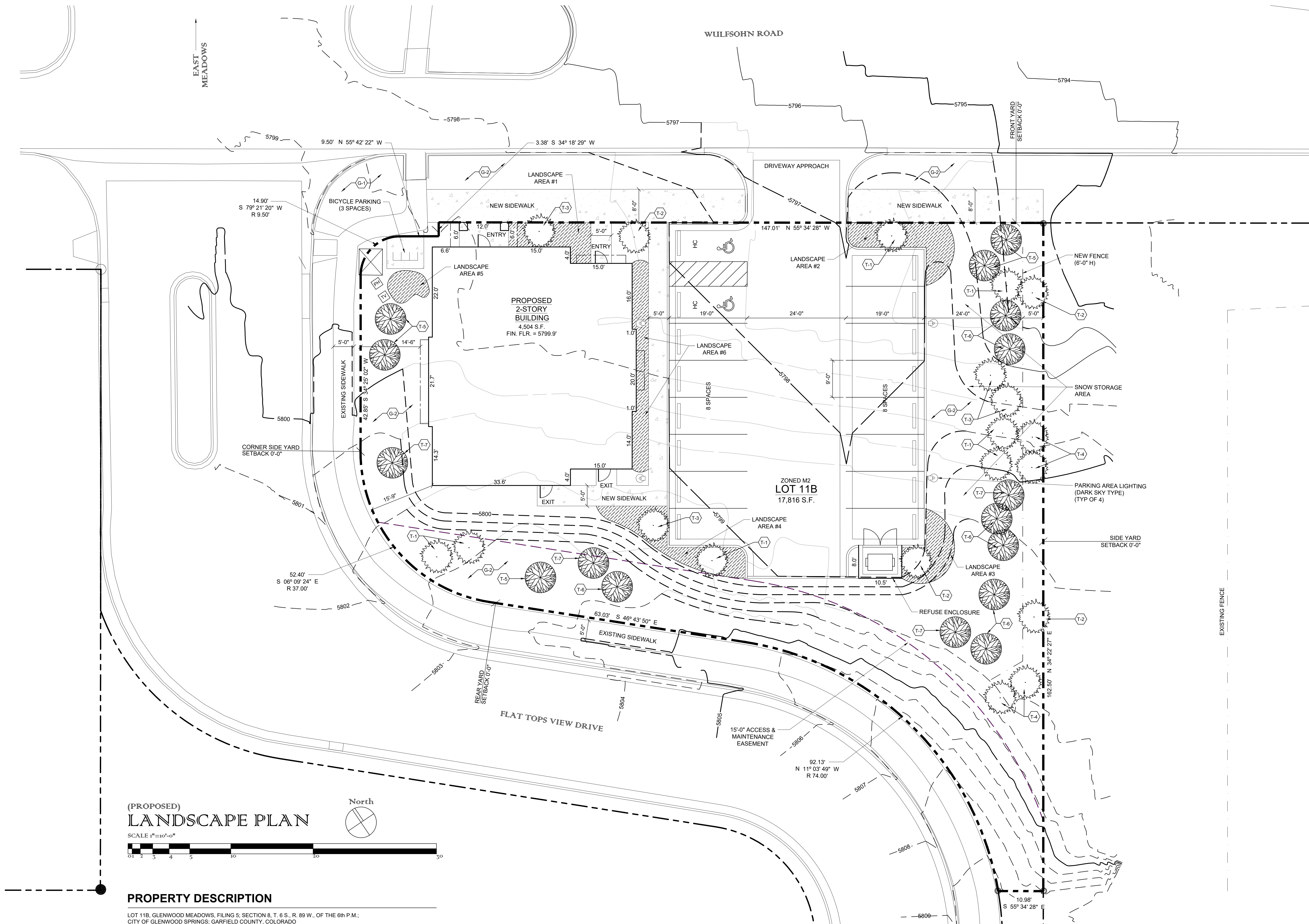
PSM

PROPOSED NEW DISPENSARY
THE KIND CASTLE
WULFSOHN ROAD
GLENWOOD SPRINGS, CO 81601

REVISIONS:

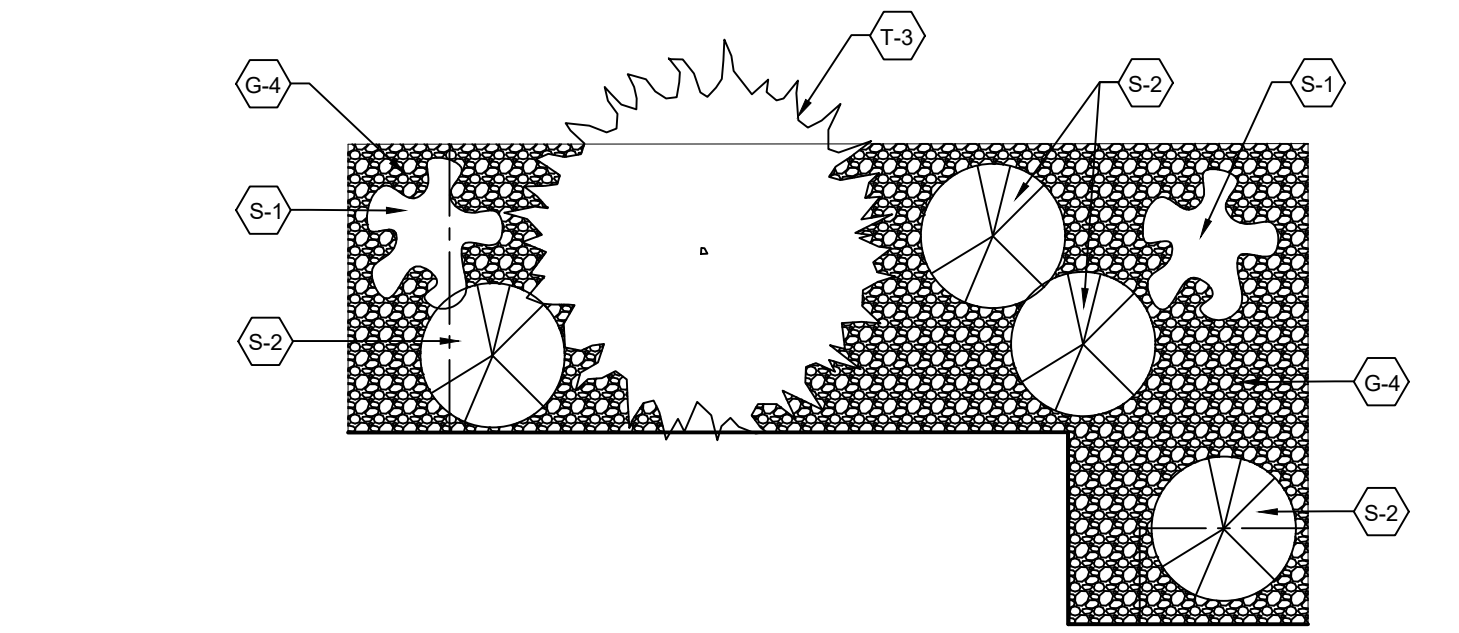
JOB NO. 19-233	DATE: 09/11/2020
CHECKED: PRG	DRAWN: JAS

SP₁
SHEET
2 OF 9

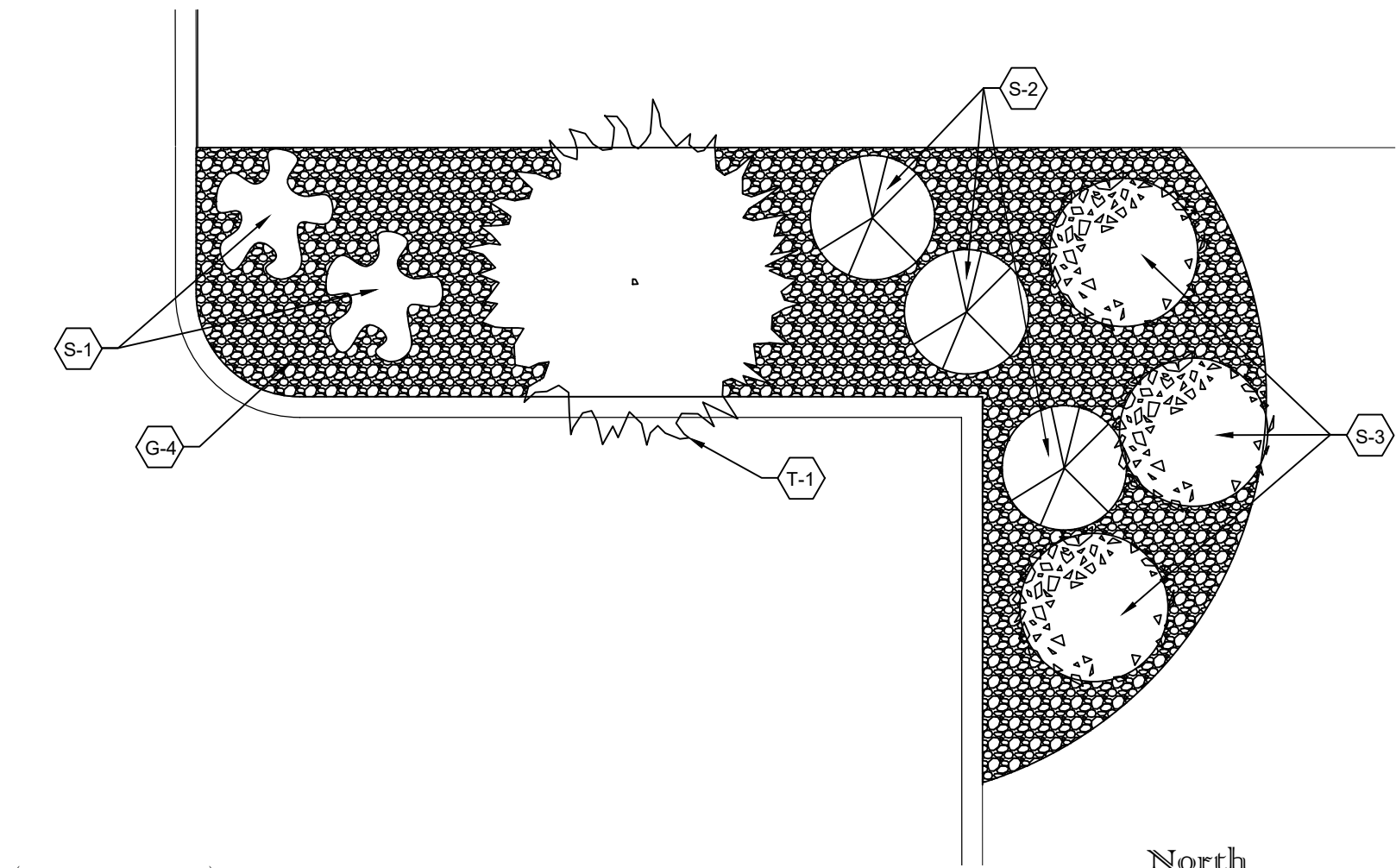


REVISIONS:

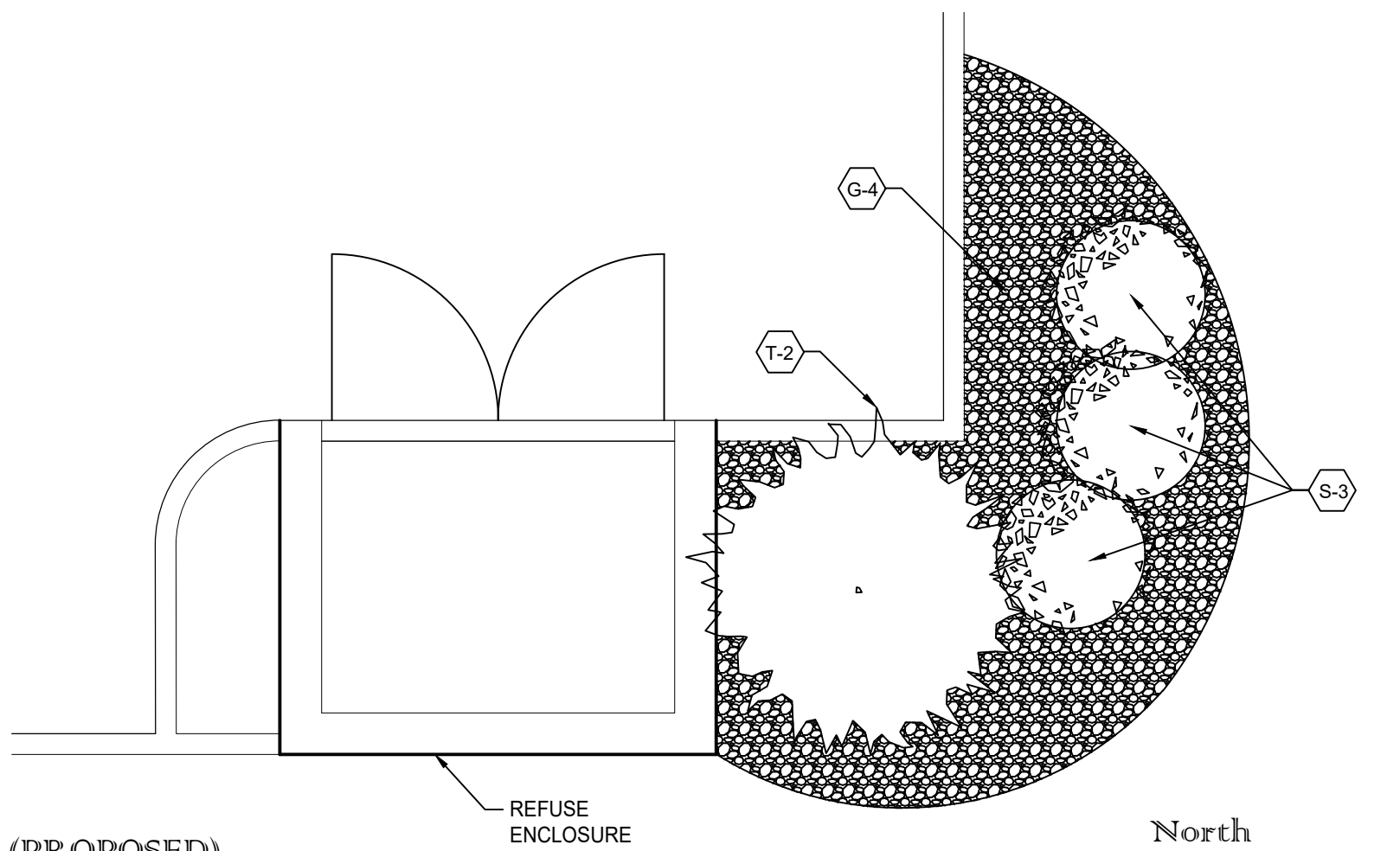
JOB NO. 19-233	DATE: 09/11/2020
CHECKED: PRG	DRAWN: JAS



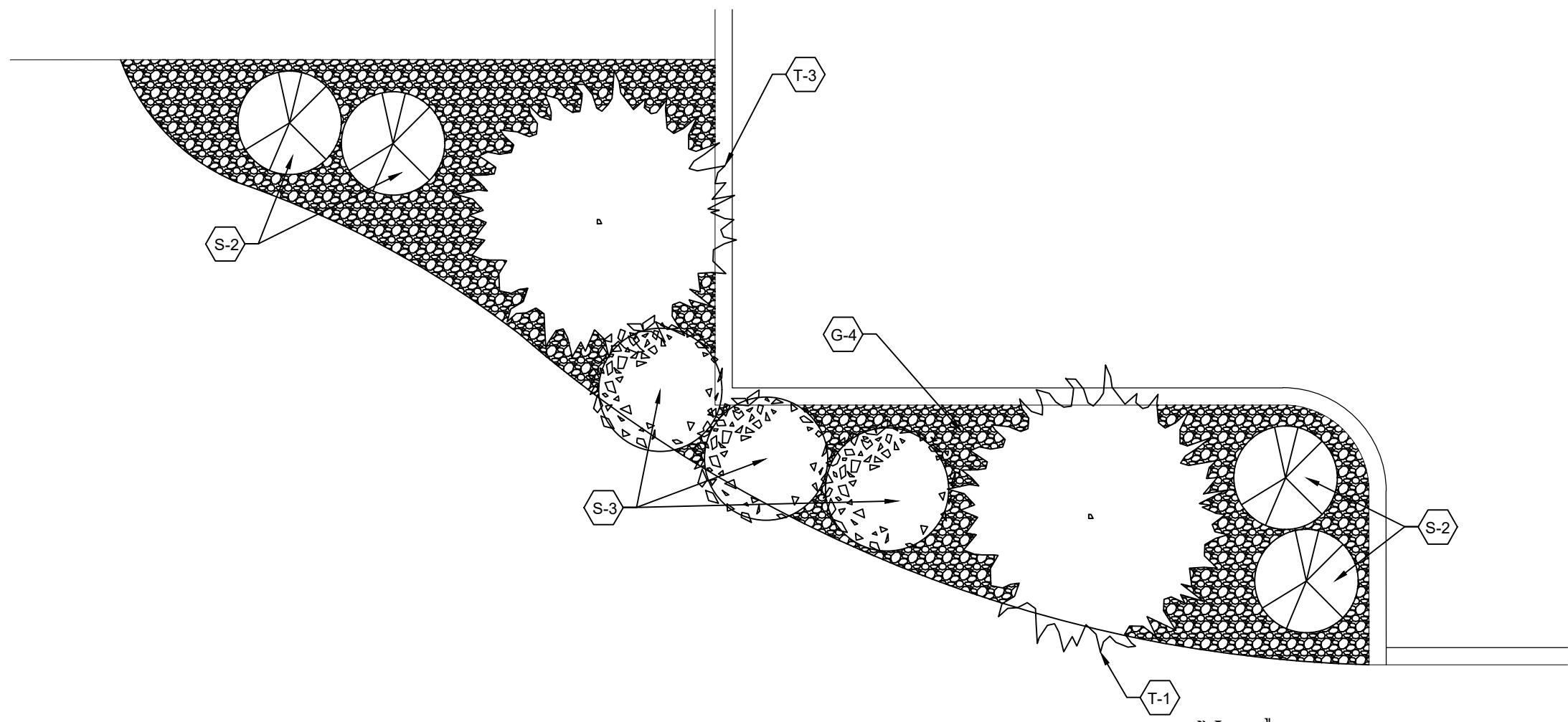
(PROPOSED)
LANDSCAPE AREA PLAN #1
SCALE 1/4"=1'-0"
140 SQUARE FEET



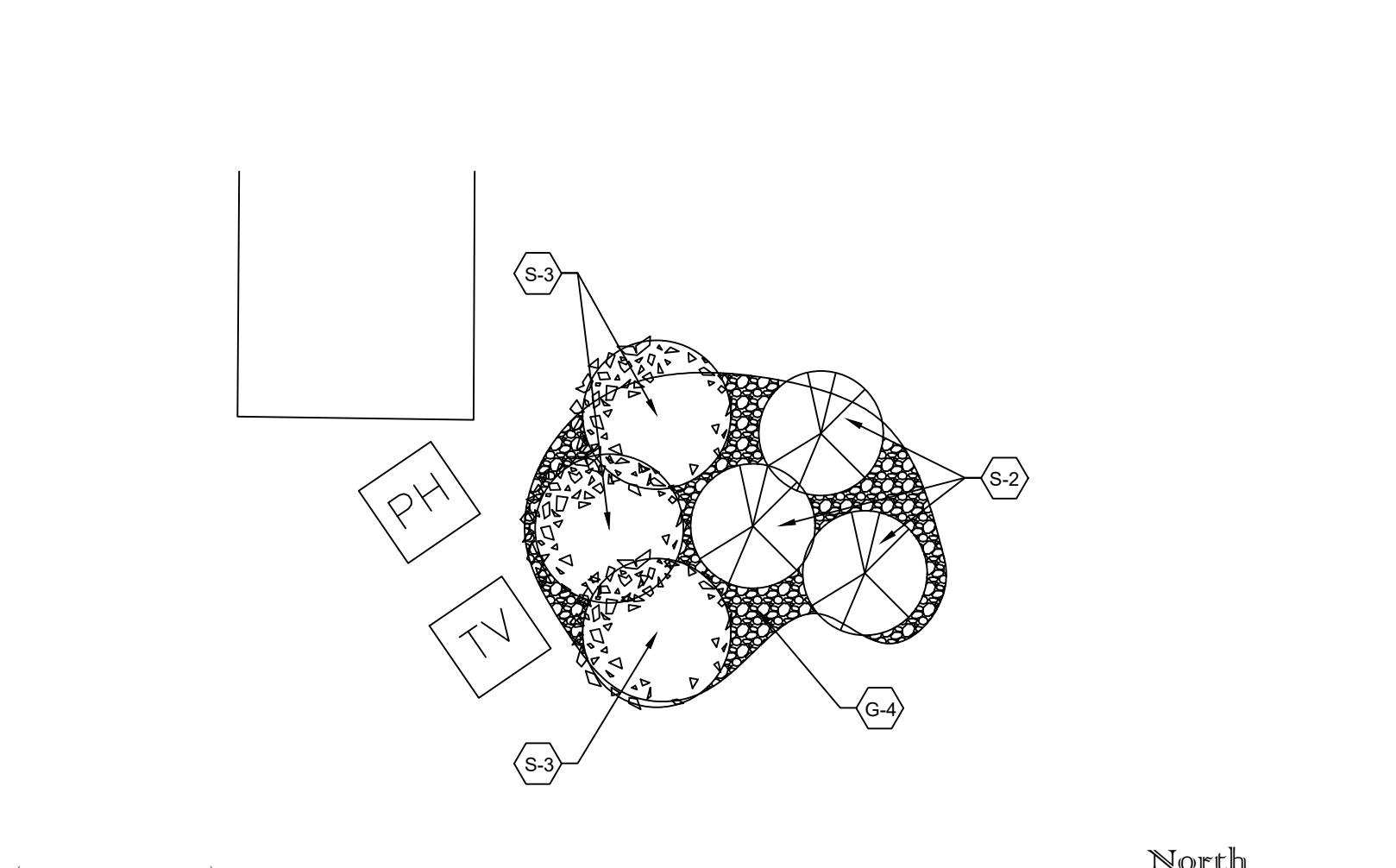
(PROPOSED)
LANDSCAPE AREA PLAN #2
SCALE 1/4"=1'-0"
197 SQUARE FEET



(PROPOSED)
LANDSCAPE AREA PLAN #3
SCALE 1/4"=1'-0"
143 SQUARE FEET



(PROPOSED)
LANDSCAPE AREA PLAN #4
SCALE 1/4"=1'-0"
217 SQUARE FEET



(PROPOSED)
LANDSCAPE AREA PLAN #5
SCALE 1/4"=1'-0"
59 SQUARE FEET

PLANT SCHEDULE

T - TREES					
KEY	SIZE	BOTANICAL NAME	COMMON NAME	QTY.	REMARKS
T-1	2 1/2"	ACER GLABRUM	ROCKY MOUNTAIN MAPLE	7	--
T-2	2 1/2"	GLEDTISIA TRIACANTHOS	SHADEMASTER HONEY LOCUST	4	--
T-3	2 1/2"	KRAUTER VESUVIUS PLUM	PLUM TREE	4	--
T-4	2 1/2"	POPULUS TREMULOIDES	QUAKING ASPEN	4	--
T-5	6'	PINUS EDULIS	PINON PINE	5	--
T-6	6'	ABIES CONCOLOR	WHITE FIR	7	--
T-7	10'	PICEA PUNGENS	COLORADO SPRUCE	4	--
S - SHRUBS					
KEY	SIZE	BOTANICAL NAME	COMMON NAME	QTY.	REMARKS
S-1	5 GAL.	BUDDLEIA	BUTTERFLY BUSH	8	--
S-2	5 GAL.	PEROVSKIA ATRIPLICIFOLIA	RUSSIAN SAGE	20	--
S-3	5 GAL.	CALAMAGROSTIS ACUTIFLORA	KARL FOERSTER FEATHER REED GRASS	12	--
G - GROUND COVERS					
KEY	DESCRIPTION	REMARKS			
G-1	NATIVE SOIL	OUTSIDE OF LIMITS OF DISTURBANCE, LEAVE NATIVE CONDITIONS IN PLACE			
G-2	UPLAND SEED MIX	N/A			
G-3	CEDAR MULCHING	CEDAR MULCHING TO BE APPLIED 4" DEEP IN AREAS INDICATED ON PLANS. INSTALL WEED MAT BELOW ALL AREAS TO RECEIVE MULCH.			
G-4	WASHED GRAVEL	3/4" WASHED GRAVEL TO BE APPLIED 2"-3" DEEP IN AREAS INDICATED ON PLANS. INSTALL WEED MAT BELOW ALL AREAS TO RECEIVE GRAVEL.			
P-1	MIXED PERENNIALS	MIXED PERENNIALS TO INCLUDE SMALL LEAF FUSSY TOES, WESTERN COLUMBINE, INDIAN PAINT BRUSH, PRAIRIE CONEFLOWER, JAMES PENSTEMMON. PROVIDE 1 GALLON CONTAINERS, SPACE AT 18" O.C. THROUGHOUT PLANTING AREA. PROVIDE CEDAR MULCH AROUND PLANTINGS.			

(PROPOSED)
LANDSCAPE AREA PLAN #6
SCALE 1/4"=1'-0"
184 SQUARE FEET

PROPERTY DESCRIPTION

LOT 11B, GLENWOOD MEADOWS, FILING 5, SECTION 8, T. 6 S., R. 89 W., OF THE 6th P.M.;
CITY OF GLENWOOD SPRINGS; GARFIELD COUNTY, COLORADO

Architecture and Planning Inc
320 Jenkins Ranch road Durango, Colorado 81301
870.692.9237
aplan@att.net

PTM

PROPOSED NEW DISPENSARY

THE KIND CASTLE

WULFSOHN ROAD
GLENWOOD SPRINGS, CO 81601

REVISIONS:			
1			
2			
3			
4			

JOB NO. 19-233	DATE: 09/11/2020	CHECKED: JAS
		PRG

LS2

SHEET
4 OF 10



P.O. Box 3945
Telluride, CO 81435
970-729-0683

SUBMISSIONS:

SUBMITTAL 2020-09-09

Kind Kastle
Glenwood Springs, CO



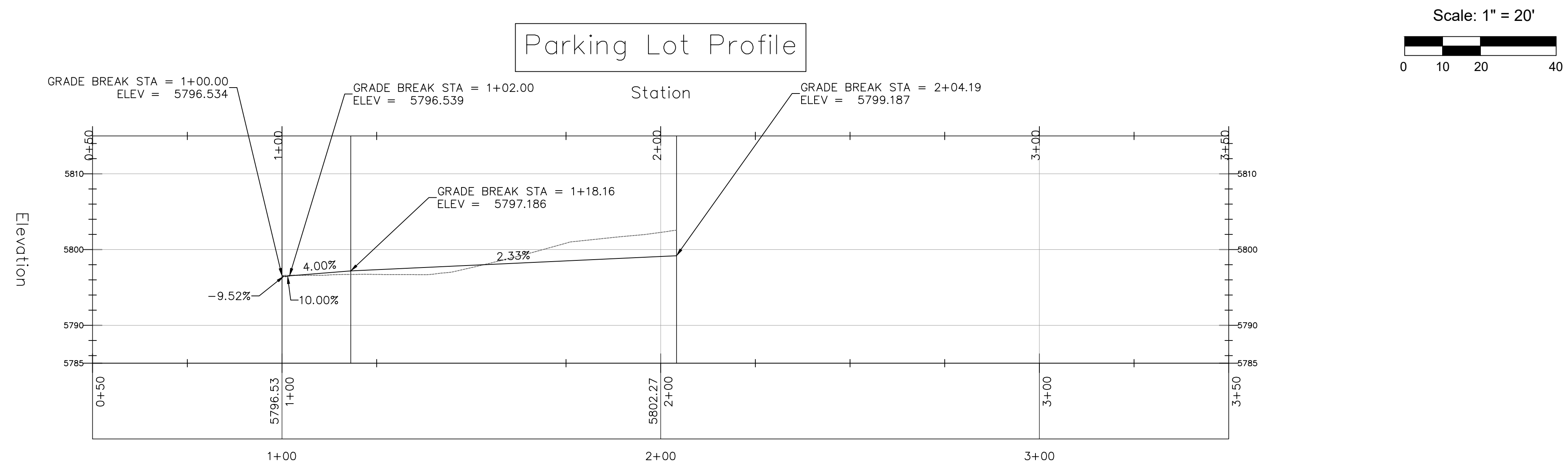
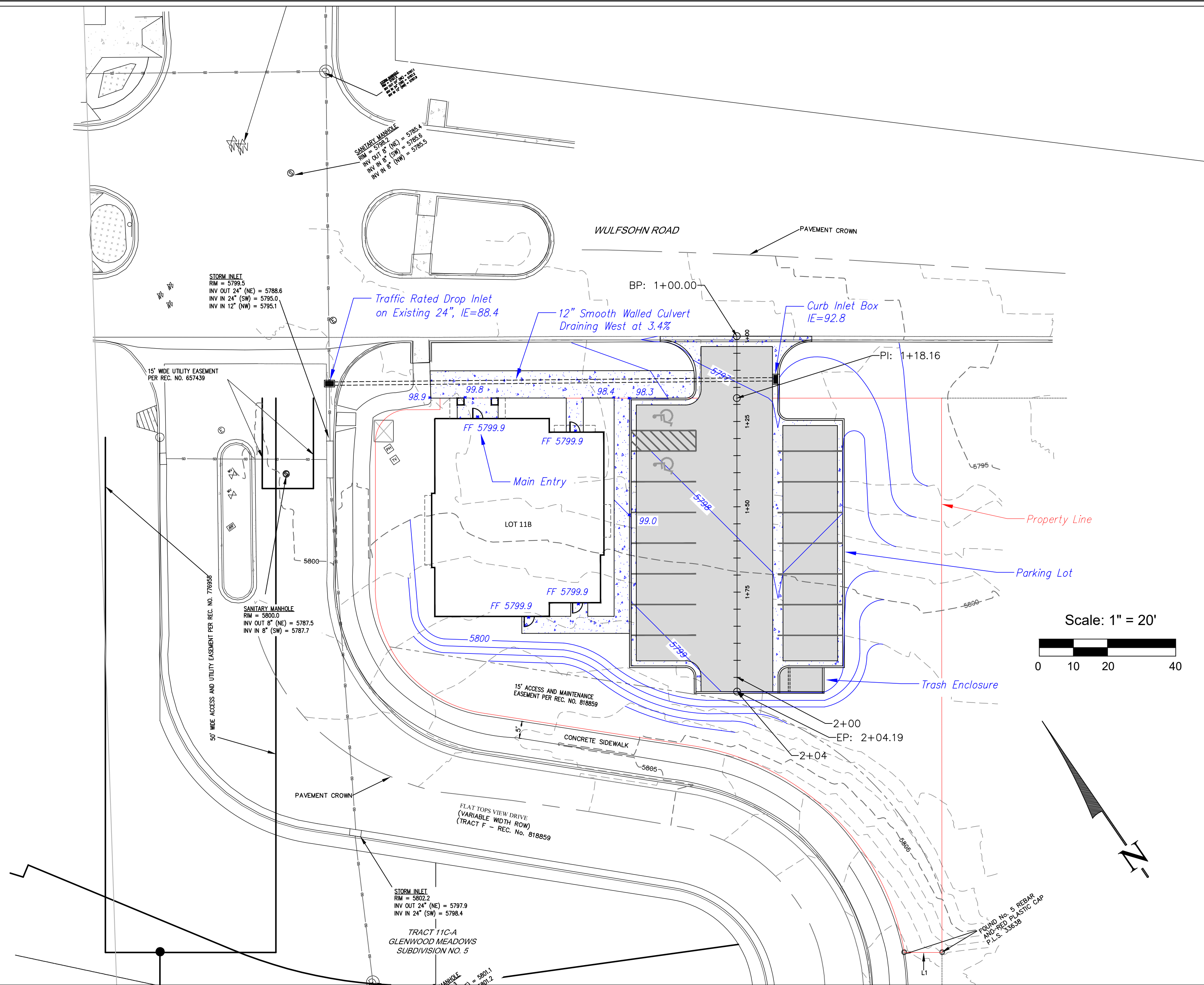
CONTRACTOR TO REVIEW AND COMPARE ALL CHAPTERS AND INTERDISCIPLINARY DRAWINGS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO ANY FIELD WORK BEING DONE IN ACCORDANCE WITH AIA DOCUMENT A201

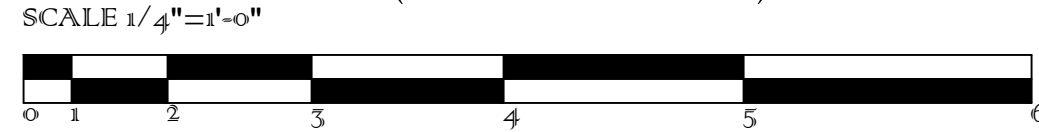
Conceptual Grading

with

Parking Lot Profile

C2

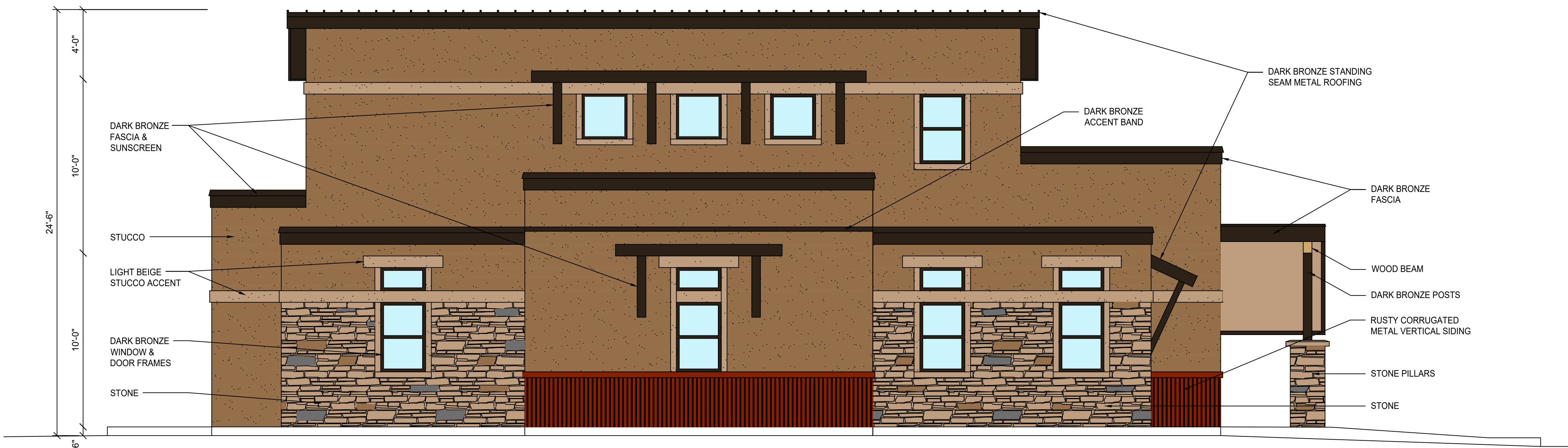
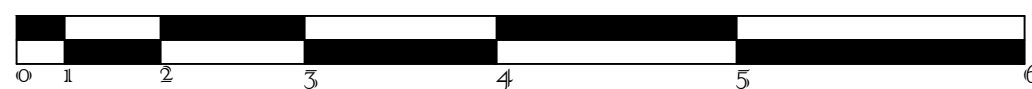






(PROPOSED)
REAR (SOUTH) ELEVATION

SCALE 1/4"=1'-0"



(PROPOSED)
SIDE (EAST) ELEVATION

SCALE 1/4"=1'-0"



GENERAL NOTES

THESE ELEVATIONS ARE CONCEPTUAL IN NATURE. INDIVIDUAL BUILDING ELEVATIONS MAY VARY, BUT WILL BE EVALUATED FOR COMPLIANCE WITH DESIGN STANDARDS CONTAINED WITHIN TITLE 070 OF THE GLENWOOD SPRINGS MUNICIPAL CODE AT THE TIME OF SUBMITTING A BUILDING PERMIT APPLICATION.

MATERIALS SUMMARY TABLE

FRONT ELEVATION (NORTH)			REAR ELEVATION (SOUTH)		
GROSS OF WALL:	SURFACE AREA (SF)	PERCENT OF NET*	GROSS OF WALL:	SURFACE AREA (SF)	PERCENT OF NET*
	1,008	100%		1,014	100%
NET* MATERIAL 1: STUCCO - DARK	455	45%	NET* MATERIAL 1: STUCCO - DARK	688	68%
NET* MATERIAL 2: STUCCO - LIGHT	66	6%	NET* MATERIAL 2: STUCCO - LIGHT	53	5%
NET* MATERIAL 3: STONE	83	8%	NET* MATERIAL 3: STONE	89	9%
NET* MATERIAL 4: METAL SIDING	88	9%	NET* MATERIAL 4: METAL SIDING	3	<1%
NET* MATERIAL 5: METAL TRIMS	160	16%	NET* MATERIAL 5: METAL TRIMS	111	11%
NET* MATERIAL 6: GUARDRAIL	36	4%	NET* MATERIAL 6: GUARDRAIL	0	0%
NET* MATERIAL 7: DOORS	58	6%	NET* MATERIAL 7: DOORS	48	5%
NET* MATERIAL 8: WINDOWS	65	6%	NET* MATERIAL 8: WINDOWS	22	2%
SIDE ELEVATION (EAST)			SIDE ELEVATION (WEST)		
GROSS OF WALL:	SURFACE AREA (SF)	PERCENT OF NET*	GROSS OF WALL:	SURFACE AREA (SF)	PERCENT OF NET*
	1,260	100%		1,129	100%
NET* MATERIAL 1: STUCCO - DARK	665	53%	NET* MATERIAL 1: STUCCO - DARK	523	46%
NET* MATERIAL 2: STUCCO - LIGHT	96	8%	NET* MATERIAL 2: STUCCO - LIGHT	56	5%
NET* MATERIAL 3: STONE	183	15%	NET* MATERIAL 3: STONE	187	17%
NET* MATERIAL 4: METAL SIDING	76	6%	NET* MATERIAL 4: METAL SIDING	132	12%
NET* MATERIAL 5: METAL TRIMS	154	12%	NET* MATERIAL 5: METAL TRIMS	110	10%
NET* MATERIAL 6: GUARDRAIL	0	0%	NET* MATERIAL 6: GUARDRAIL	0	0%
NET* MATERIAL 7: DOORS	0	0%	NET* MATERIAL 7: DOORS	0	0%
NET* MATERIAL 8: WINDOWS	85	7%	NET* MATERIAL 8: WINDOWS	119	11%

*NET EXCLUDES ROOF FORMS, AWNINGS AND SIMILAR FEATURES. INCLUDES GLAZED WINDOWS AND DOORS.

PROPERTY DESCRIPTION

LOT 11B, GLENWOOD MEADOWS, FILING 5; SECTION 8, T. 6 S., R. 89 W., OF THE 6th P.M.; CITY OF GLENWOOD SPRINGS; GARFIELD COUNTY, COLORADO



PROPOSED NEW DISPENSARY

THE KIND CASTLE

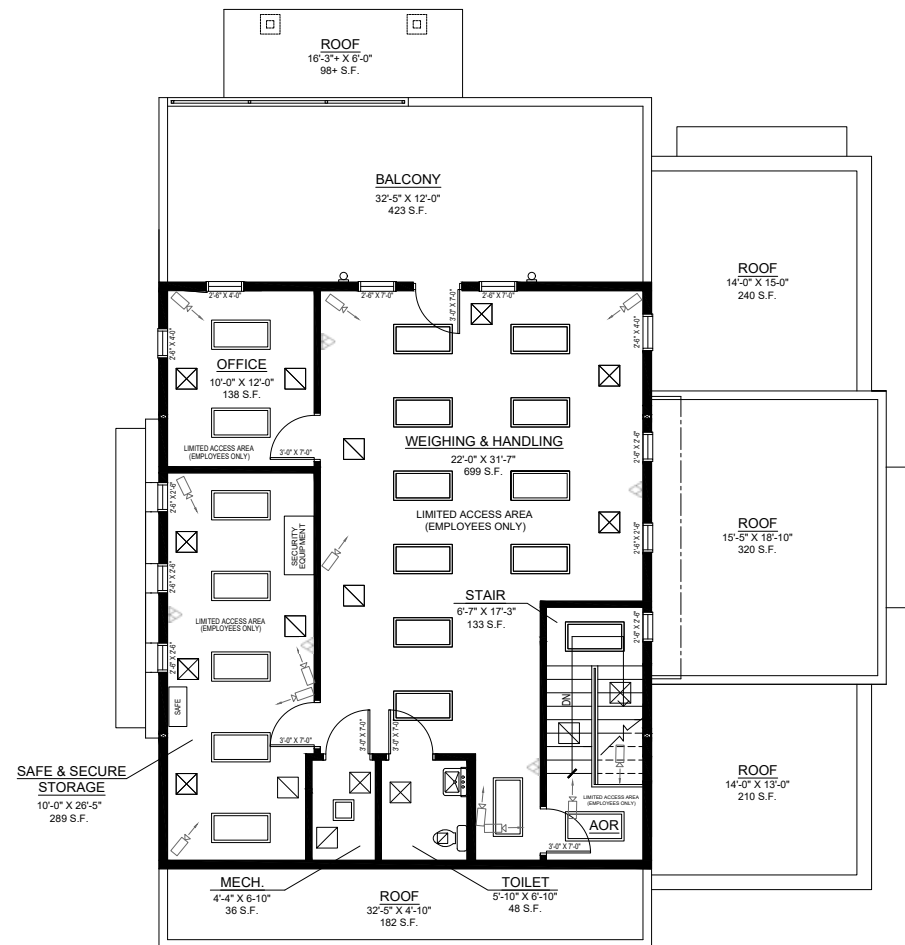
WULFSOHN ROAD
GLENWOOD SPRINGS, CO 81601

REVISIONS:		JOB NO. 19-233	DATE: 09/11/2020	CHECKED: DRAWN: JAS

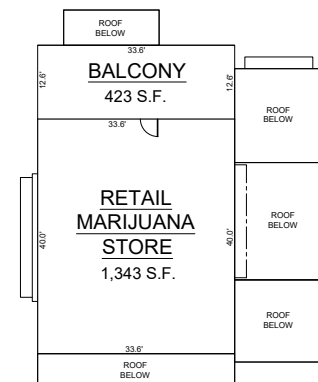
A2

SHEET 7 OF 9

(PROPOSED)
UPPER LEVEL FLOOR PLAN
 SCALE 1/4"=1'-0"
 1,343 GROSS SQUARE FEET



- KEY
- ★ LOCATION OF POSTED LICENSE
 - ⊠ ALARM KEY PADS
 - ▽ MOTION DETECTORS
 - SECURITY CAMERAS (ARROW INDICATES DIRECTION)



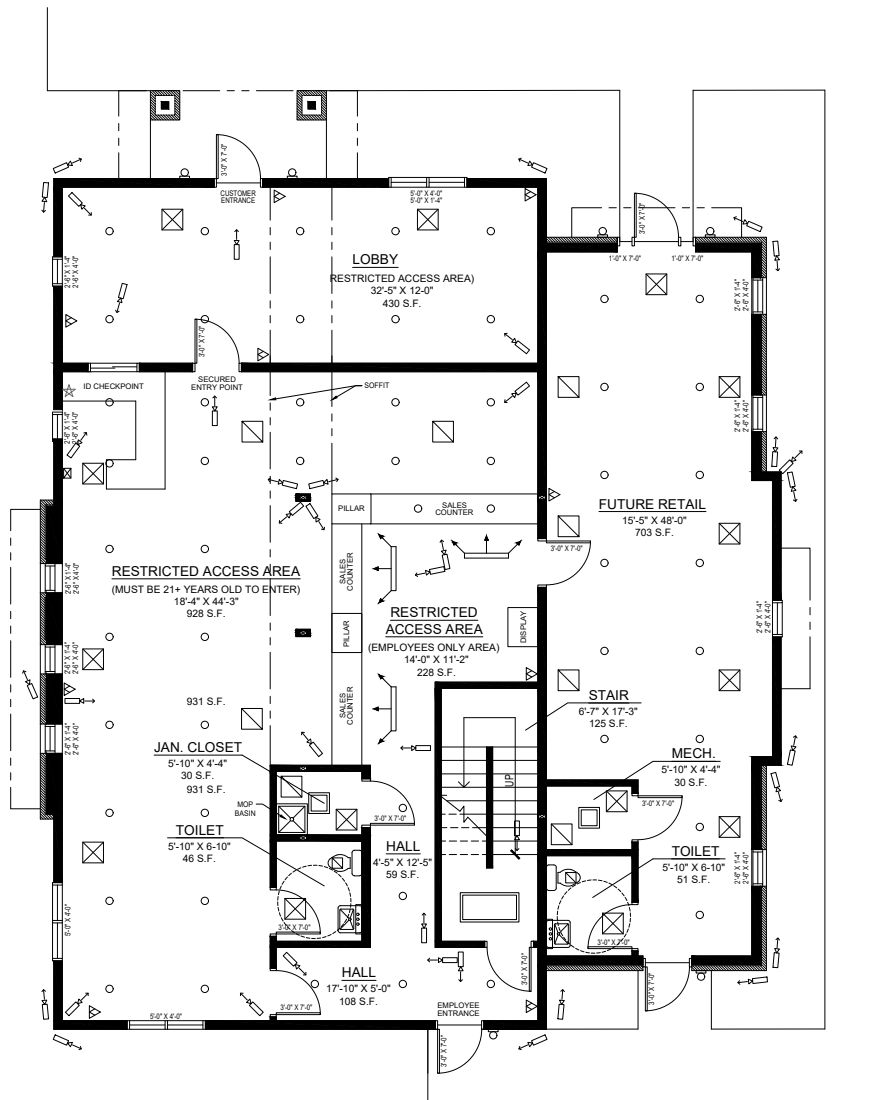
(PROPOSED)
UPPER LEVEL AREA PLAN
 1,343 GROSS SQUARE FEET

BUILDING AREA SUMMARY - GROSS FLOOR AREA

RETAIL MARIJUANA STORE (RMS):		TOTAL BUILDING:	
MAIN LEVEL	1,954 S.F.	MAIN LEVEL - RMS	1,954 S.F.
UPPER LEVEL	1,343 S.F.	MAIN LEVEL - FR	784 S.F.
RMS TOTAL	3,297 S.F.	MAIN LEVEL TOTAL	2,738 S.F.
FUTURE RETAIL (FR):		UPPER LEVEL - RMS	1,343 S.F.
MAIN LEVEL	784 S.F.	UPPER LEVEL TOTAL	1,343 S.F.
FR TOTAL	784 S.F.	MAIN LEVEL TOTAL	2,738 S.F.
		UPPER LEVEL TOTAL	1,343 S.F.
		TOTAL BUILDING	4,081 S.F.

PROPERTY DESCRIPTION

LOT 11B, GLENWOOD MEADOWS, PLING S, SECTION 8, T. 6 S., R. 85 W., OF THE 6th P.M.;
 CITY OF GLENWOOD SPRINGS, GARFIELD COUNTY, COLORADO



(PROPOSED)
MAIN LEVEL FLOOR PLAN

SCALE: 1/8" = 1'-0"

2,738 GROSS SQUARE FEET

- KEY
- ★ LOCATION OF POSTED LICENSE
 - ⊠ ALARM KEY PADS
 - ▽ MOTION DETECTORS
 - SECURITY CAMERAS (ARROW INDICATES DIRECTION)



(PROPOSED)
MAIN LEVEL AREA

2,738 SQUARE FEET

BUILDING AREA SUMMARY

RETAIL MARIJUANA STORE (RMS):		TOTAL BUILDING:	
MAIN LEVEL	1,954 S.F.	MAIN LEVEL - RMS	1,954 S.F.
UPPER LEVEL	1,343 S.F.	MAIN LEVEL - FR	784 S.F.
UPPER LEVEL - BALCONY	423 S.F.	MAIN LEVEL TOTAL	2,738 S.F.
RMS TOTAL	3,720 S.F.	UPPER LEVEL - RMS	1,343 S.F.
		UPPER LEVEL - RMS BALCONY	423 S.F.
FUTURE RETAIL (FR):		UPPER LEVEL TOTAL	1,766 S.F.
MAIN LEVEL	784 S.F.	MAIN LEVEL TOTAL	2,738 S.F.
FR TOTAL	784 S.F.	UPPER LEVEL TOTAL	1,766 S.F.
		TOTAL BUILDING	4,504 S.F.

PROPERTY DESCRIPTION

LOT 11B, GLENWOOD MEADOWS, PLING S, SECTION 8, T. 6 S., R. 85 W., OF THE 6th P.M.;
CITY OF GLENWOOD SPRINGS, GARFIELD COUNTY, COLORADO

**Engineering
Department**

**Ryan Johnson
Asst. City
Engineer**

Date: October 9, 2020

To: Community Development

Re: Kind Castle Lot 11B Glenwood Meadows No. 5 – Technical Review
Comments

Please see the following comments from Engineering for the technical review submittal.

1. Provide a Utility Plan
 - a. Show the proposed water service and sewer service locations.
2. Traffic Analysis
 - a. Given the existing configuration of Wulfsohn Rd. a westbound driver heading to the business would be tempted to turn directly into the facility through the existing striping in lieu of taking a u-turn at intersection as noted in the report. Report should consider additional alternatives to the existing configuration.
 - i. Access configuration for a Right-in Right-Out
 - ii. Is a left turn lane into the facility a feasible option and desired option?
 1. Is a left turn out of the business problematic?
 - b. As noted in the Narrative, traffic analysis should discuss whether or not a four way stop at Wulfsohn and flat top is necessary from a traffic volume and/or operational safety perspective.
 - c. What is the Peak hour of the business versus the Peak Hour on Wulfsohn?
 - d. A number of new multi family developments have occurred on Wulfsohn since 2018. Please recount Wulfsohn.
 - e. Please evaluate the following intersections for existing and proposed LOS and queue lengths.

East Wulfsohn and Midland

East Meadow Drive and Midland

West Meadow Drive and Midland

West Wulfsohn and Midland

- f. Two other projects are currently proposed in this area that will also affect these intersections. Analysis of the intersections could be coordinated between the other projects if the developer elects to do that.

To be completed for Construction Documents Review

1. Site Specific Geotechnical report should address geohazards and mitigation techniques for the proposed site, including but not limited to: hydrocompactive soils and debris flow.
 - a. The report can reference the existing mitigation for debris flow but should provide recommendations for the hydrocompactive soils, as that will affect the foundation recommendations.
 2. Major drainage report will be required and coordinated with existing drainage master plans and adjacent development drainage reports.
 - a. The report at a minimum will need to provide supporting documentation and report excerpts to address the following:
 - i. Downstream storm sewer capacity.
 - ii. Downstream pond capacity.
 - iii. Verify any offsite pass through flows that may need to be passed through the property.
 - b. Additional adjacent report documentation is can be found at the link below.
- https://glenwoodsprings-my.sharepoint.com/:f:/g/personal/ryan_johnson_cogs_us/Ehupuo-XErVOsbQPkz33wlYB3PPT7_NNU99nliWhDQwKHg?e=aboJ8E
3. Subsurface Utility Engineering
 - a. Provide S.U.E. plan per Colorado Law CRS 9-1.5-102 requirements or provide a stamped letter determining why this is not a Subsurface Utility Engineering-Required Project.

Public Works Comments:

Planning Item: 51-20, Kind Castle GW Meadows
Applicant: Chris Hawkins, Alpine Planning, LLC
Owner: Glenwood Meadows, LLC
Location: 150 Wulfsohn Road
Zone: Mixed Use Core (M2)
Parcel: 2185-081-41-004

1. The lot does not have sanitary sewer or water services to it since it is an adjusted lot from the original plans. There is no sewer within Wulfsohn Road in front of this lot. The simplest way to connect to sewer and water will be to pull it from Flat Tops View Drive. The sewer in Flat Tops View Drive is quite deep to connect to the sewer manhole in Wulfsohn. NO street cuts are shown which will need to be per City standards. None of this is shown on the plans.
2. There is a switch cabinet at this location already, but the developer will need to fill out a Load Form and submit it to the Electric department. A new transformer will need to be set at the site for the building's needs which will need to be coordinated with the Electric Department. None of this is shown on the plans.
3. Will this access be right in and right out only due to the center island and striping in Wulfsohn?
4. Sidewalk to extend across the lot? It shows stopping at the driveway.
5. Site engineer should review Flat Tops View Drive site lines at the intersection of Wulfsohn to make sure the building does not impede safe turning movements. I do not see any site lines on the plans attached.
6. Has the storm sewer being collected in inlets and hooked to the Glenwood Meadows metro district storm sewer system been approved by the district?
7. Any fire sprinkler system will need to meet the NFPA 24 design requirements.
 - Here are items that we will need to see on the plans before we can approve:
 - Since the new tap and service will be used for combined potable/fire, the service will need to be designed, installed, and tested to meet NFPA 24 requirements. Prior to construction, an engineered detail of the service line must be submitted to the City for review and approval. At a minimum, the detail must show the following:
 - 1) A plan and profile view of the piping alignments extending from the tap location to the building interior penetration.
 - 2) Materials for construction.
 - 3) Thrust restraint locations and specifications.
 - 4) A comment indicating that the design conforms to all NFPA 24 requirements .
 - 5) A note indicating that the service line must be pressure- and flow-tested in accordance with NFPA 24.
 - 6) The stamp and signature of the Professional Engineer (PE) certifying the design.

The applicant is responsible for providing the City with a signed copy of the completed “Contractor’s Material and Testing Certificate for Underground Piping” form (attached) prior to issuance of a Certificate of Occupancy. This form certifies that the service line was designed, installed, and tested in accordance with all NFPA 24 requirements

From: [Linda DuPriest](#)
To: [Trent Hyatt](#)
Subject: RE: Planning Item #51-20, Kind Castle GW Meadows Referral
Date: Monday, October 19, 2020 1:40:17 PM
Attachments: [image001.png](#)

Hi Trent:

We've got an emergency situation with the airport so sorry I couldn't be there today. The only thing I would've commented on is (per usual) bike parking.

Good luck,
Linda

From: Trent Hyatt <trent.hyatt@cogs.us>
Sent: Monday, October 19, 2020 10:10 AM
To: Doug Hazzard <doug.hazzard@cogs.us>; Emery Ellingson <emery.ellingson@cogs.us>; Greg Bak <greg.bak@cogs.us>; Gretchen E Ricehill <gretchen.ricehill@cogs.us>; Jake Velasquez <jake.velasquez@cogs.us>; Jennifer Ooton <jenn.ooton@cogs.us>; Joseph Deras <jderas@glenwoodpolice.com>; Linda DuPriest <linda.dupriest@cogs.us>; Lisa DiNardo <lisa.dinardo@cogs.us>; Matt Nuñez <matt.nunez@cogs.us>; Matthew Langhorst <matthew.langhorst@cogs.us>; Mike Hoffman <mike.hoffman@cogs.us>; Peter Hayes <peter.hayes@cogs.us>; Ryan Johnson <ryan.johnson@cogs.us>; Terri Partch <terri.partch@cogs.us>; Trent Hyatt <trent.hyatt@cogs.us>; Victor Meraz <victor.meraz@cogs.us>; Warren Hays <warren.hays@cogs.us>; Brian Smith <brian.smith@cogs.us>; Doug Hazzard <doug.hazzard@cogs.us>; Emery Ellingson <emery.ellingson@cogs.us>; Greg Bak <greg.bak@cogs.us>; Jake Velasquez <jake.velasquez@cogs.us>; Jennifer Ooton <jenn.ooton@cogs.us>; Jessica Bowser <jessica.bowser@cogs.us>; Lisa DiNardo <lisa.dinardo@cogs.us>; Matt Nuñez <matt.nunez@cogs.us>; Matthew Langhorst <matthew.langhorst@cogs.us>; Mike Hoffman <mike.hoffman@cogs.us>; Peter Hayes <peter.hayes@cogs.us>; Ryan Johnson <ryan.johnson@cogs.us>; Terri Partch <terri.partch@cogs.us>; Victor Meraz <victor.meraz@cogs.us>; Warren Hays <warren.hays@cogs.us>
Subject: RE: Planning Item #51-20, Kind Castle GW Meadows Referral

Hi all,
I only received comments on this proposal from the Engineering Department. If you have any official comments, please submit those to me ASAP. As a reminder, we have a department review meeting for this item today at 1:30PM.

Thanks,

Trent L. Hyatt
970.384.6427 (p)
trent.hyatt@cogs.us

From: [Doug Hazzard](#)
To: [Trent Hyatt](#)
Subject: RE: Planning Item #51-20, Kind Castle GW Meadows Referral
Date: Monday, October 19, 2020 10:35:21 AM
Attachments: [City of GS Line Extension Policy.pdf](#)
[Load information.pdf](#)
[image001.png](#)

Trent,

The Line extension will apply . I did not see where a transformer location was called out but it can be placed near the electrical cabinet on corner. They may have to give up a bicycle parking spot. I will need the Load information sheet filled out to determine single phase or three phase load. Pretty simple on our end.

Thanks

Doug

From: Trent Hyatt <trent.hyatt@cogs.us>
Sent: Monday, October 19, 2020 10:10 AM
To: Doug Hazzard <doug.hazzard@cogs.us>; Emery Ellingson <emery.ellingson@cogs.us>; Greg Bak <greg.bak@cogs.us>; Gretchen E Ricehill <gretchen.ricehill@cogs.us>; Jake Velasquez <jake.velasquez@cogs.us>; Jennifer Ooton <jenn.ooton@cogs.us>; Joseph Deras <jderas@glenwoodpolice.com>; Linda DuPriest <linda.dupriest@cogs.us>; Lisa DiNardo <lisa.dinardo@cogs.us>; Matt Nuñez <matt.nunez@cogs.us>; Matthew Langhorst <matthew.langhorst@cogs.us>; Mike Hoffman <mike.hoffman@cogs.us>; Peter Hayes <peter.hayes@cogs.us>; Ryan Johnson <ryan.johnson@cogs.us>; Terri Partch <terri.partch@cogs.us>; Trent Hyatt <trent.hyatt@cogs.us>; Victor Meraz <victor.meraz@cogs.us>; Warren Hays <warren.hays@cogs.us>; Brian Smith <brian.smith@cogs.us>; Doug Hazzard <doug.hazzard@cogs.us>; Emery Ellingson <emery.ellingson@cogs.us>; Greg Bak <greg.bak@cogs.us>; Jake Velasquez <jake.velasquez@cogs.us>; Jennifer Ooton <jenn.ooton@cogs.us>; Jessica Bowser <jessica.bowser@cogs.us>; Lisa DiNardo <lisa.dinardo@cogs.us>; Matt Nuñez <matt.nunez@cogs.us>; Matthew Langhorst <matthew.langhorst@cogs.us>; Mike Hoffman <mike.hoffman@cogs.us>; Peter Hayes <peter.hayes@cogs.us>; Ryan Johnson <ryan.johnson@cogs.us>; Terri Partch <terri.partch@cogs.us>; Victor Meraz <victor.meraz@cogs.us>; Warren Hays <warren.hays@cogs.us>
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Thanks,

Trent L. Hyatt
970.384.6427 (p)
trent.hyatt@cogs.us

From: Trent Hyatt
Sent: Monday, September 21, 2020 4:02 PM
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Retail Marijuana Store Special Use Permit



The
KIND+Castle
Retail Marijuana Stores

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BACKGROUND

Kind West Glenwood, LLC (“**Applicant**”) is under contract to purchase Lot 11B, Glenwood Meadows No. 5, A Minor Subdivision of Glenwood Meadows No. 4 Lot Line Adjustment (“**Site**”) as shown in Figure 1. The Applicant intends to purchase the Site and construct a new retail marijuana facility (“**Facility**”) if the Special Use Permit and required license are approved by the City of Glenwood Springs (“**City**”). The Applicant also intends to construct a future retail space on the east side of the building. The Applicant will submit the required Site/ Architectural Plan Review application and construct the Facility and associated improvements starting in the spring of 2021 if the special use permit and retail license are approved by the City. Exhibit A contains the Retail Marijuana Store Plan Summary

The Property is located in the M2 Zone District as shown in Figure 2. The City of Glenwood Springs Development Code (“**Development Code**”) Section 070.030.020, Table of Allowed Uses, establishes that a retail marijuana business is an allowed use in the M2 Zone District subject to the approval of a special use permit.

The Property contains 0.406 acres and was created by the platting and construction of Flat Tops View Drive which resulted in the small sized parcel. The Property has approximately 147 lineal feet of frontage onto the Wulfshon Road Right-of-Way on its northern border. Flat Tops View Drive wraps around the Property on the west and south sides. The Property is gently sloping except for the grading for Flat Tops View Drive that is located within a 15-foot Access and Maintenance Easement. The slopes outside of the easement area range from a high USGS elevation of 5803 and a low USGS elevation of 5795 on the northeast corner for an overall change of eight (8) feet with a 12% grade. The Property is vacant with sidewalks along Flat Tops View Drive. Access to the Site will be provided by a new primary access drive off of Wulfshon Road and the current Glenwood Meadows sidewalk system.



Figure 1. The Site

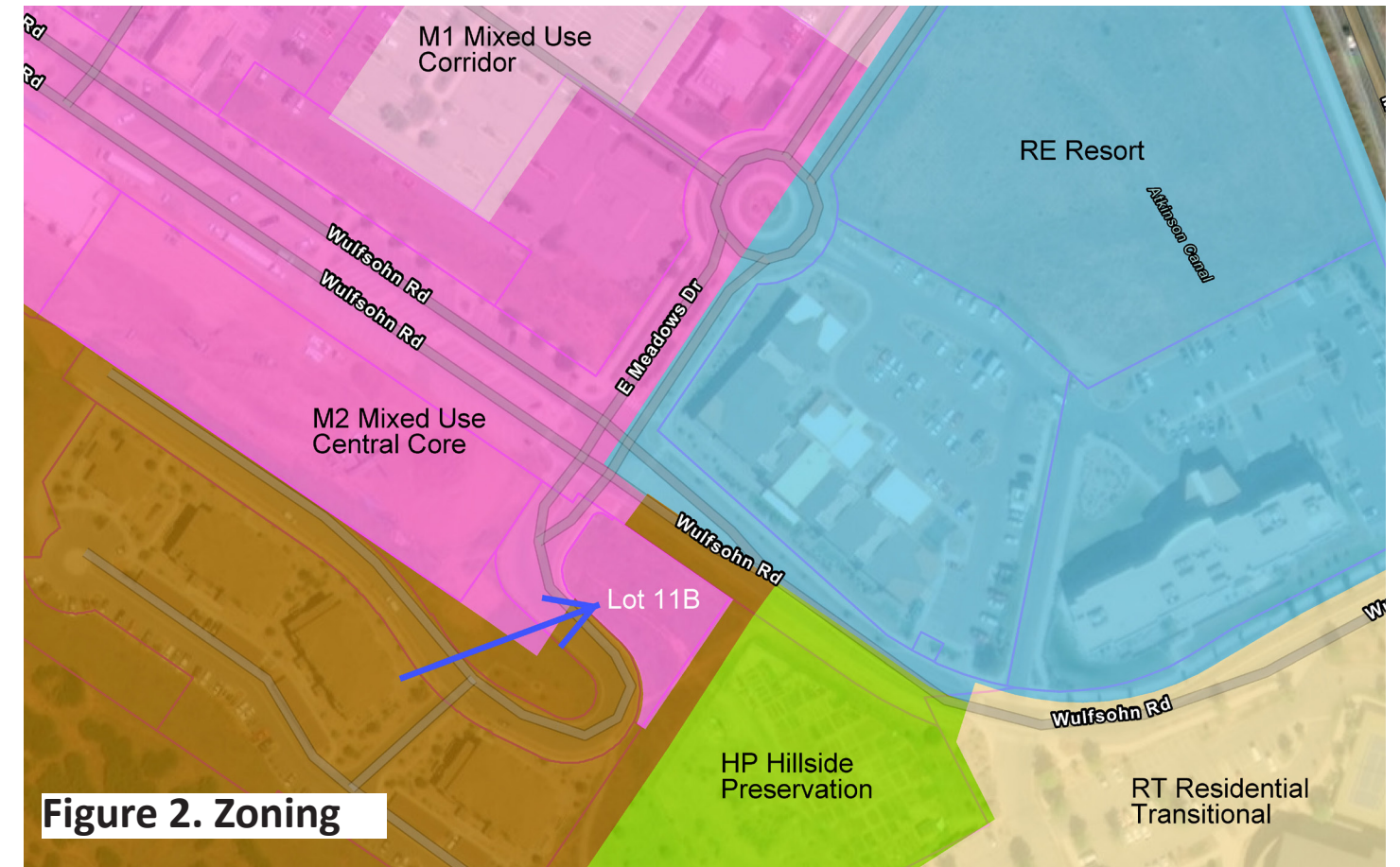


Figure 2. Zoning

SPECIAL USE PERMIT CRITERIA

Development Code Section 070.060.050(e) establishes the special use permit procedures. Section 070.060.050(e)(1) establishes the purpose of the special use permit process:

“Purpose. The special use permit procedure provides a mechanism for the City to evaluate proposed development and land uses that have unique or widely varying operating characteristics or unusual features. This procedure is intended to ensure compatibility with surrounding areas and that adequate mitigation is provided for anticipated impacts.”

This purpose and intent is important because it anticipates that mitigation can be provided to address any anticipated impacts and also assure project compatibility.

The following sections analyze how the project meets the special use permit approval criteria contained in Development Code Section 070.060.050(e)(3)(e)(2).

Section 1. Compatibility with Surrounding Area

The Facility will be compatible with the surrounding area. The Facility is located in a very intensive mixed use area that includes a resort hotel complex to the north, a regional shopping center to the northwest and high density multi-family housing to the west and south. An open space buffer exists to the east in between the Site and the Glenwood Springs Community Center. The Applicant proposes no ground or pole signage and will not depict any marijuana leaf or green crosses on its advertising.

The design of the Site and building will also assure compatibility with surrounding land uses. The Facility was located on the west side of the Site that allows for an extensive 99-foot setback to the eastern property line, and an approximate 143-foot setback to the community garden. A new fence and extensive landscaping along the eastern setback will provide a significant buffer to the community garden and Glenwood Springs Community Center.

The Residence Inn to the north is located approximately 135 feet from the Facility. The Glenwood Green Apartments are located approximately 190 feet too the south of the Facility. The Lofts at Red Mountain, located to the west of the Site, are over 75 feet from the Facility. Thus, there is a lot of space between the Facility and surrounding developments.

The Facility assures compatibility through both architectural and landscape design. The Facility is designed as a contemporary building within the design requirements of the Annexation and Development Agreement Glenwood Meadows, LLC Annexation, recorded as Reception No. 616083 in the Garfield County land records on December 6, 2002” together with all amendments thereto, hereinafter “ADA”. It therefore looks like any retail building in Glenwood Meadows within a high density, mixed use neighborhood. Extensive landscaping provides buffering to surrounding uses and blend the building and Facility use into the site. Building, walkway and parking area lighting will be dark sky compliant with no off-site glare. All windows have to be covered with an opaque security film so no pedestrians or surrounding properties can view activity inside of the store or the second floor operations.

The Facility will also ensure compatibility through its mechanical conditioning. The Facility will include four (4) activated carbon filtration, “Air Purifier T600 units”, with two (2) units serving the first retail level and two (2) units serving the upper floor that contain product storage and the product weighing and handling area. These additional carbon filtration systems are independent to the air filtration system running through the air conditioning/heating systems. The Air Purifier T600 units are an industry leading activated carbon air filtration system that are very effective at handling marijuana terpene emissions and overall odor. The Applicant’s plan described herein contains over quadruple the amount of air filtration operation and power for the size of the proposed retail space, as just Air Purifier T600 can remove odors from 2,000 square feet of space. Such odor mitigation efforts significantly exceed those deployed by other marijuana retailers in the city. The Facility mechanical, electrical and plumbing systems will be designed by qualified professionals to ensure the highest levels of performance, energy efficiency and odor prevention. Please refer to Exhibit B that contains more information on the proposed odor mitigation plan.

The Facility will also include a state of the art security, fire, and carbon monoxide systems that will be monitored by an off-site professional security firm. The Applicant will install many more cameras than required to ensure customer safety, neighborhood peace and employee accountability. In accordance with State regulations, security cameras will be installed to monitor and record all of the interior and exterior spaces of the Facility and the Site. Facility staff will be trained to view security cameras covering the customer parking areas and walkways to prevent loitering, excessive noise, or on-site consumption which is expressly prohibited by City and State laws. Staff will also periodically walk the property to ensure compatible and quiet customer use and Facility operations. Please refer to Exhibit C that contains the Retail Marijuana Store summary.

Glenwood Meadows LLC has approved the Applicant’s proposal and agreed that the Facility’s design and nature of business meets the compatibility criteria of the surrounding area and needs of the neighborhood. The Applicant’s Facility meets the demand and needs of the neighborhood including, but not limited to, the following facts: there are currently no other retail marijuana stores operating within or surrounding the Glenwood Meadows plaza; the Facility meets the City’s desired walkability and therefore reduces traffic within the city; and the Facility accommodates the new residential growth nearby, hotel patrons and surrounding businesses. All parties associated with the Applicant, including Glenwood Meadows LLC, have concurred that this site is best suited for retail sales and exceeds all compatibility requirements.

Section 2. Minimized Impacts on Surrounding Area

The impacts of the use on surrounding areas have been adequately minimized. The main impacts to surrounding properties for a retail marijuana facility typically can include odor, traffic, and parking.

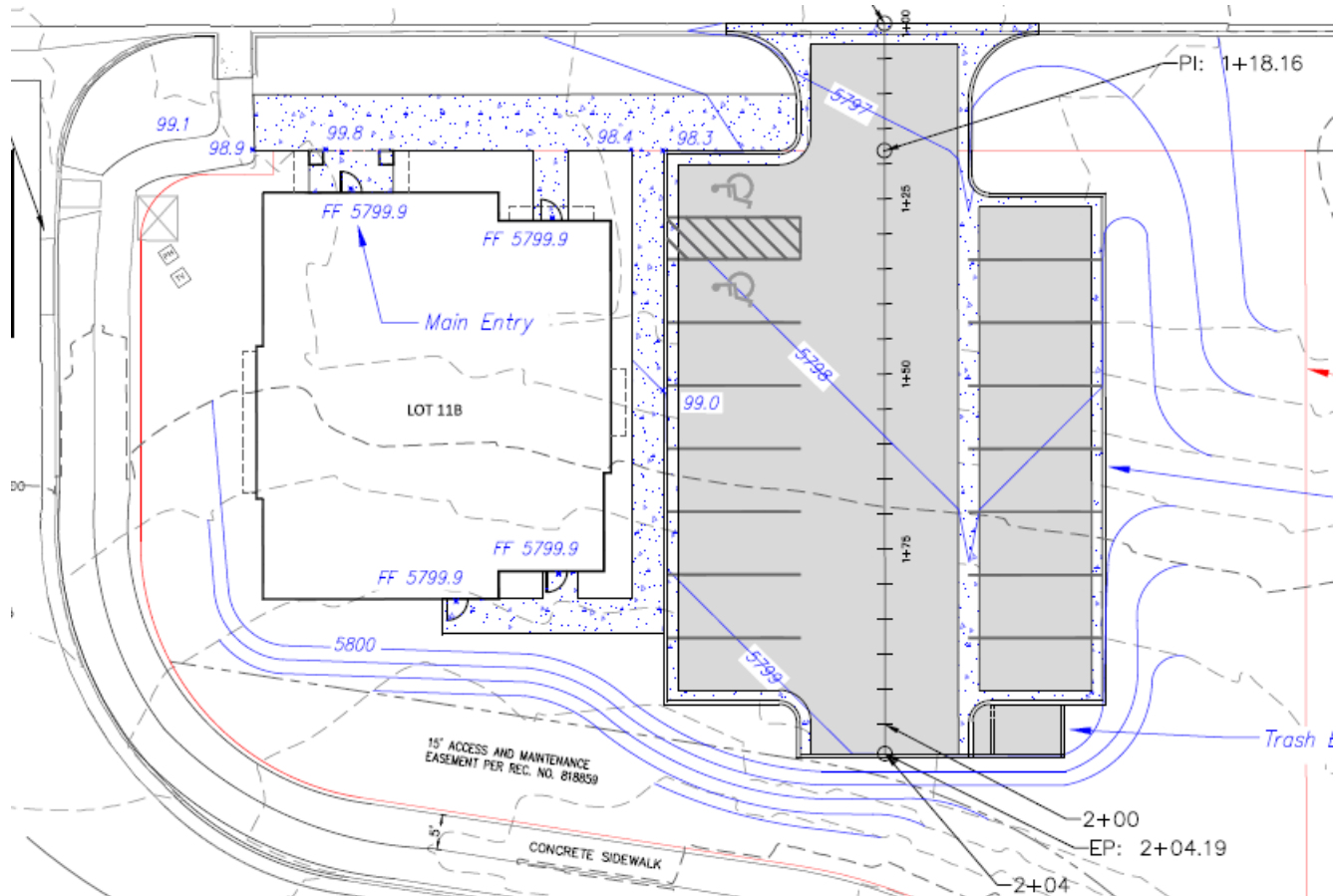
Odor. Odor will be prevented and mitigated as discussed in Section 1.

Traffic. There will be no traffic impacts since Wulfshon Road and East Meadows Drive have been designed to accommodate traffic and provide access to the Glenwood Meadows regional shopping center and mixed use development. These arterial and collector roads have great capacity for the traffic that will be generated by the store. We understand that the City may require a four-way stop at the Wulfshon Road-East Meadows Drive intersection in order to slow traffic down and allow for proper and safe flow. If this four-way stop is required by the City, the Applicant will install the stop signs as a part of the project improvements, or pay the City the equivalent money for it to install the stop signs, whichever is desired by the City.

Parking. Parking for the Facility and future retail space is calculated as follows:

Land Use	Habitable Floor Area ¹	Parking Requirement	Required Parking
Retail Marijuana Facility	2,423 sq. ft.	1 space/300 sq. ft.	9 spaces
Future General Retail	703 sq. ft.	1 space/400 sq. ft.	2 spaces
Total Required Parking			11 spaces
Parking Provided			16 spaces
Excess Parking			+5 spaces

Thus, the project provides more than sufficient parking in accordance with the City’s Parking Regulations.
1 The Development Code defines floor area as: “The total habitable horizontal area of all floors in a building. Bathroom, toilet compartments, closets, halls, storage or utility space, garages, and similar areas are not considered habitable space.”



The Facility will also be designed to fit into the neighborhood like any other retail store with the required architectural and landscaping design per the ADA, with additional mitigation provided by the eastern property fence and landscape buffering and overall site landscaping as shown on the plans. There will be no off-site glare from lighting.

Section 3. Consistency with the General Purposes of the Code

The use will be consistent with the general purpose and intent of this Code. Development Code Section 070.010.030 establishes the following purpose and intent, with comments on the proposed Facility compliance shown in *italics*:

Purpose. The general purpose of this Code is to protect the public health, safety, and welfare of the City and to implement the policies, goals, and strategies adopted by the City of Glenwood Springs, including those set forth in the Glenwood Springs Comprehensive Plan. *Over 60% of the City electorate voted to legalize marijuana under Amendment 64 in 2012 so there is broad support for the use in the city. The City adopted and modified regulations related to retail marijuana since that time with an understanding that the base rules established in the Development Code will protect the public health, safety and welfare while also limiting the number of stores in the city due to the use-specific standards. Retail marijuana facilities are only permitted via the special use permit process in 25% of the City’s zoning districts (4 of 16 zone districts) including M1, M2, CO, and I2 zone districts. Retail marijuana facilities are also subject to the retail marijuana use-specific standards contained in Development Code Section 070.030.030(e)(11) as discussed in Section 4. The Facility is located in a zone district that permits the use where it meets the use-specific standards and is compatible with surrounding uses. The Facility also generally conforms to the Glenwood Springs Comprehensive Plan as discussed in Section 5. Therefore, the proposed Facility is consistent with the purpose of the Development Code.*

Intent. More specifically, this Code is intended to create the best built environment for the City by:

1. Promoting orderly, efficient, and integrated development within the City. *The Facility will promote orderly infill development within the City in line with the underlying zoning and the specific requirements of the Development Code. The Facility will integrate a new use into the Glenwood Meadows area that is desired by more than 60% of city residents. This will allow for better diversity of desired retail uses in Glenwood Meadows and reduce traffic traveling outside of the neighborhood with strong pedestrian connectivity to existing and planned resort, high density and mixed use development. The development will be integrated into the Glenwood Meadows neighborhood through the roadway, sidewalks and the ADA design standards.*
2. Providing for a variety of housing and neighborhood types and densities at a range of housing costs, including affordable workforce housing. *No housing is proposed for the Facility because of past City concerns on mixing residential and retail marijuana uses. The Applicant also desires to maximize parking for the proposed Facility and future retail use. The Glenwood Meadows development is already providing a significant amount of housing for the City and region so there is no need to provide additional on-site housing.*
3. Regulating the development and use of land based upon the impact of such development or use on surrounding areas or the City. *The retail marijuana facility use-specific standards will ensure that there are no adverse impacts to surrounding uses. Odor, traffic, parking, lighting and use are mitigated as provided for in this narrative. City zoning rules allow for retail marijuana facilities via the special use permit process to ensure that uses are compatible with the surrounding area, ensure compliance with the use-specific standards and also require appropriate mitigation.*
4. Providing minimum standards for the design and improvement of land, subdivision, and development to ensure that each building site is capable of accommodating a structure devoted to the intended use of the land. *The Facility has been designed in accordance with the applicable standards and requirements of the Development Code as discussed in this narrative, including but not limited to the M2 Zone District dimen-*

sional standards (070.020.020); Facility use-specific standards (070.030.030); the site improvement and development standards (070.040.030); the landscaping, screening and fencing standards (070.040.050); access and circulation requirements (070.040.070); commercial building design per the ADA; and exterior lighting requirements (070.040.120)

5. Guarding against loss from natural hazards such as flooding, falling rock, landslides, snowslides, debris flow, and unstable soils. *The Glenwood Meadows development provides debris flow mitigation. The facility will be designed by a structural engineer to ensure that soils conditions are mitigated and the building will withstand expected and normal events, such as minor earthquakes. A geotechnical report will be provided with the required Architectural Review/building permit process.*
6. Ensuring equitable processing of development proposals by providing uniform procedures and standards. *The Facility is being processed and evaluated in accordance with the special use permit process; the retail marijuana facility use-specific standards; the ADA and other requirements set forth in the Development Code and Municipal Code.*
7. Providing a legally accepted record of lot lines, easements, and dedications in accordance with this Code and Colorado law. *The Site was legally created by the City through the subdivision process that was in effect at the time of platting.*

Section 4. Compliance with Applicable Standards of the Code

The use will comply with all applicable standards of the Development Code.

USE-SPECIFIC STANDARDS

The Facility will meet the use-specific standards contained in Development Code Section 070.030.030, with our

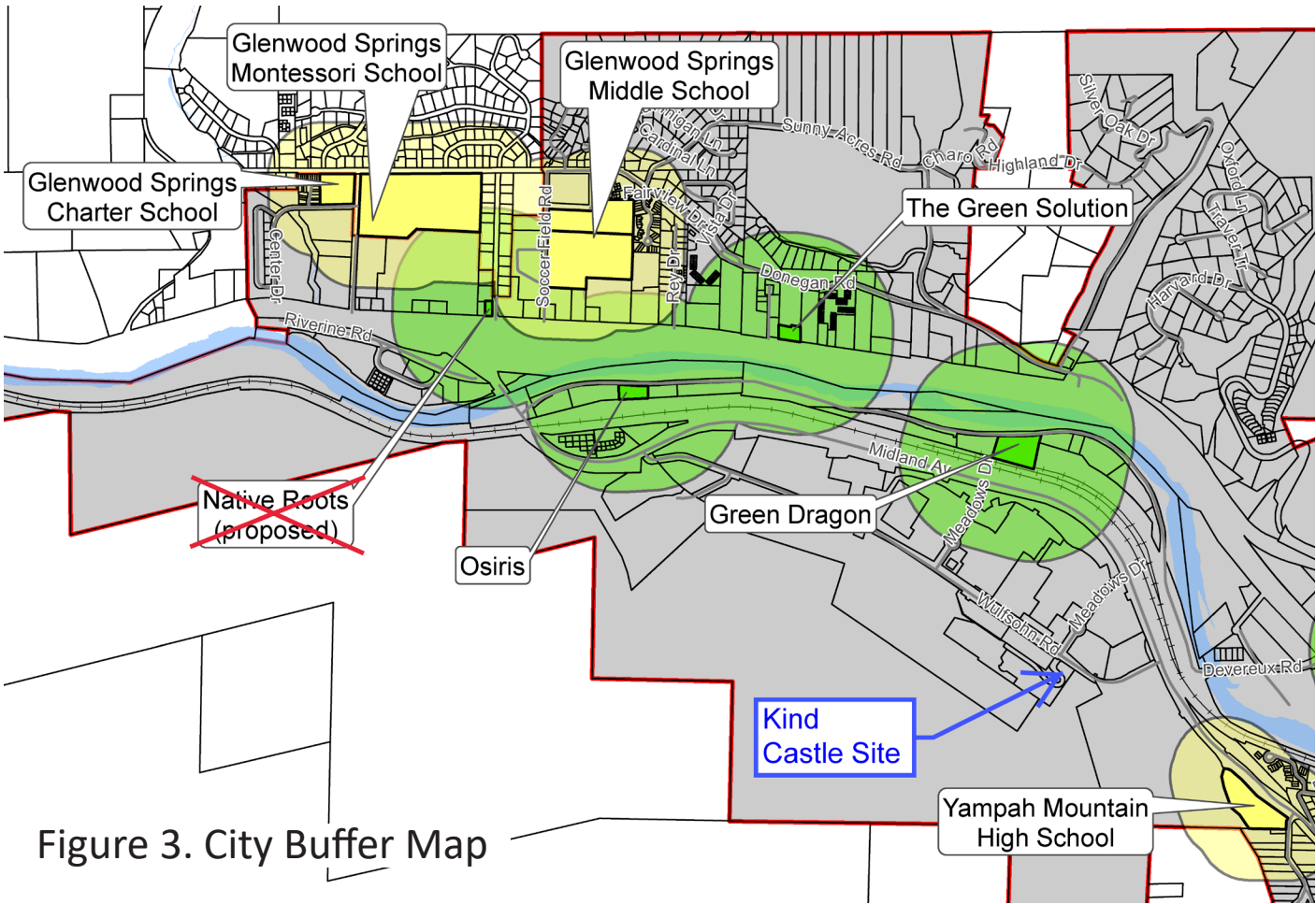


Figure 3. City Buffer Map

compliance comments shown in *italics*:

- a. Purpose. The City Council intends to allow state-licensed retail marijuana establishments to exist in Glenwood Springs in accordance with Article XVIII, Section 16 of the Colorado Constitution, Article 43.4 of Title 12 Colorado Revised Statutes, and any other applicable state laws and regulations as well as the additional local licensing requirements and other restrictions set forth in the Glenwood Springs Municipal Code. Nothing within this Article is intended to promote or condone the production, use, sale, testing or distribution of marijuana other than in compliance with applicable state law. *The Facility is proposed under the authority of Article XVIII Section 16 of the Colorado Constitution and the Colorado Marijuana Code in Colorado Revised Statutes Title 44, Article 10. The proposed Facility design and operation will comply with the Colorado Marijuana Code and the applicable requirements of the Glenwood Springs Municipal Code as discussed in this narrative. The Applicant will submit a retail marijuana license to the State of Colorado Department of Revenue Marijuana Enforcement Division ("MED").*
- b. License Required. Retail marijuana establishments shall comply with City Code Article 050.090. *The Facility will comply with the requirements of City Code Article 050.090, Retail Marijuana. The Applicant will submit a retail marijuana license and all required plans, materials and forms to the Glenwood Springs City Clerk (Local Licensing Authority). The Facility is not located within 500 feet of an existing public or private school facility where classes are held for K-12 grades. The proposed Facility is located more than 900 feet away from the closest retail marijuana facility, with the closest store, Green Dragon, approximately 1,880 feet away. The Facility is a permitted use in the M2 Zone District. Retail store staff will be trained to ensure compliance with the requirements related to the operation of a retail marijuana facility contained in Development Code Section 050.090.090. Facility staff will be extensively trained in the Colorado Retail Marijuana Code related to procurement, display, dispensing, labeling, sanitation, security and other criteria. Hours of operation will comply with the current city limitations. Facility signage will comply with the City Sign Regulations. Lighting will be designed in accordance with the Lighting Regulations. No on-site consumption will be allowed within the Facility. The Facility manager will be a designated employee of the Applicant that is listed on the required City forms. A sign will be posted in the store with all of the required warnings and notifications per City Code Section 050.090.090(b)(8).*
- c. Location of Licensed Establishment. Retail marijuana establishments shall be located only in areas of the City as allowed in the City Code. No retail marijuana establishment shall be located:
 - 1. Within five hundred (500) feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade. The distance shall be computed by a straight line measurement from the nearest property line of the school property to the nearest property line of the site housing the retail marijuana establishment. *The Facility is located over 1,700 feet from the Yampah Mountain High School property located at 695 Red Mountain Drive as shown in Figure 3, and is also located significantly out of the major transit routes to any school.*
 - 2. Within nine hundred (900) feet of another retail marijuana establishment or a medical marijuana business. A retail marijuana establishment may locate on the same licensed premises as a medical marijuana business of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other retail marijuana establishments and medical marijuana businesses. This distance shall be computed by a straight line measurement between property lines of the sites housing the two (2) facilities. *The closest existing facility is the Green Dragon that is approximately 1,880 feet away from the Site as shown in Figure 3. The other closest stores are Green Solution, Martin's Naturals, and Doctor's Garden that are more than 4,000 feet away.*
 - 3. In any zoning district not specifically permitted by the City Code. *The Facility is located in the M2 Zone District that allows retail marijuana facilities via the special use permit process.*
 - 4.

e. Requirements Related to Operation of a Retail Marijuana Establishment.

- 1. Retail marijuana establishments shall meet all operational criteria for the procurement, display, dispensing, labeling, sanitation, security, and any other criteria as required by the state pursuant to the CRMC and the City Code Article 050.090. *The Facility will meet the City and State operational requirements. The Applicant has been operating the Kind Castle in Parachute, CO for almost three years without any Town or State MED violations. The Parachute store has won many awards as one of the best stores in Colorado and attracts customers from far distances.*
- 2. In addition, all retail marijuana establishments shall comply with the following local restrictions:
 - i. Hours of operation shall be limited to between 8:00 a.m. and 7:00 p.m. Monday through Sunday. *The Facility will comply with the City hours of operation.*
 - ii. A sign permit shall be obtained through the Community Development Department. All exterior signage associated with a retail marijuana establishment will meet the standards established in the CRMC and in the City Code. *A sign permit will be obtained for the business signage. All signage will comply with the Colorado Marijuana Code, City Sign Regulations and retail marijuana ordinances.*
 - iii. Storage of retail marijuana shall be considered an accessory use. Not more than twenty-five (25) percent of the licensed premises of a retail marijuana business shall be used for such purposes. *The product storage room is located on the second floor and contains 289 sq. ft. of floor area that represents 8.8% of the total 3,297 gross sq. ft. for the Facility. The storage room size is 11.9% of the floor area of the facility per the Development Code definition of floor area.*
 - iv. No discernible odor shall be projected beyond the exterior walls of the licensed premises. *The proposed odor mitigation plan is outlined under Section 1. No odor will be projected beyond the exterior walls of the Facility. In general, retail stores typically do not generate an odor nuisance.*

OTHER DEVELOPMENT CODE REQUIREMENTS

The Facility has been designed in accordance with the applicable standards and requirements of the Development Code as discussed in this narrative, including but not limited to the M2 Zone District dimensional standards (070.020.020); facility use-specific standards (070.030.030); the site improvement and development standards (070.040.030); the landscaping, screening and fencing standards (070.040.050); off-street parking and loading standards; access and circulation requirements (070.040.070); commercial building design per the ADA; and exterior lighting regulations (070.040.120).

Drainage. Drainage for the project will be routed to the existing Glenwood Meadows storm drain and detention system that, according to Mike Maples of Glenwood Meadows, LLC, has more than adequate capacity to serve site runoff from the Site. Mr. Maples indicated that some of the hotel projects did not utilize this system which affords more than enough capacity to handle Site runoff via the storm drainage system. The development will have to pay into the Glenwood Meadows Public Improvement Fee that are earmarked to finance certain public improvements in the subdivision, including public roads, traffic and safety controls, water and sewer systems, storm drainage systems, debris flow management systems and landscaping.

Roads and Traffic. The road system has also been designed to handle all of the traffic from the anticipated development within Glenwood Meadows. A traffic impact analysis will be provided to better understand traffic flow at the Site and if a four-way stop is needed at the Wulfshon Road-Flat Tops Drive intersection. This study will be provided as a supplement to the application. The Glenwood Meadows Public Improvement Fee will help offset City road costs for the desired development as outlined in the preceding paragraph. The Applicant

will pay for the installation of the four-way stop signs.

Geotechnical. The Town approved the Glenwood Meadows Subdivision based on detailed geotechnical analyses and reports that included identification and mitigation of geologic hazards. The lots would not have been created unless the City found them buildable with identified hazard mitigation. Glenwood Meadows includes an extensive debris flow mitigation system. A site specific geotechnical analysis will be prepared for building design parameters and submitted with the required Site/Architectural Review process, and the building will be designed to meet the site specific recommendations of the geotechnical report.

Section 5. General Conformance with the Glenwood Springs Comprehensive Plan

The use is in conformance with the Comprehensive Plan and other City plans and policies. The proposed retail marijuana facility conforms to the Glenwood Springs Comprehensive Plan (“**Comprehensive Plan**”).

The proposed retail use is envisioned by the Future Land Use Map that shows Mixed-use in the Site area. The Future Land Use Descriptions indicate the following for Mixed-use areas:

“The Mixed-use land use designation allows for a variety of uses including commercial, retail, office, restaurant, entertainment and multi-family housing co-existing through design either in a horizontal or vertical fashion.”

The Facility helps maintain a small town character with a two story building surrounded by taller multi-family and commercial hotel buildings. The Facility provides a walkable retail destination that will serve existing and planned residential development in the area, and also compliments the regional shopping center to the northwest. The Facility is readily served by utility and transportation infrastructure, with all utilities either on or adjacent to the Site and a bus stop located to the west. The sidewalk system will provide direct access to Glenwood Meadows residents and visitors staying in the resort area to the north. This Facility’s location, well out of any highway corridor, will provide a local neighborhood shopping option that does not require Glenwood Meadows residents or guests to hop in a car. Ample on-site parking is available for those that have to drive to the store.

The Facility is located in the Glenwood Meadows “secondary commercial center” that provide convenient services, retail and commercial uses. The Comprehensive Plan states that “...these secondary centers are seen as opportune locations for mixed-use neighborhoods that include retail, offices and housing.” The Comprehensive Plan further states that:

“Secondary centers can be redeveloped with buildings up to 4-5 stories. Upper stories should be for residential uses. Development should be scaled in size to the surrounding neighborhoods, be walkable, and be landscaped or include buffering elements that both protect the surrounding neighborhood and integrate the mixed-use secondary center into adjacent established neighborhoods.”

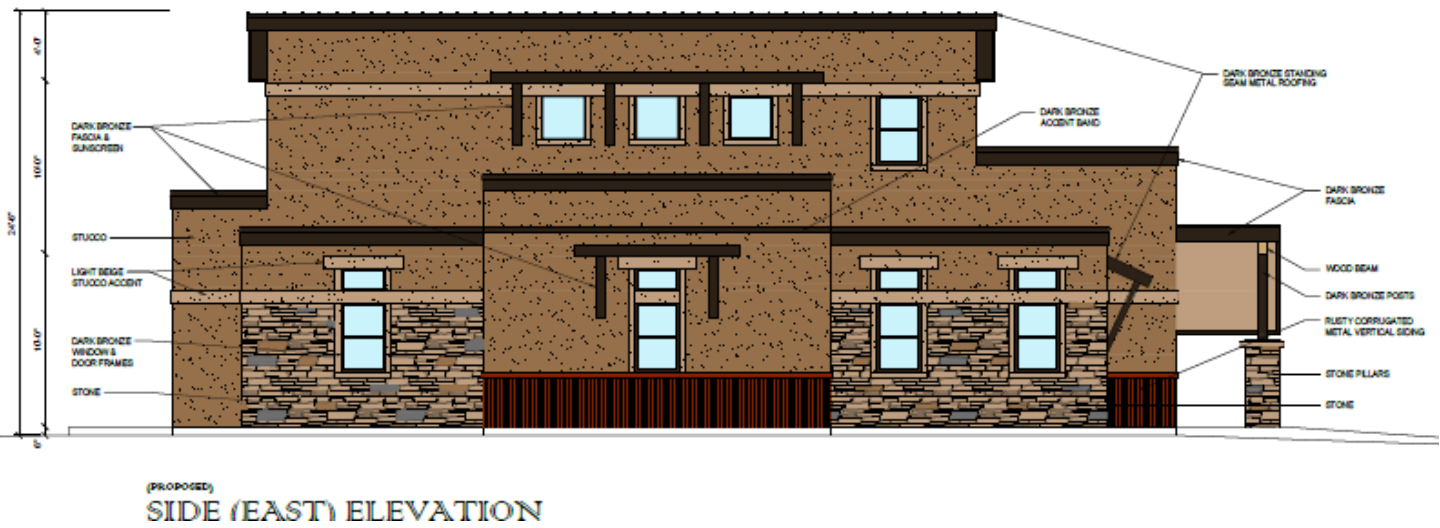
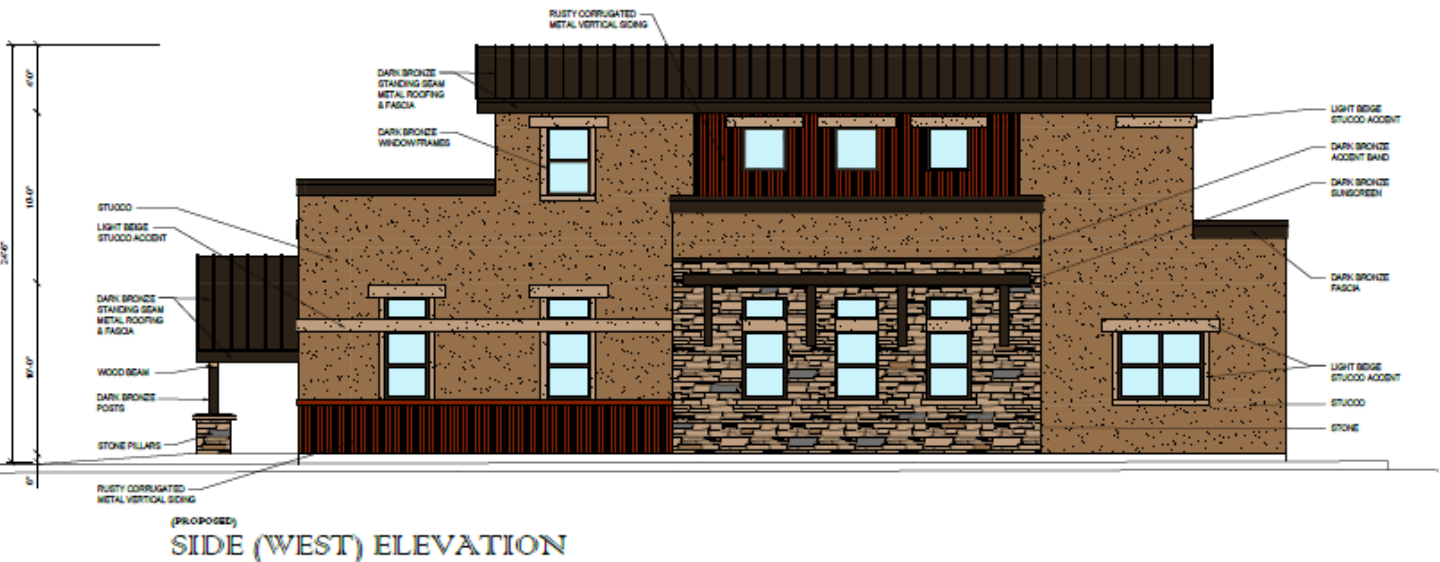
The Facility has been designed to fit the small 0.406 acre lot size that also provides a transition in height from the surrounding taller multi-family and lodging development to the open spaces uses to the east. The Facility’s smaller scale provides an appropriate-scaled pedestrian building at the eastern entry to Glenwood Meadows along Wulfshon Road. The Facility has also been designed in accordance with the ADA as required by the Development Code’s Nonresidential and Mixed-Use Site and Building Design requirements in Section 070.040.090.

The Facility is also supported by the Comprehensive Plan’s economic development policies. First, the approval of the special use permit will create an opportunity for new and expanding local business. The proposed store will employ approximately 12-15 people with good paying jobs and livable wages of \$15 to \$25 per hour or greater. The proposed store will enhance Glenwood Meadows as a regional business center that is convenient

to shoppers and lodgers patronizing Glenwood Meadows stores and hotels.

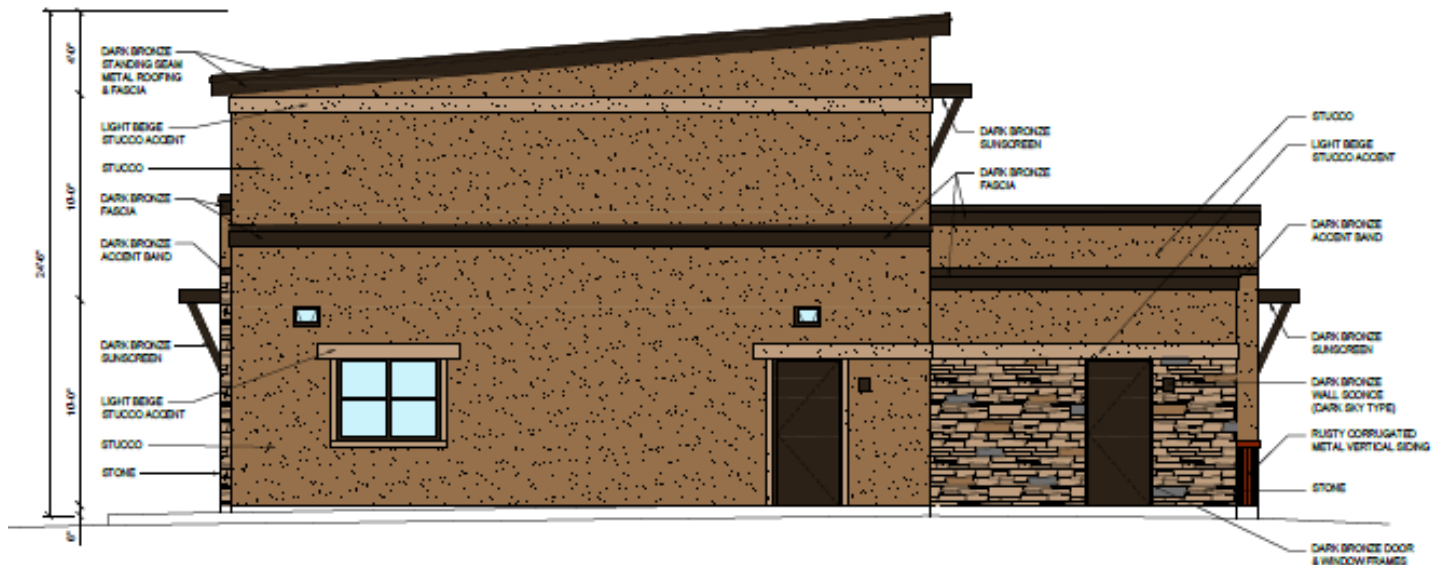
The Facility will also help Glenwood Springs to further integrate and balance the city’s multi-modal transportation system. The Store will reduce traffic from surrounding multi-family, lodging and mixed-use development that would otherwise have to drive to another store in the city. Sidewalks connect to the Facility from all of the surrounding developments. This in turn will reduce traffic on Grand Avenue and other highly congested areas of the city. It will also reduce single-occupant vehicles from leaving Glenwood Meadows that desire to purchase retail marijuana products.

The special use permit directs growth to an area where utilities and services can be efficiently provided. Water, sewer, electricity, natural gas, telecommunications and broadband are provided adjacent to the site. Police service, fire protection and emergency services are provided to the Site from the Glenwood Springs Police Department and the Glenwood Springs Fire Department. Drainage infrastructure for the overall Glenwood Meadows development is already in place. Security and fire alarm services will be provided through a security monitoring firm. The Applicant’s security system is an industry-leading system that will ensure the safety and security of the area.



The KIND+Castle

Retail Marijuana Stores



(PROPOSED)
REAR (SOUTH) ELEVATION



Uncompahgre
Engineering, LLC

VICENTE
SEDERBERG
LLP



Retail Marijuana Store Project Description

Name of Applicant

KIND WEST GLENWOOD LLC d/b/a THE KIND CASTLE

Hours of Operation

Monday – Sunday: OPEN 8am / CLOSED 7pm

Location & Zoning

TBD Wulfshon Road, Glenwood Springs, CO 81601 | M2 - Mixed Use (formerly C/2) Zoning

Lot 11B, Glenwood Meadows No. 5, A Minor Subdivision of Glenwood Meadows No. 4 Lot Line Adjustment

Description of Applicant's Services

The Applicant will provide (to end-use consumers) sales services consisting of the highest quality retail marijuana and marijuana-infused product. Products sold are organic driven. The business nature of the Applicant is a retail/recreational marijuana store and does not include medical sales, product-manufacturing or cultivation. Customers must be 21 years of age or older to enter, which will be **strictly** enforced through the Applicant's customer identification (ID) check/verification process including the first ID checkpoint upon entering into a fully secured, double-door lobby and then a second ID checkpoint at the sales counter. The Applicant will strictly enforce all of its policies and procedures, especially close oversight of its employees to screen all customers diligently for age and in-state/out-of-state residency status. The Applicant has a very strong interest in the knowledge of codes and compliance of this business and therefore the Applicant warrants that the business and its employees will at all times be in full compliance with the codes and regulations of the **City of Glenwood Springs** and the **State of Colorado's Marijuana Enforcement Division**. Furthermore, the Applicant has retained legal and consulting services to contribute to the current and future compliance of this business operation. The retail sales provided by the Applicant will solely occur at the **Applicant's physical business premises located at TBD Wulfshon Road in Glenwood Springs** and the Applicant will not provide delivery service to customers. The Applicant will offer quality products in proper packaging in a safe environment with full respect to the all industry-standard practices.

Lighting Plan (See "Floor Plan")

The Applicant's lighting plan will fully address the safety/security of the public, respective to city code, in addition to enhancing the city's nighttime character yet respecting the residents' value of the local mountain setting. The internal and external areas of the premises will have sufficient lighting for the safety of the customers, business, and surrounding residents, however, the Applicant will take precautions to ensure the safety of the installation/maintenance of such lighting fixtures. The Applicant will be in full compliance with the City code including, but not limited to, parking, display, security, and general purpose lighting structures.

Storage of Products

The Applicant will securely store its products on-site in a room identified as the Product Storage Room, which will house all inventory, receipts and the safe, located in the back portion of the building. The Product Storage Room shall consist of 9" solid brick walls, double-bolt door, and secured safe. The Product Storage Room will also have its own alarm system, only accessible to management level staff. For adequate product safety and odor control, products will be stored inside a locked safe in labeled, sealed containers in a temperature-controlled environment. The Product Storage Room, in addition to the rest of the premises, will be under video surveillance at all times. For more details regarding Applicant's policies relating to Product Storage, see "Storage of Marijuana Products" section of the "Retail Marijuana Store Plan Summary" document accompanying the Retail Marijuana Application.

Security Plan Summary

The Applicant takes security and the safety of the public very seriously. Therefore, an extensive security plan beyond industry-standard practices will govern the business establishment. The Applicant's security plan consists of alarms, video surveillance, monitoring services, motion sensors, and many other components. The Applicant will install and maintain alarms systems on-site at the premises, with remote alarm system monitoring services which operate 24 hours per day, 7 days per week. The alarm systems will have functions of 24/7 remote monitoring services, motion detection devices throughout the premises (particularly at all points of entry/exit including windows), and security measures such as a silent duress alarm in the event of robbery during business hours. Codes for alarm systems will be changed frequently. Employees must complete several training protocols such as a complete security and safety training courses. The Applicant will not employ any person that does not appear to be able to enforce Applicant's security measures.

The Applicant will install and facilitate the full use of full color, high-resolution video surveillance 24 hours per day, 7 days per week. Internally, the video surveillance will sufficiently monitor and capture such recordings of all entrances/exits of the premises, windows, lobby area, restricted areas, Security/Product Storage Room/Safe(s), points of sale, and all rooms/space within. Externally, video surveillance will capture front, side and rear views of the premises in addition to the covering all entrances/exits, surrounding area in front of the establishment, and the alley driveway. See the "Retail Marijuana Store Plan Summary" for additional supporting details relating to the Applicant's security procedures. See Applicant's Floor Plans (drawn to scale) for indications of camera locations throughout the proposed licensed premises.

ID Check Points

The Applicant's strict policy on double checking the unexpired and appropriate identification from both in-state and out-of state customers for age verification ensures no underage person shall be sold any products from this establishment. Our store in the Town of Parachute has passed underage sting operation tests conducted by the State of Colorado's Marijuana Enforcement Division (see attached letter from the MED). All entrants to the premises have their ID checked once at the door and again at the sales counter. Employees undergo extensive and frequent training and compliance enforcements measures to IDs are consistently checked properly.

Ventilation & Odor Mitigation Plan

The Applicant's licensed premises shall include an aggressive plan to mitigate any odor from permeating through the exterior of the building containing the licensed premises. The plan will include utilizing an extensive, top-quality carbon filtration system to be installed in the building at TBD Wulfshon Road, Glenwood Springs, CO 81601. The carbon odor filtration system implemented and installed by the Applicant at the licensed premises shall meet and exceed the odor mitigation requirements set forth in the City of Glenwood's Retail Marijuana Business Ordinance and expectations for such establishment. The carbon filtration shall effectively filter the air and ensure odor is never an issue. In our current operation in almost 3 years, we have never had a complaint about odor. The licensed premises shall operate a fully functional odor elimination and ventilation system as desired by the City of Glenwood Springs.

Waste Disposal Plan

Please see Applicant's "Plan Summary" and "Operations Manual" for all details related to the waste disposal plan.



CANNABIS ODOR ELIMINATION

The Kind Castle shall utilize extensive odor mitigation tactics to ensure marijuana odor is filtered from the air to avoid permeating the Applicant's marijuana store site. The new air conditioning/heating system's negative air machine (model #6560 or similar) will be equipped with an activated carbon air filter (made from 100% coconut shells) installed at the vent of the system's air return, which will powerfully draw in air from the premises and force it through the activated carbon filter and therefore eliminating odors from the premises. The filters will be changed routinely to ensure peak operation and performance at consistently and aggressively removing marijuana odor.

Additionally, The Kind Castle shall purchase and operate four (4) additional activated carbon filtration units, with two (2) in the lower level sales area and two (2) in the upstairs storage, product handling and office area. These additional carbon filtration systems are independent to the air filtration system running through the air conditioning/heating system defined above. The four (4) additional individual standalone carbon air filtration systems are called Air Purifier T600, which is known as a leading activated carbon air filtration system as most effective, especially at handling marijuana terpene emissions and overall odor. The Kind Castle's plan describes herein contains over quadruple the amount of air filtration operation and power for the size of the proposed retail space, as just Air Purifier T600 can remove odors from 2,000 square feet of space.

The following pages contain additional product information and technical specifications of the Air Purifier T600.

Air purifier T600

An air purifier specially designed to remove the odors and gases of thousands of noxious chemicals including marijuana odor, carcinogens and tars contained in smoke.

Odor Control

There are a group of compounds found in marijuana called terpenes, in addition to the well-known cannabinoids. These essential oils give marijuana their signature tastes and aromas. Terpenes evaporate quickly making them VOCs (volatile organic compounds). This quick evaporation causes their aromas to disperse easily causing that skunky smell. Your best friend in neutralizing terpenes is activated carbon. An air purifier with the maximum amount of carbon that allows for maximum dwell time of the terpenes over the surface area will remove this smell from the premises.

Why this Air Purifier is considered #1:

- Powerful 26-lb carbon filter adsorbs odors from the air and is very effective in achieving odor mitigation.
- Unique TarBarrier pre-filter traps tars, completely eliminating them from air stream.
- HEPA barrier captures 95% of airborne particles, as small as 1 micron.
- 360° air distribution allows for purification for up to 2000 sq. ft with 2 air exchanges an hour (based on 8-foot ceilings).
- Sleek machine design that will blend in with all decors.
- Easy care and maintenance

Technical Specs

Manufacturer Suggested Room Size (sq.ft)	2,000 sq.ft
Ozone Emissions	No
Performance	
System Efficiency	95% of airborne particles $\geq$ 1 microns
Fan Speed	560 CFM (Free Flow)
Air Exchange per Hour	2
Filter Configuration	

Filter Configuration

Chemical gas & odor filter

3" Carbon Filter

26-lbs, Activated Carbon Filter (100% Coconut Shell)

Replace: 24 months

Upgrade Option

26-lbs, Impregnated Carbon Filter

The potassium iodide accelerates the binding of chemicals faster and thus removes them quicker
(Potassium Iodide, Coconut Shell Carbon)

Main Particle Filter

HEPA Barrier

95% of airborne particles $\geq$ 1 micron

Replace: 1 year

General Specifications	
Dimension	23" High, 15" Wide
Colors	Black, White, Beige
Housing Materials	Powder coat steel
Fan Speed	Variable
Casters	Yes
Voltage	115 or 220 volts



Retail Marijuana Store Plan Summary

The Applicant's premises will meet and exceed the security requirements set forth by the State and City. This Plan Summary will detail extensive security measurements such as secure doors/locks, alarm system equipment/services, video surveillance, and various other measures of security enhancements and safety procedures specified herein. There are no anti-personnel devices that impede entry to the premise by emergency responders. The business does not propose to engage in the production of retail sale of food or other products for human ingestion, and no products or services will be provided at a location different than the address on the license application. There are no toxic, flammable, or other such regulated materials on site other than lighters and lighter fluid which are sold at the sales counters as indicated on the Floor Plan.

Security Alarm Systems

The Applicant's premises will install and maintain a security alarm system, provided by a reputable alarm system installation company. The entire premises will hold a total of two (2) separate alarm system units, one (1) alarm system exclusively dedicated to the Product Storage and Safe Room. Each alarm system and respective suite area of coverage will have its own alarm keypad and unique access code, in addition to accompanying support services from the alarm system provider including motion detection sensors. All perimeter entry points and windows will be covered by the alarms systems.

The Applicant's alarm system records and existing contracts will be kept current and located within the business premises. The name and contact information of the alarm system installation company will be kept available on-site at the business. The Applicant will also maintain records of the location and respective operation of each security alarm system.

Security Monitoring Services

The Applicant will fulfill the requirement by engaging in the services of a monitoring company. The monitoring company will monitor the alarm systems of the premises 24 hours a day, 7 days a week. The monitoring company will notify the Applicant and the local authorities of any identified security breach.

The Applicant's monitoring service records and existing contracts will be kept current and located in the business premises. The name and contact information of the monitoring company will be kept available on-site at the business.

Locking Mechanisms

At all points of ingress and egress at the Applicant's premises, the Applicant will use commercial-grade door locks and locking mechanisms.

Video Camera Surveillance

The Applicant will maintain video surveillance systems at the licensed premises at all times. The internal video surveillance will broadly monitor and record all entrances and exits to/from the premises, including, but not limited to, all windows, the lobby area, limited/restricted access areas, ID checkpoints, product storage and preparation room, and all points of sale. The external video surveillance will monitor and record the front and rear areas of the premises including all entrances/exits, areas surrounding the front/rear of the establishment, and the rear alley driveway.

The following summary of guidelines for the Applicant's video surveillance requirements will be applicable:

Before operating as a Retail Marijuana Store, the Applicant will install a high-specification, full color video surveillance and camera recording system. The video surveillance recording system will record in digital format and meet the

requirements specified in the Retail Code. All video surveillance records and recordings will be stored in a secure area that is only accessible to the Applicant's management staff member(s). Video surveillance equipment will, at a minimum, consist of digital or network video recorders, video monitors, digital archiving devices, a color printer capable of delivering still photos, and cameras capable of meeting the recording requirements described in the Retail Code including, but not limited to, clearly identifying all individuals entering and exiting the Applicant's premises.

The video surveillance systems will be equipped with a failure notification system that provides prompt notification to the Applicant of any prolonged surveillance interruption and/or the complete failure of the surveillance system. The Applicant will ensure that all surveillance equipment is properly functioning and maintained, so that the playback quality is suitable for viewing and the surveillance equipment is capturing the identity of all individuals and activities in the monitored areas. In addition, all video surveillance equipment will have sufficient battery backup to support a minimum of four hours of recording in the event of a power outage and the Applicant will notify the Division of any loss of video surveillance capabilities that extend beyond four hours.

The video cameras will cover all Limited/Restricted Access Areas, point-of-sale areas, security rooms, all points of ingress and egress to Limited Access Areas, all areas where Retail Marijuana or Retail Marijuana Product is displayed for sale, and all points of ingress and egress to the exterior of the Licensed Premises. Camera placement will be capable of identifying activity occurring within 20 feet of all points of ingress and egress and will allow for the clear and certain identification of any individual and activities on the Licensed Premises. At each point-of-sale location, camera coverage will enable recording of the customer(s) and employee(s) facial features with sufficient clarity to determine identity. All entrances and exits to the facility will be recorded from both indoor and outdoor vantage points. The system will be capable of recording all pre-determined surveillance areas in any lighting conditions. Areas where Retail Marijuana is stored will have camera placement in the room facing the primary entry door at a height which will provide a clear unobstructed view of activity without sight blockage from lighting hoods, fixtures, or other equipment. Cameras will also be placed at each location where weighing, packaging, transport preparation, processing, or tagging activities occur. At least one camera will be dedicated to record the access points to the secured surveillance recording area.

The surveillance room or surveillance area will be a Limited Access Area. Surveillance recording equipment will be housed in a designated, locked, and secured room with access limited to authorized employees, agents of the Division and relevant local jurisdiction, state or local law enforcement agencies. The Applicant will keep a current list of all authorized employees and service personnel who have access to the surveillance system and/or room on the Licensed Premises and also keep a surveillance equipment maintenance activity log on the Licensed Premises to record all service activity including the identity of the individual(s) performing the service, the service date and time and the reason for service to the surveillance system. Off-site Monitoring and video recording storage of the Applicant's Premises by the Applicant or an independent third-party will be performed provided the standards exercised at the remote location meet or exceed all standards for on-site Monitoring.

Video Recording/Retention Requirements

All camera views of all Limited/Restricted Access Areas will be continuously recorded 24 hours a day. The use of motion detection will only be used when the Applicant can demonstrate that monitored activities are adequately recorded. All surveillance recordings will be kept for a minimum of 40 days and be in a format that can be easily accessed for viewing. Video recordings will be archived in a format that ensures authentication of the recording as legitimately-captured video and guarantees that no alteration of the recorded image has taken place. The Applicant's surveillance system/equipment will have the capabilities to produce a color still photograph from any camera image, live or recorded, of the Licensed Premises. The date and time will be embedded on all surveillance recordings without significantly obscuring the picture.

Other Records

All records applicable to the surveillance system will be maintained on the Licensed Premises. The Applicant will maintain a map of the camera locations, direction of coverage, camera numbers, surveillance equipment maintenance activity log, user authorization list, and operating instructions for the surveillance equipment. A chronological point-of-sale transaction log will be made available to be used in conjunction with recorded video of those transactions.

In compliance with state and City regulations and in the interest of providing a safe and secure environment for our employees, our products, and our establishment, the premises will be equipped with state of the art security equipment. The Applicant has also established strict policies and procedures, which, under the direction of an on-staff expert of

the Colorado Retail Marijuana Code will guide our employees in all areas of dispensing retail marijuana with strict compliance. The security system will be monitored by an extensive camera operation both inside and outside the establishment.

Applicant's Facility Security Plan includes procedures such as:

- Building and Site Security Measures
- Security of Products
- Employee Parking Safety Precautions
- 24/7 Coverage
- Security Systems
- Maintenance of Security Systems
- Access Control / Ingress & Egress
- Perimeter Security

Applicant's Operational Security Plan shall include:

- Transactional Security
- Security Threats & Contingency Planning
- Security of Product Delivery
- Product Inventory Control
- Human Resource Procedures
- Employee Security Training
- Emergency Response Plan
- Visitor Procedures
- Neighborhood Involvement

The guidelines below shall define the security measures of the building, which includes access control hardware, intrusion detection, surveillance, and, at the main entrance at various times, security and/or management personnel for control and screening. All employees and approved contractors will enter through the front door of the building. Access in the building will be through the use of electronic access control mechanisms. This type of entrance prevents unauthorized access. Cameras will monitor and log all persons entering/exiting the facility.

Layer 1 - Entry Procedure:

- All employees will be badged and documented by the state of Colorado;
- Employees will use access credentials only to open the secure door;
- Each employee must open the door with his/her credentials, and only one employee may pass through the door at any given time;
- Vetted and registered guests may only enter the facility under the constant supervision of an employee after they have been photo ID verified, logged in, and issued a temporary ID;
- In the instance that two or more unique visitors arrive at the same time, they will be instructed via intercom to enter the foyer one at a time for processing; and
- All deliveries must be verified and logged in at the front entrance prior to receiving access to the loading areas.

Layer 2 - Entry Foyer:

- Be equipped with a secured lobby with hardened door to separate the public entry from the secure areas;
- Be equipped with a duress alarm;
- Have a log in sheet stating name, date, time, and reason for entry to the security room for legitimate purposes with the entry being witnessed by a security camera;
- Be locked at all times, even during business hours;

- Be equipped with “daytime annunciating” entry/exit doors;
- Have all activity in this area visible on CCTV and monitored by the security office;
- Have motion detectors located inside; and
- Have high security contacts on all doors.

Layer 3 - Entrance into the Office Area

The third layer of control will be to segregate with doors and hardware the generally accessible public area (reception area) to the staff-only areas. Additionally, staff-only areas will also have highly restricted access. Staff Morning Procedures:

- Office staff will be restricted electronically to only the office areas;
- Office staff will wear photo ID badges at all times;
- General office doors will not use key card access control;
- Dispensary staff will wear photo ID badges at all times;
- Dispensary staff will continue to limited and restricted access areas;

The Office Area Will:

- Be equipped with motion sensors;
- Be segregated by a hardened door with an electronic lock;
- Be accessible to all levels of office employees;
- Be equipped with hardwired and wireless emergency police call devices;
- Have video cameras monitoring all doors (exit and entrance) and doors accessing limited and restricted access areas where marijuana is for sale or stored;
- Include the security office that will remain locked and will not be used for any other function; and
- Have limited access to security, surveillance, and access control systems areas

Layer 4 - Access, Shipping and Receiving

Moving throughout the facility will be restricted even among staff. Employees will only be permitted in areas where they have specific tasks assigned. Employees will be given electronic access cards permitting entry to specific location(s) at specific time(s). Only the minimum number of staff required to perform assigned tasks will be permitted in the restricted access areas.

Staff Restrictions:

- Any picture or video recording on the premises is strictly prohibited;
- Having an access-controlled door leading from the limited access areas to the restricted areas;
- Allowing access only by assigned access credentials provided to workers as they are assigned tasks in specific areas;
- Granting and monitoring access by access credentials to the minimum number of authorized personnel needed (whether production personnel or laboratory employees) in secured areas where marijuana is kept;
- A clear schedule of who should be in what room, at what time, that will be monitored both visually and electronically by the facility manager and/or director of security;

Receiving Area Will:

- Have all doors controlled electronically;
- Be on a “time lock” to only be accessible during regular business hours;
- Have all doors remain closed and locked when not in use; and
- Have all working areas equipped with CCTV surveillance.

Layer 5 - Secure Safe Room

- Highly secure safe for all finished product with locking day gate;
- Must be opened by the manager or two employees with two unique codes simultaneously;

- Will be on a “time lock” to only be accessible during regular business hours;
- Inventory will be performed and documented at the close of each day on the contents of the safe room;
- Keep all equipment, safe(s), and any areas used for the storage of marijuana, securely locked unless in use;
- All lighting and security devices within the safe will be on emergency power; and
- Entrance doors to the safe will be monitored by CCTV.

General Security Guidelines

- Background checks for current/new employees, particularly if the employee is to be accessing restricted areas;
- Request employees to watch for suspicious activities and ask persons they don’t recognize to identify themselves and state their reasons for being at the facility;
- Adopt a security whistleblower protection policy;
- Retrieve keys and employment identification cards from an employee and change computer access passwords when their employment ends;
- Arrange for a 24-hour security service when there are unusual or extraordinary challenges to security;
- Keep all access system credentials, access codes, access cards, passwords, etc., secure and accessible only to specifically authorized personnel;
- Arrange for prompt and safe disposal of materials;
- All employees will be trained on emergency procedures;
- Post emergency response numbers, including fire, law enforcement, and executive team in several locations in the facility; and
- Quarterly security reviews with the Director of Security (and security staff) as it is their responsibility to constantly review and make recommendations for improvement to the Applicant’s Security Policy and Procedure Manual with all recommended changes being reviewed and approved by the members prior to implementation.

Summary of Restricted and Limited Access Areas

Limited Access Areas

"Limited Access Area" means a building, room, or other contiguous area upon the Licensed Premises where Retail Marijuana is grown, cultivated, stored, weighed, packaged, sold, or processed for sale, under control of the Licensee. All persons in a Limited Access Area will be required to hold and properly display a current license badge issued by the Division at all times. Proper display of the license badge will consist of wearing the badge in a plainly visible manner, at or above the waist, and with the photo of the Licensee visible. In addition, the business will be compliant with provisions from the Retail Code “Visitors in Limited Access Areas” and “Required Signage”.

Point of Sale: Restricted Access Areas

“Restricted Access Area” means a designated and secure area within a Licensed Premises in a Retail Marijuana Store where Retail Marijuana and Retail Marijuana Product are sold, possessed for sale, and displayed for sale, and where no one under the age of 21 is permitted. Purchases Only Within Restricted Access Area. A customer must be physically present within the Restricted Access Area of the Retail Marijuana Store’s Licensed Premises to purchase Retail Marijuana or Retail Marijuana Product. The purpose of this rule is to establish that a Retail Marijuana Store must control and safeguard access to certain areas where Retail Marijuana and Retail Marijuana Product will be sold to the general public and prevent the diversion of Retail Marijuana and Retail Marijuana Product to people under 21 years of age.

A. Identification of Restricted Access Area. All areas where Retail Marijuana or Retail Marijuana Product are sold, possessed for sale, displayed, or dispensed for sale will be identified as a Restricted Access Area and will be clearly identified by the posting of a sign which will be not less than 12 inches wide and 12 inches long, composed of letters not less than a half inch in height, which will state, “Restricted Access Area – No One Under 21 Years of Age Allowed.”

B. Customers in Restricted Access Area. The Restricted Access Area must be supervised by a Licensee at all times when customers are present to ensure that only persons who are 21 years of age or older are permitted to enter. When allowing a customer access to a Restricted Access Area, Owners and Occupational Licensees will make

reasonable efforts to limit the number of customers in relation to the number of Owners or employees in the Restricted Access Area at any time.

C. Display of Retail Marijuana. The display of Retail Marijuana and Retail Marijuana Product for sale is allowed only in Restricted Access Areas. Any product displays that are readily accessible to the customer must be supervised by the Owner or Occupational Licensees at all times when customers are present.

Summary of Transportation Plan

No stops will be made when transporting marijuana. The transportation manager will ensure that all delivery times and routes are randomized, vehicles and equipment are in good working order, and personnel are appropriately trained. Prior to transport, a dispensary agent will weigh, inventory, and account for on video and in the inventory system all marijuana products. The dispensary agent responsible for packaging a delivery order will complete a shipping manifest in a form and manner dictated by state regulations, for retention by the origination location, carry a copy of the manifest with the products being transported, and obtain an original signature from the recipient. Each delivery vehicle will be equipped with two-way radio communication equipment. The transportation manager will ensure the system is operational and each dispensary agent who transports marijuana is trained appropriately on its use, according to our Transportation Plan and any applicable state and local regulations. Radio contact will be maintained throughout the delivery day.

The transportation manager will be responsible for reporting all transportation events. In case of an emergency stop or an accident, a detailed reporting in the transportation event log must be recorded describing the reason for the event, the duration, the location, and any activities of personnel exiting the vehicle. In accordance with state regulations, the Applicant will immediately report any loss or theft of marijuana or marijuana products to state regulators and local law enforcement.

Summary of Inventory Plan

The Applicant will utilize comprehensive seed-to-sale tracking of all inventory and will use METRC, a compliant software system designed specifically for the intricacies of the marijuana industry. This will have online systems for Point of Sale (POS) tracking, inventory, sales, grow management, and other components of the business. Using a comprehensive tracking system will allow for real time reporting and oversight, as well as preventing diversion by increasing accountability. Inventory control will be implemented to account for all products. Inventory levels will be based on projected demand and adjusted to anticipate future customer needs. Beginning inventories will be established based on demand projected in this business plan. The Inventory Manager, along with department managers, will be responsible for tracking marijuana and marijuana-related products. All damaged, defective, expired, or contaminated marijuana will await disposal.

The Inventory Manager will ensure that inventory management and point of sale systems are accurate and capable of producing the necessary reports upon request. The Inventory Manager will direct department managers to perform certified inventory counts on a regular basis utilizing a cycle count method. A cycle count requires the department manager to perform a complete count of the inventory over a period of time counting inventory groups individually. The Inventory Manager, in consultation with Mark Mayasich, the Chief of Operations, along with back-up support by Trevor McGarvey Attorney-at-Law, will verify adherence to law, procedures, and policies. Department managers are responsible for the accurate recording of all inventory transactions in their department. The Compliance Officer is responsible to ensure that our cumulative inventory of usable marijuana reflects the projected needs of customers. Inventory limits will be adjusted by the Compliance Officer to anticipate future customer needs.

Inventory Tracking Requirements

1. In addition to all other tracking requirements set forth in these rules, a Licensee will utilize MITS to ensure its waste materials are identified, weighed and tracked while on the Licensed Premises until disposed of.
2. All Retail Marijuana waste must be weighed before leaving any Retail Marijuana Establishment. A scale used to weigh Retail Marijuana waste prior to entry into the MITS system will be certified in accordance with measurement standards established in Article 14 of Title 35, C.R.S. See Rule R 309 – Retail Marijuana Establishments: Marijuana Inventory Tracking Solution (MITS).
3. A Licensee is required to maintain accurate and comprehensive records regarding waste material that accounts for, reconciles, and evidences all waste activity related to the disposal of Marijuana.

Compliance / Quality Assurance / Quality Control

Quality Assurance and Quality Control. The Applicant will establish and follow a strict quality assurance and quality control program to ensure sufficient monitoring of laboratory processes and quality of results reported.

Identification

The purpose of this rule is to establish guidelines for the acceptable forms of identification for verifying the lawful sale of Retail Marijuana or Retail Marijuana Product.

Acceptable Forms of Identification for Retail Sales

Valid Identification to Verify Age Only. A Licensee will refuse the sale of Retail Marijuana or Retail Marijuana Product to anyone, unless such person can produce a form of valid identification of 21 years of age. If the identification contains a picture and date of birth, the kind and type of identification deemed adequate will be limited to the following, so long as such identification is valid and not expired:

1. An operator's, chauffeur's or similar type driver's license, issued by any state within the United States, any U.S. Territory;
2. An identification card, issued by any state for the purpose of proof of age using requirements similar to those in sections 42-2-302 and 42-2- 303, C.R.S.;
3. A United States military identification card;
4. A passport; or
5. Enrollment card issued by the governing authority of a federally recognized Indian tribe located in the state of Colorado, if the enrollment card incorporates proof of age requirements similar to sections 42-2-302 and 42-2- 303, C.R.S.
6. See below for valid identification to verify Colorado residency.

Valid Identification to Verify Colorado Residency. A Licensee will refuse the sale of more than one quarter of an ounce of Retail Marijuana or its equivalent in Retail Marijuana Product to anyone, unless such person can produce a form of valid identification of Colorado residency. As long as it contains a picture and date of birth, the kind and type of identification deemed adequate to establish Colorado residency for purchase will be limited to the following:

1. Valid state of Colorado driver's license;
2. Valid state of Colorado identification card; or
3. Any other valid government-issued picture identification that demonstrates that the holder of the identification is a Colorado resident.
4. No combination of identification or documents may be used to establish residency.

Storage of Marijuana Products

The Applicant's storage area will be climate controlled and have adequate lighting, ventilation, temperature, humidity, space, and equipment. There will be a separate area for storage of marijuana that is outdated, damaged, deteriorated, mislabeled, contaminated, or whose containers or packing have been opened or breached, until such products are destroyed.

All storage areas will be maintained in a clean and orderly condition, be free from infestation of pests, and maintained in accordance with the security requirements of Colorado state law. All marijuana in the process of preparation, transport or analysis will be housed and stored in locked containers and only the minimum number of specifically authorized employees will have access. At the end of the day, all marijuana and infused products will be returned to a secure, locked, and designated area. All safes, vaults, and any other equipment areas used for the storage of marijuana and MIPs will be securely locked and protected from entry at all times except for the actual time required to remove or replace marijuana. Surveillance cameras will be pointed directly at these storage areas and the camera's field for view will not be obstructed.

Storage and transportation of finished products will be held under conditions that protect them against physical, chemical, and microbial contamination as well as against deterioration of them or their container. Marijuana will be stored in approved, secured, and surveyed areas and each will be labeled with a batch number throughout the process.

Storage and Display Limitations. The business will not display Retail Marijuana and Retail Marijuana Product outside of a designated Restricted Access Area or in a manner in which Retail Marijuana or Retail Marijuana Product can be seen from outside the Licensed Premises. Storage of Retail Marijuana and Retail Marijuana Product will otherwise be maintained in Limited Access Areas or Restricted Access Area.

Labeling / Packaging

The Applicant will comply with the State Licensing Authority's Retail Code to regulate and establish labeling and secure packaging requirements for Retail Marijuana, Retail Marijuana concentrates, and Retail Marijuana Product. The Applicant will ensure that all Retail Marijuana and Retail Marijuana Product are sold and delivered to lawful consumers in packaging that is not easily opened by children.

Waste Disposal

That waste is properly removed and the operating systems for waste disposal are maintained in an adequate manner so that they do not constitute a source of contamination in areas where Retail Marijuana or Retail Marijuana Product are exposed. That the Licensee provides adequate screening or other protection against the entry of pests. Rubbish will be disposed of so as to minimize the development of odor

and minimize the potential for the waste becoming an attractant, harborage, or breeding place for pests. A Retail Marijuana Testing Facility will properly dispose of all Samples it receives, that are not transferred to another Retail Marijuana Testing Facility, after all necessary tests have been conducted and any required period of storage.

A. All Applicable Laws Apply. Retail Marijuana and Retail Marijuana Product waste must be stored, secured, locked, and managed in accordance with all applicable federal, state, and local statutes, regulations, ordinances, or other requirements.

B. Liquid Waste. Liquid waste from Retail Marijuana Establishments will be disposed of in compliance with the applicable Water Quality Control Division statutes and regulations.

C. Hazardous Waste. Disposal of hazardous and chemical waste must be conducted in a manner consistent with federal, state and local laws.

D. Waste Must Be Made Unusable and Unrecognizable. Retail Marijuana and Retail Marijuana Product waste must be made unusable and Unrecognizable prior to leaving the Licensed Premises.

E. Methods to Make Waste Unusable and Unrecognizable. Retail Marijuana and Retail Marijuana Product waste will be rendered unusable and Unrecognizable through one of the following methods:

Grinding and incorporating the marijuana waste with non-consumable, solid wastes listed below such that the resulting mixture is at least 50 percent non-marijuana waste: Paper waste; Plastic waste; Cardboard waste; Food waste; Grease or other compostable oil waste; Bokashi or other compost activators; Other wastes approved by the Division that will render the Retail Marijuana waste unusable and Unrecognizable; and Soil. After Waste is Made Unusable and Unrecognizable. Licensees will not dispose of Retail Marijuana waste in an unsecured waste receptacle not in possession and control of the Licensee. After the Retail Marijuana waste is made unusable and Unrecognizable, then the rendered waste will be:

1. Disposed of at a solid waste site and disposal facility that has a Certificate of Designation from the local governing body;

2. Deposited at a compost facility that has a Certificate of Designation from the Department of Public Health and Environment; or

3. Composted on-site at a facility owned by the generator of the waste and operated in compliance with the Regulations Pertaining to Solid Waste Sites and Facilities (6 CCR 1007-2, Part 1) in the Department of Public Health and Environment.

G. Proper Disposal of Waste. A Licensee will not dispose of Retail Marijuana and Retail Marijuana Product waste in an unsecured waste receptacle not in possession and control of the Licensee.

All marijuana or marijuana containing items that are disposed shall be placed in the secured dumpster outside that shall be locked at all times.

Health & Safety

The Applicant and its employees will remain diligent at keeping intact the public's interest of health and safety. All state and City regulations pertaining to the health and safety of the public will be followed.

The site building will be maintained in a sanitary condition as follows;

- Toxic cleaning compounds, sanitizing agents, and pesticide chemicals will be identified, held, and stored in a manner that protects against contamination of Retail Marijuana or Retail Marijuana Product and in a manner that is in accordance with any applicable local, state, or federal law, rule, regulation or ordinance.
- All operations in the receiving, inspecting, transporting, segregating, preparing, manufacturing, packaging, and storing of Retail Marijuana or Retail Marijuana Product will be conducted in accordance with adequate sanitation principles.
- Employees are provided with adequate and readily accessible toilet facilities that are maintained in a sanitary condition and good repair.
- Retail Marijuana or Retail Marijuana Product that can support the rapid growth of undesirable microorganisms will be held in a manner that prevents the growth of these microorganisms.

Records / Reporting / Taxes

All business records will be accurately kept up-to-date and on-site. All taxes will be remitted in a timely and accurate fashion.

Testing requirements

The Applicant will ensure that all products sold or offered from its premises meet all current and future product testing and standardization requirements.

THE KIND CASTLE DISPENSARY GLENWOOD MEADOWS

TRAFFIC ASSESSMENT

Project Location: Lot 11B Glenwood Meadows No. 5
Glenwood Springs, CO

Prepared By: APEX Consulting Engineers, LLC
1000 N 9th Street, Suite 44
Grand Junction, CO 81501

Report Date: September 15, 2020



APEX
CONSULTING ENGINEERS
CIVIL ENGINEERS • MANAGEMENT • DEVELOPMENT



Kind Castle Dispensary, Glenwood Meadows
Traffic Assessment

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A. ITE Trip Generation Data

1. Introduction

APEX Consulting Engineers LLC (APEX) prepared this Traffic Assessment for the proposed 2.4 ksf marijuana dispensary to be located at Lot 11B Glenwood Meadows No. 5, Glenwood Springs, Colorado. The Traffic Assessment is being provided in conformance with the City of Glenwood Springs Engineering Design Standards § 5.11.2 for proposed commercial developments that are expected to generate less than 100 peak hour trips.

This Traffic Assessment will determine the expected increase in traffic volume entering and exiting the proposed access to/from Wulfsohn Road.

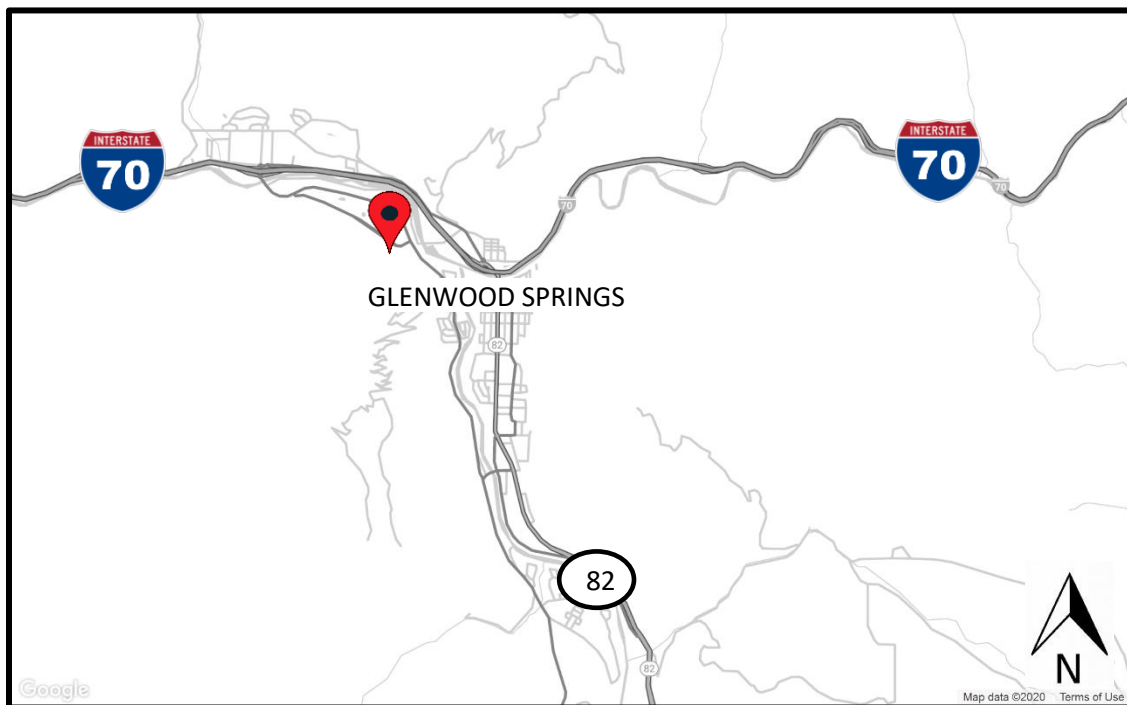
2. Project Location and Description

As shown in Figures 1, 2 and 3, the proposed Project is be located on the south side of Wulfsohn Road and will require a new access.

The Project is a proposed 2.4 ksf marijuana dispensary to be built within an existing mixed use development that currently contains multi-family housing and commercial businesses.

The proposed business will not open until 9:00 am or later, therefore morning peak hour traffic is not evaluated in this assessment.

Figure 1 - Site Location Map



3. Site Access

The site proposes one access, to Wulfsohn Road as shown in Figures 2 and 3.

Figure 2 –Project Site Access

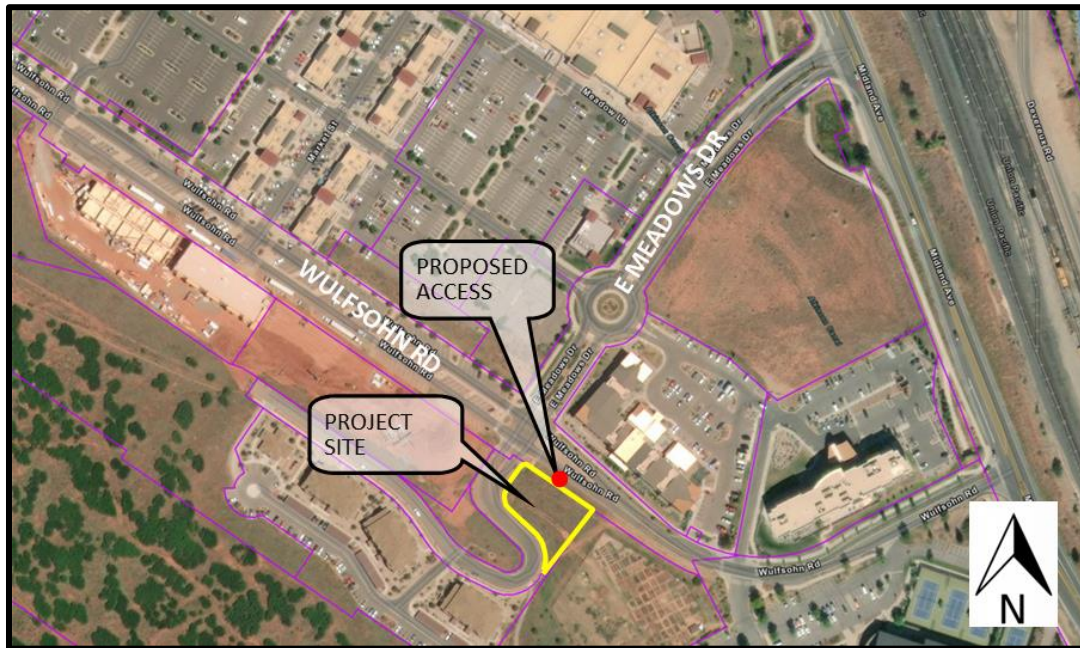
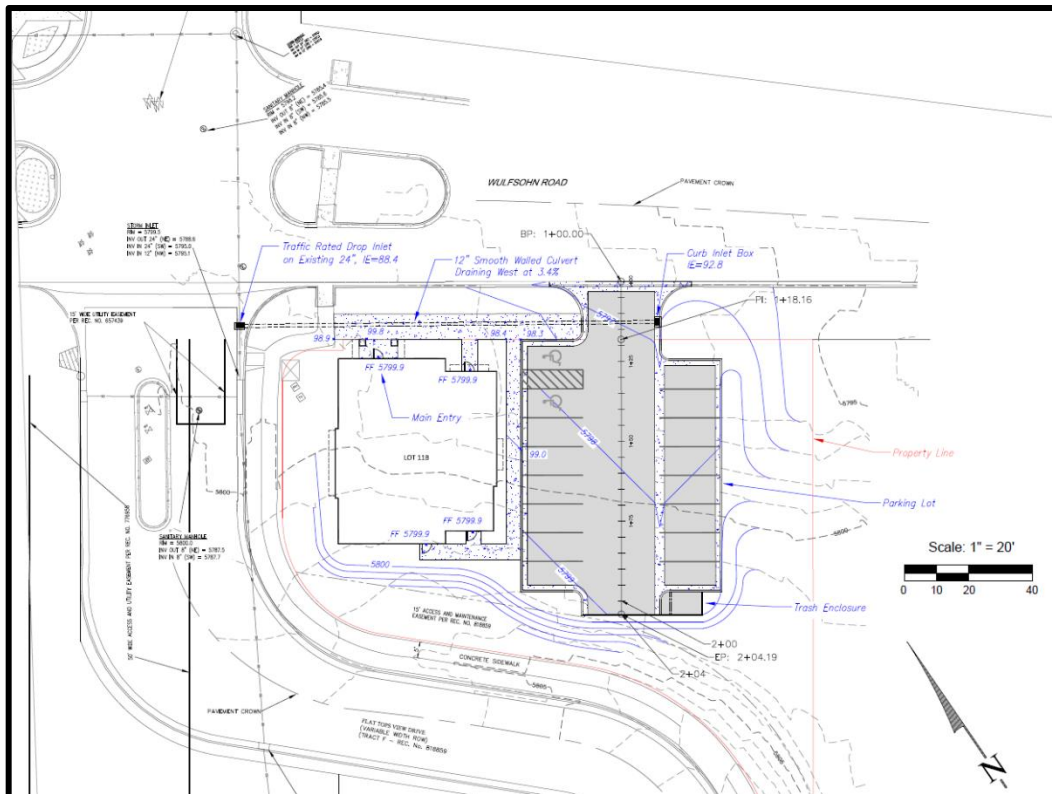


Figure 3 –Site Plan



4. Trip Generation

Land Use Code (LUC) 882, Marijuana Dispensary, from the Institute of Transportation Engineers (ITE) Trip Generation Manual 10TH Edition, is used to determine trip generation. In all peak hour cases, the higher of “either peak hour of roadway” or “peak hour of generator” is used, providing a conservative assessment. In this case, peak hour of generator is used. Trip generation traffic calculations from the ITETripGen Web-based App are attached in Appendix A.

Table 1 – Peak Hour Project Traffic Volumes

Period	Inbound	Outbound	Total
Peak Hour	36	36	72
ADT			606

5. Access

Wulfsohn Road is a two-lane roadway with a solid striped (painted) re-direct taper at the location of the proposed access. The current striping would make westbound left turns into the proposed driveway a prohibited maneuver. However, u-turns are allowed at the intersection of E Meadows Drive and Flat Tops View Drive, allowing westbound traffic to gain access to the Project driveway. If u-turns are not allowed in the future, westbound traffic have the ability to turn right on E Meadows Drive and utilize the round-a-bout to change directions at Wulfsohn Road.

A previous Traffic Impact Study¹ reported the 2018 daily traffic volumes along Wulfsohn Road to range between 1,700 and 1,760 VPD. The same study estimated future growth on the south side of Wulfsohn Road to be from residential development at 15 units per acre and would result in traffic volumes along Wulfsohn Road to be 2,400 and 2,600 VPD where the capacity is 12,000 to 16,000 VPD.

Wulfsohn Road has current and future traffic capacity for the proposed project.

6. Adjacent Intersections

The intersection of E Meadows Drive/Flat Tops View Drive and Wulfsohn Road is approximately 100' to the east. The intersection is a two way stop controlled intersection with stop signs on the north-south roadways (E Meadows Drive and Flat Tops View Drive). Eastbound traffic on Wulfsohn Road has a speed limit of 15 mph whereas west bound traffic has a speed limit of 25 mph until the intersection of Wulfsohn Road and E Meadows Drive. The volume to capacity ratio of roadway are low and the intersection should easily absorb the additional traffic.

The 2018 TIS indicated the intersection of Midland Avenue and Wulfsohn Road operates at a Level of Service (LOS) E and approaches the criteria required to warrant a traffic signal. The additional traffic generated from the proposed dispensary will add traffic to this intersection.

7. Parking

The project will provide 16 parking spaces including 2 ADA reserved spaces. This provides 14 spaces for customers and two for employees. The parking spaces allow for a turnover rate of approximately 22 minutes per parking stall, allowing for one ADA vacancy, during the PM peak hour.

¹ Felsburg Holt & Ullevig, *Wulfsohn Road Commercial Building Traffic Impact Study*, 2018

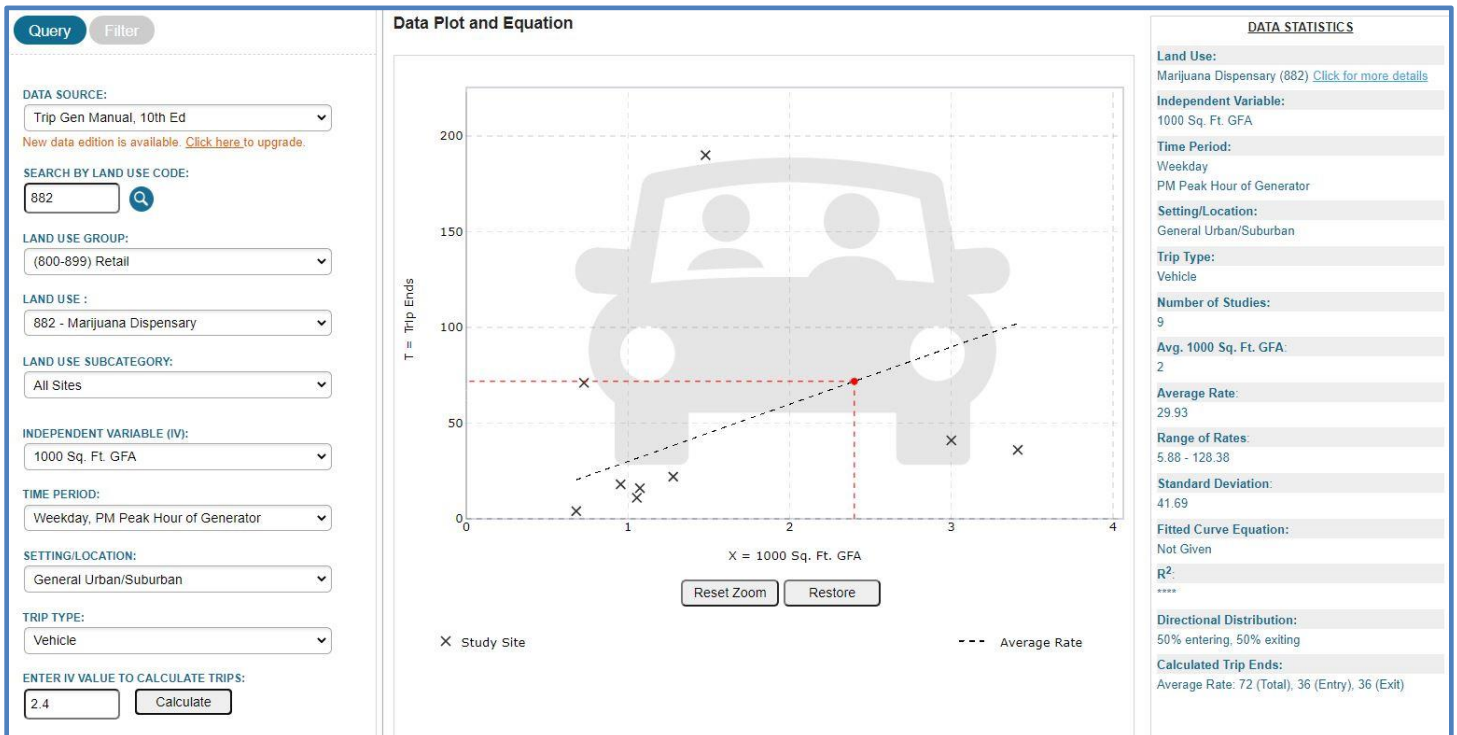
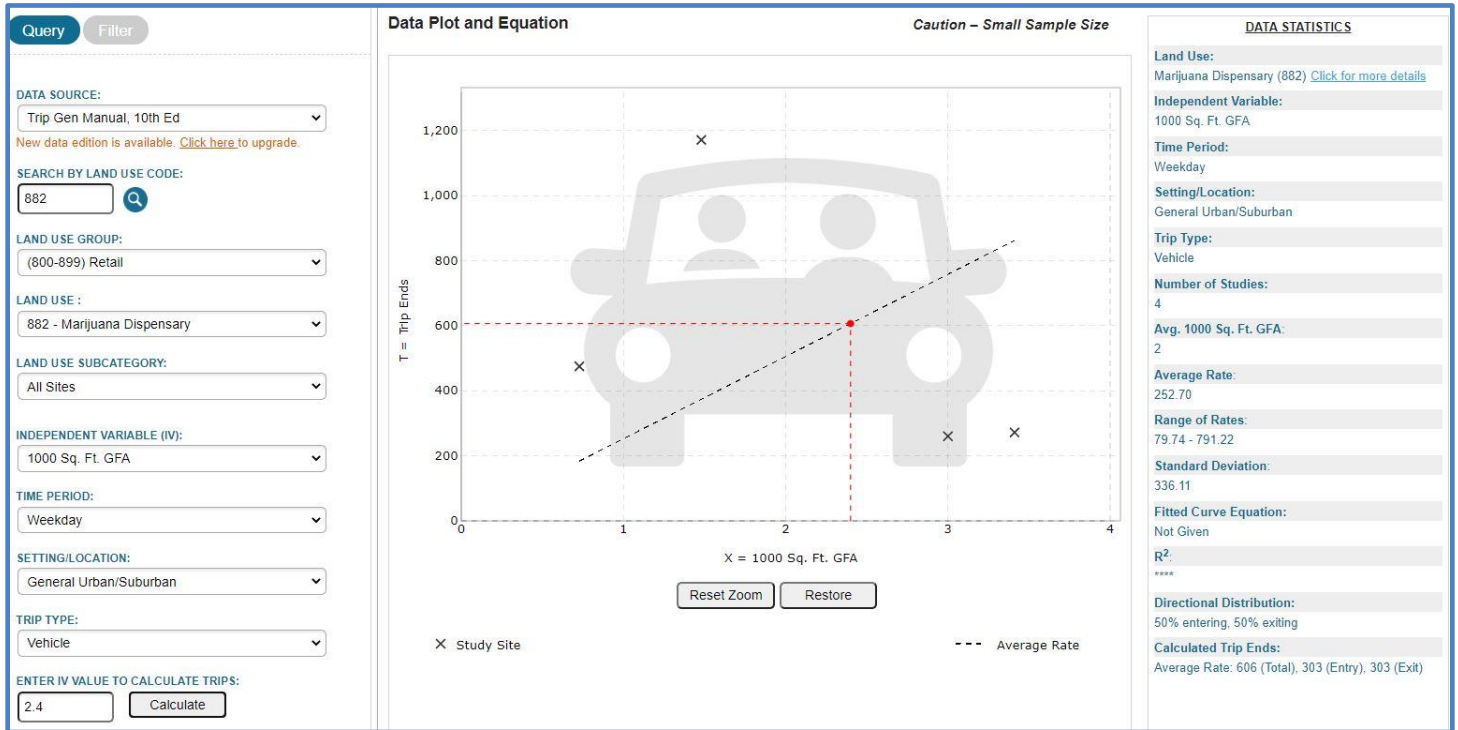


8. Conclusions

- The project will contribute an estimated 606 VPD to Wulfsohn Road.
- The project will contribute an estimated 72 trips/hour during the weekday PM peak hour.
- Proposed Project parking is adequate.
- Wulfsohn Road has adequate current and future capacity for the additional Project traffic.



Appendix A



SPECIAL WARRANTY DEED

Doc Fee: 706.51

GLENWOOD LAND COMPANY, LLC, a Colorado limited liability company ("Grantor"), with an address of 525 East Cooper, Aspen, Colorado 81611, for \$10.00 and other good and valuable consideration, the receipt and sufficiency of which are hereby acknowledged, hereby grants, sells and conveys unto GLENWOOD MEADOWS, LLC, a Colorado limited liability company ("Grantee"), with an address of 525 East Cooper, Aspen, Colorado 81611, that certain real property located in the County of Garfield, State of Colorado, described in Exhibit A attached hereto and made a part hereof (the "Property"), together with all improvements located thereon, subject only to the matters set forth in Exhibit B attached hereto and made a part hereof (the "Permitted Exceptions").

To have and to hold the Property unto Grantee, and its successors and assigns, forever. Grantor shall warrant and forever defend the title to the Property, except for the Permitted Exceptions, against all persons claiming by, through or under the Grantor.

~~EXECUTED~~ DATED this 16 day of OCTOBER, 1998.

GLENWOOD LAND COMPANY, LLC, a Colorado limited liability company

By: _____
Name: Robert Duncan Macgregor
Title: Manager

STATE OF COLORADO)
) ss.
COUNTY OF GARFIELD)

The foregoing instrument was acknowledged before me this 10th day of July, 1998, by Robert Duncan Macgregor, as the Manager of Glenwood Land Company, LLC, a Colorado limited liability company.

Witness my hand and official seal.

Notary Public

My commission ~~expires~~.

expirés.
MARGARET R. JOY
NOTARY PUBLIC
STATE OF COLORADO
12/2

My Commission Expires 12/23/2000

Return to Tom Hartert ^{REC.}
802 Grand
GWS, CO 8160

EXHIBIT A

PARCEL # 1

ALL THAT PROPERTY LOCATED IN THE NW1/4NW1/4 AND E1/2NW1/4 OF SECTION 8; THAT PORTION OF THE NE1/4 OF SAID SECTION 8 LOCATED SOUTH OF THE SOUTHERLY RIGHT OF WAY LINE OF THE MIDLAND AVENUE RIGHT OF WAY ACQUISITION; THAT PORTION OF THE NW1/2NW1/4 OF SECTION 9 LOCATED SOUTH OF THE SOUTHERLY RIGHT OF WAY LINE OF SAID MIDLAND AVENUE RIGHT OF WAY ACQUISITION; THAT PORTION OF THE SE1/4 OF THE NW1/4 OF SAID SECTION 9 BEING TRIANGULAR IN SHAPE BOUNDED ON THE WEST BY THE WESTERLY LINE OF THE SE1/4NW1/4 OF SAID SECTION 9, ON THE NORTH BY THE SOUTHERLY LINE OF SAID MIDLAND AVENUE RIGHT OF WAY ACQUISITION AND ON THE SOUTH BY THE WEST LINE OF THE RED MOUNTAIN DRIVE RIGHT OF WAY; THAT PORTION OF THE S1/2S1/2 OF SECTION 5 LOCATED SOUTH OF THE SOUTHERLY RIGHT OF WAY LINE OF SAID MIDLAND AVENUE RIGHT OF WAY ACQUISITION; THAT PORTION OF THE SE1/4SE1/4 OF SECTION 6 LOCATED SOUTH OF THE SOUTHERLY RIGHT OF WAY LINE OF SAID MIDLAND AVENUE RIGHT OF WAY ACQUISITION AND THE DENVER & RIO GRANDE WESTERN RAILROAD RIGHT OF WAY; ALL LOCATED IN TOWNSHIP 6 SOUTH RANGE 89 WEST OF THE SIXTH PRINCIPAL MERIDIAN. THE PROPERTY AS MENTIONED ABOVE BEING MORE PARTICULARLY DESCRIBED BY NOTES AND BOUNDS FOR ACRES AS FOLLOWS:

BEGINNING AT THE SOUTH EAST CORNER OF SAID SECTION 6; THENCE S. 75 DEGREES 55'37" W. A DISTANCE OF 1316.16 FEET ALONG THE SOUTHERLY LINE OF SAID SECTION 6 TO THE EAST 1/16 CORNER COMMON TO SAID SECTIONS 6 AND 7; THENCE N 01 DEGREES 07'55" E. A DISTANCE OF 1253.02 FEET ALONG THE WEST LINE OF THE SE1/4SE1/4 OF SAID SECTION 6 TO THE SOUTHERLY RIGHT OF WAY LINE OF SAID DENVER RIO GRANDE WESTERN RAILROAD RIGHT OF WAY; THENCE ALONG SAID RIGHT OF WAY THE FOLLOWING (2) COURSES: THENCE N 86 DEGREES 20'21" E. A DISTANCE OF 58.27 FEET; THENCE N 83 DEGREES 38'11" E. A DISTANCE OF 480.07 FEET TO THE SOUTHERLY RIGHT OF WAY LINE OF SAID MIDLAND AVENUE RIGHT OF WAY ACQUISITION; THENCE ALONG SAID RIGHT OF WAY THE FOLLOWING (47) COURSES, THENCE S 44 DEGREES 35'36" E. A DISTANCE OF 72.53 FEET; THENCE S 53 DEGREES 05'31" E. A DISTANCE OF 232.27 FEET; THENCE ALONG THE ARC OF A CURVE TO THE LEFT A DISTANCE OF 580.98 FEET, HAVING A RADIUS OF 886.00 FEET, THE CHORD OF WHICH BEARS S 72 DEGREES 00'20" E. A DISTANCE OF 570.45 FEET; THENCE N 89 DEGREES 04'52" E. A DISTANCE OF 386.55 FEET; THENCE N 02 DEGREES 31'09" E. A DISTANCE OF 137.01 FEET; THENCE N 62 DEGREES 28'35" E. A DISTANCE OF 379.58 FEET; THENCE N 70 DEGREES 48'52" E. A DISTANCE OF 233.77 FEET; THENCE ALONG THE ARC OF A CURVE TO THE RIGHT A DISTANCE OF 138.27 FEET, HAVING A RADIUS OF 400.00 FEET, THE CHORD OF WHICH BEARS N 80 DEGREES 43'03" E. A DISTANCE OF 137.58 FEET; THENCE S 89 DEGREES 22'47" E. A DISTANCE OF 220.22 FEET; THENCE S 04 DEGREES 05'51" W. A DISTANCE OF 84.94 FEET; THENCE S 88 DEGREES 31'25" E. A DISTANCE OF 23.12 FEET; THENCE N 84 DEGREES 12'24" E. A DISTANCE OF 200.33 FEET; THENCE N 13 DEGREES 23'18" E. A DISTANCE OF 18.04 FEET; THENCE S 79 DEGREES 23'12" E. A DISTANCE OF 693.85 FEET; THENCE S 15 DEGREES 25'14" W. A DISTANCE OF 81.35 FEET; THENCE S 72 DEGREES 50'58" E. A DISTANCE OF 30.90 FEET; THENCE S 87 DEGREES 49'17" E. A DISTANCE OF 164.69 FEET; THENCE N 47 DEGREES 19'34" E. A DISTANCE OF 37.65 FEET; THENCE S 74 DEGREES 59'10" E. A DISTANCE OF 294.83 FEET; THENCE S 08 DEGREES 06'53" E. A DISTANCE OF 62.58 FEET; THENCE S 77 DEGREES 46'27" E. A DISTANCE OF 150.75 FEET; THENCE N 14 DEGREES 20'16" E. A DISTANCE OF 62.19 FEET; THENCE S 79 DEGREES 08'31" E. A DISTANCE OF 369.05 FEET; THENCE S 18 DEGREES 40'31" E. A DISTANCE OF 29.17 FEET; THENCE S 66 DEGREES 03'20" E. A DISTANCE OF 143.01 FEET; THENCE N 25 DEGREES 40'28" E. A DISTANCE OF 42.89 FEET; THENCE S 74 DEGREES 19'33" E. A DISTANCE OF 339.00 FEET; THENCE S 76 DEGREES 23'22" E. A DISTANCE OF 559.87 FEET; THENCE ALONG THE ARC OF A CURVE TO THE RIGHT A DISTANCE OF 1104.51 FEET, HAVING A RADIUS OF 1190.00 FEET, THE CHORD OF WHICH BEARS S 48 DEGREES 18'54" E. A DISTANCE OF 1065.29 FEET; THENCE S 21 DEGREES 43'32" E. A DISTANCE OF 179.06 FEET; THENCE S 68 DEGREES 16'28" W. A DISTANCE OF 31.36 FEET; THENCE S 23 DEGREES 26'58" E. A DISTANCE OF 78.06 FEET; THENCE N 68 DEGREES 16'28" E. A DISTANCE OF 29.01 FEET; THENCE S 21 DEGREES 43'32" E. A DISTANCE OF 764.09 FEET; THENCE ALONG THE ARC OF A CURVE TO THE LEFT A DISTANCE OF 272.23 FEET, HAVING A RADIUS OF 1165.00 FEET, THE CHORD OF WHICH BEARS S 28 DEGREES 25'10" E. A DISTANCE OF 271.61 FEET; THENCE S 35 DEGREES 06'49" E. A DISTANCE OF 21.42 FEET; THENCE S 54 DEGREES 53'11" W. A DISTANCE OF 22.69 FEET; THENCE S 41 DEGREES 40'06" E. A DISTANCE OF 43.30 FEET; THENCE N 54 DEGREES 53'11" E. A DISTANCE OF 17.75 FEET; THENCE S 30 DEGREES 38'49" E. A DISTANCE OF 271.37 FEET; THENCE S 41 DEGREES 04'38" E. A DISTANCE OF 291.31 FEET; THENCE S 23 DEGREES 28'47" E. A DISTANCE OF 143.44

6/21/2005

EXHIBIT A

FEET THENCE ALONG THE ARC OF A CURVE TO THE LEFT A DISTANCE OF 95.96 FEET, HAVING A RADIUS OF 100.00 FEET, THE CHORD OF WHICH BEARS S 50 DEGREES 58'14" E, A DISTANCE OF 92.32 FEET; THENCE S 78 DEGREES 27'41" E, A DISTANCE OF 27.55 FEET; THENCE ALONG THE ARC OF A CURVE TO THE RIGHT A DISTANCE OF 42.95 FEET, HAVING A RADIUS OF 200.00 FEET, THE CHORD OF WHICH BEARS S 72 DEGREES 18'32" E, A DISTANCE OF 42.87 FEET; THENCE S 50 DEGREES 12'25" E, A DISTANCE OF 131.37 FEET; THENCE S 43 DEGREES 26'37" E, A DISTANCE OF 220.95 FEET; THENCE S 54 DEGREES 38'36" E, A DISTANCE OF 373.46 FEET TO THE WESTERLY RIGHT OF WAY LINE OF SAID RED MOUNTAIN DRIVE RIGHT OF WAY; THENCE ALONG SAID RIGHT OF WAY THE FOLLOWING (2) COURSES: THENCE ALONG THE ARC OF A CURVE TO THE LEFT A DISTANCE OF 35.93 FEET, HAVING A RADIUS OF 124.00 FEET, THE CHORD OF WHICH BEARS S 32 DEGREES 55'05" W, A DISTANCE OF 35.80 FEET TO THE EAST LINE OF THE SW1/4NW1/4 OF SAID SECTION 9; THENCE ALONG SAID EAST LINE S 00 DEGREES 39'40" W, A DISTANCE OF 82.52 FEET TO THE CENTER WEST 1/16 CORNER OF SAID SECTION 9; THENCE LEAVING SAID RIGHT OF WAY AND ALONG THE EAST WEST CENTER LINE OF SAID SECTION 9 N 88 DEGREES 48'57" W, A DISTANCE OF 1318.50 FEET TO THE EAST 1/6 CORNER OF SAID SECTION 8; THENCE ALONG THE EAST WEST CENTER LINE OF SAID SECTION 8 N 89 DEGREES 15'16" W, A DISTANCE OF 2735.32 FEET TO THE CENTER 1/4 CORNER OF SAID SECTION 8; THENCE CONTINUING ALONG THE EAST WEST CENTER LINE OF SAID SECTION 8 N 89 DEGREES 15'16" W, A DISTANCE OF 1363.17 FEET TO THE CENTER WEST 1/16 CORNER OF SAID SECTION 8; THENCE ALONG THE WEST LINE OF THE SE1/4NW1/4 OF SAID SECTION 8 N 01 DEGREES 13'52" E, A DISTANCE OF 1360.08 FEET TO THE NW1/16 CORNER OF SAID SECTION 8; THENCE ALONG THE SOUTH LINE OF THE NW1/4NW1/4 OF SAID SECTION 8 N 88 DEGREES 45'05" W, A DISTANCE OF 1367.66 FEET TO THE NW1/16 CORNER COMMON TO SAID SECTIONS 7 AND 8; THENCE ALONG THE WEST LINE TO THE NW CORNER OF SAID SECTION 8 N 01 DEGREES 04'56" E, A DISTANCE OF 1369.98 FEET, TO THE POINT OF BEGINNING.

PARCEL # 2

A PARCEL OF LAND SITUATED IN SECTIONS 5 AND 6, TOWNSHIP 6 SOUTH RANGE 89 WEST OF THE SIXTH PRINCIPAL MERIDIAN, COUNTY OF GARFIELD, STATE OF COLORADO, SAID PARCEL BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGINNING AT A CONCRETE RIGHT OF WAY MONUMENT FOUND ON THE SOUTHERLY RIGHT OF WAY OF THE DENVER AND RIO GRANDE WESTERN RAILROAD WHENCE THE SLM ALUM. CAP IN PLACE AND PROPERLY MARKED FOR THE SOUTHWEST CORNER OF SAID SECTION 5 BEARS S 23 DEGREES 29'57" W 1196.71 FEET; THENCE ALONG SAID SOUTHERLY RIGHT OF WAY THE FOLLOWING (2) COURSES: THENCE S 83 DEGREES 38'11" W 901.40 FEET; THENCE N 82 DEGREES 19'49" W 183.30 FEET; THENCE S 83 DEGREES 38'11" W 7.05 FEET LEAVING SAID SOUTHERLY RIGHT OF WAY; THENCE ALONG THE NORTHERLY RIGHT OF WAY OF THE MIDLAND AVE. RIGHT OF WAY ACQUISITION THE FOLLOWING (8) COURSES: THENCE S 42 DEGREES 36'53" E 17.15 FEET; THENCE ALONG A CURVE TO THE LEFT HAVING A RADIUS OF 710.00 FEET AND AN ARC LENGTH OF 677.38 FEET, THE CHORD OF WHICH BEARS S 69 DEGREES 56'33" E 551.91 FEET; THENCE N 82 DEGREES 43'44" E 80.93 FEET; THENCE ALONG THE ARC OF A CURVE TO THE LEFT HAVING A RADIUS OF 500.00 FEET AND AN ARC LENGTH OF 194.98 FEET, THE CHORD OF WHICH BEARS N 71 DEGREES 32'27" E 193.75 FEET; THENCE N 50 DEGREES 23'10" E 285.42 FEET; THENCE N 56 DEGREES 38'55" E 230.74 FEET; THENCE N 75 DEGREES 17'22" E 201.74 FEET; THENCE N 06 DEGREES 21'45" W 19.87 FEET LEAVING SAID NORTHERLY RIGHT OF WAY; THENCE ALONG SAID SOUTHERLY RIGHT OF WAY THE FOLLOWING (3) COURSES: THENCE S 83 DEGREES 38'11" W 132.79 FEET; THENCE S 86 DEGREES 30'11" W 209.30 FEET; THENCE S 80 DEGREES 38'11" W 98.60 FEET TO THE POINT OF BEGINNING.

PARCEL # 3

A PARCEL OF LAND SITUATED SOUTHERLY OF THE DENVER & RIO GRANDE WESTERN RAILROAD (DARGO) RIGHT OF WAY AND NORTHERLY OF THE MIDLAND AVENUE ACQUISITION RIGHT OF WAY (MIDLAND ROW), LOCATED IN THE SW1/4NW1/4 OF SECTION 9, TOWNSHIP 6 SOUTH, RANGE 89 WEST OF THE SIXTH PRINCIPAL MERIDIAN, COUNTY OF GARFIELD, STATE OF COLORADO, THE PARCEL BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGINNING AT THE SOUTHEAST CORNER OF SAID PARCEL WHENCE THE BRASS CAP FOUND AT THE CENTER WEST 1/16 CORNER OF SECTION 9 BEARS S 00 DEGREES 39'40" W 281.52 FEET; THENCE N 00 DEGREES 39'40" E 418.89 FEET; ALONG THE EASTERLY BOUNDARY OF SAID PARCEL ALSO BEING THE EASTERLY LINE OF THE SW1/4NW1/4 OF SAID SECTION 9 TO THE SOUTHERLY LINE OF SAID DARGO RIGHT OF WAY; THENCE NORTHWESTERLY ALONG THE SAID RIGHT OF WAY THE FOLLOWING (4) COURSES: N 70 DEGREES 07'49" W 195.45 FEET; THENCE N 68 DEGREES 27'19" W 235.00 FEET; THENCE S 24 DEGREES 32'11" W 25.00

6230043

EXHIBIT A

FEET; THENCE ALONG A CURVE TO THE RIGHT HAVING A RADIUS OF 1507.40 FEET AND AN ARC DISTANCE OF 551.12 FEET, THE CHORD OF WHICH BEARS N 54 DEGREES 59'25" W 548.06 FEET; THENCE S 54 DEGREES 50'12" W 20.12 FEET ALONG THE WESTERLY BOUNDARY OF SAID PARCEL ALSO BEING THE NORTHERLY BOUNDARY OF THE MIDLAND ROW; THENCE SOUTHEASTERLY ALONG SAID NORTHERLY BOUNDARY THE FOLLOWING (10) COURSES; S 35 DEGREES 05'32" E 163.12 FEET; THENCE ALONG A CURVE TO THE LEFT HAVING A RADIUS OF 950.00 FEET AND AN ARC DISTANCE OF 250.42 FEET, THE CHORD OF WHICH BEARS S 42 DEGREES 39'59" E 249.83 FEET; THENCE S 50 DEGREES 13'26" E 91.26 FEET; THENCE S 39 DEGREES 45'34" W 10.00 FEET; THENCE S 50 DEGREES 13'26" E 18.00 FEET; THENCE N 39 DEGREES 46'34" E 10.00 FEET; THENCE S 50 DEGREES 13'26" E 259.08 FEET; THENCE ALONG A CURVE TO THE RIGHT HAVING A RADIUS OF 1050.00 FEET AND AN ARC DISTANCE OF 133.74 FEET, THE CHORD OF WHICH BEARS S 46 DEGREES 34'29" E 133.65 FEET; THENCE S 42 DEGREES 55'33" E 112.05 FEET; THENCE ALONG A CURVE TO THE LEFT HAVING A RADIUS OF 715.00 FEET AND AN ARC DISTANCE OF 138.66 FEET, THE CHORD OF WHICH BEARS S 50 DEGREES 53'08" E 198.02 FEET TO THE POINT OF BEGINNING.

EXCEPTING THEREFROM:

A PARCEL OF LAND SITUATED IN THE SOUTHWEST QUARTER OF THE NORTHWEST QUARTER OF SECTION 9, TOWNSHIP 6 SOUTH, RANGE 89 WEST OF THE SIXTH PRINCIPAL MERIDIAN, COUNTY OF GARFIELD, STATE OF COLORADO SAID PARCEL OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS

COMMENCING AT THE STREET MONUMENT AT THE INTERSECTION OF 8TH STREET AND MIDLAND AVENUE;

THENCE N 51 DEGREES 15'40" W. 681.29 FEET TO THE WEST CENTER SIXTEENTH CORNER OF SAID SECTION 9, THE TRUE POINT OF BEGINNING

THENCE N 18 DEGREES 48'57" W ALONG THE SOUTHERLY LINE OF SAID SOUTHWEST QUARTER OF THE NORTHWEST QUARTER 361.88 FEET

THENCE LEAVING SAID SOUTHERLY LINE W 33 DEGREES 57'41" W 159.48 FEET

THENCE N 50 DEGREES 07'49" W 109.72 FEET

THENCE N 19 DEGREES 54'08" W 208.92 FEET

THENCE N 12 DEGREES 15'22" W 109.78 FEET

THENCE N 17 DEGREES 34'08" E 81.85 FEET TO A POINT IN THE SOUTHERLY RIGHT OF WAY OF MIDLAND AVENUE;

THENCE THE FIVE (5) FOLLOWING COURSES ALONG SAID SOUTHERLY RIGHT OF WAY

1. S 78 DEGREES 26'55" E 27.55 FEET

2. ALONG THE ARC OF A CURVE TO THE RIGHT HAVING A RADIUS OF 200.00 FEET, A CENTRAL ANGLE 12 DEGREES 18'18" AND A DISTANCE OF 42.95 FEET (CHORD BEARS S 72 DEGREES 17'46" E 42.67 FEET)

3. S 50 DEGREES 12'40" E 131.37 FEET

4. S 43 DEGREES 25'51" E 229.36 FEET

5. S 54 DEGREES 37'50" E 373.48 FEET

THENCE ALONG THE ARC OF A NON-TANGENT CURVE TO THE LEFT HAVING A RADIUS OF 114.00 FEET, A CENTRAL ANGLE OF 16 DEGREES 35'59" AND A DISTANCE OF 35.93 FEET (CHORD BEARS S 32 DEGREES 55'06" W. 35.80 FEET) TO A POINT ON THE EASTERLY LINE OF SAID SOUTHWEST QUARTER OF THE NORTHWEST QUARTER

THENCE S 00 DEGREES 39'40" W ALONG SAID EASTERLY LINE 82.52 FEET TO THE TRUE POINT OF BEGINNING.

COUNTY OF GARFIELD
STATE OF COLORADO

Uncompahgre Engineering, LLC
P.O. Box 3945
Telluride, CO 81435
dballode@msn.com (970) 729-0683

September 21, 2020

To: Chris Hawkins
Alpine Planning LLC

RE: Kind Castle Development, Lot 11B, Filing 5, Glenwood Meadows, Glenwood Springs, CO

Dear Chris:

The proposed Kind Castle lot is within the master-planned Glenwood Meadows development. That development contemplated various areas that were to be developed and their master drainage plan had to assume that all of those lots were at ultimate build-out conditions. That's done so that individual lot owners and developers aren't required to upsize the storm drain system every time a new impervious area is built. It was already contemplated and accounted for.

This lot was part of the Glenwood Meadows development and, therefore, the run-off at its ultimate build-out condition was accounted for in the sizing of that storm drain system. The hydrologic and hydraulic details of the interior of the lot are not final, but the general drainage plan is as follows:

The final site plan will show a large landscaped buffer on all sides. This landscaping will have a fairly low run-off coefficient. Roof gutters and downspouts will be incorporated into the plan so that any concentrated water off of the new impervious building area can be intercepted and dissipated. The northern sidewalk will slope to a greenbelt along Wulfsohn Road. The run-off from the eastern sidewalk and parking lot will be directed to a curb inlet (a large capacity inlet) which will capture the required storm. That inlet will tie in to the existing master storm system via an additional storm junction box near the intersection of Wulfsohn and Flat Top.

Although the hydrology and hydraulic final calculations are not in their final form, this site is so small that the stormwater can easily be intercepted or mitigated. The eastern sidewalk and parking lot is only 0.13 acres. Using the Rational Method, a 25-year storm will generate approx. 0.5 cfs. I'm showing that amount draining to a 12" culvert with a capacity of 2.4 cfs. The roof will be mitigated in the landscaping.

I understand that the final design will require that the city stormwater standards must be followed and a full analysis must be completed. But this site generates so little water that I'd like to defer the full analysis until final landscaping and building details are available.

Please let me know if you need any additional information or clarification.

Sincerely,



David Ballode, P.E.
Uncompahgre Engineering, LLC





Planning & Zoning Commission Staff Report

Date: October 27, 2020

To: Planning & Zoning Commission

From: Gretchen Ricehill, Asst. Director Community Development Dept.

Subject: #54-20- Location & Extent Review – Colorado Department of Transportation – 202 Centennial Street

REQUEST	Consideration of a Location & Extent application for the redevelopment of the existing Colorado Department of Transportation maintenance facility.
APPLICANT	Colorado Department of Transportation
OWNER	Colorado Department of Transportation
ZONE	M1-Mixed Use Corridor
LOCATION:	202 Centennial Street
SURROUNDING LAND USES	North: Interstate ROW; Commercial South: Office; Industrial East: Park West: Resort/ hot springs
LOT SIZE	This property is currently part of the Interstate 70 right of way

ACTION ITEM SUMMARY

Action 1: Consideration of a Location & Extent Review application for the redevelopment of the existing Colorado Department of Transportation maintenance facility including the construction of an 11,000 square foot vehicle storage and office building; a 2,100 square foot communication building with 60-foot tall tower; and relocation of existing fuel and magnesium chloride containment structures.

Staff Recommendation: Staff recommends that the Planning and Zoning Commission approve the Location & Extent Review application with findings and conditions on page 7 of this report.

PROJECT SUMMARY

This Location and Extent application considers improvements to an existing, CDOT-owned maintenance facility located at 202 Centennial Street, immediately adjacent to Interstate 70 and bordering Devereux Road. Currently improvements on the site include two maintenance/office buildings, a communication building, and storage structures. CDOT and the Iron Mountain Hot Springs (IMHS) have negotiated a land trade whereby approximately 3.35 acres of land will be transferred to IMHS in exchange for approximately 9,000 sf of condominiumized office space on the ground floor of the new Caverns Village building, currently under construction. The Caverns Village development and the minor subdivision allowing the subsequent land trade were approved by the Commission and Council earlier this year.



After the land trade, CDOT's site will be a rectangular-shaped parcel of about 3.1 acres. This and the expired life span of the existing building necessitates demolition of the existing communication and maintenance buildings, and consolidation and reorganization of operations onto a smaller site. To that end, CDOT proposes construction of a new 11,000 square foot maintenance/office/vehicle storage building, and a new 2,100 square foot communication building with 60-foot tower. It will also move the existing fuel storage and magnesium chloride tanks and install a gate at its access driveway to improve security.

Below are current photos of the property.



View from Devereux Road, looking west/northwest. The tan building closest to the camera is scheduled for demolition.



Looking north, across parking lot currently owned by CDOT but to be transferred to Iron Mountain Hot Springs per the land transfer agreement.



Looking south/southeast from Interstate 70. From right to left: Current communication building and tower (scheduled for demolition); tanks (to be relocated); existing sand storage building (to remain).

Review Process & Approval Criteria:

The Planning and Zoning Commission considers Location and Extent Review applications for all public and quasi-public special districts based on whether the application is consistent with applicable Comprehensive Plan goals and policies, consistency with the general purpose and intent of the Municipal Code and how well the application minimizes adverse impacts to health, safety and welfare of inhabitants of surrounding areas and the city.

i. Consistency with the Comprehensive Plan and other City plans and policies.

The Comprehensive plan provides a vision for the development and redevelopment of parcels within the city. The following are statements supportive of the redevelopment of the CDOT maintenance facility site:

- *Direct Growth to Areas Where Utilities and Services Can be Provided Efficiently*

This vision statement encourages directing growth inward through infill and redevelopment. The CDOT proposal redevelops its existing maintenance facility, consolidating and organizing various uses onto a smaller and more efficiently utilized parcel.

- *Direct Development to Locations and Forms that are Cost-effective to Serve*
- *Direct Growth to Areas Where Utilities and Services Can be Provided Efficiently*
- *Address Transportation Needs and Provide Multiple Convenient Travel Choices*
- *Values and Vision for Public Utilities and Services*

This site provides CDOT and the State patrol with convenient, immediate access onto Interstate 70 where the departments can quickly respond to emergencies and on-going transportation maintenance needs. The infrastructure and capacities of the site currently meet the needs of both state departments. The proposed changes fit within the site acreage and do not affect the utility capacities within the site.

The Comprehensive Plan's Future Land Use map identifies this site as "commercial" which includes a wide range of general retail and services for both regional and local markets. As noted above, this site includes a mix of offices, vehicle maintenance, communication, and storage. The property is zoned M1-Mixed Use Corridor which supports a wide range of uses including government offices, contractor offices & equipment storage yard; warehousing; automotive repair. Fuel storage and automobile wash facilities are allowed by special review.

ii. Is consistent with the general purpose and intent of this Code.

The proposed improvements comply with Title 070 with the following exceptions:

Allowed Uses

The proposed uses include fuel storage and automobile wash bay, and a 60-foot tall communication tower which exceeds the maximum 40-foot height. These uses would normally require a special review however, as a government entity CDOT is submitting this application as a Location and Extent review. As noted above, the purpose of a Location and Extent Review is to allow review of public and quasi-public entities in relation to the applicable policies and goals of the Comprehensive Plan, the purpose & intent of the Code, and minimization of adverse impacts. The fuel storage and automobile wash are current uses on this property and are considered necessary to support CDOT's maintenance of the highway system. The added 20-foot height of the communication tower, CDOT indicates, is necessary for the radio waves to better clear existing trees to the south and west and to maintain a proper line of sight with other microwave towers on Sunlight Mountain.

Impacts of these uses on adjacent properties are minimized due to the property's relative remoteness from commercial and residential uses and the fact that it sits several feet lower than surrounding roadways and most properties in the vicinity. Additionally, as shown in the photo at right, the communication tower's open, tapered design helps minimize its visual impact.



Common Open Space and Landscaping

Ten percent of the lot area, or about 13,500 square feet, is required to be common open space. Areas counted as open space include formal plantings and gardens; squares, courtyards, plazas; and required buffer and landscape areas. Similarly, in the M1 zoning district fifteen percent of the lot area (20,250 sf) is required to consist of landscaping. The project proposes to maintain existing and native vegetation and screening along the I-70 and Devereux Road frontages. CDOT estimated that this existing vegetated area and newly landscaped areas amount to approximately 11 percent (14,800 sf) of the site.

The Code also requires that a development of this size plant 50 trees, 200 shrubs and 405 perennials. As shown in the photographs above, there are several existing trees and shrubs along the Devereux Road

and Interstate 70 frontages. CDOT proposes maintaining the existing trees and shrubs and does not propose planting additional vegetation as the site is not irrigated.

Finally, CDOT proposes installing new chain link fencing along the south/southeast and west/northwest property lines. The Code allows chain link if vinyl coated in green, black, or brown and when used in conjunction with tightly spaced plants. The adjacent Caverns Village development's approved landscape plan calls for plantings between its parking spaces and CDOT's fencing including trees in each landscape island, and red twig dogwood and potentilla shrubs.



Exterior & Security Lighting

Although building elevation plans depict exterior lights, no exterior lighting information was submitted with this application. Staff recommends a condition of approval requiring that all new exterior lighting conform to 070.040.100 such that wall lighting is fully shielded and parking lot lighting fixtures are full cut off.

Non-residential Design Standards

CDOT's proposed vehicle storage and office and communications buildings are pre-engineered metal structures both with standing seam metal roofs. The city's non-residential design standards, section 070.040.090 requires certain architectural treatments when building elevations exceed 30 feet in length to visually break up continuous wall planes. Acceptable design treatments include building articulation (24-inch minimum wall plane difference); window/door glazing, change in material texture; awnings/canopies; and change in building height. The primary issue is the north 227-foot long elevation adjacent to Interstate 70. Along with the metal wall covering, this elevation includes a short 3-foot tall concrete foundation wall. Staff recommends that CDOT add another element along this publicly visible elevation to better meet the intent of the design standards. CDOT reported that it is considering adding pilasters.

The communication building also exceeds 30 feet without the required design treatments. The building footprint is about 40' x 50'. All but the east elevation has architectural treatments that help to break up the wall plane. However, this east elevation which faces Devereux Road, will be fairly-well screened from view as shown in the photo below. Additionally, there is no public sidewalk along this side of the street. Plans (not related to this development) call for the sidewalk to be on the south (Two Rivers Park) side. So, this communication building will only briefly be visible to passing vehicular traffic. For these reasons, staff does not recommend adding architectural treatments on this east elevation.



In conclusion, although the proposal is not in strict compliance with all aspects of the Development Code as noted above, the project is found to be consistent with the Code's general purpose and intent provisions namely, it is supported by many of the goals as stated in the Comprehensive Plan and as will be noted below, the use protects the public health, safety, and welfare of the city and region.

iii. Minimizes adverse impacts to the health, safety, and welfare of the inhabitants of the surrounding areas and the city.

The proposed project does not negatively impact surrounding properties or the health, safety and welfare of the city's inhabitants. The use as a highway/interstate maintenance facility serves to protect the public health, safety and welfare of the city and region. Further, CDOT's development along with that of the adjacent Caverns Village, improves and organizes access to and from all properties along Centennial Drive. Traffic to both sites will be along a specifically defined, organized private roadway with proper curb and gutter, and sidewalk. Currently this access is unclear, undefined and a hazard to pedestrians.

REVIEWING AGENCY COMMENTS

Comments were received from the City's Public Works, Engineering, Electric, and Community Development departments and are attached for your review. CDOT is currently working with the departments to revise plans as required.

REQUIRED ACTIONS

Action 1: Consideration of a Location & Extent Review application for construction of an 11,000 square foot vehicle storage and office building; a 2,100 square foot communication building with 60-foot tall tower; and relocation of existing fuel and magnesium chloride containment structures.

Action Alternatives

The Planning and Zoning Commission may approve, approve with conditions, deny or continue this application to a future hearing with a request for additional specific information which it finds necessary to determine compliance with the Municipal Code and city goals and policies.

Staff Recommendation: Staff recommends that the Planning and Zoning Commission approve the Location & Extent Review application with suggested findings and conditions on page 7 of this report.

Suggested Findings:

1. The proposal is consistent with the Comprehensive Plan and other City plans and policies;
2. The proposal is consistent with the general purpose and intent of this Code; and
3. The proposal minimizes adverse impacts to the health, safety, and welfare of the inhabitants of the surrounding areas and the city.

Suggested Conditions:

1. The owner shall comply with all verbal representations made in the public hearings, as outlined in all application materials, and/or as revised per conditions herein and approved by the Community Development Director.
2. Prior to construction, applicant/owner shall submit building and/or grading permit, as required.
3. The owner shall revise architectural plans of the office and vehicle storage (identified on the plans as VCF) to include articulations such as pilasters along the north elevation to break up the continuous wall plane.
4. The owner shall provide exterior building color samples for Community Development Director review and approval at the time of building and/or grading permit submission. The proposed colors shall be earth-tone and the metal roof shall have a matte finish.
5. The owner shall provide lighting manufacturer information for all exterior lighting for Community Development Director review and approval. Lighting shall be reviewed and approved by the Community Development Director prior to installation. Building wall lighting shall be shielded and parking lot lighting fixtures shall be full cut off.
6. The owner shall revise plans to specify that the chain link fence is vinyl coated in either black, brown or green.
7. Any future physical changes to or improvements on the property are subject to the review and approval of the Community Development Director for compliance with applicable requirements of the Glenwood Springs Municipal Code.



LAND USE APPLICATION

City of Glenwood Springs
Economic and Community Development Dept.
101 West 8th Street
Glenwood Springs, CO 81601
970-384-6411

Ordinance Amendments		Development Permits	
Rezoning	☐	Site/Architectural Plan Review	
Rezoning to Planned Unit Development (PUD)	☐	Administrative	☐
Annexation	☐	Minor	☐
Condominiumization	☐	Major	☐
Annexation	☐	Master Plan	☐
Street Vacation	☐	Construction Plans	☐
	☐	Location & Extent	☒
Subdivisions	☐	Right of Way Encroachment License	☐
Minor Subdivision	☐	Floodplain Development Permit	☐
Preliminary Plat	☐		☐
Final Plat	☐	Flexibility and Relief Procedures	☐
Vacation of Right-of-Way	☐	Variance	☐
	☐	Administrative Adjustment	☐
	☐	Appeal	☐

Applicant/Property Owner Information:			
Applicant		Owner	
Erin Lucero, Architect		Colorado Department of Transportation	
Relationship to Owner		Owner	
Employee		Colorado Department of Transportation	
Applicant Address		Owner Address	
2829 W. Howard Place, Denver, CO 80204		2829 W. Howard Place, Denver, CO 80204	
Phone 720.737.6591	Email Address erin.lucero@state.co.us	Phone 720.737.6591	Email Address erin.lucero@state.co.us

Property Information:	
Address	Parcel Number
202 Centennial St., Glenwood Springs, CO 81601	
Subdivision	Lot and Block Number
	Situated in lots 6 & 7 of Section 9, Township 6 South, Range 89 West, 6th Principal Meridian, Glenwood Springs
Zone District Institutional	Existing Land Use CDOT Maintenance Yard

General Project Description:
Please see attached Project Description.

Signatures:		
The owner or applicant must be present at the hearing. All public hearings must be properly noticed according to G.S.M.C. Section 070.010.030. Signatures of all owners of the property must appear before the application is accepted. Partnerships or corporations may have the authorized general partner or corporate officer sign the application. (Attach additional incorporation documents if necessary.) I declare under the penalty of perjury that the above information is true and correct to the best of my knowledge.		
	Applicant	Date
	Owner	Date
	Owner	Date



COLORADO
Department of Transportation
Office of the Chief Engineer

2829 W. Howard Place
Denver, CO 80204-2305

TO: Gretchen Ricehill, Assistant Economic/Community Development Director, City of Glenwood Springs

FROM: Rob Levinson, Architect I, CDOT

DATE: August 11, 2020

RE: Project Statement for CDOT Centennial St Maintenance Site

The Colorado Department of Transportation (CDOT) has been making it easier for Coloradans and visitors to get around Glenwood Springs since at least 1937, when CDOT paved the first 4-mile section of Highway 82 from Glenwood Springs to Aspen. In order to continue to serve the community of Glenwood Springs, CDOT is planning to replace two of its local maintenance buildings that have reached the end of their life-cycles.

CDOT, as a State entity, is governed by State Statute for capital construction under the State Buildings Program. Under this specific statutory authority, local building permits are not required for state-owned properties; all existing codes, however, must be met. CDOT submits every set of construction plans to an independent code consultant for code compliance. The State Plumbing and Electrical inspectors conduct their respective inspections and final sign offs along with final sign off from the independent code consultant. Construction plans are also reviewed by the local fire marshal and the facilities are subsequently inspected upon completion. Additionally, CDOT makes every attempt to comply with all local zoning and planning requirements, and is submitting this Development Plan to the City of Glenwood Springs as part of the planning requirements identified in the pre-application process.

Site / Zoning

The proposed project is located at the existing CDOT maintenance site at 202 Centennial Street in Glenwood Springs, Colorado. The site currently houses a small communications building, a sand shed, two vehicle storage and maintenance buildings, a magnesium-chloride containment structure, a fuel tank and several storage sheds.

The existing 1,500 SF communications building and a 7,000 SF vehicle storage and maintenance building are proposed to be demolished to make way for the new buildings, and as part of a negotiated land trade, which will transfer the southern 3.35 acres (approximately) of CDOT property to Iron Mountain Hot Springs, LLC, the neighboring land-owner to the west and south. The existing fuel tank and magnesium-chloride containment structure will remain on the site, but are proposed to move north, in order to remain on the CDOT property after the land trade is complete.

Following the land trade, CDOT will be left with a long and narrow 3.1 acre site, bordered on the north by I-70, on the east by Deveraux Road, and by private property on the south and west sides. The total property area is 143,086 SF, with 8,237 of the western-most portion already being developed under separate permit by Iron Mountain Hot Springs, LLC, leaving CDOT with a developable area of 132,837 SF. All access to the site will be via a gated entrance from Centennial Street through access easements currently being negotiated with Iron Mountain Hot Springs, LLC.

The site is currently zoned as M1 Mixed-Use Corridor. The function of this site is to be a maintenance and service facility which is a permitted use by right in M-1 zoning.

CDOT has completed a NEPA determination on the site to identify any known wetlands, hazmat, sensitive species, paleontological, archeological or historical concerns that could impact development of the site. No such items were identified at this location.

As part of the development plan, CDOT is submitting a drainage report for review. CDOT will go through the CDPHE permit process for storm water management during construction.

Building Design

CDOT is proposing to replace the existing 7,000 SF maintenance building with an 11,000 sf 10-bay Vehicle Storage Facility (VSF) to store and wash trucks and provide associated office space for 9 staff. The Vehicle Storage Facility will also include one bay dedicated to the Colorado State Patrol for vehicle storage and evidence storage. CDOT is also proposing to replace the existing 1,500 SF Communications Building with a new 2,100 SF Communications Building and 60-foot tall microwave communications tower. (The existing 35-foot tall tower at the northwest corner of the property will be demolished.) The Communications Building serves as a critical link in both CDOT's and CSP's communications networks linking the western slope of Colorado with the eastern slope via the Eisenhower and Johnson Tunnels. Utilizing redundant systems of microwave-relay and fiber optics, the system helps keep I-70 open year-round, and a viable component of the U.S. interstate highway system. The Tower height has been determined by State OIT staff as the required height to maintain line of sight with the other microwave on Sunlight Mountain. The Vehicle Storage Facility and Communications Building shall be pre-engineered metal building construction with a single slope standing-seam metal shed roof. The maximum building height shall be 28 feet tall.

Utilities

The buildings are not proposed to be sprinklered, as CDOT plans to keep the building fire areas smaller than the IFC code requirements for storage of non-commercial vehicles. CDOT, in coordination with the Glenwood Springs Fire Department, proposes one new fire hydrant on the site with a new dead-end water line service to meet the flow requirements of the site. CDOT is proposing to connect to City of Glenwood Springs utilities for power, water and sewer at the site. CDOT is also proposing to connect to Black Hills Energy for natural gas service.

Landscape / Site Design

Through preliminary discussions with Glenwood Springs (Ms. Ricehill) CDOT understands that there are no landscaping requirements for the site based on its use as a heavy equipment storage yard and its inherent inhospitable nature to landscaping. However, CDOT does propose to add some landscaping area to the site, as well as maintaining the existing landscaping primarily the north edge of the site abutting I-70. The total percentage of the developable area to be landscaped with vegetation is approximately 11% (15,000 SF), with an additional 20% (27,000 SF) of pervious (gravel) surface .

If you have any questions, please do not hesitate to call me at 720-550-0033.
Thank you.

Rob Levinson

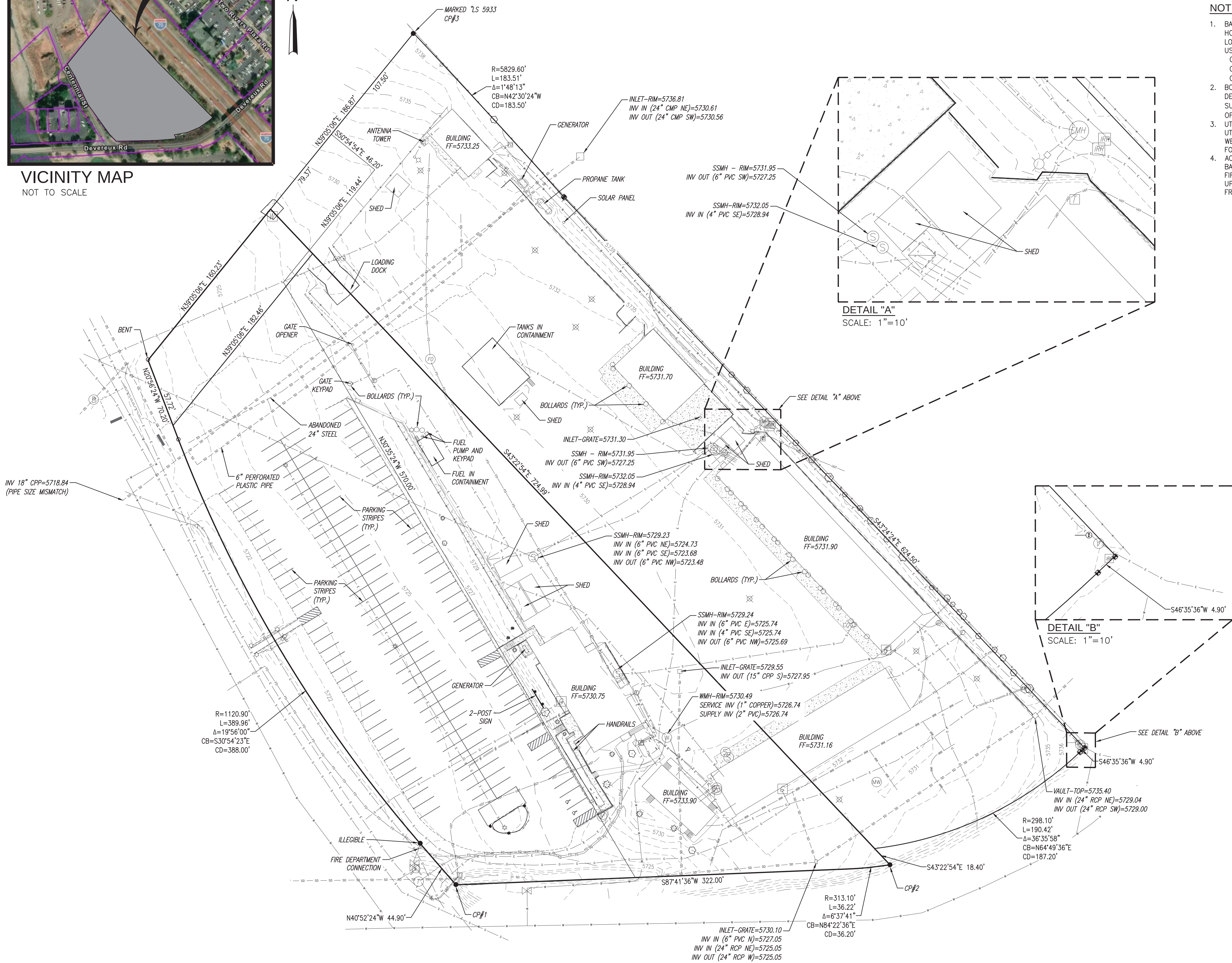
Architect I
Property Management
P: 720-550-0033
E: robert.levinson@state.co.us



VICINITY MAP
NOT TO SCALE

TOPOGRAPHIC SURVEY

SITUATED IN LOTS 6 & 7 OF SECTION 9,
TOWNSHIP 6 SOUTH, RANGE 89 WEST, 6TH PRINCIPAL MERIDIAN,
GLENWOOD SPRINGS, GARFIELD COUNTY, COLORADO



NOTES:

1. BASIS OF BEARING AND COORDINATE SYSTEM: CITY OF GLENWOOD SPRINGS HORIZONTAL AND VERTICAL DATUM TAKEN FROM "CAVERNS VILLAGE" PROJECT LOCATED TO THE NORTHWEST OF THE SUBJECT PROPERTY. CONTROL POINTS USED FOR THIS SURVEY ARE AS FOLLOWS:
CP#1 N:42112.15 E:16020.49 EL:5727.54
CP#2 N:42128.56 E:16378.22 EL:5734.44
CP#3 N:42813.55 E:15985.33 EL:5737.91
2. BOUNDARY OF THE SUBJECT PARCELS AND RIGHT-OF-WAY ARE AS DETERMINED IN "GLENWOOD MAINTENANCE SITE - BOUNDARY AND ROW SURVEY" DEPOSITED IN THE GARFIELD COUNTY RECORDER AND CLERKS OFFICE AS NUMBER 720 DATED 8/15/2006.
3. UTILITY LOCATIONS WERE PROVIDED BY CDOT AS PART OF A SUBSURFACE UTILITY ENGINEERING INVESTIGATION CONDUCTED IN JANUARY 2020. UTILITIES WERE VERIFIED AS POSSIBLE DURING THIS SURVEY. SEE S.U.E. INVESTIGATION FOR ADDITIONAL INFORMATION.
4. ACCORDING TO COLORADO LAW, YOU MUST COMMENCE ANY LEGAL ACTION BASED UPON ANY DEFECT IN THIS SURVEY WITHIN THREE YEARS AFTER YOU FIRST DISCOVERED SUCH DEFECT. IN NO EVENT MAY ANY ACTION BASED UPON ANY DEFECT IN THIS SURVEY BE COMMENCED MORE THAN TEN YEARS FROM THE DATE OF THE CERTIFICATION SHOWN HEREON.

LEGEND

	PROPERTY BOUNDARY		RIGHT-OF-WAY LINE
	99 YR. LEASE LINE		EXISTING EDGE OF ASPHALT
	EXISTING EDGE OF CONCRETE		EXISTING STORM DRAIN LINE/PIPE
	EXISTING SANITARY SEWER LINE		EXISTING FIBER OPTIC LINE
	EXISTING UNDERGROUND POWER		EXISTING GAS LINE
	EXISTING WATER LINE		EXISTING FENCE
	EXISTING RETAINING WALL/STRUCTURE		EXISTING SURFACE MAJOR CONTOUR
	EXISTING SURFACE MINOR CONTOUR		SANITARY SEWER MANHOLE
	WATER MANHOLE		CLEAN OUT
	ELECTRIC METER		ELECTRIC BOX
	POWER POLE		LIGHTS/LIGHT POLES
	TRANSFORMER		ELECTRICAL MANHOLE
	TELEPHONE PEDESTAL/BOX		TELEPHONE MANHOLE
	FIBER OPTIC MANHOLE		GAS METER
	WATER VALVE		FIRE HYDRANT
	WATER METER		IRRIGATION BOX
	IRRIGATION RISER		IRRIGATION VALVE
	BORE HOLE		MONITOR WELL
	BOLLARD		SIGNS AND DELINEATORS
	TREES		

◆ FOUND 3" ALUMINUM CDOT CAP
● FOUND 1.5" ALUMINUM CAP MARKED "LS 31551" OR AS NOTED
○ FOUND #5 BAR WITH NO CAP

Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

TOPOGRAPHIC SURVEY

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

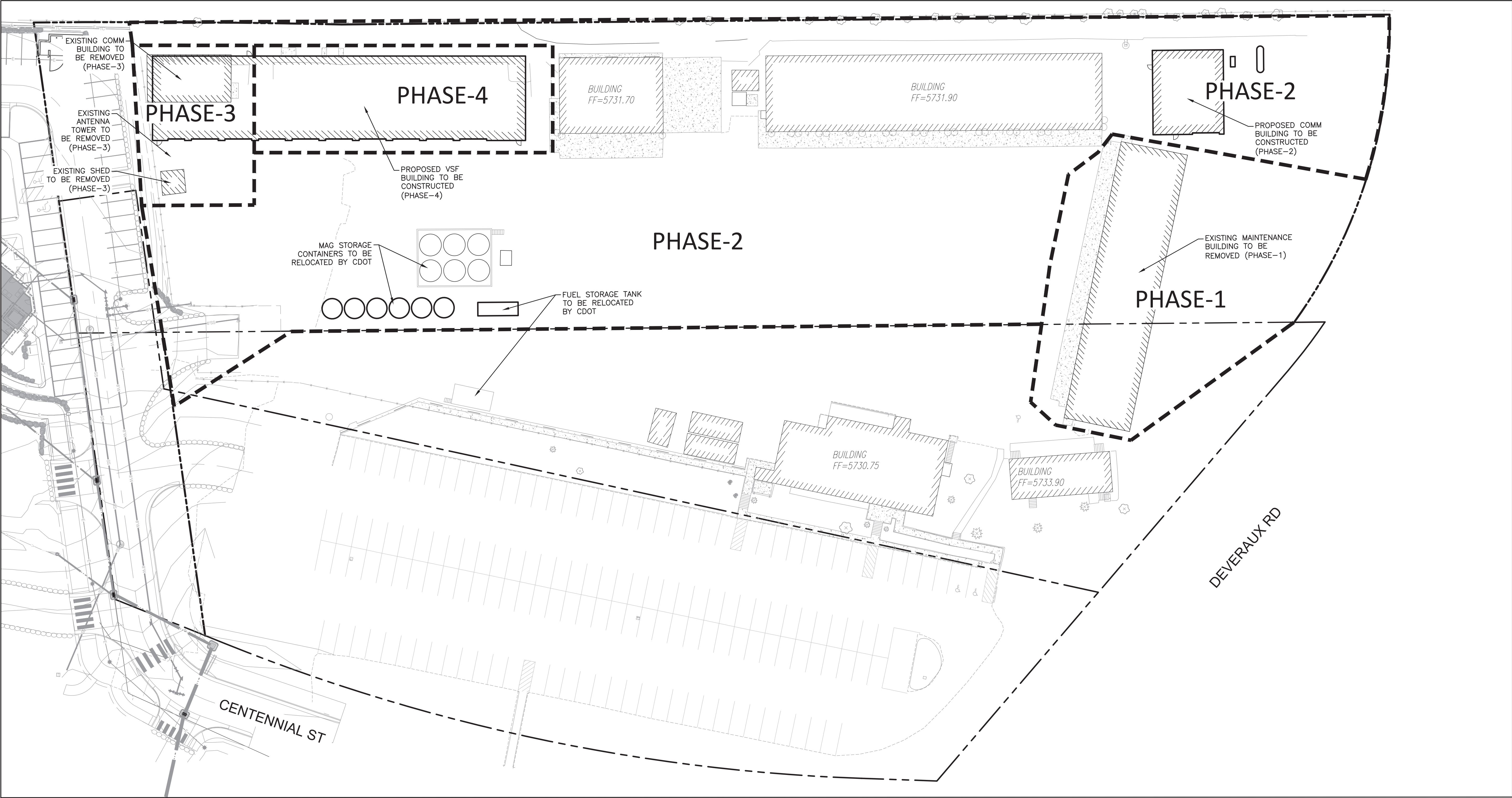
VORTEX
ENGINEERING, INC.
• CONSTRUCTION MANAGERS & SITE PLANNERS
• PROJECT MANAGERS
• CIVIL & CONSULTING ENGINEERS
861 Rood Avenue
Grand Junction, CO 81501
Phone: (970) 245-9051
Fax: (970) 245-7639

DRAWN BY: AL

CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

DRAWING NUMBER

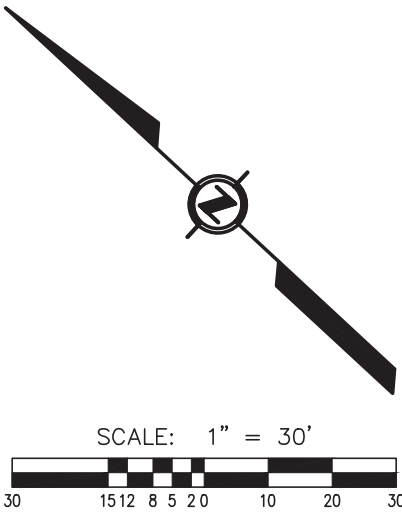
C0.1



NOTES:

- 1. COORDINATION WITH CDOT PERSONNEL WILL BE MAINTAINED THROUGHOUT THE PROJECT.
- 2. ALL EXISTING BUILDINGS TO REMAIN OPERATIONAL DURING THE PROJECT. SITE ACCESS TO BE MAINTAINED.
- 3. PROPOSED COMM BUILDING TO BE COMPLETELY OPERATIONAL AND CONNECTED WITH THE REST OF THE BUILDINGS ON SITE PRIOR TO THE REMOVAL OF THE EXISTING COMM BUILDING.
- 4. COORDINATE RELOCATION OF TANKS AND GENERATOR WITH CDOT.
- 5. ALL UTILITIES WILL BE INSTALLED PRIOR TO THE START OF PHASE -4.

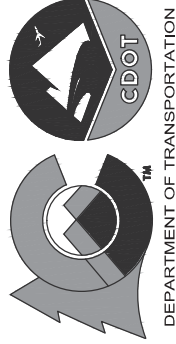
EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.



DATE	NO.	DESCRIPTION
08/12/2020	1	65% CONSTRUCTION DOCS
08/09/2020	2	85% CONSTRUCTION DOCS

Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550


DEPARTMENT OF TRANSPORTATION

PHASING PLAN

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

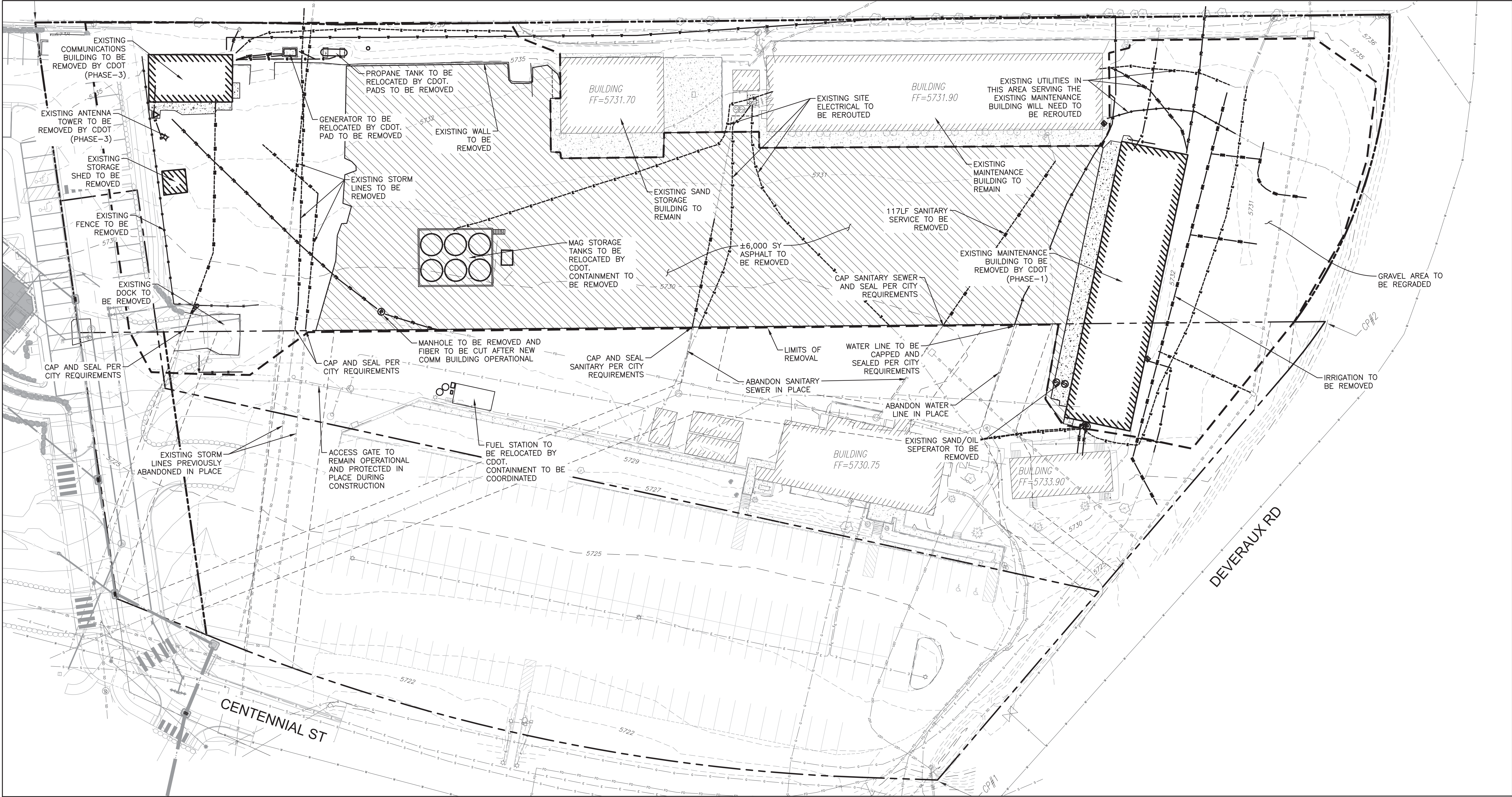
COLLINS ENGINEERS

455 Sherman Street, Suite 100
Denver, Colorado 80203
voice: (303) 447-0300
fax: (303) 447-9141
www.collinsengr.com

DRAWN BY: JLS

CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

DRAWING NUMBER
C0.2



EXISTING TO REMAIN

PROPERTY LINE

LOT LINE

EASEMENT

REMOVAL LIMITS

BUILDINGS

CONTOURS

CONCRETE

ASPHALT

STORM SEWER

SANITARY SEWER

WATER

IRRIGATION

ELECTRICAL

FIBER/COM

TO BE REMOVED

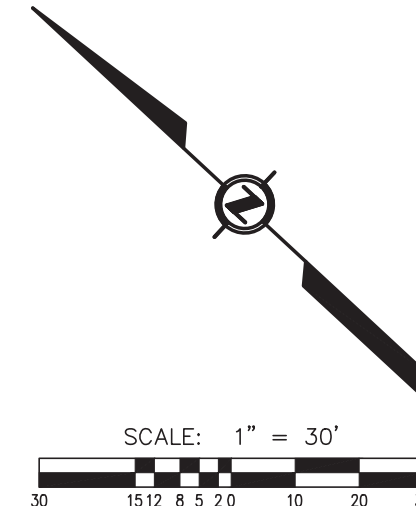
LEGEND

NOTES:

UTILITIES TO BE ABANDONED IN PLACE WILL BE APPROVED BY THE CITY.

CITY OF GLENWOOD SPRINGS IS TO DISCONNECT ALL WATER AND SANITARY MAINS AND SERVICES UNLESS CONTRACTOR IS PRE-APPROVED BY THE CITY.

EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.



Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

DEMOLITION PLAN

Glenwood Springs VSF
202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

COLLINS ENGINEERS

455 Sherman Street, Suite 100
Denver, Colorado 80203
Phone: (303) 447-0090
Fax: (303) 447-9141
www.collinsengr.com

DATE

NO.

DESCRIPTION

1

65% CONSTRUCTION DOCS

08/12/2020

2

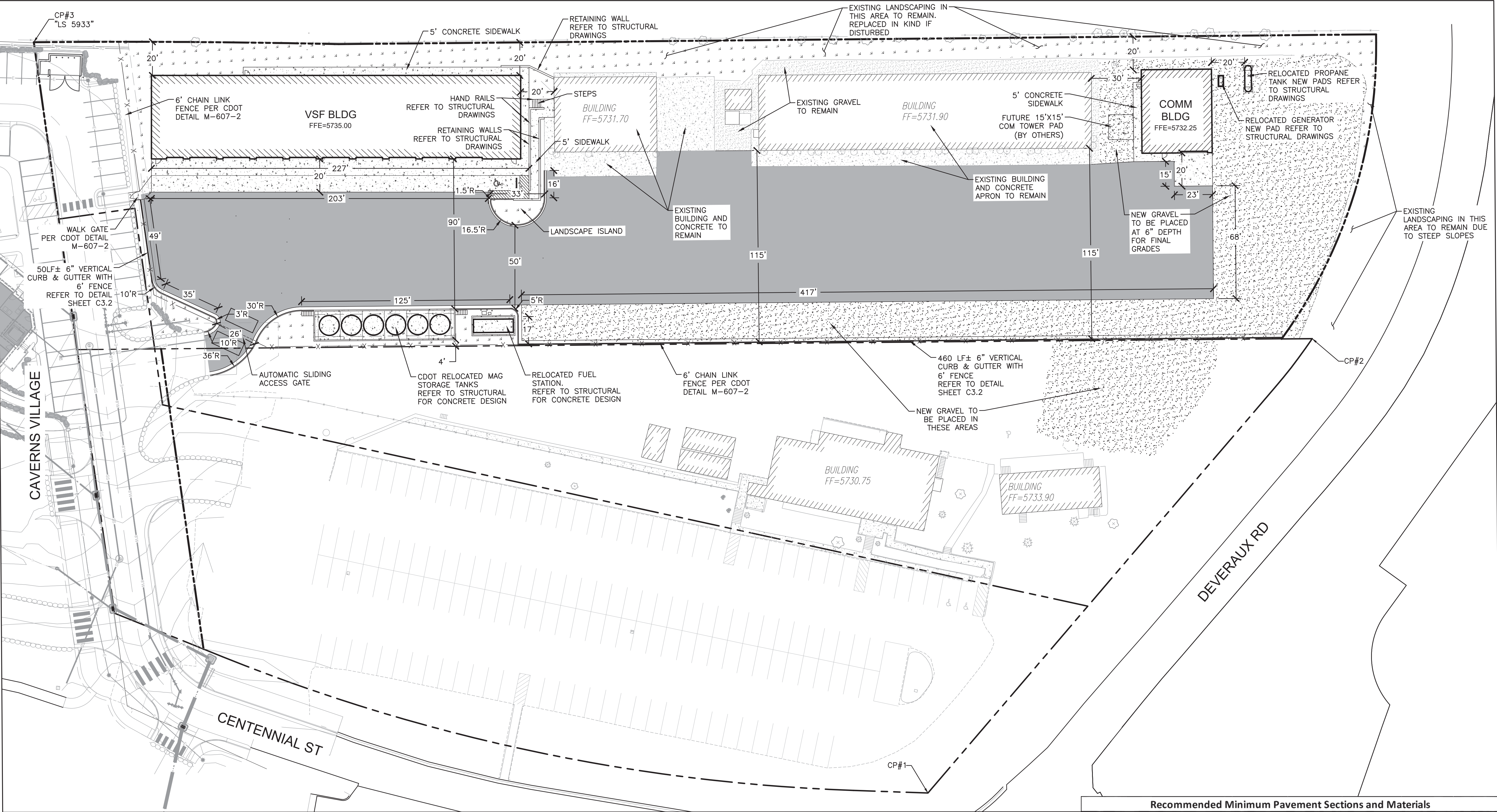
85% CONSTRUCTION DOCS

08/09/2020

DRAWN BY: JLS

CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

DRAWING NUMBER
C0.3



EXISTING

LEGEND

PROPERTY LINE

LOT LINE

EASEMENT

BUILDINGS

CONCRETE

ASPHALT

GRAVEL

LANDSCAPING

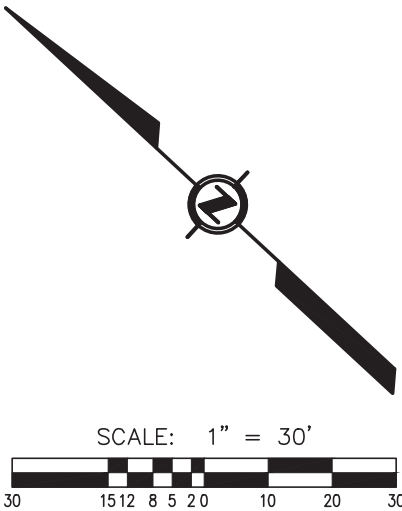
PROPOSED

NOTES:
1. ALL NEW AND DISTURBED LANDSCAPING TO BE REPLACED IN KIND.

Recommended Minimum Pavement Sections and Materials					
Traffic Area	Alternative	Preliminary Pavement Thickness (inches)			
		Asphalt Concrete Surface	Aggregate Base Course	Portland Cement Concrete	Total
Glenwood Springs Facility Lot - 75% SUT	A	6	-	-	6
	B	4	8	-	12
	C ¹	-	-	5	5
Glenwood Springs Facility Lot - 75% MUT	A	7.5	-	-	7.5
	B	5	10	-	15
	C ¹	-	-	6	6

1. The minimum pavement section thickness per ACI
Reference: Terracon GeoReport "CDOT Glenwood Springs Facility Improvements"

BASIS OF BEARING AND COORDINATE SYSTEM: CITY OF GLENWOOD SPRINGS HORIZONTAL AND VERTICAL DATUM
TAKEN FROM "CAVERNS VILLAGE" PROJECT LOCATED TO THE NORTHWEST OF THE SUBJECT PROPERTY. CONTROL POINTS USED FOR THIS SURVEY ARE AS FOLLOWS:
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CP#2 N:42128.56 E:16378.22 EL:5734.44
CP#3 N:42813.55 E:15985.33 EL:5737.91



SITE PLAN

Glenwood Springs VSF

202 CENTENNIAL ST.

GLENWOOD SPRINGS, COLORADO 81601

Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

CDOT

DEPARTMENT OF TRANSPORTATION

DATE	NO.	DESCRIPTION
08/12/2020	1	65% CONSTRUCTION DOCS
08/09/2020	2	85% CONSTRUCTION DOCS

COLLINS ENGINEERS

455 Sherman Street, Suite 160
Denver, Colorado 80203
Phone: (303) 447-0090 Fax: (303) 447-9141
www.collinsengr.com

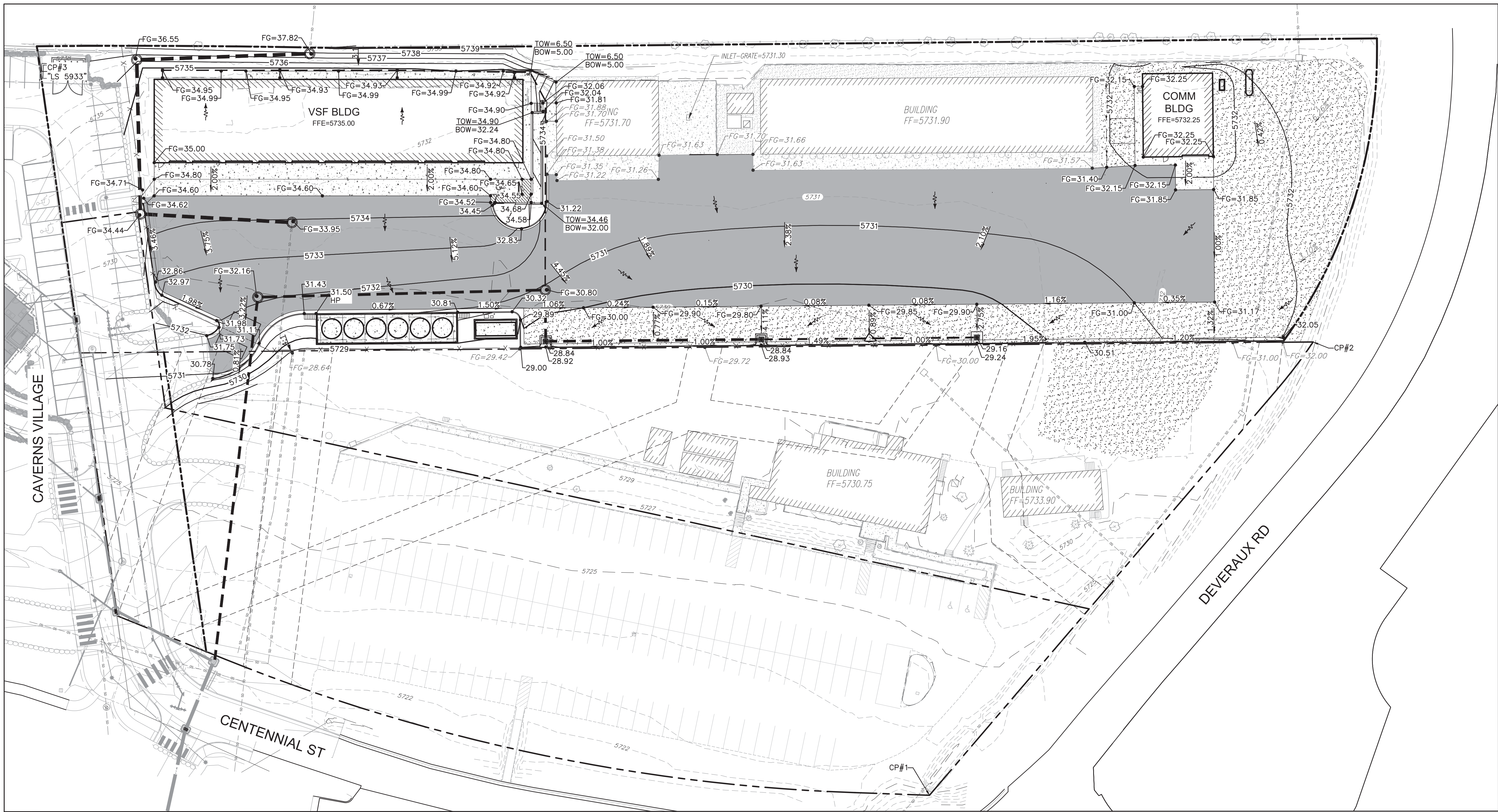
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CDOT PROJECT NO. 23332
SAP BUILDING # 1000/XXX

DRAWING NUMBER

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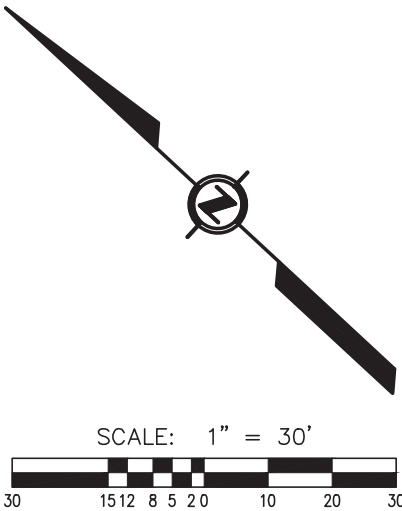
Packet Page 503



EXISTING		LEGEND		PROPOSED	
		PROPERTY LINE			
		LOT LINE			
		EASEMENT			
		BUILDINGS			
		CONTOURS			
		CONCRETE			
		ASPHALT			
		GRAVEL			
		LANDSCAPING			
		STORM SEWER			
		FLOW ARROW			

BASIS OF BEARING AND COORDINATE SYSTEM: CITY OF GLENWOOD SPRINGS HORIZONTAL AND VERTICAL DATUM TAKEN FROM "CAVERNS VILLAGE" PROJECT LOCATED TO THE NORTHWEST OF THE SUBJECT PROPERTY. CONTROL POINTS USED FOR THIS SURVEY ARE AS FOLLOWS:
CP#1 N:42112.15 E:16020.49 EL:5727.54
CP#2 N:42128.56 E:16378.22 EL:5734.44
CP#3 N:42813.55 E:15985.33 EL:5737.91

EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.



DATE	NO.	DESCRIPTION
08/12/2020	1	65% CONSTRUCTION DOCS
08/09/2020	2	85% CONSTRUCTION DOCS

Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

GRADING PLAN

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

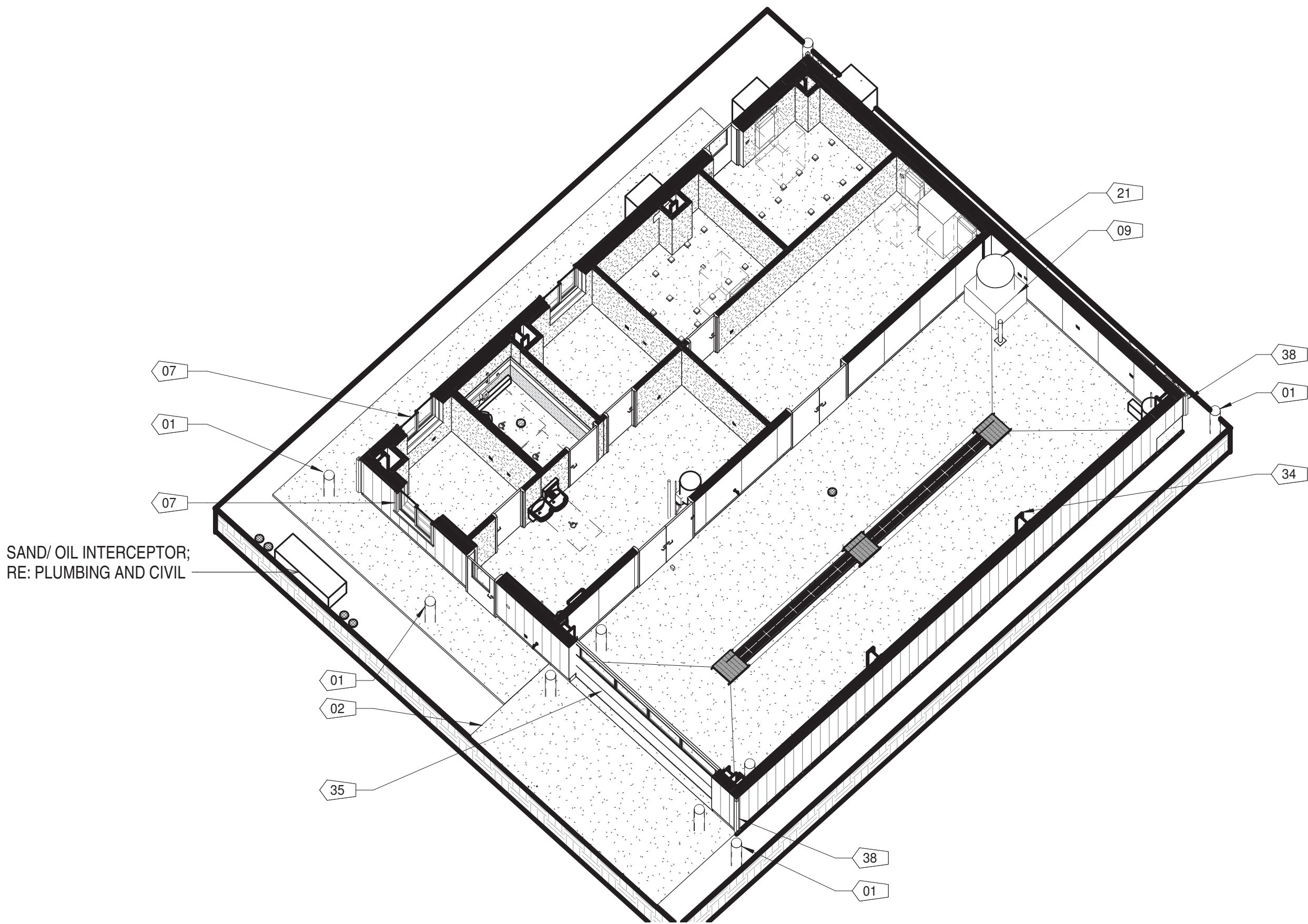
COLLINS ENGINEERS

455 Sherman Street, Suite 100
Denver, Colorado 80203
voice: (303) 447-0090
fax: (303) 447-9141
www.collinsengr.com

DRAWN BY: JLS

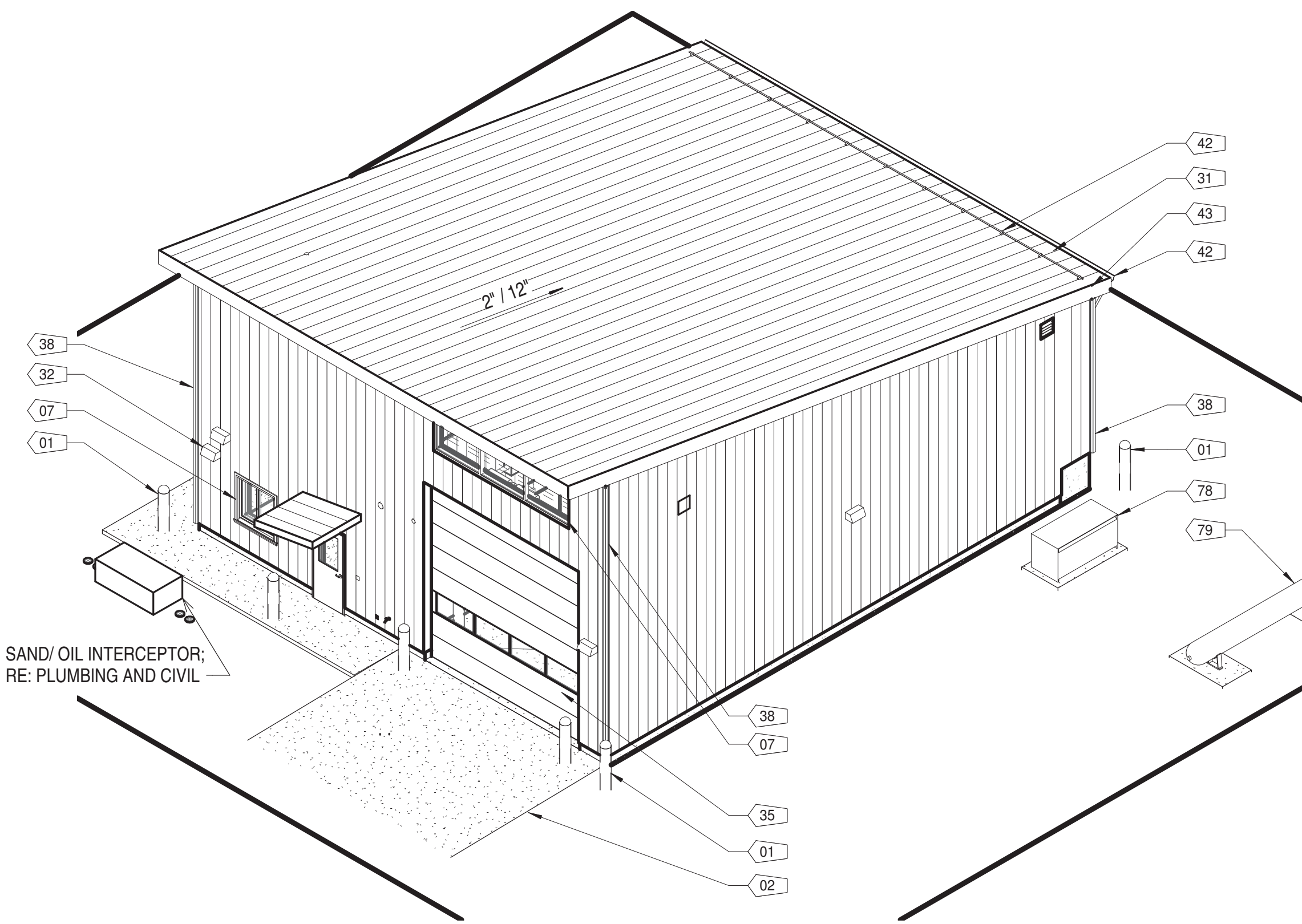
CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

DRAWING NUMBER
C2.1



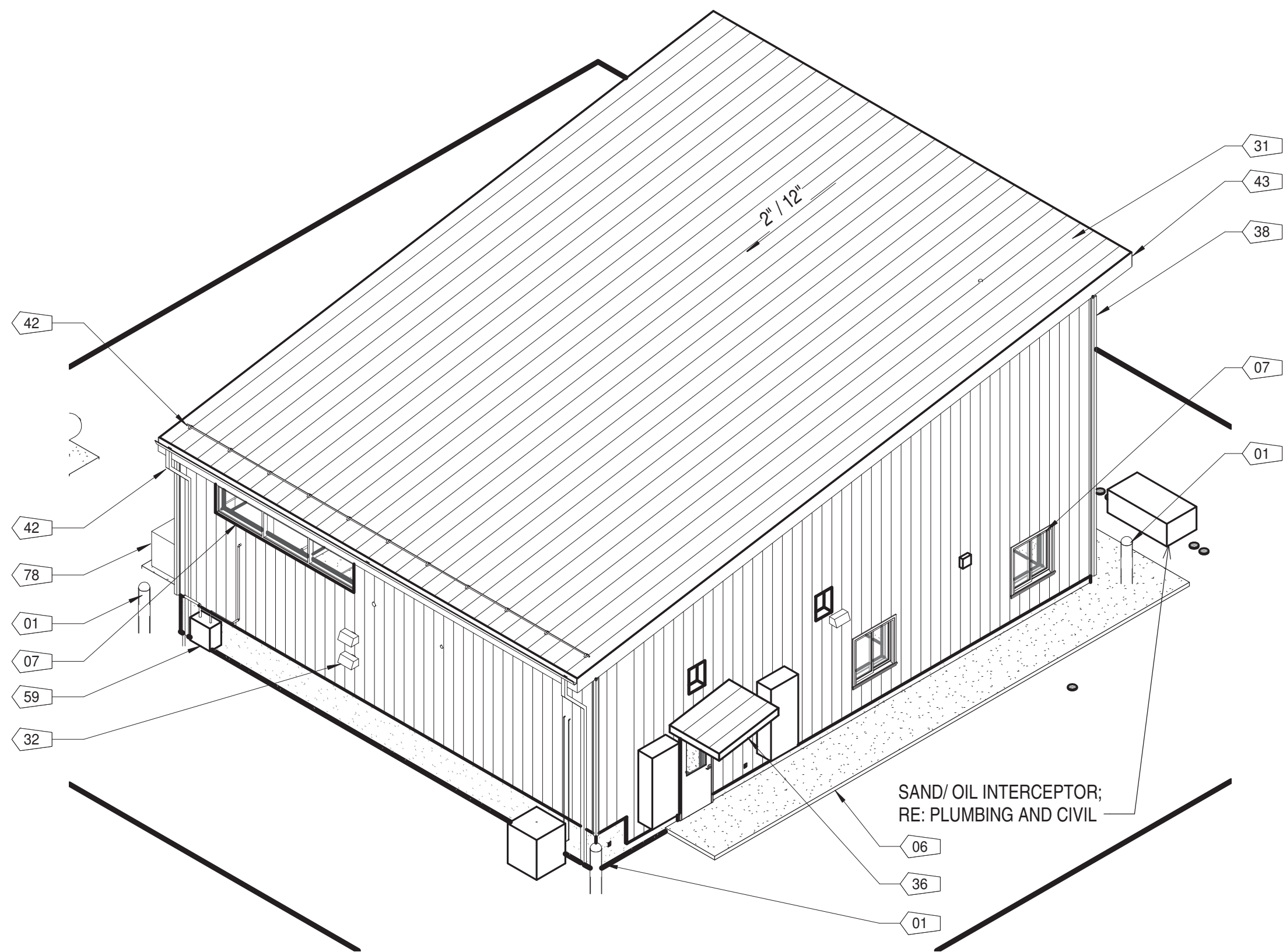
3 | 3D AXONOMETRIC - CUT FLOOR PLAN, FIRST FLOOR

C-A3.1 |



1 | 3D AXONOMETRIC - COMMUNICATION, SOUTHEAST

C-A3.1 |



2 | 3D AXONOMETRIC - COMMUNICATION, NORTHWEST

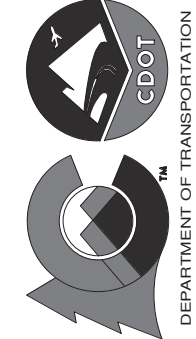
C-A3.1 |

MASTER KEY NOTES

1. TYPICAL 10" DIA. CONC. FILLED STEEL BOLLARD, PT-4, RE: A4.0 FOR BOLLARDS AT OH DOORS AND APRON, RE: CIVIL FOR OTHER EXTERIOR BOLLARDS. BOLLARDS AT ALL EXTERIOR EQUIPMENT AND METERS.
2. CONCRETE SLAB ON GRADE, RE:1/A2.1, CIVIL AND STRUCTURAL DWGS. SEAL CONCRETE ACCORDING TO FINISH SCHEDULE.
3. FREEZEPROOF WALL HYDRANT RE: PLUMB
4. SLAB SHALL SLOPE TO DRAIN & NOT EXCEED ICC A117.1 CROSS-SLOPE REQUIREMENTS.
5. CONCRETE CURB, RE: STRUCT.
6. 5' WIDE CONC. LANDING AND SIDEWALK EXTEND TO CONC. APRON
7. SCHEDULED WINDOW
8. TRENCH DRAIN, RE: STRUCT., SEE ALSO KEYNOTE 58
9. 4" MIN" CONCRETE UTILITY PAD, RE: STRUCTURAL
10. METAL BUILDING ON ENGINEERED FOUNDATION, RE: STRUCTURAL FOR FOUNDATION
11. 500 GAL. WASTE OIL TANK (WT-1) CONNECT TO WASTE OIL PUMP IN VSF
12. STATIONARY PRESSURE WASHER (PW-1), VENT TO SIDEWALL
13. ACCESSIBLE PARKING SIGNAGE RE: CIVIL
14. EXTENT OF RETRACTED OH DOOR, ABOVE
15. REFER TO A4.3 FOR TYPICAL BOLLARD PLACEMENT CRITERIA AT OH DOORS. FOR BOLLARDS ON SITE REFER TO CIVIL DETAILS.
16. 20" DEEP HVY DTY ADJUSTABLE BLACK MELAMINE WALL SHELVING - 3 SHELVES
17. REQUIRED CLEAR AREA AT DOORS DO NOT EXCEED SLOPE OR CROSS SLOPE REQUIREMENTS AT ACCESSIBLE ROUTE
18. FLAMMABLE MATERIAL STORAGE CABINET
19. WASTE OIL DIAPHRAM PUMP (WT-1)
20. WASTE OIL COLLECTION BASIN (WT-1)
21. 120 GAL. AIR COMPRESSOR (AIR-1)
22. WATER TREATMENT SYSTEM TRAFFIC RATED MANHOLE COVERS, RE: CIVIL
23. CO/NO2 SENSOR AND PANEL. PROVIDE WATERPROOF IN WASH BAY.
24. PAVEMENT STRIPING
25. FURR COLUMNS / BASES AT ADMIN TYP TO 6" ABOVE CEILING
26. CONC. FOUNDATION WALL, RE: STRUCT
27. PILASTER, RE: STRUCTURAL
28. FRP-2
29. FRP-1 WITH MP-1 ABOVE 8'-6" AFF
30. EPOXY PAINT SYSTEM AT EXPOSED METAL AT WASH BAY. RE: SPECS
31. PREFINISHED METAL SEAM, COLOR MP-3. USE ONLY CDOT- APPROVED CONCEALED FASTENER ROOF SYSTEM. PINNED AT PEAK.
32. SCHEDULED LIGHT FIXTURE.
33. PREFINISHED METAL WALL PANEL, COLOR MP-2, TYPICAL
34. PAINT ALL EXPOSED STRUCTURAL STEEL TYP. RE: FINISH SCHEDULE
35. INSULATED SECTIONAL DOOR. COORDINATE LOCATION OF MOTOR OPERATOR AT EACH DOOR WITH STRUCTURE.
36. 5'-0" X 6'-0" CANOPY BY PEMB, MANUF. COLOR=MP-3, TYPICAL ALL EXTERIOR MAN DOORS, NO GUTTER. 6" DEEP, 2:12 SLOPE, SUPPORT FROM WALL STRUCTURE. RE: DETAIL ON A4.2 FOR FLASHING
37. PREFINISHED METAL TRIM, MP-3, TYPICAL
38. CORNER TRIM, MP-2, TYP ALL EXT. CORNERS.
39. PRESSURE WASHER FLUE, RE: MECH
40. FINISH GRADE RE: CIVIL
41. WATER TREATMENT SYSTEM, ALL TANKS AND PIPING INTO BUILDING BY CONTRACTOR. RE: PLUMB FOR DIMENSIONS AND STUB-OUT DIAGRAM. ACTUAL TREATMENT SYSTEM BY CDOT SUB CONTRACTOR. CONTRACTOR TO COORDINATE INSTALLATION WITH CDOT SUB.
42. PROVIDE SNOW GUARDS AT ROOF WITH GUTTER AND DOWNSPOUTS FOR DRAINAGE
43. PREFINISHED METAL FASCIA, COLOR MP-3
44. WATER TREATMENT SYSTEM TANKS, PROVIDED BY GC RE: MECH.
45. ROD BRACING FRAME, PER METAL BLDG MFR (RE: STRUCT DRAWINGS) PAINT AT ALL EXPOSED LOCATIONS. AT WASH BAY USE EPOXY PAINT.
46. 200 AMP ELECTRICAL PANEL FOR WTS
47. STRIPE 3'-0 IN FRONT OF ELEC PANEL, PT-4
48. FURNITURE, N.I.C.
49. ELECTRICAL PANELS. RE: ELEC.
50. HOSE BIBB
51. SEWER CONNECTION. RE: CIVIL / PLUMB
52. WATER HEATER TANK ON CURB
53. WATER CONNECTION. RE: CIVIL AND MECH
54. KNOX BOX. COORDINATE WITH AHJ.
55. EMERGENCY SHOWER/EYE WASH (ESEW)
56. WASTE OIL PIPING, SECURED TO WALL
57. RADIANT HEATER INTAKE VENT, MP-2
58. 2'X 2' X 2' CATCH BASIN. CONNECTION TO SAND/OIL INTERCEPTOR TANK, PROVIDE SEDIMENT BUCKET. RE: MECH
59. GAS METER, RE: CIVIL AND MEP DWGS.
60. COMPRESSED AIR OUTLET TYP
61. LOUVERED OPENING, RE: MECH
62. INFRA RED RADIANT TUBE HEATER, RE: MECH
63. POLYETHELENE DUCT AND POWER VENTILATOR W/ CO/NO2 DETECTOR. PROVIDE MANUAL OVERRIDE RE: MECH
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71. PROVIDE ADA ACCESSIBLE EXIT SIGNAGE AT LATCH SIDE OF EXIT DOORS.
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73. DUAL ELECTRIC WATER COOLER. MAINTAIN 15" FROM CENTERLINE OF ADA UNIT
74. 20A DEDICATED OUTLET FOR BLOCK HEATER
75. RESTROOM VENT RE: MECH
76. VENT STACK +36"
77. MECH DUCTWORK; FURR AROUND MECH DUCTWORK IN LOCATIONS SHOWN; LOCATION OF DUCT PENETRATIONS, VIF. RE: STRUCTURAL FOR ADDITIONAL SLAB PENETRATION INFORMATION
78. EMERGENCY GENERATOR; RE: ELECTRICAL
79. FUEL STORAGE TANK; RE: MECHANICAL
80. 8'-0" HIGH, CHAIN-LINK FENCE ENCLOSURE

Colorado Department of Transportation

PROPERTY MANAGEMENT
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3D AXONOMETRICS - COMM

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

D2C
ARCHITECTS

DRAWN BY: REC

CDOT PROJECT NO. 23332
SAP BUILDING # 1000/XXX

DRAWING NUMBER

C-A3.1

MASTER KEY NOTES

1. TYPICAL 10" DIA. CONC. FILLED STEEL BOLLARD, PT-4, RE: A4.0 FOR BOLLARDS AT OH DOORS AND APRON, RE: CIVIL FOR OTHER EXTERIOR BOLLARDS. BOLLARDS AT ALL EXTERIOR EQUIPMENT AND METERS.
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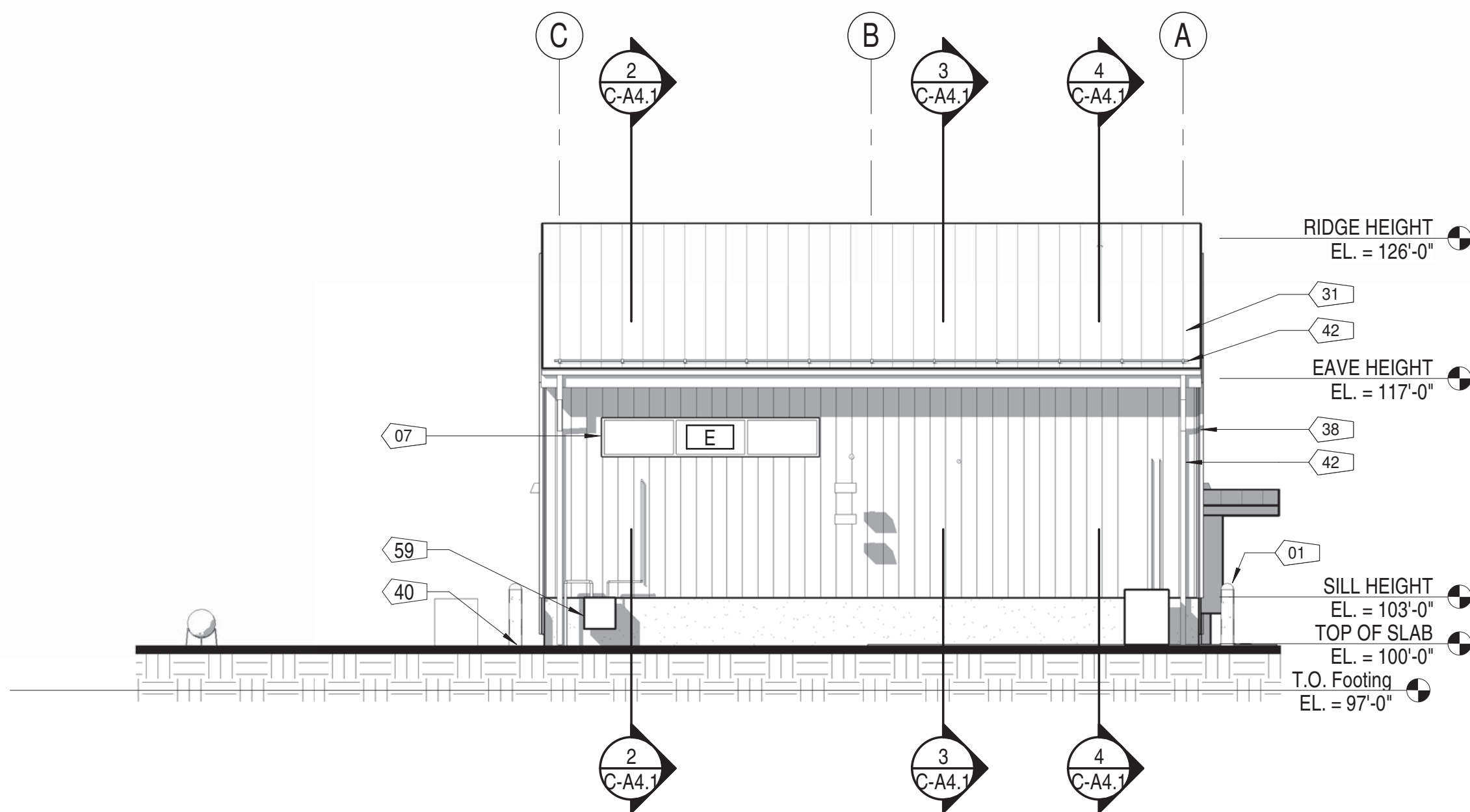
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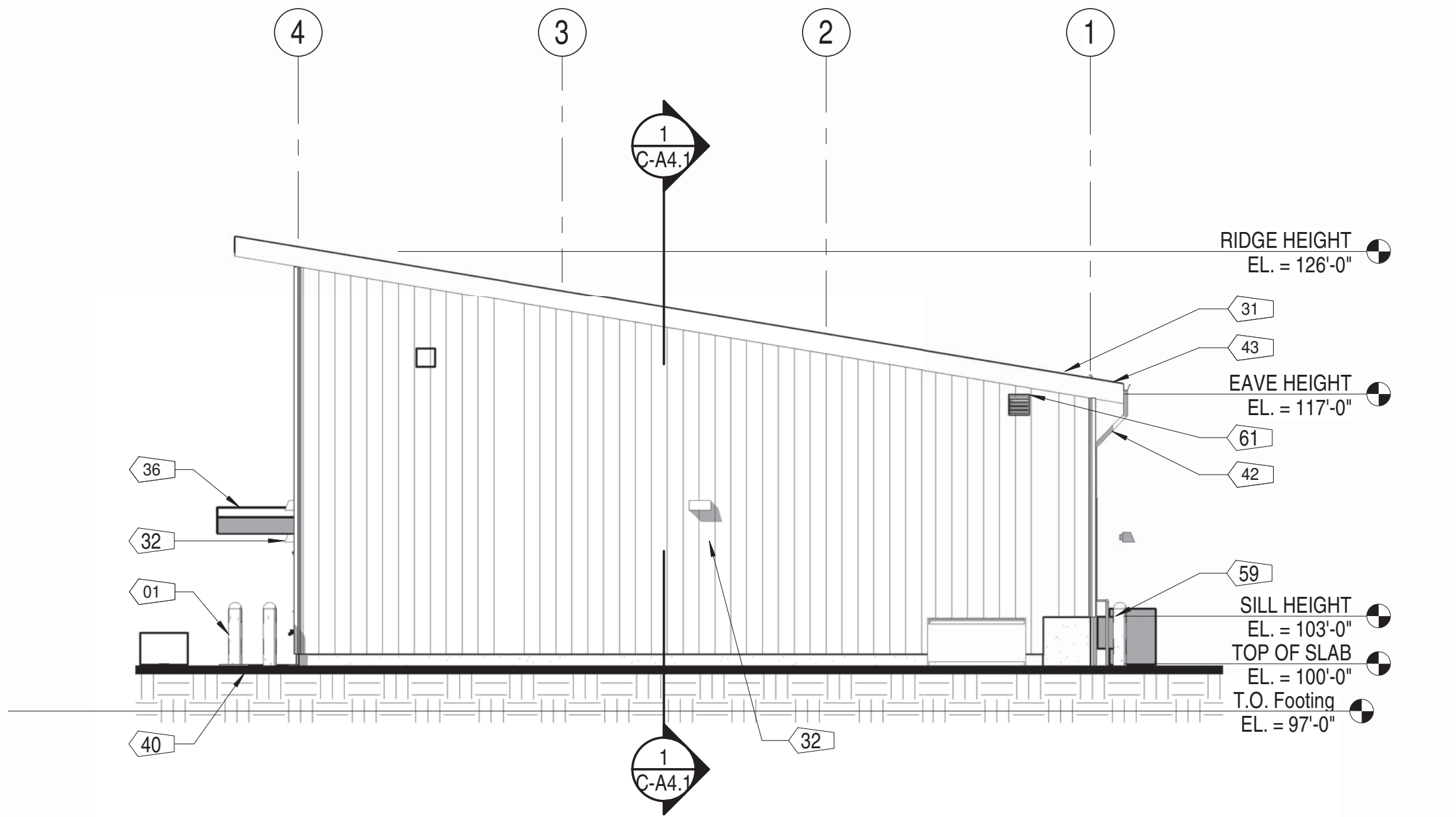
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47. STRIPE 3'-0 IN FRONT OF ELEC PANEL, PT-4
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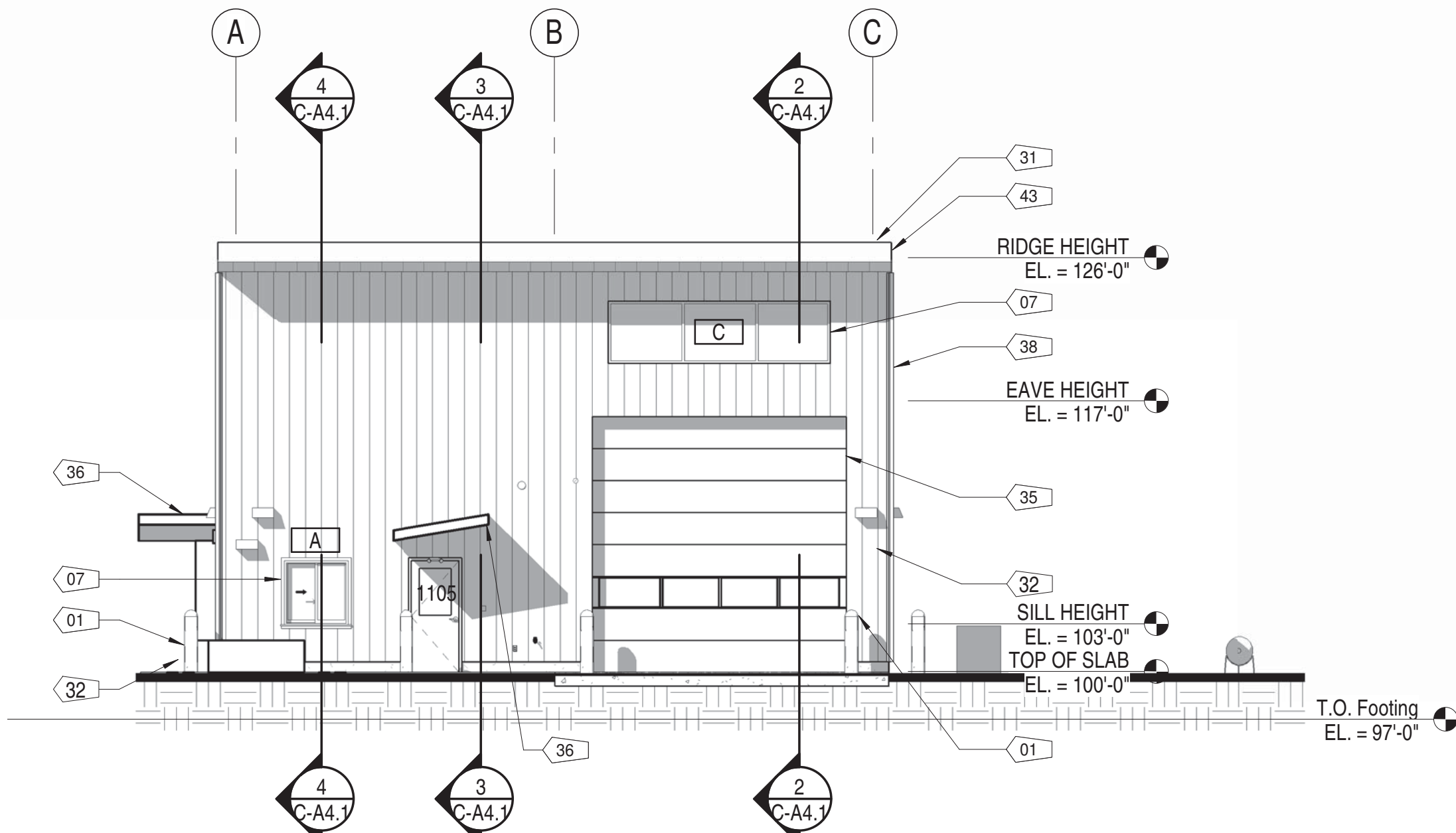
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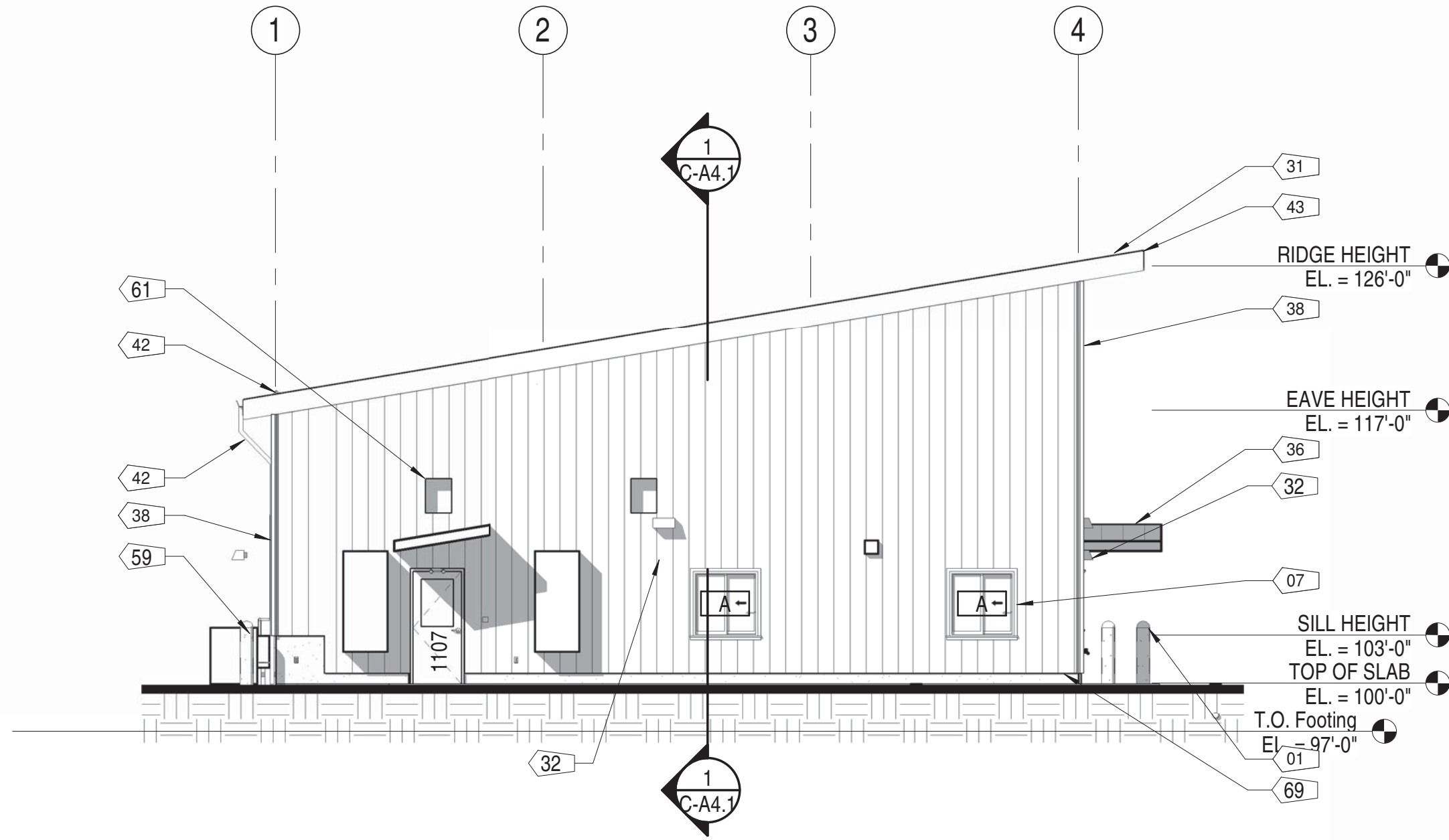
2 | BUILDING ELEVATION - COMMUNICATION, NORTH
C-A3.2 1/8" = 1'-0"



3 | BUILDING ELEVATION - COMMUNICATION, EAST
C-A3.2 1/8" = 1'-0"



1 | BUILDING ELEVATION - COMMUNICATION, SOUTH
C-A3.2 1/8" = 1'-0"



4 | BUILDING ELEVATION - COMMUNICATION, WEST
C-A3.2 1/8" = 1'-0"

Colorado Department of Transportation

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BUILDING ELEVATIONS - COMM

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

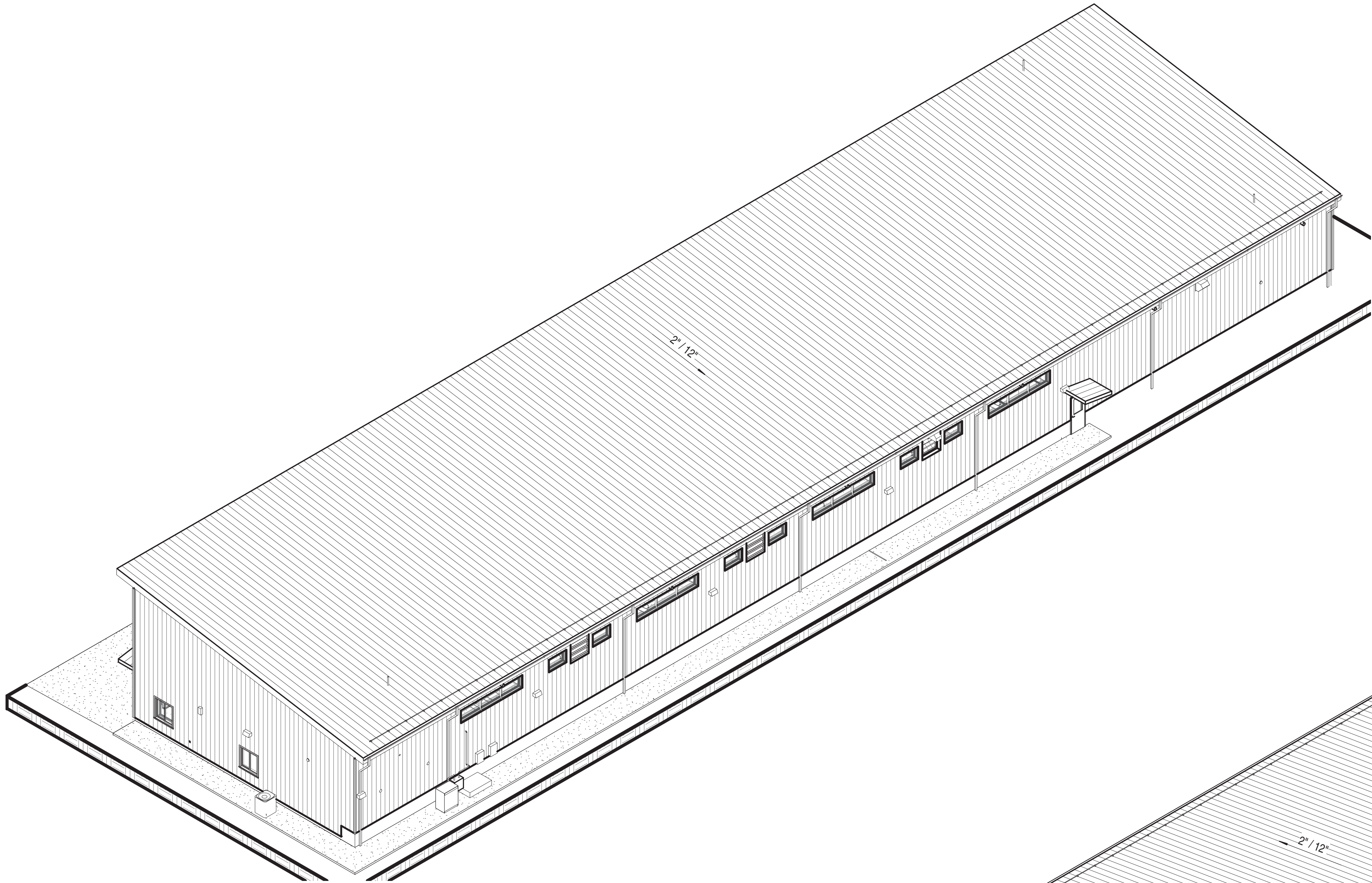
D2C
ARCHITECTS

DRAWN BY: REC

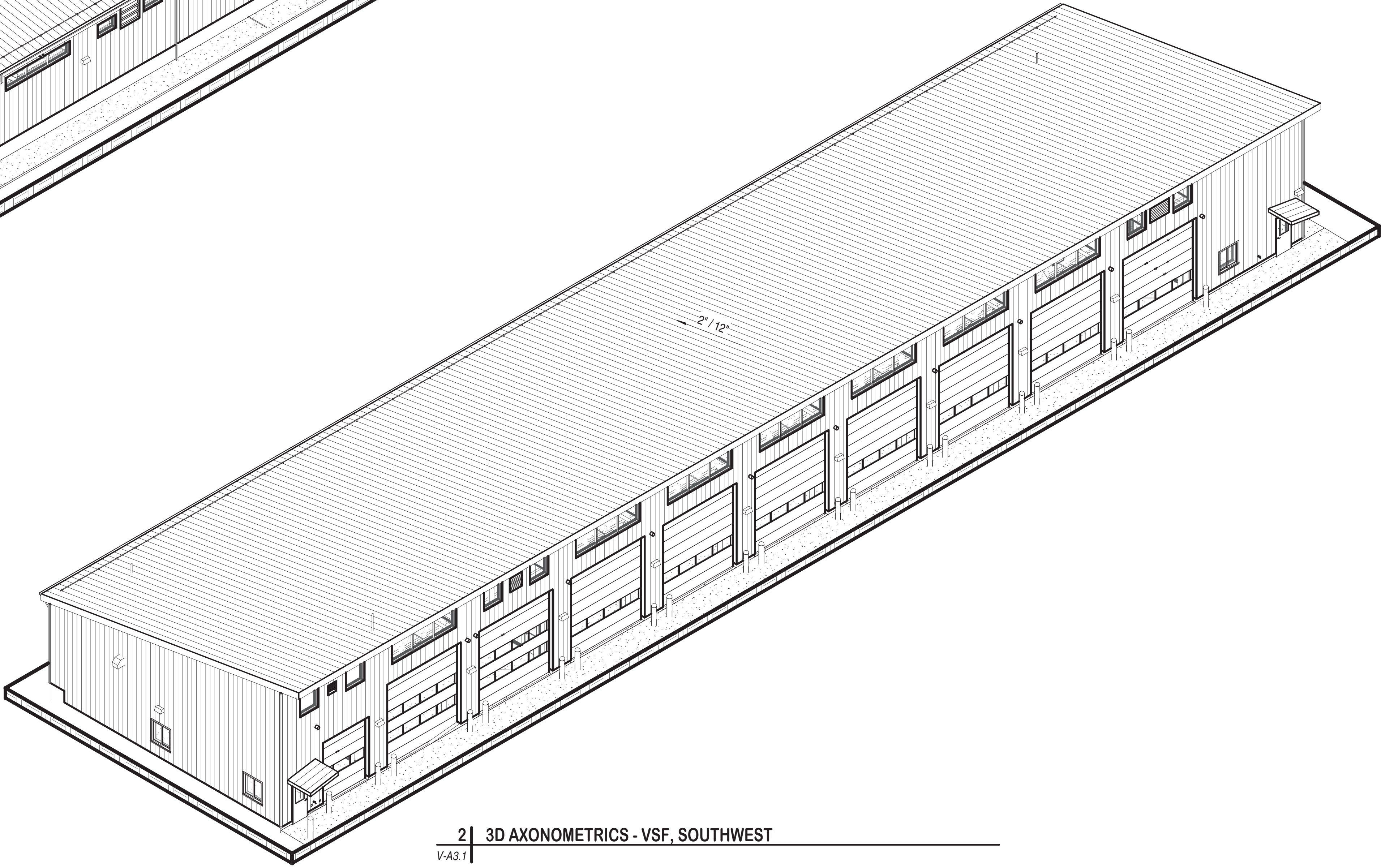
CDOT PROJECT NO. 23332
SAP BUILDING # 1000/XXX

DRAWING NUMBER

C-A3.2



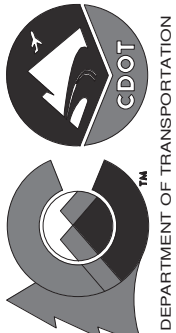
1 | 3D AXONOMETRICS - VSF, NORTHEAST
V-A3.1



2 | 3D AXONOMETRICS - VSF, SOUTHWEST
V-A3.1

DATE	NO.	DESCRIPTION
06/12/2020	1	85% CONSTRUCTION DOCS
07/24/2020	2	85% CONSTRUCTION DOCS

Colorado Department of Transportation



CDOT
DEPARTMENT OF TRANSPORTATION

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3D AXONOMETRICS - VSF

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D2C

ARCHITECTS

DRAWN BY: REC

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23332
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DRAWING NUMBER
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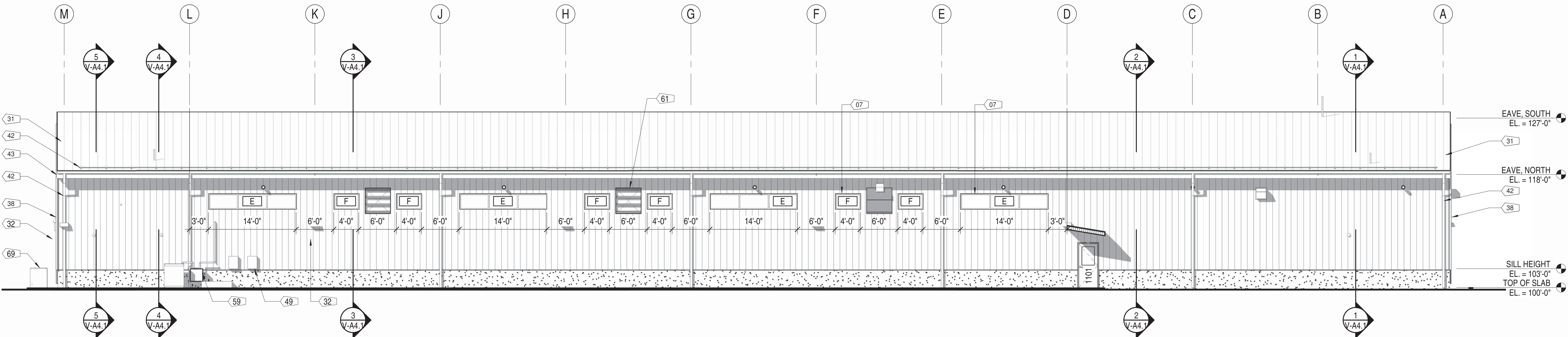
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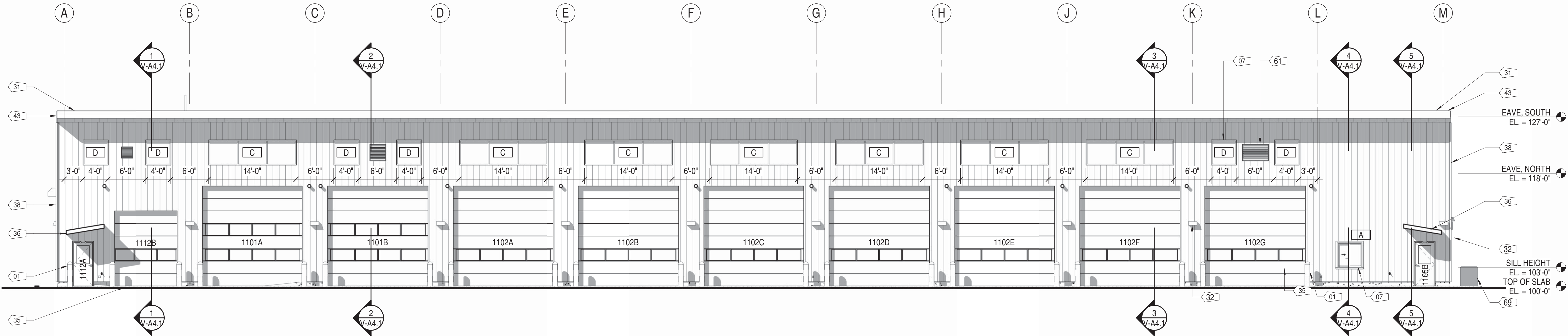
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42. CLIMATE ZONE 5: PROVIDE GUTTER AND DOWNSPOUT, AS WELL AS, SNOW GUARDS
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65. 3'-6" TALL 4" PIPE BOLLARD, PAINT PT-4 W/ 8 X 8 X 1/4" BASE PLATE ANCHORED W/ (4) 1/2" MIN ANCHOR BOLTS INTO CONC FLOOR

66. RADIANT HEATER EXHAUST VENT
67. EXHAUST/VENTILATION FAN RE: MECH
68. EXTERIOR OUTLET RE: ELEC
69. A/C CONDENSING UNIT (CU) BOLTED TO PAD
70. PROVIDE PLAM WINDOW SILL AT WINDOW
71. PROVIDE ADA ACCESSIBLE EXIT SIGNAGE AT LATCH SIDE OF EXIT DOORS.
72. DOMESTIC WATER LINE AND BACK FLOW PREVENTOR. RE: PLUMBING AND CIVIL.
73. DUAL ELECTRIC WATER COOLER. MAINTAIN 15" FROM CENTERLINE OF ADA UNIT
74. 20A DEDICATED OUTLET FOR BLOCK HEATER
75. RESTROOM VENT RE: MECH
76. VENT STACK +36"
77. MECH DUCTWORK: FURR AROUND MECH DUCTWORK IN LOCATIONS SHOWN: LOCATION OF DUCT PENETRATIONS, VIF. RE: STRUCTURAL FOR ADDITIONAL SLAB PENETRATION INFORMATION
78. EMERGENCY GENERATOR; RE: ELECTRICAL
79. FUEL STORAGE TANK; RE: MECHANICAL
80. 8'-0" HIGH, CHAIN-LINK FENCE ENCLOSURE



1 | BUILDING ELEVATION - VSF, NORTH
V-A3.2 1/8" = 1'-0"



2 | BUILDING ELEVATION - VSF, SOUTH
V-A3.2 1/8" = 1'-0"

Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL 4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

BUILDING ELEVATIONS - VSF

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

D2C
ARCHITECTS

DRAWN BY: REC

CDOT PROJECT NO. 23332
SAP BUILDING # 1000/XXX

DRAWING NUMBER

V-A3.2

MASTER KEY NOTES

1. TYPICAL 10" DIA. CONC. FILLED STEEL BOLLARD, PT-4, RE: A4.0 FOR BOLLARDS AT OH DOORS AND APRON, RE: CIVIL FOR OTHER EXTERIOR BOLLARDS. BOLLARDS AT ALL EXTERIOR EQUIPMENT AND METERS.
2. CONCRETE SLAB ON GRADE, RE:1/A2.1, CIVIL AND STRUCTURAL DWGS. SEAL CONCRETE ACCORDING TO FINISH SCHEDULE.
3. FREEZEPROOF WALL HYDRANT RE: PLUMB
4. SLAB SHALL SLOPE TO DRAIN & NOT EXCEED ICC A117.1 CROSS-SLOPE REQUIREMENTS.
5. CONCRETE CURB, RE: STRUCT.
6. 5' WIDE CONC. LANDING AND SIDEWALK EXTEND TO CONC. APRON
7. SCHEDULED WINDOW
8. TRENCH DRAIN, RE: STRUCT., SEE ALSO KEYNOTE 58
9. 4" MIN' CONCRETE UTILITY PAD, RE: STRUCTURAL
10. METAL BUILDING ON ENGINEERED FOUNDATION, RE: STRUCTURAL FOR FOUNDATION
11. 500 GAL. WASTE OIL TANK (WT-1) CONNECT TO WASTE OIL PUMP IN VSF

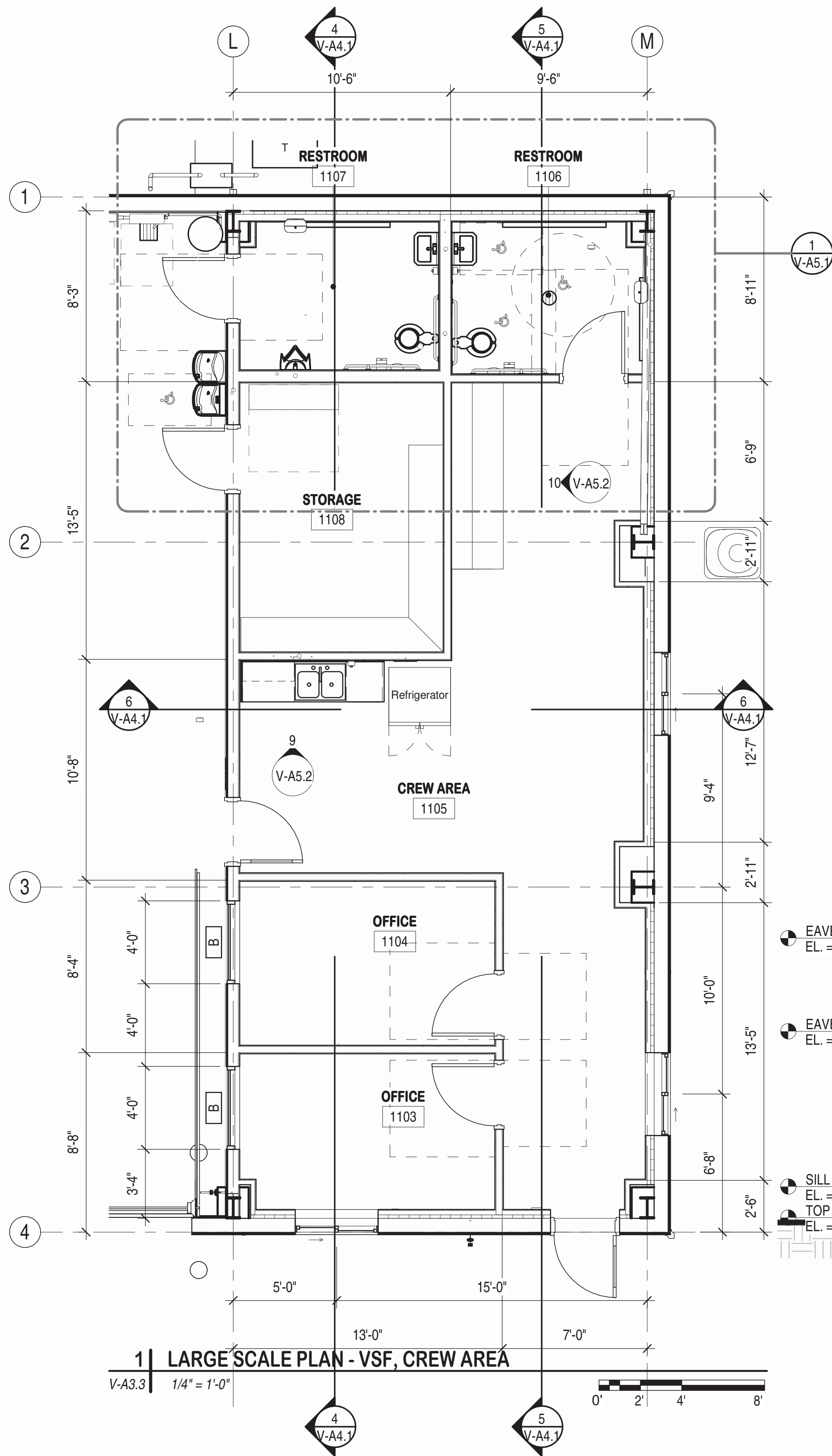
12. STATIONARY PRESSURE WASHER (PW-1), VENT TO SIDEWALL
13. ACCESSIBLE PARKING SIGNAGE RE: CIVIL
14. EXTENT OF RETRACTED OH DOOR, ABOVE
15. REFER TO A4.3 FOR TYPICAL BOLLARD PLACEMENT CRITERIA AT OH DOORS. FOR BOLLARDS ON SITE REFER TO CIVIL DETAILS.
16. 20" DEEP HVY DTY ADJUSTABLE BLACK MELAMINE WALL SHELVING - 3 SHELVES
17. REQUIRED CLEAR AREA AT DOORS DO NOT EXCEED SLOPE OR CROSS SLOPE REQUIREMENTS AT ACCESSIBLE ROUTE
18. FLAMMABLE MATERIAL STORAGE CABINET
19. WASTE OIL DIAPHRAM PUMP (WT-1)
20. WASTE OIL COLLECTION BASIN (WT-1)
21. 120 GAL. AIR COMPRESSOR (AIR-1)
22. WATER TREATMENT SYSTEM TRAFFIC RATED MANHOLE COVERS, RE: CIVIL
23. CO/NO2 SENSOR AND PANEL. PROVIDE WATERPROOF IN WASH BAY.
24. PAVEMENT STRIPING

25. FURR COLUMNS / BASES AT ADMIN TYP TO 6" ABOVE CEILING
26. CONC. FOUNDATION WALL, RE: STRUCT
27. PILASTER, RE: STRUCTURAL
28. FRP-2
29. FRP-1 WITH MP-1 ABOVE 8'-6 AFF
30. EPOXY PAINT SYSTEM AT EXPOSED METAL AT WASH BAY, RE: SPECS
31. PREFINISHED METAL ROOF, STANDING SEAM, COLOR MP-3. USE ONLY CDOT- APPROVED CONCEALED FASTENER ROOF SYSTEM. PINNED AT PEAK.
32. SCHEDULED LIGHT FIXTURE.
33. PREFINISHED METAL WALL PANEL, COLOR MP-2, TYPICAL
34. PAINT ALL EXPOSED STRUCTURAL STEEL TYP. RE: FINISH SCHEDULE
35. INSULATED SECTIONAL DOOR. COORDINATE LOCATION OF MOTOR OPERATOR AT EACH DOOR WITH STRUCTURE.
36. 5'-0" X 6'-0" CANOPY BY PEMB, MANUF. COLOR=MP-3, TYPICAL ALL EXTERIOR MAN DOORS, NO GUTTER. 6" DEEP, 2:12 SLOPE, SUPPORT FROM WALL STRUCTURE. RE: DETAIL ON A4.2 FOR FLASHING

37. PREFINISHED METAL TRIM, MP-3, TYPICAL
38. CORNER TRIM, MP-2, TYP ALL EXT. CORNERS.
39. PRESSURE WASHER FLUE, RE: MECH
40. FINISH GRADE RE: CIVIL
41. WATER TREATMENT SYSTEM, ALL TANKS AND PIPING INTO BUILDING BY CONTRACTOR. RE: PLUMB FOR DIMENSIONS AND STUB-OUT DIAGRAM. ACTUAL TREATMENT SYSTEM BY CDOT SUB CONTRACTOR. CONTRACTOR TO COORDINATE INSTALLATION WITH CDOT SUB.
42. CLIMATE ZONE 5: PROVIDE GUTTER AND DOWNSPOUT, AS WELL AS, SNOW GUARDS
43. PREFINISHED METAL FASCIA, COLOR MP-3
44. WATER TREATMENT SYSTEM TANKS, PROVIDED BY GC RE: MECH.
45. ROD BRACING FRAME, PER METAL BLDG MFR (RE: STRUCT DRAWINGS) PAINT AT ALL EXPOSED LOCATIONS. AT WASH BAY USE EPOXY PAINT.
46. 200 AMP ELECTRICAL PANEL FOR WTS
47. STRIPE 3'-0 IN FRONT OF ELEC PANEL, PT-4
48. FURNITURE, N.I.C.
49. ELECTRICAL PANELS, RE: ELEC.
50. HOSE BIBB

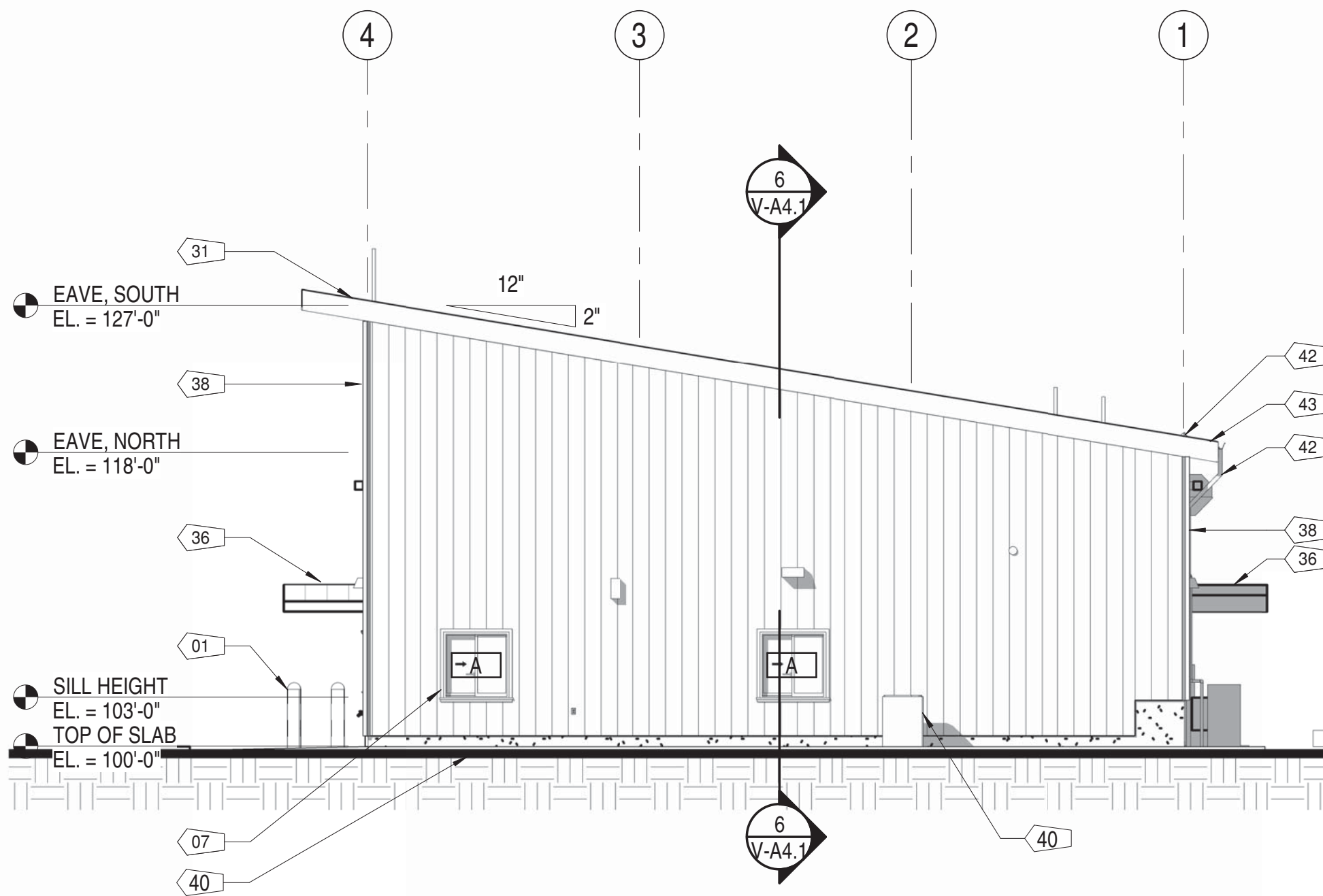
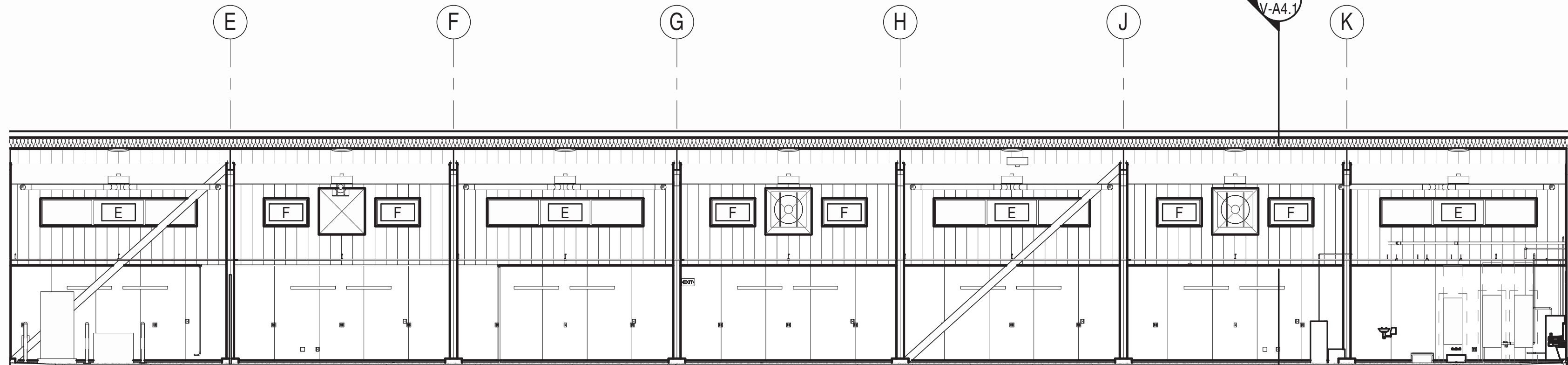
51. SEWER CONNECTION, RE: CIVIL / PLUMB
52. WATER HEATER TANK ON CURB
53. WATER CONNECTION, RE: CIVIL AND MECH
54. KNOX BOX. COORDINATE WITH AHJ.
55. EMERGENCY SHOWER/EYE WASH (ESEW)
56. WASTE OIL PIPING, SECURED TO WALL
57. RADIANT HEATER INTAKE VENT, MP-2
58. 2'X 2' X 2' CATCH BASIN. CONNECTION TO SAND/OIL INTERCEPTOR TANK, PROVIDE SEDIMENT BUCKET. RE: MECH
59. GAS METER, RE: CIVIL AND MEP DWGS.
60. COMPRESSED AIR OUTLET TYP
61. LOUVERED OPENING, RE: MECH
62. INFRA RED RADIANT TUBE HEATER, RE: MECH
63. POLYETHELENE DUCT AND POWER VENTILATOR W/ CO/NO2 DETECTOR. PROVIDE MANUAL OVERRIDE RE: MECH
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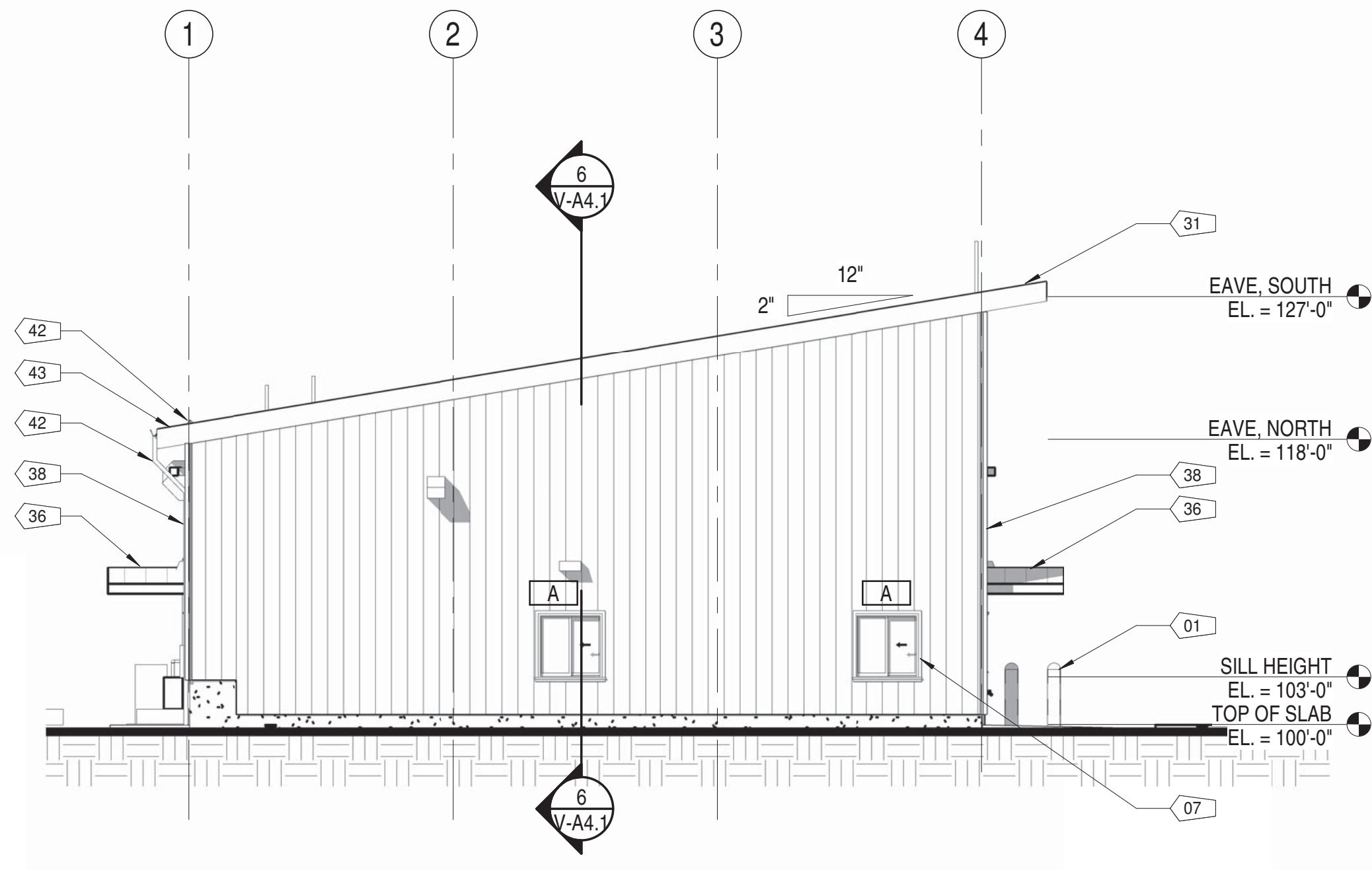
2 | INTERIOR ELEVATION - VEHICLE STORAGE, BACK WALL

V-A3.3 1/8" = 1'-0"



3 | BUILDING ELEVATION - VSP, EAST

V-A3.3 1/8" = 1'-0"



4 | BUILDING ELEVATION - VSP, WEST

V-A3.3 1/8" = 1'-0"

Colorado Department of Transportation

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BUILDING ELEVATIONS - VSF

Glenwood Springs VSF

202 CENTENNIAL ST.
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D2C
ARCHITECTS

DRAWN BY: REC

CDOT PROJECT NO. 23332
SAP BUILDING # 1000/XXX

DRAWING NUMBER

V-A3.3

DRAINAGE REPORT

FOR THE

**GLENWOOD SPRINGS VEHICLE STORAGE FACILITY
202 CENTENNIAL ST
GLENWOOD SPRINGS, COLORADO 81601**

IN

CITY OF GLENWOOD SPRINGS, COLORADO

Prepared For:

**COLORADO DEPARTMENT OF TRANSPORTATION
2829 W HOWARD PL
DENVER, COLORADO 80204**

Prepared By:

**COLLINS ENGINEERS, INC.
455 SHERMAN STREET, SUITE 160
DENVER, CO 80203
(303) 447-0090
ATTN.: BRYAN FRANTZ, P.E.
COLLINS JOB NO. 12383-25**

July 17, 2020

I hereby certify that this report (plan) for the Final Utility Design of CDOT Glenwood Springs VSF was prepared by me (or under my direct supervision) in accordance with the provisions of the City of Glenwood Springs Engineering Standards for the Responsible Parties thereof. I understand that the City of Glenwood Springs does not and shall not assume liability for utilities designed by others.

Bryan Frantz, P.E.
Registered Professional Engineer
State of Colorado No. 0037362

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FINAL DRAINAGE REPORT
FOR
CDOT Glenwood Springs Vehicle Storage Facility
IN
CITY OF GLENWOOD SPRINGS, COLORADO

PURPOSE

This report provides hydrologic and hydraulic concepts for the proposed site improvements for the Glenwood Springs Vehicle Storage Facility (VSF) located in City of Glenwood Springs, Colorado. This report has been prepared in accordance with the 2019 Colorado Department of Transportation (CDOT) Drainage Design Manual, the 2018 Glenwood Engineering Standards, along with other criteria manuals referenced herein and standard engineering practices.

I. General Location and Description

A. Location

1. The project is located within the City of Glenwood Springs, County of Garfield, Colorado and lies in Lot 6 and 7 of Section 9, Township 6 South, Range 89 West of the Sixth Principal Meridian. The VSF project site is located west of I-70, east of the Colorado River, and north of Devereux Road.
2. The site is surrounded by commercial and government uses. Currently there is an existing business to the north and a government state patrol station west of the site. The property is bordered by Devereux Road to the south and I-70 to the east of the site.
3. The site limits are shown on the Vicinity Map, located in Appendix A.

B. Description of Property

1. The overall site is located on approximately 3.12 acres. The proposed site plan consists of the access drive, new vehicle storage building (11,168 sq. ft.), relocated communications building (2,110 sq. ft.) and magnesium chloride storage silos (796 sq. ft. total), parking lot, and an extended detention basin for permanent water quality control.

This final drainage report intends to provide hydrologic and hydraulic calculations for the proposed site improvements that are included as part of the development associated with the platting and site improvements for the additional lots.

No major drainageways exist onsite.

2. Existing soils were determined by the U.S. Department of Agriculture- Natural Resources Conservation Service. The existing soils on site have a rating of hydraulic soil group A and B, with a 50% split between both types of the existing soil of the site. Please see **Appendix A** for the soils map. Existing grades of the site are mainly moderately flat slopes ranging from 1 to 3%. Ground cover consists mostly loose gravel and asphaltic pavement.

II. DRAINAGE BASINS AND SUB-BASINS

A. Major Drainage Basins

1. Floodplain: The project site is located in Flood Zone C: "Area of Minimal Flooding" per FEMA Panel 1432 of 1900; Community Panel Number: 080071 1432 C, revised October 15, 1985.
2. On-Site Major Basins: There are no existing on-site major basins.

Water Quality Capture Volume (WQCV) Pond - Basin (A-1) is approximately 2.84 acres. The area for Basin A-1 includes the tributary area to the WQCV extended detention basin (EDB). This area includes the existing, proposed, and relocated buildings as well as the surrounding landscaped areas, gravel and asphalt pavement.

Generally, the stormwater drains from northeast to southwest across the site. A water quality and detention pond will be located along the south corner of the property. Basin A-1 is tributary to the water quality and detention pond.

The proposed outfall from the detention pond will be discharged to the southeast at a proposed connection to an existing stormwater sewer network to the Colorado River.

Basins Flowing Offsite (O-1) is approximately 0.28 acres. The area for Basin A-2 consists of the access drive and the adjacent transition portion of the asphalt lot and perimeter landscape areas that slope to meet the existing grades to the north.

3. Existing Off-Site Outfalls:

There are no existing off-site outfall locations.

III. DRAINAGE DESIGN CRITERIA

A. Regulations

1. 2019 Colorado Department of Transportation (CDOT) Drainage Design Manual Documents
2. 2018 Glenwood Engineering Standards

B. Hydrological Criteria

1. Rainfall Source

One-Hour Point Rainfall data was provided in Chapter 4, Section 6 of the Glenwood Engineering Standards. Intensities for various durations and frequencies in the City of Glenwood Springs are as follows:

<u>Event Frequency</u>	<u>1-Hour Rainfall (in)</u>
1-Year	0.385
2-Year	0.510
5-Year	0.706
10-Year	0.858
25-Year	1.05
50-Year	1.19
100-Year	1.32

2. Calculation Method

The Rational Method was used to compute peak runoff. This method is defined as follows:

$$Q = CIA$$

Where: Q = Peak Discharge in Cubic Feet per Second (cfs)

C = Runoff Coefficient

I = Rainfall Intensity

A = Drainage Area in Acres

The runoff coefficient for each basin was determined based on Table 6-3 of the Urban Storm Drainage Criteria Manual Volume 1. The peak discharge was determined based on the times of concentration calculated from Standard Form SF-1 for each sub-basin. Hydrologic design calculations can be found in ***Appendix B***.

3. Design storm frequencies

The major and minor storms analyzed for this project were the 5-yr and 100-yr storms.

C. Hydraulic Criteria

1. Detention volume calculation method.

The WQCV was determined using the general equation provided in Section 16.3.1.1 of the CDOT 2019 Drainage Design Manual Chapter 16 – Permanent Water

Quality. 100 year storm event volumes were calculated with the use of MHFD - Detention Basin Design Workbook.

2. The 100-year storm was used to analyze the impacts of larger storm events on the detention basin, the emergency spillway, and adjacent properties.
3. 5-year and 100-year HGL's for the storm sewers have been provided for this project.
4. The downstream receiving drainageway for the proposed site will be south through the existing stormwater sewer network to the Colorado River.

D. Waivers from Criteria

No waivers are requested.

IV. DRAINAGE FACILITY PLAN

A. General Concept

1. Storm water runoff from the Glenwood Springs VSF site drains from northeast to southwest, flowing overland before ultimately discharging into the proposed water quality detention basin in the southwest corner of the property. The water quality detention basin discharges to the existing stormwater sewer south of the site where it connects with the Colorado River.
2. Water quality is provided for the site in the form of extended detention basin and has been designed to limit downstream impacts. The design volume for the detention basin has been established to handle the WQCV as well as the volume for a 100 year storm event. The proposed discharge from the water quality detention basin has been analyzed in the post development condition to ensure site overflows will be routed to existing overland flow routes away from existing infrastructure.

B. Specific Details

1. Basin Discussion

There is limited storm water management and no existing ponds on the project site. Storm water has previously drained from the northeast to the southwest and collected in an inlet routing to the stormwater sewer network running along the south of the site.

C. Stormwater Storage Facilities:

Water Quality Pond

As previously mentioned, the site currently has limited form of storm water management. As part of this project, a new water quality detention pond is proposed in the southwest corner of the site. The storm water pond has been designed per CDOT requirements and has been sized to accommodate stormwater flows from the entire tributary area of the site to meet CDOT minimums for WQCV as well as provide capacity for a 100 year storm event.

Flows were routed using the Hydraflow Storm Sewers Extension for AutoCAD and the storm water pond volume was sized using the WQCV general equation provided in Chapter 16 of the CDOT Drainage Design Manual.

Table A-1: Water Quality / Detention Pond Stage Storage

Criteria	Provided Storage (ac-ft)	Max Stage (ft)
Initial Stage	0	5726.23'
Zone 1 (WQCV)	0.067	5728.47'
Zone 2 (100 year)	0.184	5729.51'

Based on results provided by the UD-Detention design tool, the pond meets volume requirements. Please refer to **Appendix C** for complete design calculations.

Due to the proposed conditions of the development resulting in an overall reduction in site imperviousness, detention for the 100 year storm is not provided for this project. Existing impervious hardscape area comprises 75.65% of the site, and the proposed developments will result in a reduction of the impervious area to a total of 71.87%. Refer to the Composite 'C' Factors (Comparison) chart in **Appendix B** for details on areas of imperviousness and site runoff factors.

	% Imperviousness
Existing Condition	75.64
Proposed Condition	71.87

Micropool

The micropool located just outside the control structure at the base of the pond has been designed to hold approximately 1% of the 5 year stormwater volume. The volume of the 5 year storm was determined to be 0.111 ac-ft (4835 cu. ft.), and the micropool holds a total of 48 cu. ft. The CDOT Drainage Design Manual specifies that micropools should not exceed a volume equal to 1% of the 5 year storm intensity.

D. Release Rate Analysis:

Water quality has been provided for this project in the form of a water quality and detention pond with controlled release. The runoff sheet flows across the site to a series of curb cuts that drain to the proposed treatment pond. The treatment pond has been designed utilizing the Mile High Flood District Basin Design workbook to drain the required treatment volume within 40 hours through the use of a controlled outlet structure and orifice plate. The pond is designed to detain the WQCV and limits discharge through the use of a rectangular restrictor plate with circular orifices to control outflow.

Table A-2: Release Rate Analysis

Outflow Controls: Orifice Plate: (3) 0.28 sq. inch holes (9/16" diameter) @ 5726.23', 5726.98', and 5727.72'		
Control Frequency	Peak In Flow (cfs)	Peak Out Flow (cfs)
WQCV	N/A (taken from pond design spreadsheet)	0.0
100-Year	3.9	2.8

Based on the above results, the water quality pond has been designed to meet CDOT requirements of a controlled discharge.

V. CONCLUSIONS

A. Compliance with Standards

The Final Drainage Report for the Glenwood Springs VSF site was prepared in accordance with the 2019 Colorado Department of Transportation (CDOT) Drainage Design Manual.

This Drainage Report has discussed the on-site drainage improvements including water quality treatment and detention as well as off-site discharge.

B. Summary of Concept

1. The site improvements surrounding the proposed and existing buildings are designed to create positive drainage away from the buildings.
2. The storm water drainage system for this site has been designed to safely convey the 100-year storm through the site to the water quality and detention pond. Volume control will be provided by the proposed storm water detention pond.
3. The proposed site improvements will have no adverse effect on the adjacent property and downstream conditions and should mitigate downstream flooding hazards.

VI. LIST OF REFERENCES

Colorado Department of Transportation (CDOT) Drainage Design Manual, 2019

Urban Storm Drainage Criteria Manual; Volumes 1, 2 and 3. Mile High Flood District (latest revision).

Glenwood Engineering Standards (Approved August 2, 2018)

FIRM (Flood Insurance Rate Map), Panel 1432 of 1900; Community Panel Number: 080071 1432 C, revised October 15, 1985.

APPENDIX A

Maps

Site Vicinity Map



VICINITY MAP

SCALE: 1" = 500'



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CDOT GLENWOOD SPRINGS VSF
GLENWOOD SPRINGS, COLORADO
VICINITY MAP

Soils Map

Soil Map—Rifle Area, Colorado, Parts of Garfield and Mesa Counties



Soil Map may not be valid at this scale.

Map Scale: 1:1,810 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey
Packet Page 523

7/8/2020
Page 1 of 3

MAP LEGEND

Area of Interest (AOI)

Soils

-  Soil Survey Areas
-  Soil Map Unit Polygons
-  Soil Map Unit Lines
-  Soil Map Unit Points

Special Point Features

-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features

Water Features

-  Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

-  Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Rifle Area, Colorado, Parts of Garfield and Mesa Counties
Survey Area Data: Version 13, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 24, 2015—Nov 2, 2015

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
8	Atencio-Azeltine complex, 1 to 3 percent slopes	4.1	100.0%
Totals for Area of Interest		4.1	100.0%

Engineering Properties

This table gives the engineering classifications and the range of engineering properties for the layers of each soil in the survey area.

Hydrologic soil group is a group of soils having similar runoff potential under similar storm and cover conditions. The criteria for determining Hydrologic soil group is found in the National Engineering Handbook, Chapter 7 issued May 2007(<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Listing HSGs by soil map unit component and not by soil series is a new concept for the engineers. Past engineering references contained lists of HSGs by soil series. Soil series are continually being defined and redefined, and the list of soil series names changes so frequently as to make the task of maintaining a single national list virtually impossible. Therefore, the criteria is now used to calculate the HSG using the component soil properties and no such national series lists will be maintained. All such references are obsolete and their use should be discontinued. Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonal high water table, saturated hydraulic conductivity after prolonged wetting, and depth to a layer with a very slow water transmission rate. Changes in soil properties caused by land management or climate changes also cause the hydrologic soil group to change. The influence of ground cover is treated independently. There are four hydrologic soil groups, A, B, C, and D, and three dual groups, A/D, B/D, and C/D. In the dual groups, the first letter is for drained areas and the second letter is for undrained areas.

The four hydrologic soil groups are described in the following paragraphs:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Depth to the upper and lower boundaries of each layer is indicated.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."

Classification of the soils is determined according to the Unified soil classification system (ASTM, 2005) and the system adopted by the American Association of State Highway and Transportation Officials (AASHTO, 2004).

The Unified system classifies soils according to properties that affect their use as construction material. Soils are classified according to particle-size distribution of the fraction less than 3 inches in diameter and according to plasticity index, liquid limit, and organic matter content. Sandy and gravelly soils are identified as GW, GP, GM, GC, SW, SP, SM, and SC; silty and clayey soils as ML, CL, OL, MH, CH, and OH; and highly organic soils as PT. Soils exhibiting engineering properties of two groups can have a dual classification, for example, CL-ML.

The AASHTO system classifies soils according to those properties that affect roadway construction and maintenance. In this system, the fraction of a mineral soil that is less than 3 inches in diameter is classified in one of seven groups from A-1 through A-7 on the basis of particle-size distribution, liquid limit, and plasticity index. Soils in group A-1 are coarse grained and low in content of fines (silt and clay). At the other extreme, soils in group A-7 are fine grained. Highly organic soils are classified in group A-8 on the basis of visual inspection.

If laboratory data are available, the A-1, A-2, and A-7 groups are further classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-7-5, or A-7-6. As an additional refinement, the suitability of a soil as subgrade material can be indicated by a group index number. Group index numbers range from 0 for the best subgrade material to 20 or higher for the poorest.

Percentage of rock fragments larger than 10 inches in diameter and 3 to 10 inches in diameter are indicated as a percentage of the total soil on a dry-weight basis. The percentages are estimates determined mainly by converting volume percentage in the field to weight percentage. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Percentage (of soil particles) passing designated sieves is the percentage of the soil fraction less than 3 inches in diameter based on an oven-dry weight. The sieves, numbers 4, 10, 40, and 200 (USA Standard Series), have openings of 4.76, 2.00, 0.420, and 0.074 millimeters, respectively. Estimates are based on laboratory tests of soils sampled in the survey area and in nearby areas and on estimates made in the field. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Liquid limit and plasticity index (Atterberg limits) indicate the plasticity characteristics of a soil. The estimates are based on test data from the survey area or from nearby areas and on field examination. Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

References:

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Report—Engineering Properties

Absence of an entry indicates that the data were not estimated. The asterisk "*" denotes the representative texture; other possible textures follow the dash. The criteria for determining the hydrologic soil group for individual soil components is found in the National Engineering Handbook, Chapter 7 issued May 2007 (<http://directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba>). Three values are provided to identify the expected Low (L), Representative Value (R), and High (H).

Engineering Properties—Rifle Area, Colorado, Parts of Garfield and Mesa Counties														
Map unit symbol and soil name	Pct. of map unit	Hydrologic group	Depth	USDA texture	Classification		Pct Fragments		Percentage passing sieve number—				Liquid limit	Plasticity index
					Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		
			<i>In</i>				<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>	<i>L-R-H</i>
8—Atencio-Azeltine complex, 1 to 3 percent slopes														
Atencio	50	B	0-11	Sandy loam	SC, SC-SM	A-2, A-4	0- 0- 0	0- 0- 0	100-100-100	100-100-100	60-65-70	30-35-40	25-28-30	5-8 -10
			11-23	Gravelly sandy clay loam	GC, GC-GM, SC, SC-SM	A-1, A-2, A-4	0- 5- 10	0- 8- 15	60-70-80	55-65-75	45-58-70	20-30-40	25-28-30	5-8 -10
			23-28	Gravelly sandy loam	GM, SM	A-1, A-2	0- 5- 10	0- 8- 15	60-70-80	55-65-75	35-43-50	20-25-30	25-28-30	NP-3 -5
			28-60	Extremely cobbly sand	GP, GP-GM, SP, SP-SM	A-1	0-15- 30	30-58-85	20-50-80	15-45-75	10-30-50	0- 5- 10	—	NP
Azeltine	45	A	0-18	Gravelly sandy loam	GC, GC-GM, SC, SC-SM	A-1, A-2	0- 5- 10	0- 8- 15	60-70-80	55-65-75	35-43-50	20-25-30	25-28-30	5-8 -10
			18-60	Extremely gravelly sand	GW, GW-GM	A-1	0-15- 30	0-18- 35	15-23-30	10-18-25	5-10- 15	0- 3- 5	—	NP

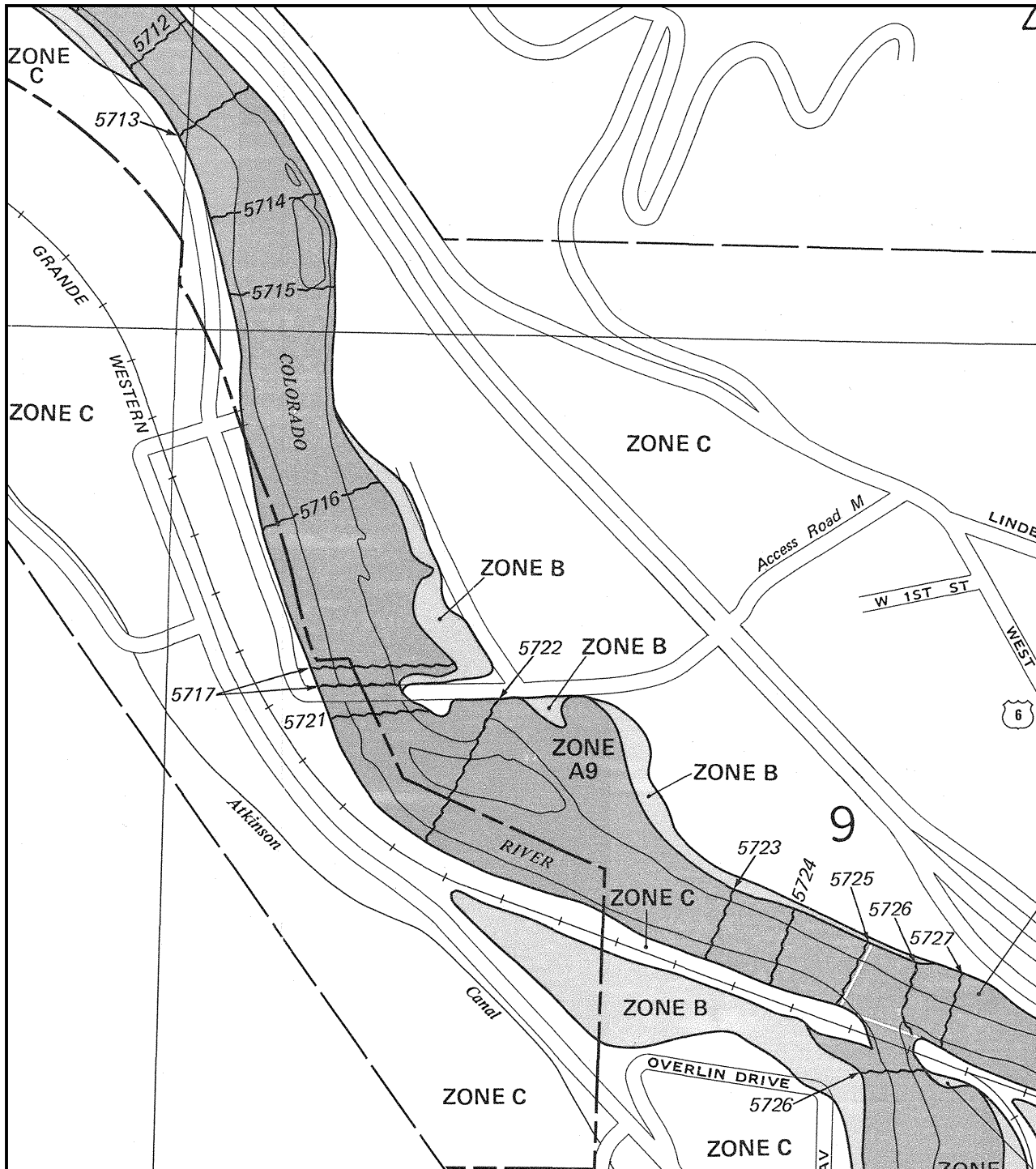
Data Source Information

Soil Survey Area: Rifle Area, Colorado, Parts of Garfield and Mesa Counties

Survey Area Data: Version 13, Jun 5, 2020



FEMA Map



4



APPROXIMATE SCALE IN FEET

500 0 500

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

CITY OF
GLENWOOD SPRINGS,
COLORADO
GARFIELD COUNTY

PANEL 1432 OF 1900
(SEE MAP INDEX FOR PANELS NOT PRINTED)

COMMUNITY-PANEL NUMBER
080071 1432 C

MAP REVISED:
OCTOBER 15, 1985



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

APPENDIX B

Hydrologic Computations

COMPOSITE 'C' FACTORS (DEVELOPED)																											
LOCATION:	Glenwood Springs					Preliminary Drainage Report								BY: NAA		Soil Type: B						DATE:	7/14/2020				
BASIN						GRAVEL				ROOFS				PAVED				LAWNS				COMPOSITE C FACTOR				% IMPERVIOUS	
DESIGNATION	GRVL	ROOFS	PAVED	LAWNS	TOTAL	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR		
				Imperviousness =		40				90				100				2									
A1	0.381	0.582	1.340	0.540	2.84	0.29	0.32	0.38	0.61	0.74	0.76	0.78	0.84	0.84	0.86	86.00	0.89	0.01	0.01	0.07	0.44	0.59	0.61	40.76	0.76	71.30	
O1	0.000	0.000	0.230	0.049	0.28	0.29	0.32	0.38	0.61	0.74	0.76	0.78	0.84	0.84	0.86	86.00	0.89	0.01	0.01	0.07	0.44	0.69	0.71	70.91	0.81	82.79	
																						COMPOSITE I, %				TOTAL A, ac	
																						72.32				3.12	

COMPOSITE 'C' FACTORS (COMPARISON)																											
LOCATION:	Glenwood Springs					Preliminary Drainage Report								BY:	NAA		Soil Type:	D				DATE:	7/14/2020				
BASIN						GRAVEL				ROOFS				PAVED				LAWNS				COMPOSITE C FACTOR				% IMPERVIOUS	
DESIGNATION	GRVL	ROOFS	PAVED	LAWNS	TOTAL	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR	2YR	5 YR	10 YR	100 YR		
				Imperviousness =		40				90				100				5									
Existing DP1	0.600	0.426	1.510	0.304	2.84	0.29	0.32	0.38	0.61	0.74	0.76	0.78	0.84	0.84	0.86	86.00	0.89	0.01	0.01	0.07	0.44	0.62	0.64	45.93	0.78	75.65	
Developed DP1	0.381	0.582	1.340	0.540	2.84	0.29	0.32	0.38	0.61	0.74	0.76	0.78	0.84	0.84	0.86	86.00	0.89	0.01	0.01	0.07	0.44	0.59	0.61	40.76	0.76	71.87	

STANDARD FORM SF-1 TIME OF CONCENTRATION (DEVELOPED)														REMARKS	
LOCATION: Glenwood Springs			Preliminary Drainage Report				BY: NAA			DATE: 7/14/2020				FORMULAS:	
SUB-BASIN DATA			INIT./OVERLAND TIME (Ti)			TRAVEL TIME (Tt)					TOTAL	Tc Check (Urbanized Basins)		FINAL Tc	* $T_i = 0.395 (1.1 - C_5) L^{0.5} / S^{1/3}$ ** $V = C_v (S/100)^{0.5}$
DESIGNATION	C5	AREA (AC)	LENGTH (FT)	SLOPE %	Ti (Min.)*	GRASS/ PAVED	LENGTH (FT)	SLOPE %	VEL. (FPS)**	Tt (Min.)	Ti+Tt (Min.)	LGTH. (FT)	Tc = (L/180) + 10	(minutes)	
A1	0.61	2.84	115	1.50	8.49	PAVED	239	0.50	1.41	2.82	11.31	354.00	11.97	11.31	

STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE, STANDARD FOR SF-2)
DESIGN STORM: 100-YEAR DEVELOPED

Calc. by: NAA

Chk'd by: BMF

Date: 7/14/2020

CDOT Glenwood Springs

Preliminary Drainage Report

Glenwood Springs

DESIGN POINT	DIRECT RUNOFF							TOTAL RUNOFF					REMARKS
	BASIN	AREA (AC)	COEFF. (C)	Tc (Min.)	C*A	I (in./hr.)	Q (cfs)	Sum Area	Tc (min)	I (in./hr.)	Sum C*A	Total Q (cfs)	
2	A1	2.84	0.76	11.31	2.15	6.72	14.45	3.12	11.31	6.72	2.38	15.98	
3	O1	0.28	0.81	0.00	0.23	12.18	2.75	0.28	0.00	12.18	0.23	2.75	

Water Quality Capture Volume (WQCV):

Where: V = Water Quality Capture Volume (acre-feet)

I = developed basin imperviousness (%)

A = tributary area (acres)

= 71.30 %

= 2.84 acres

Water Quality Capture Volume

Volume Formula per UDFCD (Volume 3)

$$V_{WQCV} = (WQCV/12) * A * 1.0$$

Where: V_{WQCV} = storage volume

WQCV = $1.0 * (0.91i^3 - 1.19i^2 + 0.78i)$ (40-hr Drain Time)

A = tributary area (acres)

= 0.28 in

$$V_{WQCV} = 0.0665 \text{ ac-ft}$$

$$= 2,897 \text{ ft}^3$$

Glenwood Springs = 0.28

$$\frac{WQCV \text{ (Other)}}{V_{WQCV \text{ (OTHER)}}} = \frac{0.28 \text{ (WQCV/0.43)}}{0.0433 \text{ ac-ft}}$$

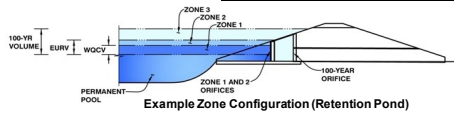
$$= 1,887 \text{ ft}^3$$

APPENDIX C

Hydraulic Computations

MHFD-Detention, Version 4.02 (February 2020)

Basin ID:



Example Zone Configuration (Retention Pond)

Selected BMP Type =	EDB	
Watershed Area =	2.84	acres
Watershed Length =	550	ft
Watershed Length to Centroid =	249	ft
Watershed Slope =	0.015	ft/ft
Watershed Imperviousness =	71.74%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Group C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths = User Input		

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	0.067	acre-feet
Excess Urban Runoff Volume (EURV) =	0.264	acre-feet
2-yr Runoff Volume ($P1 = 0.51$ in.) =	0.029	acre-feet
5-yr Runoff Volume ($P1 = 0.71$ in.) =	0.102	acre-feet
10-yr Runoff Volume ($P1 = 0.86$ in.) =	0.128	acre-feet
25-yr Runoff Volume ($P1 = 1.05$ in.) =	0.167	acre-feet
50-yr Runoff Volume ($P1 = 1.19$ in.) =	0.198	acre-feet
100-yr Runoff Volume ($P1 = 1.32$ in.) =	0.230	acre-feet
500-yr Runoff Volume ($P1 = 1.59$ in.) =	0.292	acre-feet
Approximate 2-yr Detention Volume =	0.075	acre-feet
Approximate 5-yr Detention Volume =	0.111	acre-feet
Approximate 10-yr Detention Volume =	0.144	acre-feet
Approximate 25-yr Detention Volume =	0.166	acre-feet
Approximate 50-yr Detention Volume =	0.174	acre-feet
Approximate 100-yr Detention Volume =	0.184	acre-feet

Zone 1 Volume (WQCV) =	0.067	acre-feet
Zone 2 Volume (100-year - Zone 1) =	0.117	acre-feet
Select Zone 3 Storage Volume (Optional) =		acre-feet
Total Detention Basin Volume =	0.184	acre-feet
Initial Surge Volume (ISV) =	user	ft ³
Initial Surge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	Ht/V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

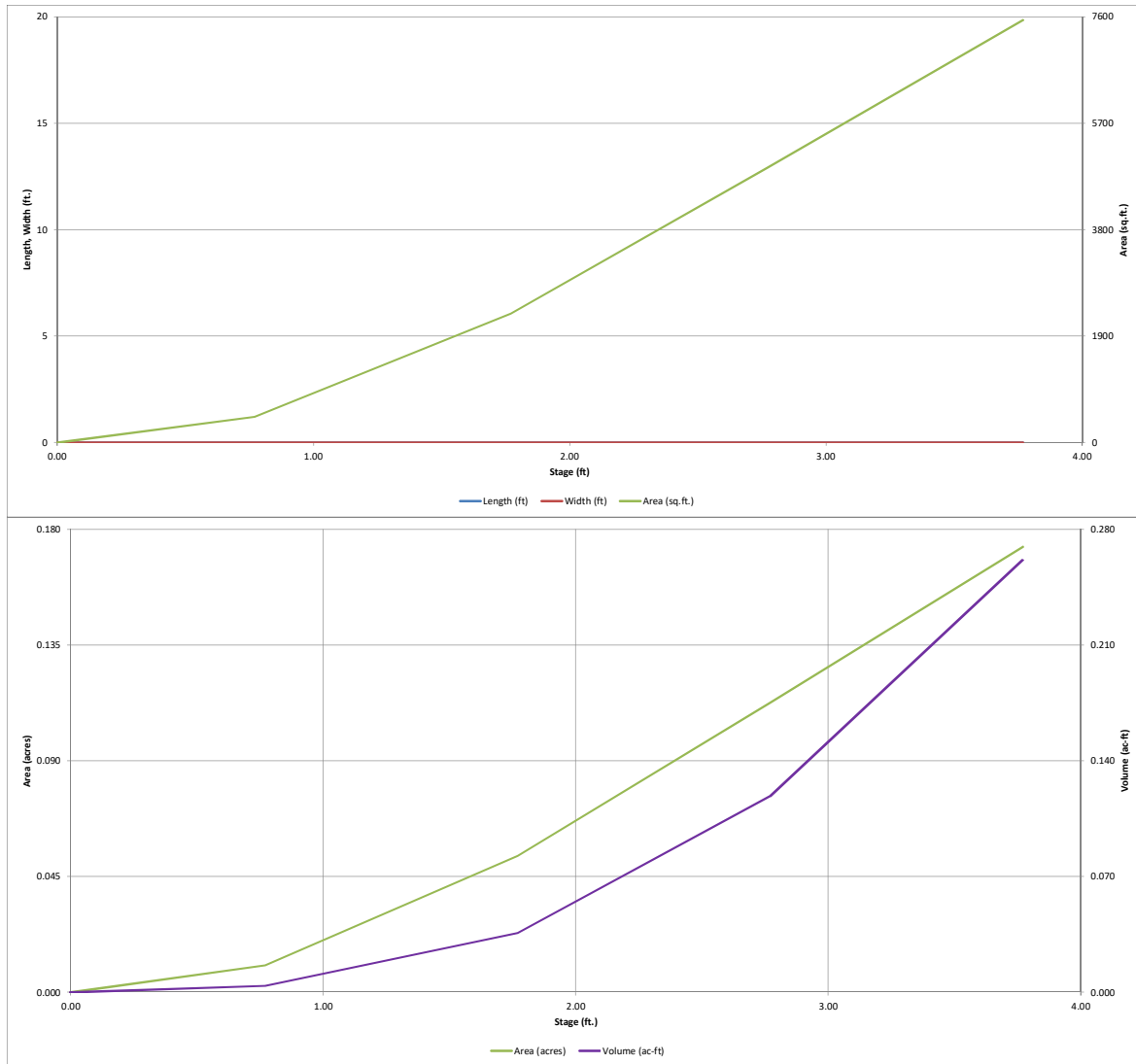
Initial Surcharge Area (A_{SV})	=	user	ft ²
Surcharge Volume Length (L_{SV})	=	user	ft
Surcharge Volume Width (W_{SV})	=	user	ft
Depth of Basin Floor (H_{FLOOR})	=	user	ft
Length of Basin Floor (L_{FLOOR})	=	user	ft
Width of Basin Floor (W_{FLOOR})	=	user	ft
Area of Basin Floor (A_{FLOOR})	=	user	ft ²
Volume of Basin Floor (V_{FLOOR})	=	user	ft ³
Depth of Main Basin (H_{MAIN})	=	user	ft
Length of Main Basin (L_{MAIN})	=	user	ft
Width of Main Basin (W_{MAIN})	=	user	ft
Area of Main Basin (A_{MAIN})	=	user	ft ²
Volume of Main Basin (V_{MAIN})	=	user	ft ³
Calculated Total Basin Volume (V_{TOTAL})	=	user	acre-feet

	acre-feet
	acre-feet
0.51	inches
0.71	inches
0.86	inches
1.05	inches
1.19	inches
1.32	inches
1.59	inches

[illegible]

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.02 (February 2020)

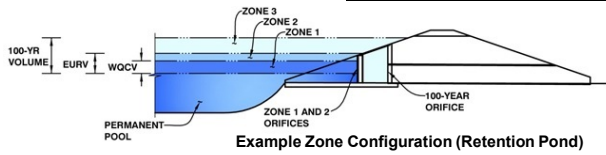


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-DETENTION, Version 4.02 (February 2020)

Project: _____

Basin ID: _____



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.24	0.067	Orifice Plate
Zone 2 (100-year)	3.28	0.117	Weir&Pipe (Circular)
Zone 3			
Total (all zones)		0.184	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = sq. inches (diameter = 9/16 inch)

Calculated Parameters for Plate
WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.75	1.49					
Orifice Area (sq. inches)	0.28	0.28	0.28					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = inches

Calculated Parameters for Vertical Orif
Vertical Orifice Area = ft²
Vertical Orifice Centroid = feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

Overflow Weir Front Edge Height, H_o = ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = feet
Overflow Weir Grate Slope = H:V
Horiz. Length of Weir Sides = feet
Overflow Grate Open Area % = %, grate open area/total area
Debris Clogging % = %

Calculated Parameters for Overflow W
Height of Grate Upper Edge, H_u = feet
Overflow Weir Slope Length = feet
Grate Open Area / 100-yr Orifice Area =
Overflow Grate Open Area w/o Debris = ac-ft
Overflow Grate Open Area w/ Debris = ac-ft

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter = inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pl
Outlet Orifice Area = ft²
Outlet Orifice Centroid = feet
Half-Central Angle of Restrictor Plate on Pipe = degrees

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = feet
Spillway End Slopes = H:V
Freeboard above Max Water Surface = feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres
Basin Volume at Top of Freeboard = acre-ft

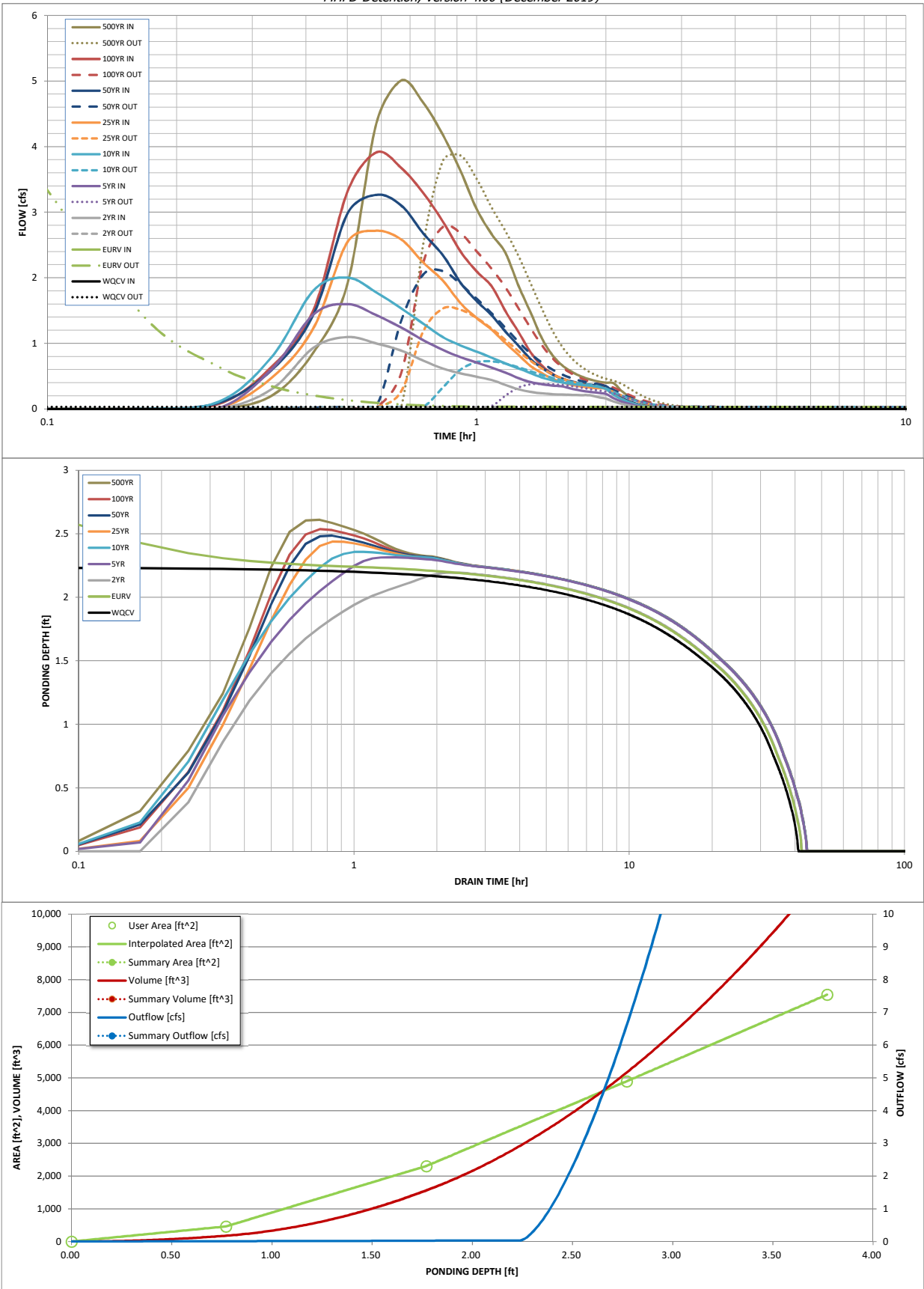
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AI)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	0.51	0.71	0.86	1.05	1.19	1.32
One-Hour Rainfall Depth (in) =	N/A	N/A	0.069	0.102	0.128	0.167	0.198	0.230
CUHP Runoff Volume (acre-ft) =	0.067	0.224	0.069	0.102	0.128	0.167	0.198	0.230
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.069	0.102	0.128	0.167	0.198	0.230
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.0	0.0	0.1	0.4	0.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.00	0.01	0.04	0.14	0.27
Peak Inflow Q (cfs) =	N/A	N/A	1.1	1.6	2.0	2.7	3.3	3.9
Peak Outflow Q (cfs) =	0.0	15.6	0.0	0.4	0.7	1.5	2.1	2.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	34.6	35.3	15.1	5.3	3.6
Structure Controlling Flow =	Overflow Weir 1	Overflow Weir 1	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	36	33	38	38	37	35	34	33
Time to Drain 99% of Inflow Volume (hours) =	39	38	40	41	41	40	40	39
Maximum Ponding Depth (ft) =	2.24	3.55	2.19	2.31	2.36	2.44	2.49	2.54
Area at Maximum Ponding Depth (acres) =	0.08	0.16	0.08	0.09	0.09	0.09	0.10	0.10
Maximum Volume Stored (acre-ft) =	0.067	0.225	0.063	0.073	0.077	0.084	0.088	0.093

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Depotion, Version 4.00 (December 2019)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:15:00	0.00	0.00	0.00	0.01	0.11	0.02	0.09	0.08	0.21
	0:20:00	0.00	0.00	0.30	0.61	0.80	0.48	0.60	0.65	0.87
	0:25:00	0.00	0.00	0.94	1.44	1.81	1.24	1.45	1.52	1.92
	0:30:00	0.00	0.00	1.10	1.60	2.00	2.55	2.98	3.32	4.35
	0:35:00	0.00	0.00	1.00	1.43	1.78	2.72	3.27	3.91	5.01
	0:40:00	0.00	0.00	0.89	1.24	1.53	2.58	3.10	3.68	4.67
	0:45:00	0.00	0.00	0.74	1.05	1.30	2.23	2.68	3.30	4.18
	0:50:00	0.00	0.00	0.62	0.90	1.10	1.96	2.35	2.86	3.63
	0:55:00	0.00	0.00	0.54	0.79	0.97	1.61	1.93	2.40	3.05
	1:00:00	0.00	0.00	0.49	0.71	0.87	1.39	1.64	2.10	2.66
	1:05:00	0.00	0.00	0.44	0.63	0.78	1.21	1.43	1.87	2.38
	1:10:00	0.00	0.00	0.37	0.55	0.69	1.00	1.18	1.49	1.88
	1:15:00	0.00	0.00	0.30	0.47	0.62	0.83	0.97	1.18	1.48
	1:20:00	0.00	0.00	0.26	0.41	0.55	0.66	0.76	0.87	1.08
	1:25:00	0.00	0.00	0.24	0.38	0.48	0.55	0.63	0.66	0.82
	1:30:00	0.00	0.00	0.23	0.36	0.44	0.46	0.53	0.54	0.66
	1:35:00	0.00	0.00	0.22	0.34	0.40	0.41	0.46	0.46	0.56
	1:40:00	0.00	0.00	0.22	0.31	0.38	0.37	0.42	0.41	0.50
	1:45:00	0.00	0.00	0.21	0.28	0.37	0.34	0.39	0.37	0.45
	1:50:00	0.00	0.00	0.21	0.26	0.35	0.33	0.37	0.35	0.42
	1:55:00	0.00	0.00	0.18	0.25	0.34	0.32	0.36	0.33	0.40
	2:00:00	0.00	0.00	0.16	0.23	0.30	0.31	0.35	0.33	0.39
	2:05:00	0.00	0.00	0.11	0.16	0.22	0.22	0.25	0.24	0.28
	2:10:00	0.00	0.00	0.08	0.12	0.15	0.16	0.18	0.17	0.20
	2:15:00	0.00	0.00	0.06	0.08	0.11	0.11	0.13	0.12	0.14
	2:20:00	0.00	0.00	0.04	0.06	0.07	0.08	0.09	0.08	0.10
	2:25:00	0.00	0.00	0.03	0.04	0.05	0.05	0.06	0.06	0.07
	2:30:00	0.00	0.00	0.02	0.02	0.03	0.04	0.04	0.04	0.04
	2:35:00	0.00	0.00	0.01	0.02	0.02	0.02	0.02	0.02	0.03
	2:40:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.01
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.02 (February 2020)

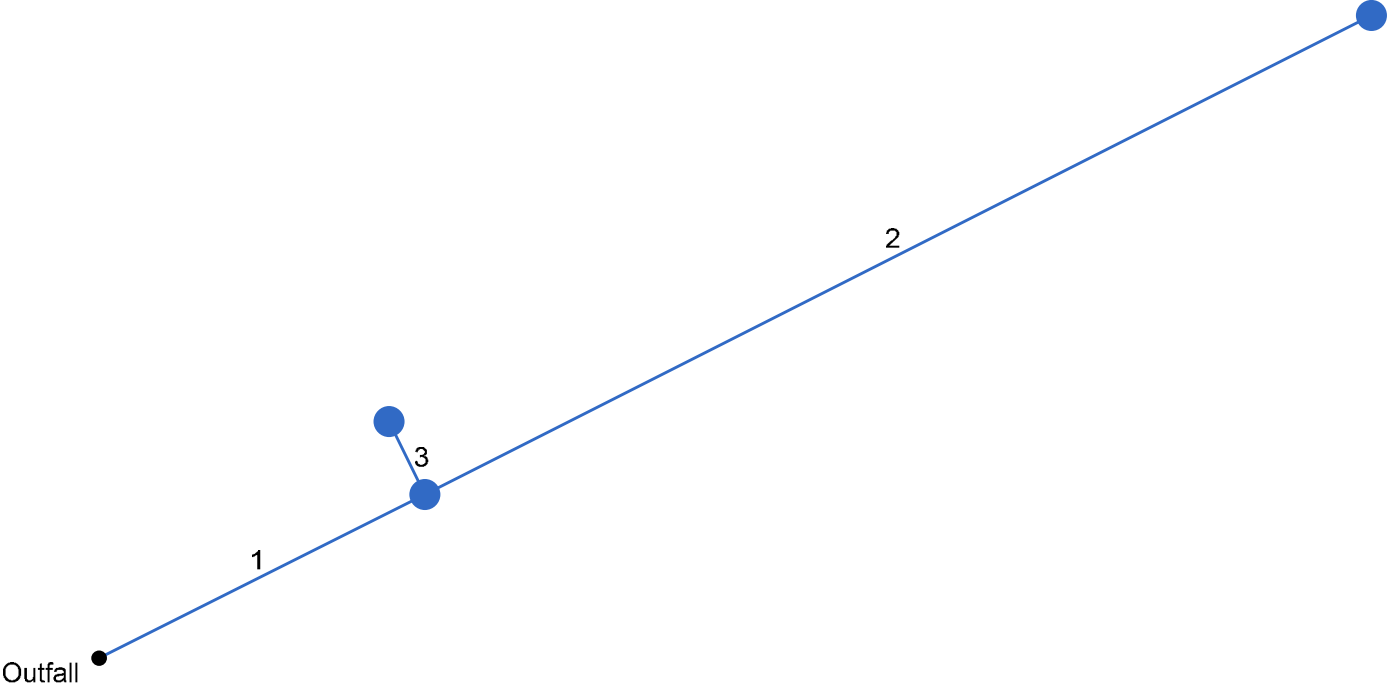
Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

[illegible]

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



CDOT Glenwood Springs VSF
5 Year Storm Analysis

Project File: New.stm	Number of lines: 3	Date: 7/14/2020
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Storm Sewer Summary Report

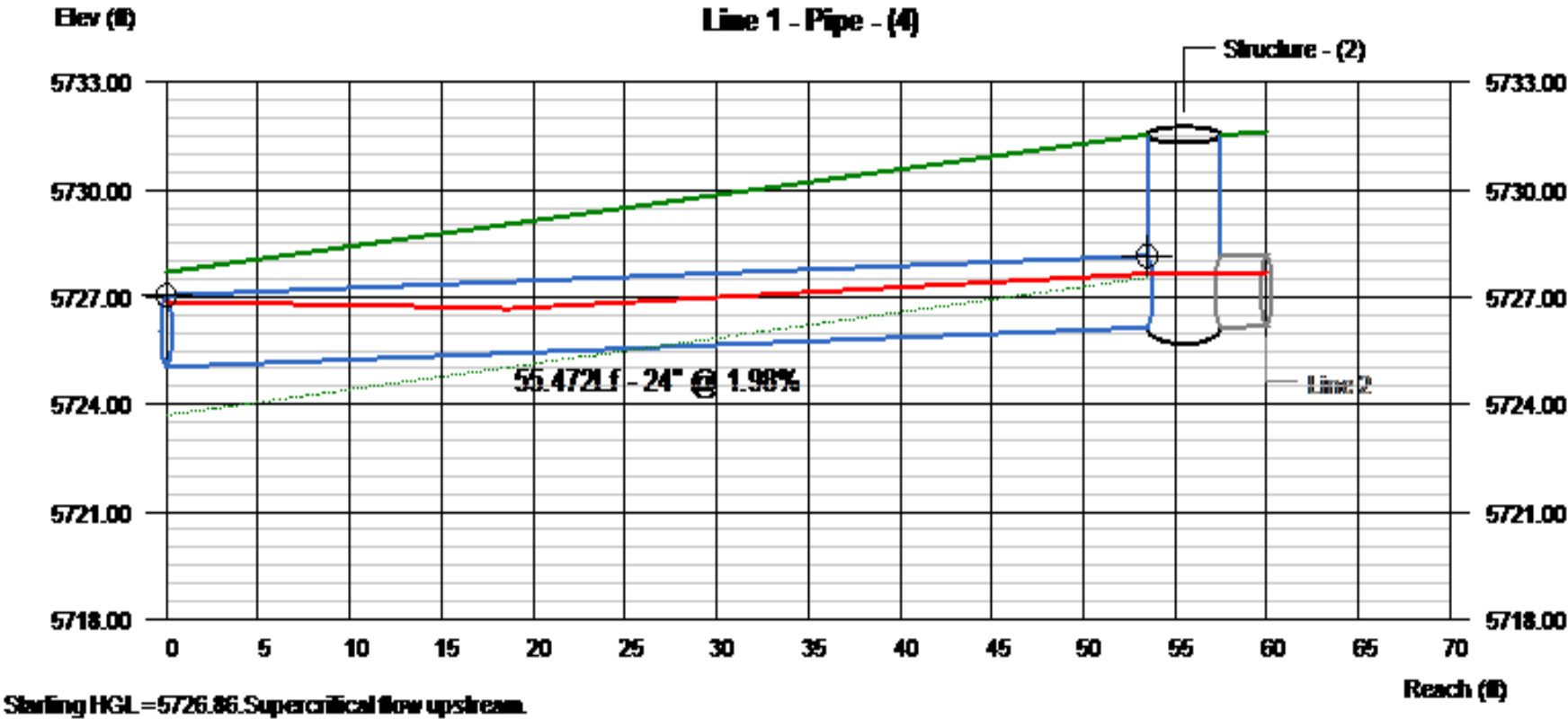
Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1	Pipe - (4)	17.48	24	Cir	55.472	5725.05	5726.15	1.983	5726.86	5727.66	n/a	5727.66 j	End	Manhole
2	Pipe - (3)	6.70	24	Cir	161.611	5726.15	5729.00	1.764	5727.66	5729.92	n/a	5729.92 j	1	Manhole
3	Pipe - (1)	11.23	24	Cir	14.596	5726.15	5726.23	0.549	5727.66	5727.43	n/a	5727.43	1	Manhole
Project File: New.stm									Number of lines: 3			Run Date: 7/14/2020		
NOTES: Return period = 5 Yrs. ; j - Line contains hyd. jump.														

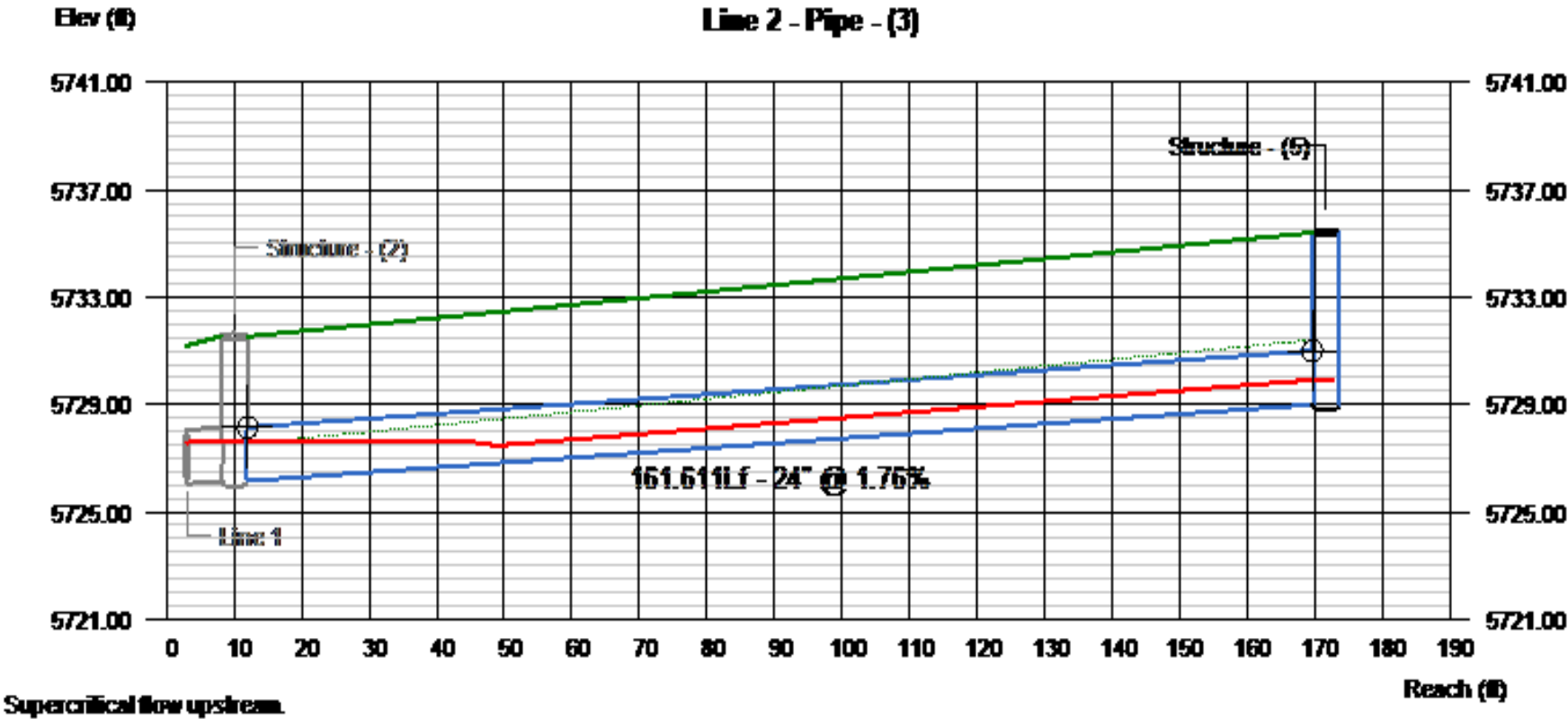
Storm Sewer Tabulation

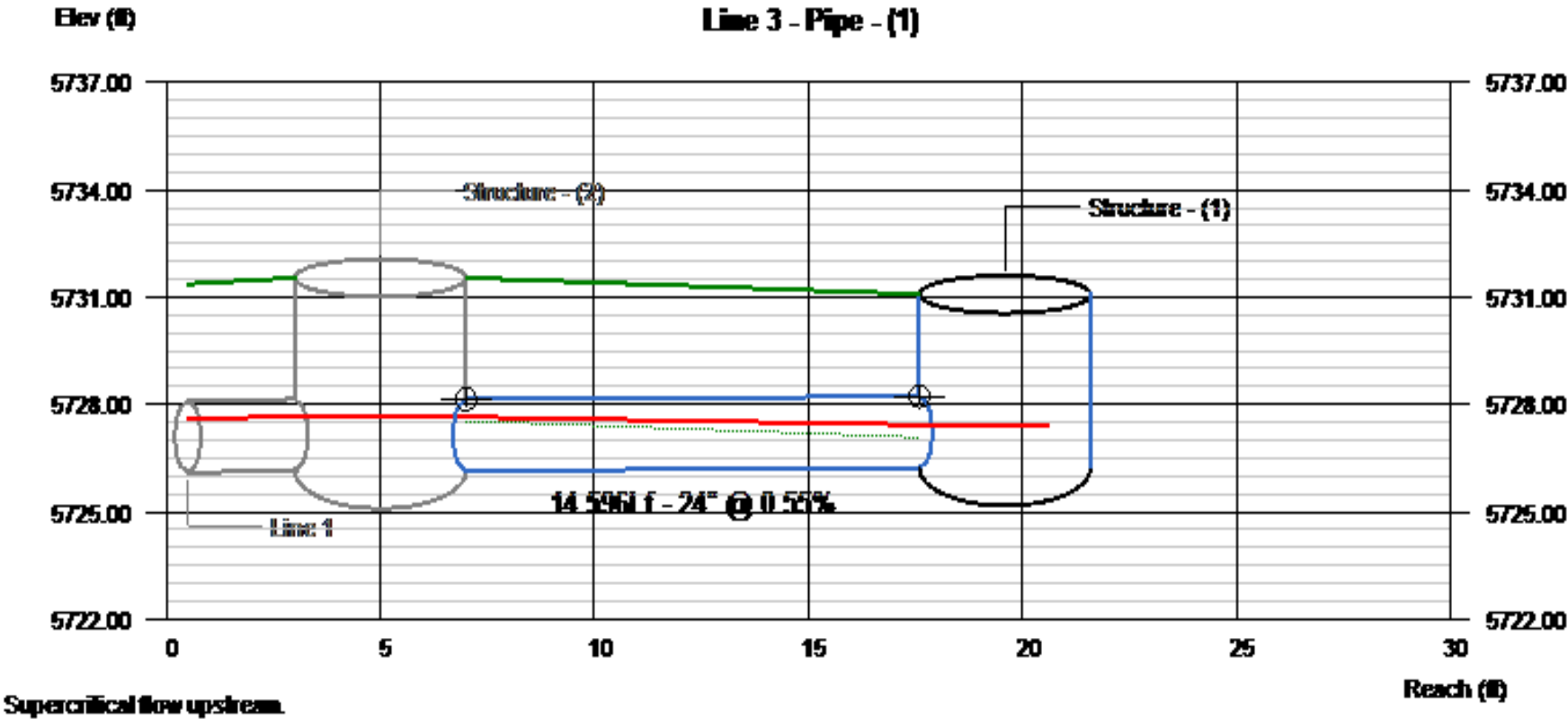
Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (l)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Dn	Up	Dn	Up	Dn	Up	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	55.472	0.00	4.43	0.00	0.00	3.37	0.0	11.4	5.2	17.48	31.85	6.37	24	1.98	5725.05	5726.15	5726.86	5727.66	5727.70	5731.54	Pipe - (4)
2	1	161.611	1.59	1.59	0.76	1.21	1.21	9.4	9.4	5.5	6.70	30.04	3.71	24	1.76	5726.15	5729.00	5727.66	5729.92	5731.54	5735.40	Pipe - (3)
3	1	14.596	2.84	2.84	0.76	2.16	2.16	11.3	11.3	5.2	11.23	16.75	5.06	24	0.55	5726.15	5726.23	5727.66	5727.43	5731.54	5731.08	Pipe - (1)
Project File: New.stm																Number of lines: 3				Run Date: 7/14/2020		
NOTES:Intensity = 79.26 / (Inlet time + 14.60) ^ 0.84; Return period =Yrs. 5 ; c = cir e = ellip b = box																						

Hydraulic Grade Line Computations

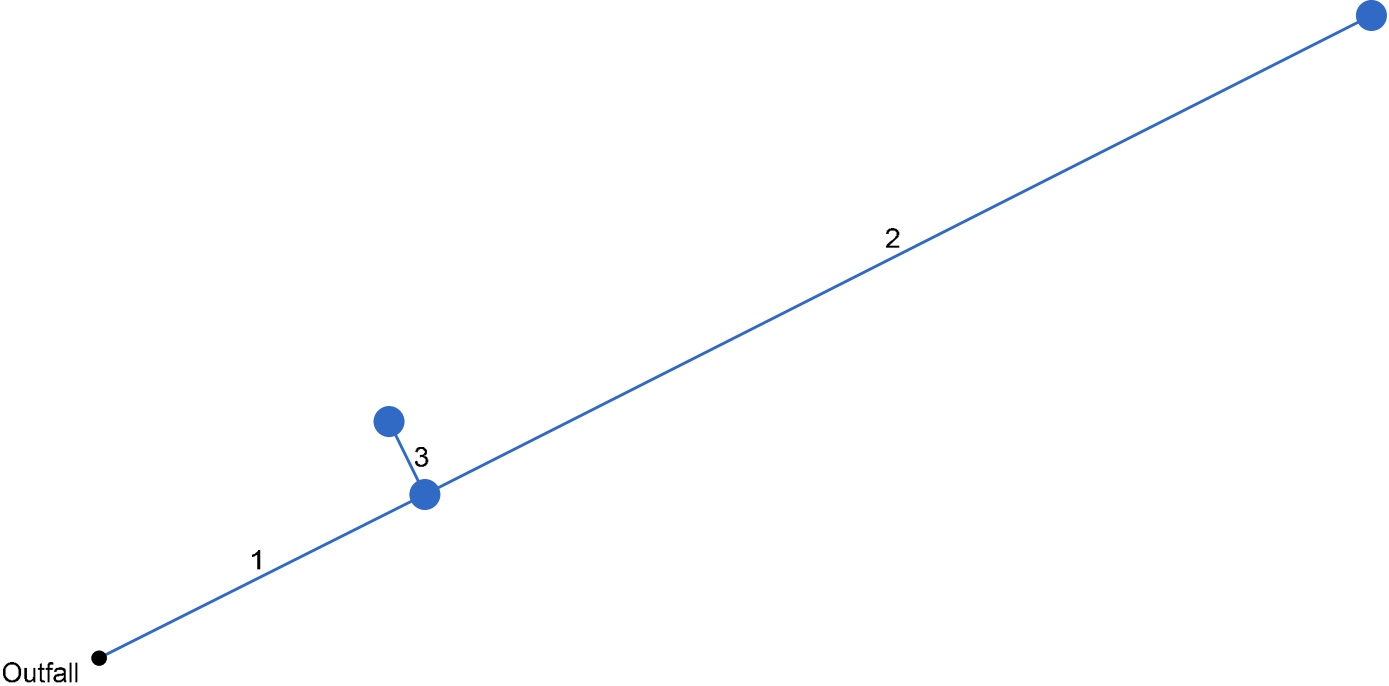
Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
(in)	(cfs)										(ft)											(K)	(ft)
1	24	17.48	5725.05	5726.86	1.81	2.54	5.85	0.74	5727.60	0.000	55.472	5726.15	5727.66 j	1.51**	2.54	6.89	0.74	5728.39	0.000	0.000	n/a	0.98	0.72
2	24	6.70	5726.15	5727.66	1.51	1.40	2.64	0.35	5728.01	0.000	161.61	5729.00	5729.92 j	0.92**	1.40	4.77	0.35	5730.27	0.000	0.000	n/a	1.00	0.35
3	24	11.23	5726.15	5727.66	1.51	1.97	4.43	0.51	5728.16	0.000	14.596	5726.23	5727.43	1.20**	1.97	5.70	0.51	5727.94	0.000	0.000	n/a	1.00	n/a
Project File: New.stm														Number of lines: 3					Run Date: 7/14/2020				
Notes: ; ** Critical depth.; j-Line contains hyd. jump ; c = cir e = ellip b = box																							







Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



CDOT Glenwood Springs VSF
100 Year Storm Analysis

Project File: New.stm	Number of lines: 3	Date: 7/14/2020
-----------------------	--------------------	-----------------

Storm Sewer Summary Report

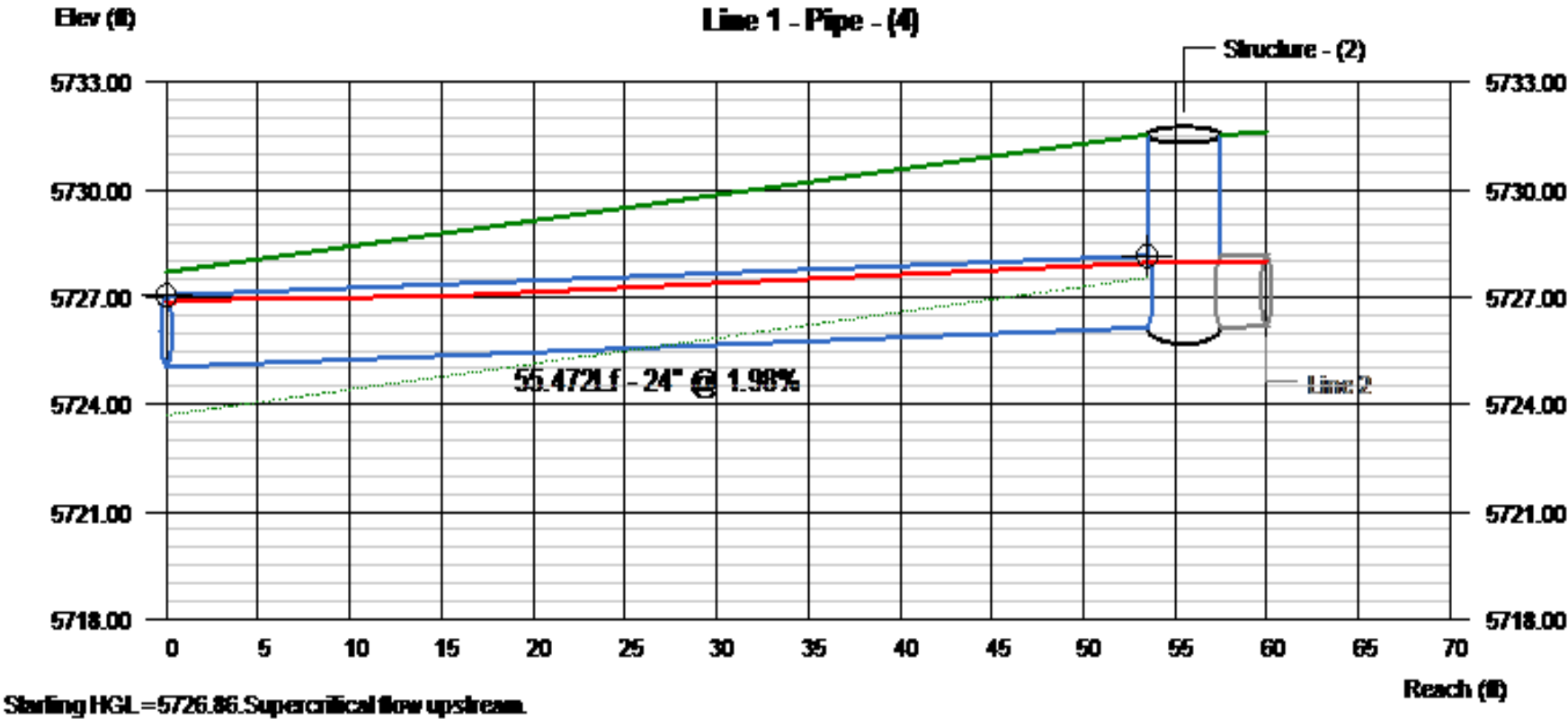
Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1	Pipe - (4)	27.08	24	Cir	55.472	5725.05	5726.15	1.983	5726.86	5727.96	n/a	5727.96 j	End	Manhole
2	Pipe - (3)	10.28	24	Cir	161.611	5726.15	5729.00	1.764	5727.96	5730.15	n/a	5730.15 j	1	Manhole
3	Pipe - (1)	17.38	24	Cir	14.596	5726.15	5726.23	0.549	5727.96	5728.03	0.53	5728.56	1	Manhole
Project File: New.stm									Number of lines: 3			Run Date: 7/14/2020		
NOTES: Return period = 100 Yrs. ; j - Line contains hyd. jump.														

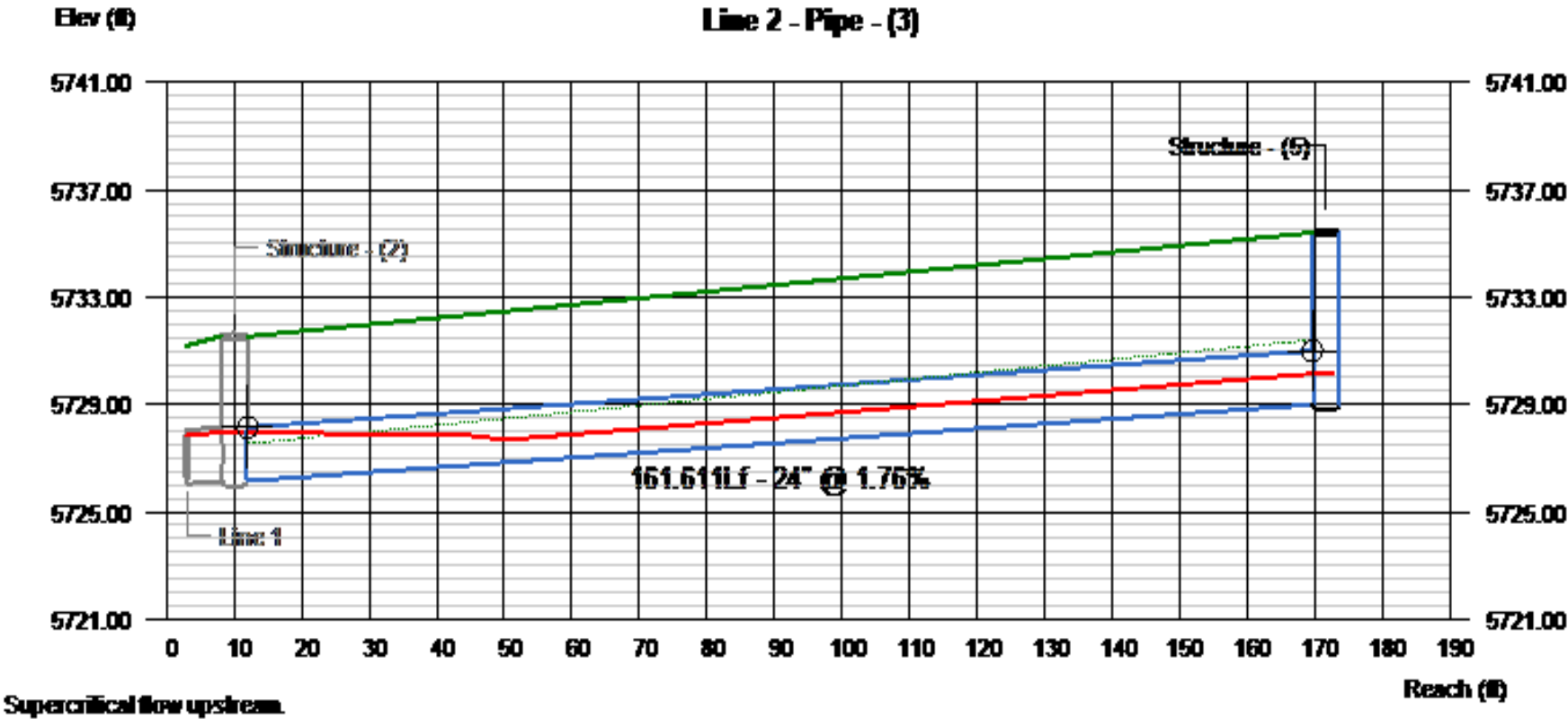
Storm Sewer Tabulation

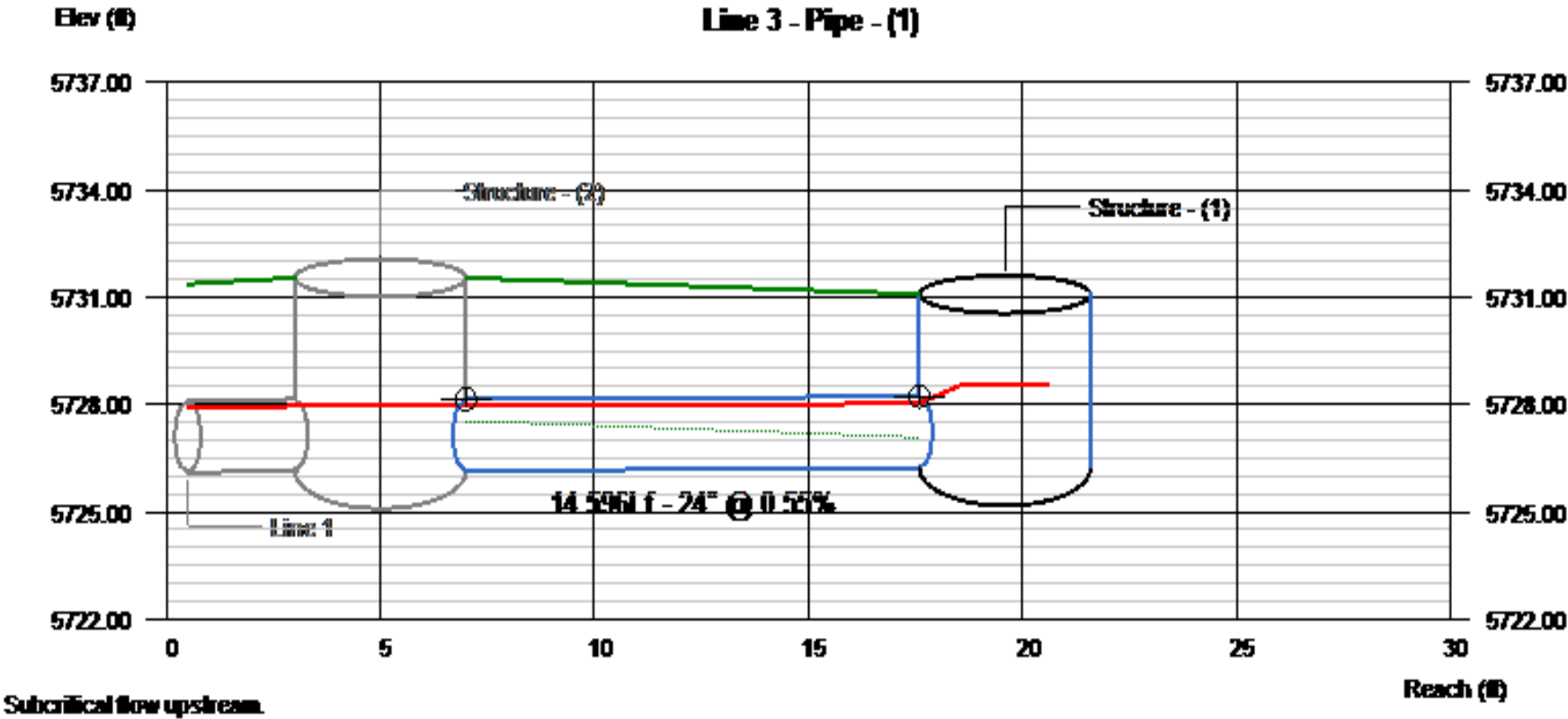
Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr (min)	Total (min)	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
1	End	55.472	0.00	4.43	0.00	0.00	3.37	0.0	11.3	8.0	27.08	31.85	9.06	24	1.98	5725.05	5726.15	5726.86	5727.96	5727.70	5731.54	Pipe - (4)
2	1	161.611	1.59	1.59	0.76	1.21	1.21	9.4	9.4	8.5	10.28	30.04	4.48	24	1.76	5726.15	5729.00	5727.96	5730.15	5731.54	5735.40	Pipe - (3)
3	1	14.596	2.84	2.84	0.76	2.16	2.16	11.3	11.3	8.1	17.38	16.75	5.82	24	0.55	5726.15	5726.23	5727.96	5728.03	5731.54	5731.08	Pipe - (1)
Project File: New.stm																Number of lines: 3				Run Date: 7/14/2020		
NOTES:Intensity = 127.16 / (Inlet time + 17.80) ^ 0.82; Return period =Yrs. 100 ; c = cir e = ellip b = box																						

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	24	27.08	5725.05	5726.86	1.81	2.99	9.06	1.28	5728.14	0.000	55.472	5726.15	5727.96 j	1.81**	2.99	9.06	1.28	5729.24	0.000	0.000	n/a	0.98	n/a
2	24	10.28	5726.15	5727.96	1.81	1.86	3.44	0.47	5728.43	0.000	161.61	5729.00	5730.15 j	1.15**	1.86	5.52	0.47	5730.62	0.000	0.000	n/a	1.00	n/a
3	24	17.38	5726.15	5727.96	1.81	2.99	5.82	0.53	5728.48	0.518	14.596	5726.23	5728.03	1.80	2.98	5.83	0.53	5728.56	0.519	0.519	0.076	1.00	0.53
Project File: New.stm														Number of lines: 3					Run Date: 7/14/2020				
Notes: ; ** Critical depth.; j-Line contains hyd. jump ; c = cir e = ellip b = box																							

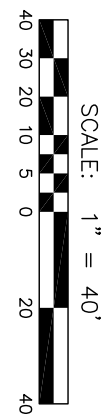
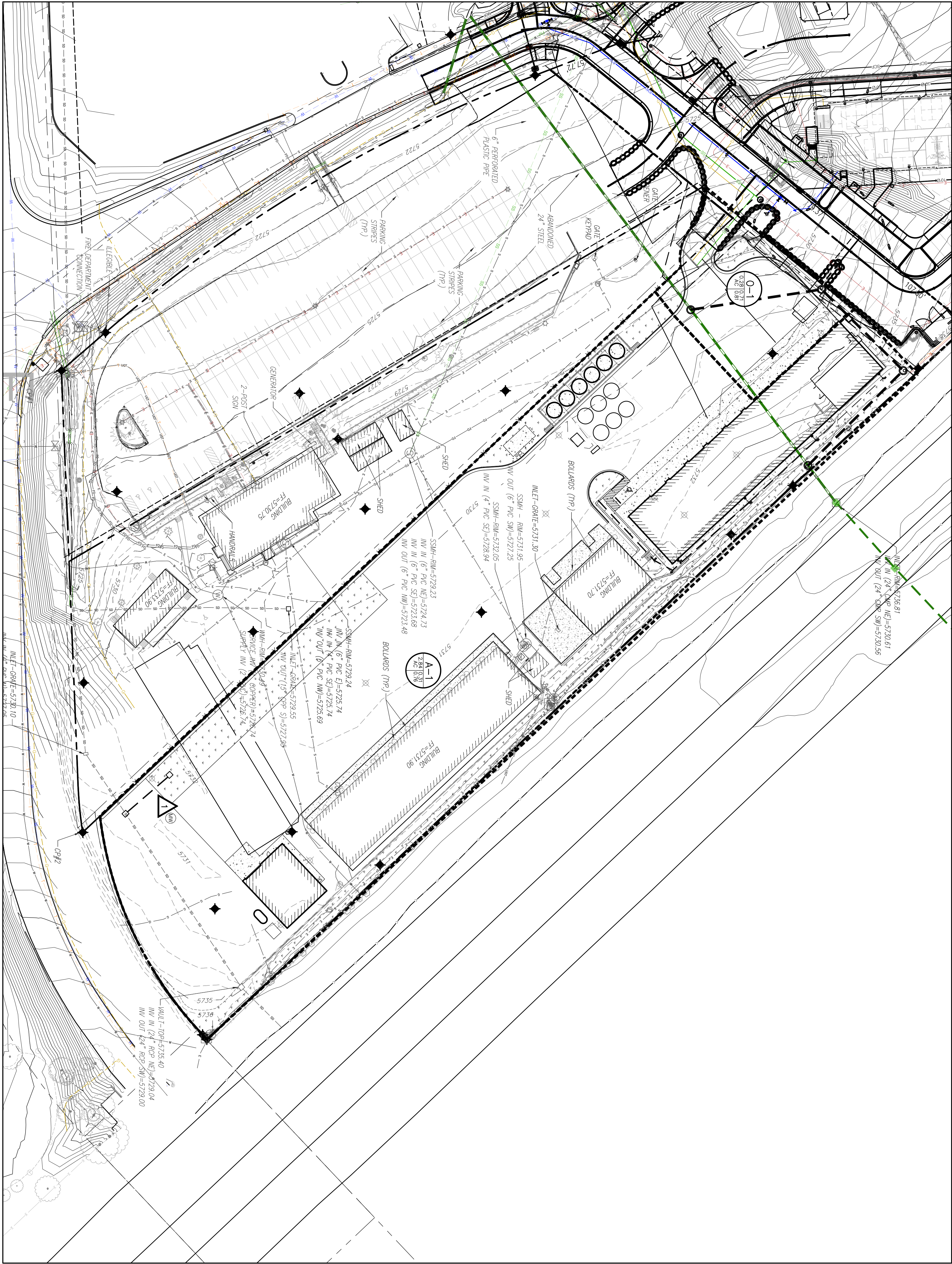






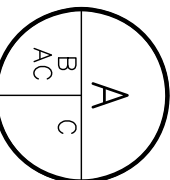
APPENDIX D

Final Drainage Plan

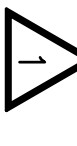


LEGEND

A = BASIN DESIGNATION
B = AREA IN ACRES
C = 5-YR "C"
100-YR "C"



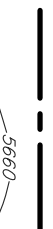
DESIGN POINT DESIGNATION



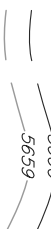
PROPOSED BASIN BOUNDARY



PROPERTY LINE



EXISTING CONTOURS



FLOW ARROW



COLLINS ENGINEERS INC.

455 Sherman Street, Suite 160
Denver, Colorado 80203
voice: (303) 447-0080
fax: (303) 447-9141
web: collinsengr.com



Know what's below.
Call before you dig.

REV. NO.	DATE	REVISIONS DESCRIPTION
1		
2		
3		
4		
5		
6		
7		

2020 REVISIONS

PROPOSED DRAINAGE MAP

CDOT GLENWOOD SPRINGS VSF
CONSTRUCTION PLANS

THORNTON, CO

C-P 1.0

drawn by: NAA
checked by: BMF
approved by: BMF
CA/OC by: 12383-25-27
Project No.: 12383-25-27
Drawing No.: 07/14/2020
date:

**Engineering
Department**

**Ryan Johnson
Asst. City
Engineer**

Date: September 18, 2020

To: Community Development

Re: 202 Centennial CDOT Facility - Location and Extent Review

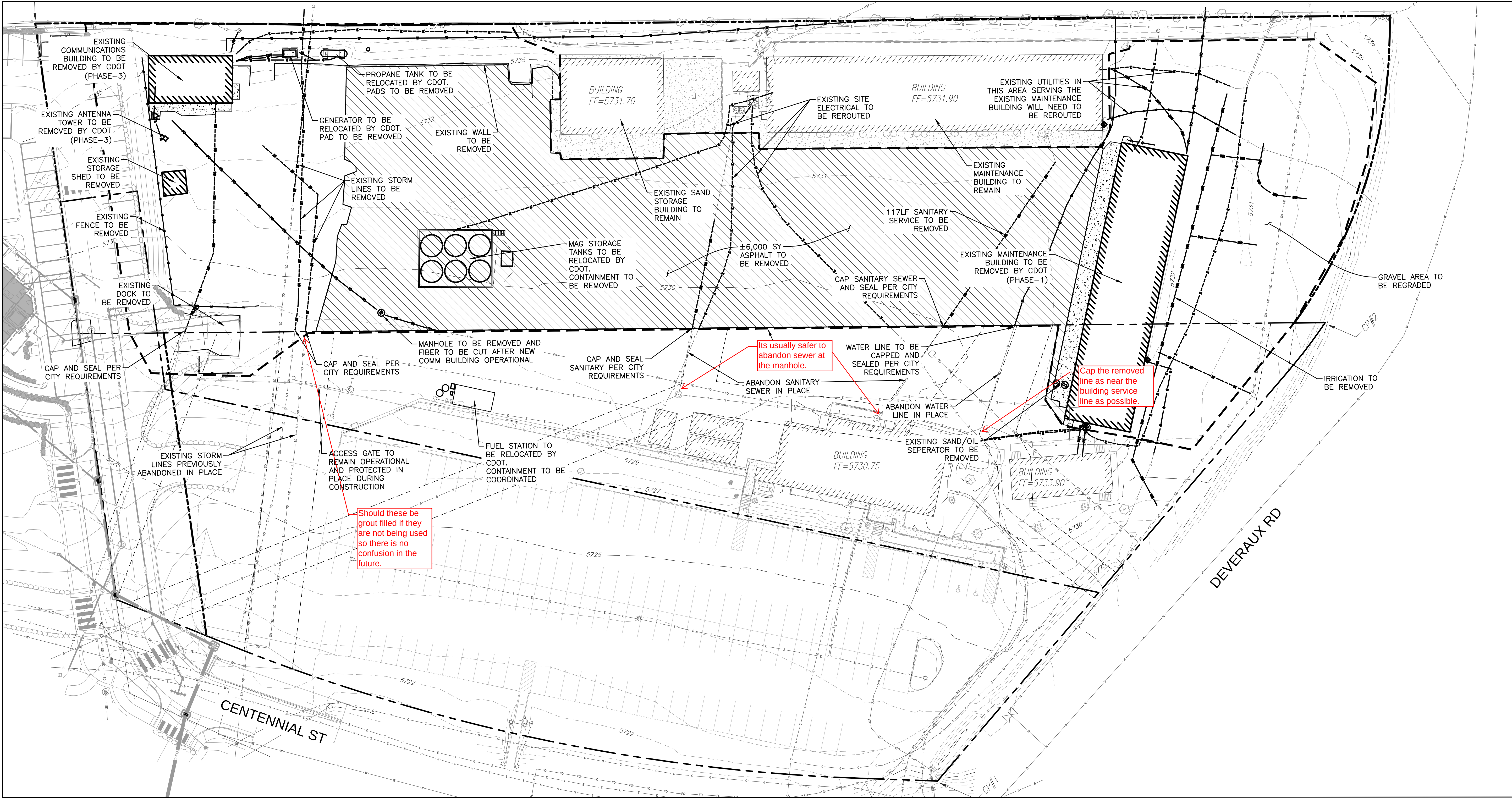
Please see the following engineering comments for the proposed redevelopment.

Comments to be addressed prior to Grading Permit Issuance:

1. A grading permit will be required for this project.
2. Provide a drainage letter for the proposed improvements to verify compliance with the Master Drainage Report completed by Boundaries Unlimited.
 - a. Include descriptions of compliance with assumed impervious areas and connection points to the caverns village/centennial street system.
 - b. Report should also address the storm sewer reroute around proposed VSP building.
 - i. Does this reduction in slope cause capacity issues for I-70 drainage and properties to the north (if any)?
3. The stormwater connection to the offsite inlet will require a drainage easement granted from adjacent property owners.
4. Tracer wire should be included on all underground utilities.

At the completion of construction provide:

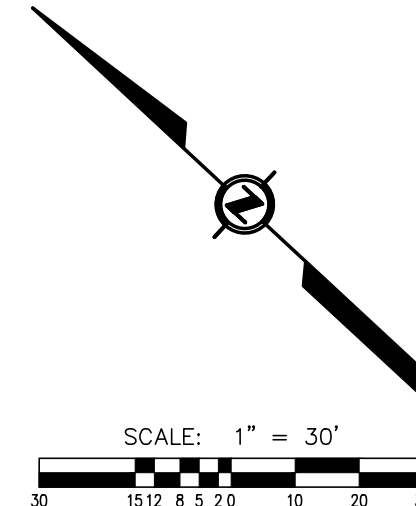
5. Stamped certification letter from the engineer for the improvements made to the property.
6. Maintenance plan and agreement for any drainage improvements.
7. As-Built Survey with Improvement Survey Plat. AutoCAD with PDF's can be submitted initially. Once approved, a Final Mylar copy shall be provided.



EXISTING TO REMAIN		LEGEND		TO BE REMOVED	
---	---	PROPERTY LINE		---	
---	---	LOT LINE		---	
---	---	EASEMENT		---	
---	---	REMOVAL LIMITS		---	
---	---	BUILDINGS		---	
---	---	CONTOURS		---	
---	---	CONCRETE		---	
---	---	ASPHALT		---	
---	---	STORM SEWER		---	
---	---	SANITARY SEWER		---	
---	---	WATER		---	
---	---	IRRIGATION		---	
---	---	ELECTRICAL		---	
---	---	FIBER/COM		---	

NOTES:
UTILITIES TO BE ABANDONED IN PLACE WILL BE APPROVED BY THE CITY.
CITY OF GLENWOOD SPRINGS IS TO DISCONNECT ALL WATER AND SANITARY MAINS AND SERVICES UNLESS CONTRACTOR IS PRE-APPROVED BY THE CITY.

EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.



Colorado Department of Transportation

PROPERTY MANAGEMENT
2829 W. HOWARD PL., FL4
DENVER, CO 80204
Phone: 303-757-9011 FAX: 303-512-5550

DEMOLITION PLAN

Glenwood Springs VSF
202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

COLLINS ENGINEERS

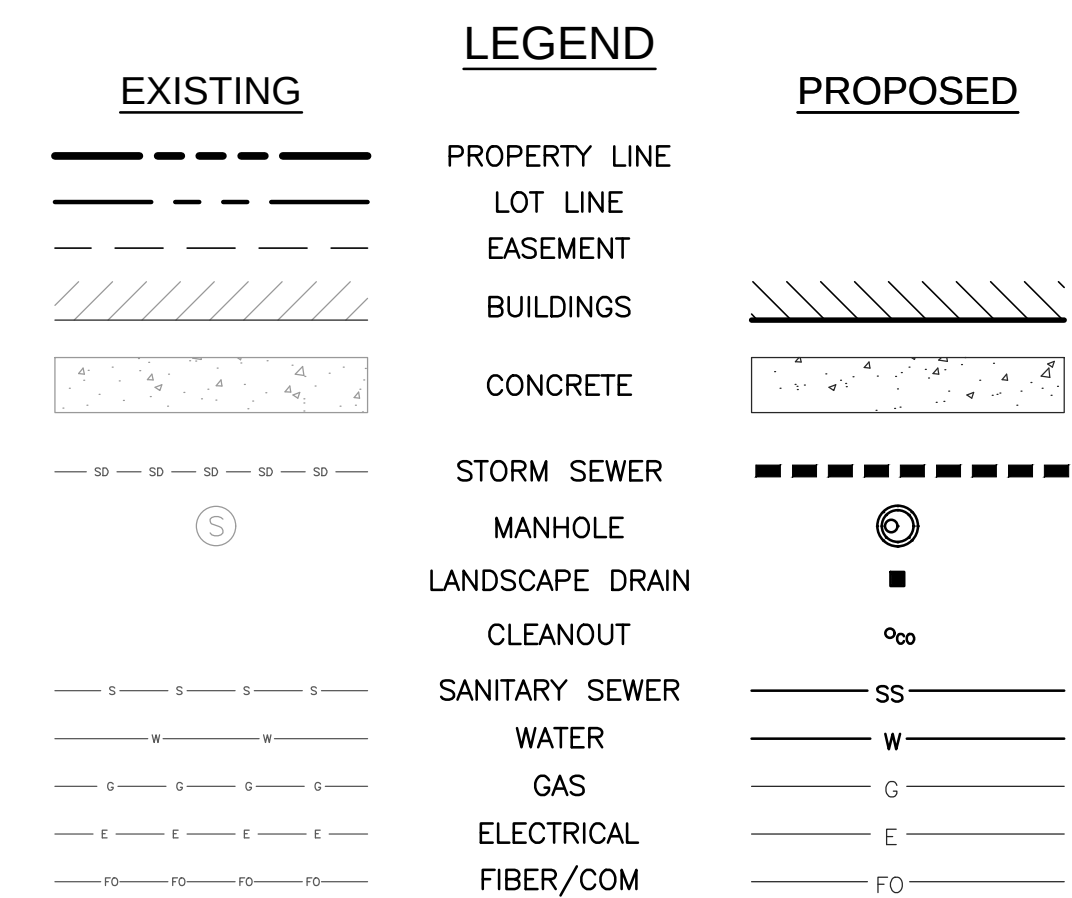
455 Sherman Street, Suite 100
Denver, Colorado 80203
Phone: (303) 447-0000
Fax: (303) 447-9141
www.collinsengr.com

DATE	NO.	DESCRIPTION
06/12/2020	1	65% CONSTRUCTION DOCS
08/06/2020	2	85% CONSTRUCTION DOCS

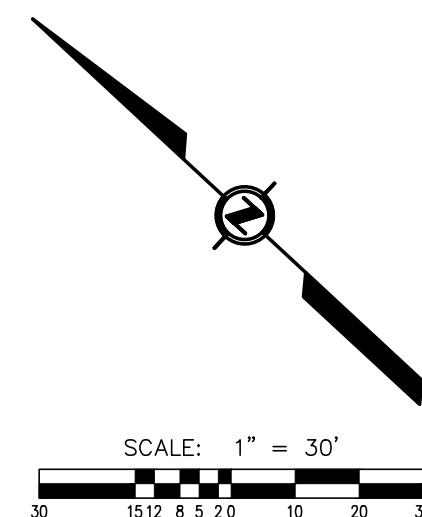
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CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

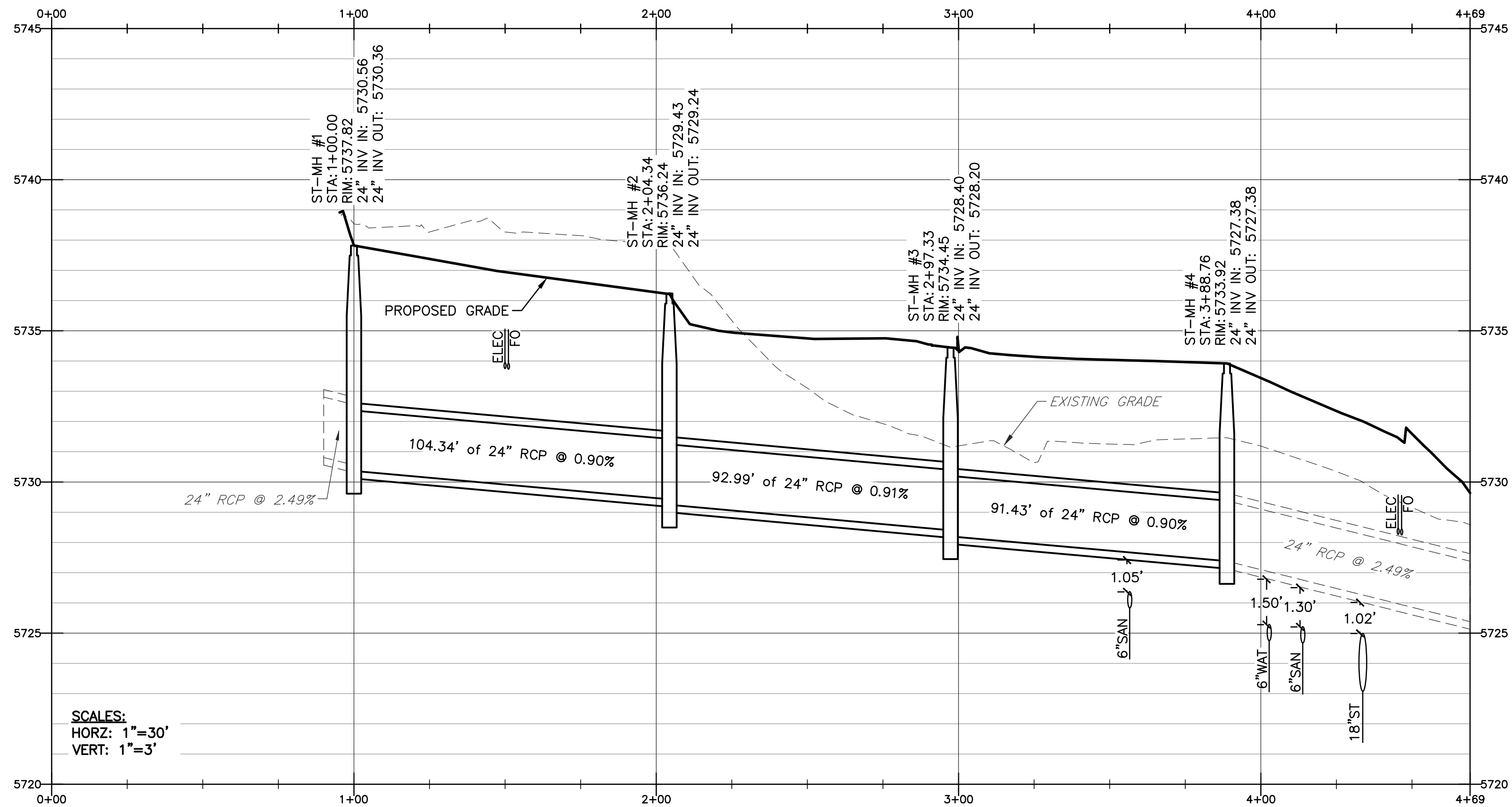
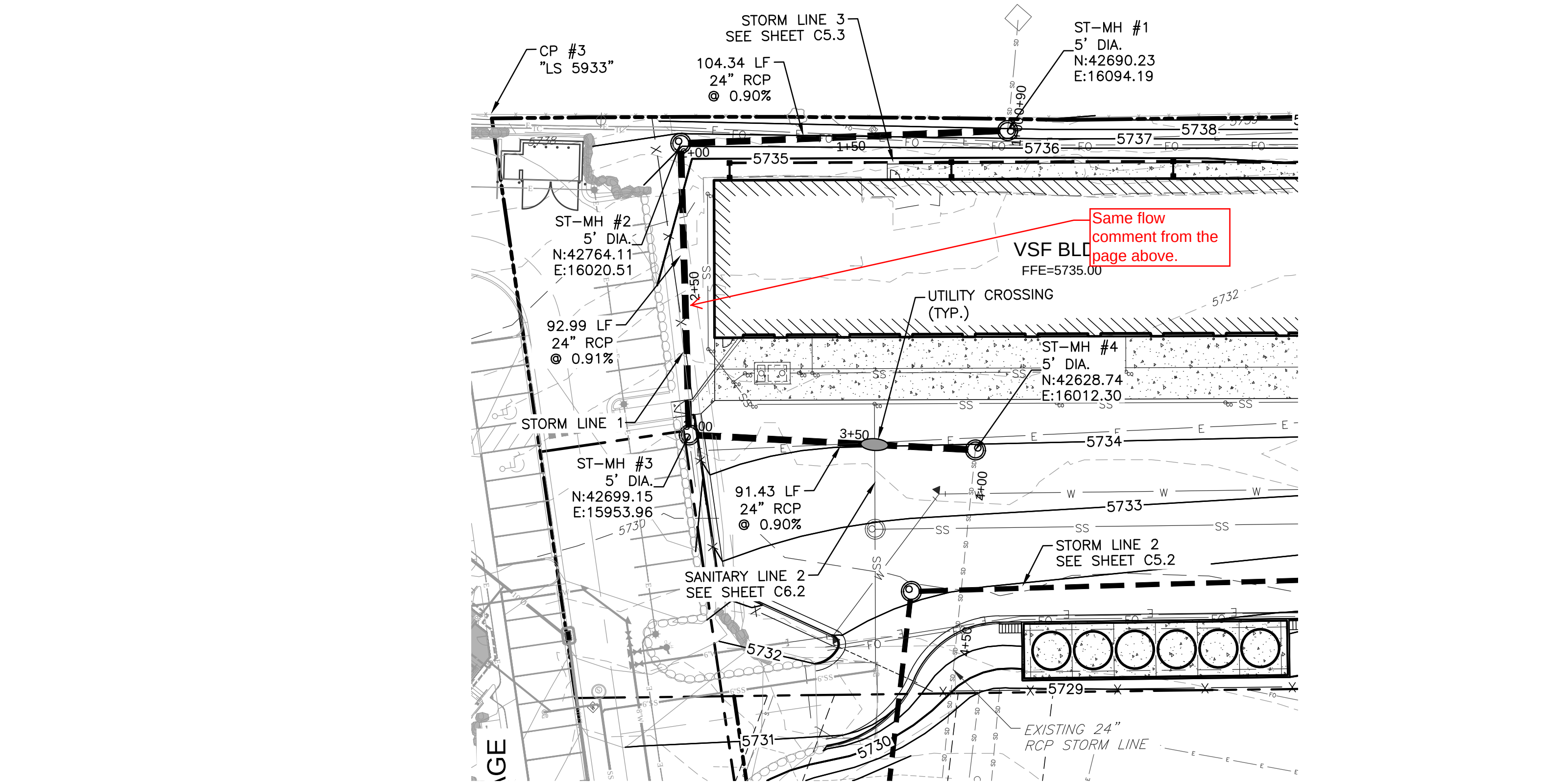
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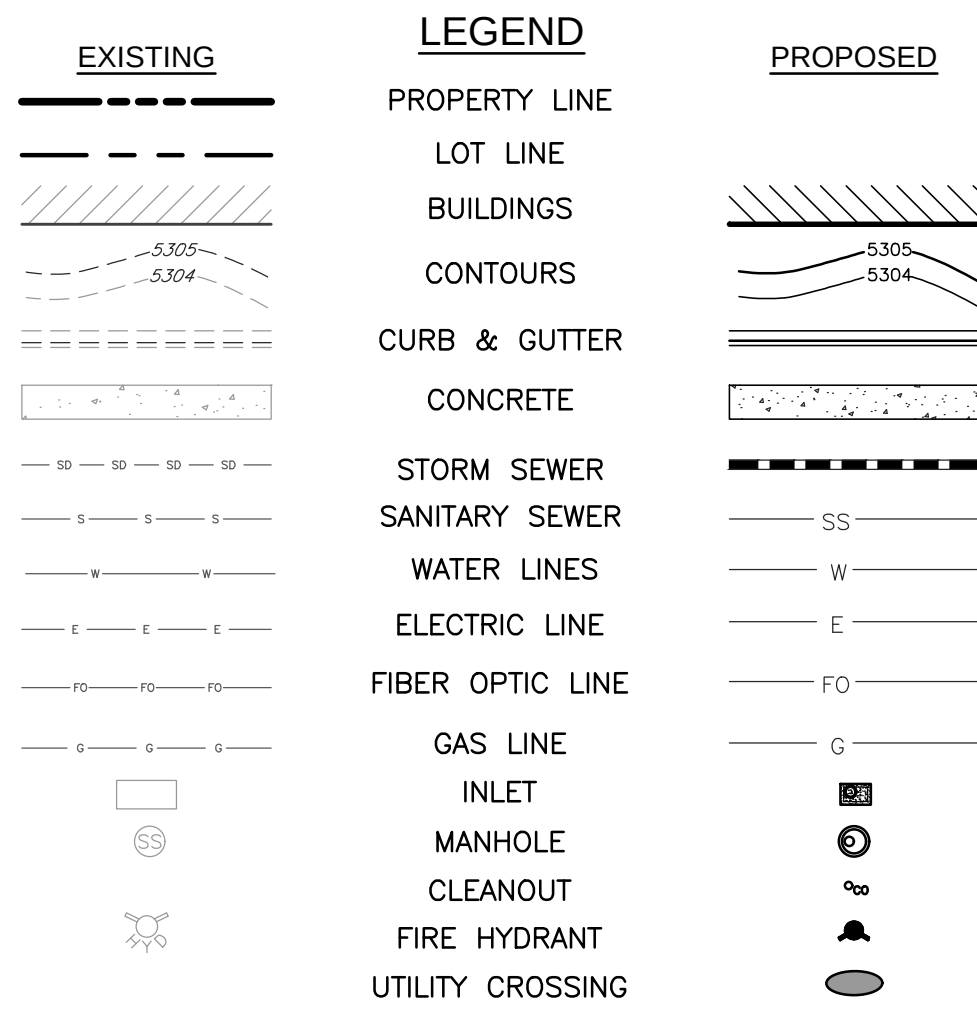
EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.



DRAWN BY: JLS
CDOT PROJECT NO. 23332 SAP BUILDING # 1000/XX
DRAWING NUMBER C4.1



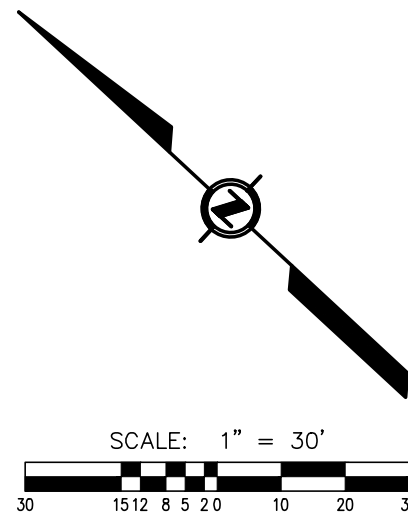
STORM LINE 1
OFFSITE STORM SEWER REROUTE



BASIS OF BEARING AND COORDINATE SYSTEM: CITY OF GLENWOOD SPRINGS HORIZONTAL AND VERTICAL DATUM TAKEN FROM "CAVERNS VILLAGE" PROJECT LOCATED TO THE NORTHWEST OF THE SUBJECT PROPERTY. CONTROL POINTS USED FOR THIS SURVEY ARE AS FOLLOWS:
CP#1 N:42112.15 E:16020.49 EL:5727.54
CP#2 N:42128.56 E:16378.22 EL:5734.44
CP#3 N:42813.55 E:15985.33 EL:5737.91

EXISTING UTILITY LOCATIONS: COLLINS ENGINEERS INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS HOWEVER THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.

- NOTES:**
- REFERENCE POINTS FOR STRUCTURES ARE AS FOLLOWS:
 - CENTER OF MANHOLES / CLEANOUTS
 - CENTER OF TOP BACK OF CURB FOR CURB INLETS
 - CENTER OF BOX FOR AREA INLETS
 - STORM / SANITARY SEWER PIPE LENGTHS ARE MEASURED AS FOLLOWS:
 - CENTER OF MANHOLES / CLEANOUTS
 - CENTER OF INLET BOX (CURB AND GRATE)
 - LENGTHS LISTED ARE 2D MEASUREMENTS
 - ALL STORM SEWER TO BE CLASS III RCP UNLESS NOTED OTHERWISE.
 - ALL STORM SEWER TO HAVE WATER TIGHT JOINTS PER ASTM C443 STANDARD USING RUBBER GASKETS.
 - ALL PROPOSED STORM SEWER SYSTEMS ARE PRIVATE UNLESS NOTED OTHERWISE.
 - ALL PROPOSED SANITARY SEWER SYSTEMS ARE PRIVATE UNLESS NOTED OTHERWISE.
 - NO CONNECTIONS TO EXISTING WATER SYSTEM SHALL BE MADE UNTIL THE NEW LINES HAVE BEEN TESTED AND ACCEPTED BY THE DISTRICT.
 - TRACER WIRE IS REQUIRED ON ALL WATERLINES. COORDINATE ALL UTILITY CONNECTIONS AT BUILDING WITH PLUMBING PLANS.
 - CONTRACTOR MUST OBTAIN NPDES WATER DISCHARGE PERMIT FROM COLORADO DEPARTMENT OF HEALTH.
 - SHORTHAND GRADING NOTATION FOR SPOT ELEVATIONS: 5295.95=95.95
 - ALL CONCRETE STRUCTURES AND PIPE WILL BE CDOT SPECIFIED AND SHALL BE CONSTRUCTED WITH SULFATE RESISTANT TYPE-V CONCRETE WITH XYPEX C-500.



DATE	NO.	DESCRIPTION
06/12/2020	1	65% CONSTRUCTION DOCS
08/06/2020	2	85% CONSTRUCTION DOCS

Colorado Department of Transportation

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CDOT
DEPARTMENT OF TRANSPORTATION

STORM SEWER PLAN & PROFILE

Glenwood Springs VSF

202 CENTENNIAL ST.
GLENWOOD SPRINGS, COLORADO 81601

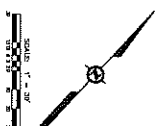
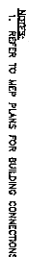
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www.collinsengr.com

DRAWN BY: JLS

CDOT PROJECT NO.
23332
SAP BUILDING #
1000/XXX

DRAWING NUMBER
C5.1





MEMO

TO: Robert Levenson
CDOT

FROM: Gretchen Ricehill
Community Development Department

DATE: September 21, 2020

RE: Planning File #54-20- CDOT Facility - 202 Centennial

This is a location and extent application and it is understood that the purpose of this type of review is to evaluate the proposed development based on whether it is

- consistent with applicable Comprehensive Plan goals and policies,
- consistent with the general purpose and intent of the Municipal Code and
- how well the application minimizes adverse impacts to health, safety and welfare of inhabitants of surrounding areas and the city.

The development is not required to meet the letter of the Municipal Code and in particular, its architectural design standards.

I understand that the buildings will be similar to the existing: tan metal sided, with white overhead doors, and what appears to be a galvanized metal roof.

Among other things, the general intent of the Nonresidential and Mixed-Use Building Design Standards, 070.040.090, is to encourage -

- the incorporation of a palette of building colors that add visual interest to the development parcel and that is compatible with the surrounding natural environment.

And, the intent is to avoid –

- building elements, materials and colors that are out of character with the community; and
- large expanses of blank, flat wall surfaces.

The proposed tan as the primary building color is no issue. I recommend that the overhead garage doors be changed from white to a darker shade of beige, or another earth toned color. Further, I recommend that CDOT not use a shiny galvanized metal roof. Instead, select a beige

*City of Glenwood Springs Community Development Department
101 W 8th Street
Glenwood Springs, Colorado 81601
Tel: 970-384-6400
FAX: 970-945-8582*

or other color with a matte finish. This is consistent with what we require of other commercial developments in the community.

Finally, to avoid large expanses of blank walls, I recommend considering any architectural feature that would break up the north elevation of the new VCF building. Features could include awnings/sun shades or columns or pilasters with enough of a wall plane change to have the effect of casting shadows.

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Planning and Zoning Commission Report

Date: October 27, 2020
To: Planning and Zoning Commission
From: Trent L. Hyatt, Senior Planner
Subject: #63-20 – Marijuana Related Uses Code Amendment

REQUEST	Consideration of a Code Amendment to Sections 070.030.030(e)(10), (11), and (f)(3) of the <i>Glenwood Springs Municipal Code</i> regarding the City's marijuana related special use regulations.
APPLICANT	City of Glenwood Springs
ZONE	Applies to all non-residential zone districts

ACTION ITEM:

The Planning and Zoning Commission (Commission) is the recommending body to City Council on any and all revisions to the Title 070 of the *Glenwood Springs Municipal Code* (Code).

Action 1 - Code Amendment - to revise the marijuana related special use regulations in Sections 070.030.030(e)(10), (11), and (f)(3) of the Glenwood Springs Municipal Code.

Staff recommendation: *Staff recommends approval of the Code Amendment with the findings on page 5.*

BACKGROUND/ANALYSIS

Proposal

The City has been operating under the current marijuana regulations since September 2015 (excluding minor modifications associated with the development code redraft in 2018). The basic premise of our regulations is the requirement for a special use review/permit to evaluate compliance with the various use specific regulations and the general special use approval criteria. These standards are provided in ***bold italics*** below.

Section 070.030.030(e)(10) addresses medical marijuana businesses whereas Section 070.030.030(e)(11) addresses retail marijuana establishments. However, the language below is representative of both sections.

b. License Required

Medical marijuana businesses shall comply with City Code Article 050.090.

c. Location of a Licensed Business

Medical marijuana businesses shall be located only in areas of the city as allowed in the City Code. No medical [or retail] marijuana businesses shall be located:

1. *Within 500 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade. The distance shall be property computed by a straight line measurement from the nearest property line of the school to the nearest property line of the site housing the retail marijuana establishment;*
 2. *Within 900 feet of another medical marijuana business or any retail marijuana establishment. A medical marijuana business may locate on the same licensed premises as a retail marijuana establishment of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other medical marijuana businesses and retail marijuana establishments. This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities; and*
 3. *In any zoning district not specifically permitted by the City Code.*
- d. *Requirements Related to Operation of a Medical Marijuana Business*
1. *Medical marijuana businesses shall meet all operational criteria for the procurement, dispensing, labeling, sanitation, security, and any other criteria as required by the state pursuant to the Colorado Medical Marijuana Code and the City Code Article 050.080.*
 2. *In addition, all medical marijuana businesses shall comply with the following local restrictions:*
 - i. *Medical marijuana businesses shall limit their hours of operation to between 8:00 a.m. and 7:00 p.m. Monday through Sunday or as otherwise limited by state law;*
 - ii. *Medical marijuana businesses shall apply for a sign permit through the Community Development Department. All exterior signage associated with a medical marijuana business will meet the standards established in the Colorado Medical Marijuana Code and in the City Code. In addition, no sign associated with a medical marijuana business shall use the word “marijuana,” “cannabis,” or any other word or phrase commonly understood to refer to marijuana or a graphic/image of any portion of a marijuana plant, or paraphernalia associated with medicinal marijuana use unless immediately preceded by the word “medical” or the message of such sign includes the words “for medical use” or “for medicinal purposes” in letters that are no smaller than the largest letter on the sign; and*
 - iii. *Marijuana plants, products, accessories, and associated paraphernalia contained in a medical marijuana facility shall not be visible from a public sidewalk, public street or right-of-way, or any other public place.*
 - iv. *No discernible odor shall be projected beyond the exterior walls of the licensed premises.*

Section 070.030.030(f)(3) of the Code addresses marijuana cultivation facilities as follows:

No marijuana cultivation facility shall be located:

- a. *Within 500 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade. This distance shall be computed by a straight line measurement from the nearest property line of the school property to the nearest property line of the site housing the marijuana cultivation facility; or*
- b. *Within 900 feet of another marijuana cultivation facility, medical marijuana business, or retail marijuana establishment. This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities.*
- c. *No discernible odor shall be projected beyond the exterior walls of the licensed premises.*

Section 070.060.050(e)(3)e.2 of the Code outlines the review criteria for all Land Uses as depicted below.

- i. The use will be compatible with the surrounding area;***
- ii. The impacts of the use on surrounding areas have been adequately minimized;***
- iii. The use will be consistent with the general purpose and intent of this Code;***
- iv. The use will comply with all applicable standards of this Code;***
- v. The use is in conformance with the Comprehensive Plan and other City plans and policies; and***
- vi. The use minimizes adverse impacts to the health, safety, and welfare of the inhabitants of the surrounding areas and the city.***

Proposed Code Amendment

Following the review of various marijuana related business special use applications, the Commission has discussed potentially reevaluating the regulations. More recently, City Council requested the Commission review the regulations and provide recommendations for revisions thereto, if applicable. The Commission conducted work sessions related to the issue on July 28, August 25, and September 15 of this year where various maps, proposed buffers, and other potential changes were considered. At the last of these work sessions, staff was directed to bring forth code amendments related to the following:

- 1. An increased buffer of 1000 feet between/from marijuana related business and schools;**
- 2. The addition of buffers for parks, mental health facilities, and drug treatment facilities;**
- 3. Language addressing occurrences where a marijuana related business ceases operation at a specific site; and**
- 4. The addition of a marijuana related business density maximum related to the City's population.**

The attached proposed revised language attempts to address the Commission's requests. Additions to existing language are underlined and deletions are ~~struck through~~. Staff has also included revised references to various guiding state regulations and terms. Additionally, we provide a density cap related to cultivation which maintains the current number of facilities. Future revisions to the City licensing requirements, as outlined in Title 050, will likely result from any the current Code Amendment.

Code Amendment Approval Criteria

Section 070.060.040(c)(3)d.3 of the Code outlines the approval criteria for revision thereto. Each of the criteria are listed in **bold** below along with comments from staff.

i. Is consistent with the Comprehensive Plan and other City policies;

"Promote long-term, sustainable, diverse, economic development" and "maintain Glenwood Springs as the regional tourism, retail, commercial and governmental center of Garfield County" are community goals outlined in the executive summary and subsequent chapters of the Glenwood Springs Comprehensive Plan. Specific objectives and strategies related to these goals are outlined in the table below.

Comprehensive Plan Goal	Objective and Strategy
-Promote long-term, sustainable, diverse economic development	-Attract diverse businesses and industries (Chapter 4, Page 50)
Staff Comment: The proposed text amendment helps to promote new business types as the proliferation of marijuana related uses potentially limits retail space availability.	
-Maintain Glenwood Springs as the regional tourism, retail, commercial and governmental center of Garfield County	-Attract diverse businesses and industries (Chapter 4, Page 50)
Staff Comment: The proposed text amendment helps to promote new business types as the proliferation of marijuana related uses potentially limits retail space availability.	

ii. Does not conflict with other provisions of this Code or other provisions in the Glenwood Springs Municipal Code;

It does not appear the proposal conflicts with other provisions of the Code. Instead, the proposal provides additional guidance on how to manage the specific use.

iii. Is necessary to address a demonstrated community need;

iv. Is necessary to respond to substantial changes in conditions and/or policy; and

As discussed above, concerns over the proliferation of the use have been raised by the Commission for a number of years now and more recently by City Council. While additional instances of the use have occurred, tax revenues have not necessarily seen a relative increase.

v. Is consistent with the general purpose and intent of this Code.

The proposal appears to be consistent with the general purpose and intent of the Code outlined in Section 070.010.030. The subject Code Amendment is simply an update to existing similar standards already addressed in the Code and supports the “promoting orderly, efficient, and integrated development within the city” and “regulating the development and use of land based upon the impact of such development or use on surrounding areas or the city” intents outlined therein.

Reviewing Agency Comments

No official comments have been received regarding the proposal. However, staff worked with the Attorney’s office and Police Department in preparation of the revisions.

ACTION ALTERNATIVES AND STAFF RECOMMENDATION:

Section 070.060.040(c)(3)d.1 of the Code explains, “*The Planning Commission shall review the Code amendment application and recommend approval, approval with conditions, or denial of the amendment in accordance with 070.060.030(g) and the approval criteria in paragraph 3 below [addressed by staff above].* The Planning Commission may also continue the hearing.

Action 1 -Code Amendment - to revise the marijuana related special use regulations in Sections 070.030.030(e)(10), (11), and (f)(3) of the Glenwood Springs Municipal Code.

Staff recommendation: Staff recommends **approval** of the Code Amendment with the **findings** below:

Recommended Findings:

1. The proposed amendment is consistent with the Comprehensive Plan and other City policies as outlined on pages 3-5 of the staff report;
2. The proposed amendment does not conflict with other provisions of this Code or other provisions in the Glenwood Springs Municipal Code as outlined on page 4 of the staff report;
3. The proposed amendment is necessary to address a demonstrated community need as outlined on page 4 of the staff report;
4. The proposed amendment is necessary to respond to substantial changes in conditions and/or policy as outlined on page 4 of the staff report; and
5. The proposed amendment is consistent with the general purpose and intent of this Code as outlined on page 4 of the staff report.

(10) Medical Marijuana Business

a. Purpose

The City Council intends to regulate the use, acquisition, production, and distribution of medical marijuana in a manner consistent with Article XVIII, Section 14 of the Colorado Constitution, Article ~~10 43.3~~ of Title ~~44 12~~ Colorado Revised Statutes and any other applicable laws and regulations of the state. Nothing within this article is intended to promote or condone the production, use, sale, or distribution of medical marijuana other than in compliance with applicable state law.

b. License Required

Medical marijuana businesses shall comply with the City Code Article 050.080.

c. Location of a Licensed Business

Medical marijuana businesses shall be located in areas of the city only as allowed in the City Code. The ability to operate a medical marijuana business on any given property shall cease upon a three year period of the use not occurring thereon. No medical marijuana business shall be located:

1. Within ~~500~~1,000 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade, park, mental health facility, or drug treatment facility. The distance shall be computed by a straight line measurement from the nearest property line of the school property, park, mental health facility, or drug treatment facility to the nearest property line of the site housing the medical marijuana business; ~~or~~
2. Within ~~900~~1,000 feet of another medical marijuana business, ~~or any retail marijuana business establishment, or a marijuana cultivation facility~~. A medical marijuana business may locate on the same licensed premises as a retail marijuana ~~business establishment or marijuana cultivation facility~~ of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other medical marijuana businesses, ~~and retail marijuana business establishments, and marijuana cultivation facilities~~. This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities.
3. In any zoning district not specifically allowed by the City Code.

d. Requirements Related to Operation of a Medical Marijuana Business

1. Medical marijuana businesses shall meet all operational criteria for the procurement, dispensing, labeling, sanitation, security, and any other criteria as required by the state pursuant to the Colorado ~~Medical Marijuana Code~~ and the City Code Article 050.080.

2. In addition, all medical marijuana businesses shall comply with the following local restrictions:
 - i. Medical marijuana businesses shall limit their hours of operation to between 8:00 a.m. and 7:00 p.m. Monday through Sunday or as otherwise limited by state law;
 - ii. Medical marijuana businesses shall apply for a sign permit through the Community Development Department. All exterior signage associated with a medical marijuana business will meet the standards established in the Colorado ~~Medical Marijuana Code~~, the Code of Colorado Regulations, and in the City Code. In addition, no sign associated with a medical marijuana business shall use the word "marijuana," "cannabis," or any other word or phrase commonly understood to refer to marijuana or a graphic/image of any portion of a marijuana plant, or paraphernalia associated with medicinal marijuana use unless immediately preceded by the word "medical" or the message of such sign includes the words "for medical use" or "for medicinal purposes" in letters that are no smaller than the largest letter on the sign; and
 - iii. Marijuana plants, products, accessories, and associated paraphernalia contained in a medical marijuana facility shall not be visible from a public sidewalk, public street or right-of-way, or any other public place.
 - iv. No discernible odor shall be projected beyond the exterior walls of the licensed premises.

e. **Density**

The number of marijuana businesses permitted to operate within the City shall be based on population. A maximum of one medical and/or retail marijuana business shall be permitted for every 1,000 people living within the City or a fraction thereof (i.e. a maximum of two marijuana businesses is allowed for a population of 2500 people). Population shall be based on the decennial census conducted by the United States Census Bureau. In all other years, population shall be based on annual estimates by the Colorado State Demography Office.

(11) **Retail Marijuana Business Establishment**

a. **Purpose**

The City Council intends to allow state-licensed retail marijuana ~~businesses establishments~~ to exist in Glenwood Springs in accordance with Article XVIII, Section 16 of the Colorado Constitution, Article ~~10.43.4~~ of Title 44 ~~12~~ Colorado Revised Statutes, and any other applicable state laws and regulations as well as the additional local licensing requirements and other restrictions set forth in the Glenwood Springs Municipal Code. Nothing within this Article is intended to promote or condone the production, use, sale, testing or distribution of marijuana other than in compliance with applicable state law.

b. **License Required**

Retail marijuana ~~businesses~~ establishments shall comply with City Code Article 050.090.

c. **Location of Licensed ~~Business~~ Establishment**

Retail marijuana ~~businesses~~ establishments shall be located only in areas of the city as allowed in the City Code. The ability to operate a retail marijuana business establishment on any given property shall cease upon a three year period of the use not occurring thereon. No retail marijuana ~~business~~ establishment shall be located:

1. Within ~~500~~1,000 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade, park, mental health facility, or drug treatment facility. The distance shall be computed by a straight line measurement from the nearest property line of the school, park, mental health facility, or drug treatment facility property to the nearest property line of the site housing the retail marijuana ~~business~~ establishment;
2. Within ~~900~~1,000 feet of another retail marijuana ~~business~~ establishment, ~~or a medical marijuana business, or a marijuana cultivation facility.~~ A retail marijuana ~~business~~ establishment may locate on the same licensed premises as a medical marijuana business or marijuana cultivation facility of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other retail marijuana ~~businesses~~ establishments, ~~and medical marijuana businesses, and marijuana cultivation facilities.~~ This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities. ; and
3. In any zoning district not specifically permitted by the City Code.

d. **Requirements Related to Operation of a Retail Marijuana ~~Business~~ Establishment**

1. Retail marijuana ~~businesses~~ establishments shall meet all operational criteria for the procurement, display, dispensing, labeling, sanitation, security, and any other criteria as required by the state pursuant to the Colorado Marijuana Code ~~CRMC~~ and the City Code Article 050.090.
2. In addition, all retail marijuana ~~businesses~~ establishments shall comply with the following local restrictions:
 - i. Hours of operation shall be limited to between 8:00 a.m. and 7:00 p.m. Monday through Sunday; and
 - ii. A sign permit shall be obtained through the Community Development Department. All exterior signage associated with a retail marijuana ~~business~~ establishment will meet the standards established in the Colorado Marijuana Code and in the City Code.

- iii. Storage of retail marijuana shall be considered an accessory use. Not more than 25 percent of the licensed premises of a retail marijuana business shall be used for such purposes.
- iv. No discernible odor shall be projected beyond the exterior walls of the licensed premises.

e. **Density**

The number of marijuana businesses permitted to operate within the City shall be based on population. A maximum of one retail and/or medical marijuana business shall be permitted for every 1,000 people living within the City or a fraction thereof (i.e. a maximum of two marijuana businesses is allowed for a population of 2500 people). Population shall be based on the decennial census conducted by the United States Census Bureau. In all other years, population shall be based on annual estimates by the Colorado State Demography Office.

Section 070.030.030(f)

(3) **Marijuana Cultivation**

- a. No marijuana cultivation facility shall be located or operated:
 - 1. Within ~~500~~1,000 feet of any existing public or private school facility where classes are held for children aged kindergarten through the 12th grade, park, mental health facility, or drug treatment facility. This distance shall be computed by a straight line measurement from the nearest property line of the school property, park, mental health facility, or drug treatment facility to the nearest property line of the site housing the marijuana cultivation facility; ~~or~~
 - 2. Within ~~900~~1,000 feet of another marijuana cultivation facility, medical marijuana business, or retail marijuana ~~business~~establishment. A marijuana cultivation facility may locate on the same licensed premises as a medical marijuana business or retail marijuana business establishment of the same class and same ownership, only if the licensed premises is located to meet the distance requirements from other retail marijuana businesses establishments, medical marijuana businesses, and marijuana cultivation facilities. This distance shall be computed by a straight line measurement between property lines of the sites housing the two facilities The ability to operate a marijuana cultivation facility on any given property shall cease upon a three year period of the use not occurring thereon;
- 3. In a zoning district where it is not specifically permitted by the City Code.
 - b. The ability to operate a marijuana cultivation facility on any given property shall cease upon a three year period of the use not occurring thereon.
 - c. No discernible odor shall be projected beyond the exterior walls of the licensed premises.

- d. The number of marijuana cultivation facilities permitted to operate within the City shall be based on population. A maximum of one marijuana cultivation facility shall be permitted for every 5,000 people living within the City or a fraction thereof (i.e. a maximum of two retail marijuana businesses is allowed for a population of 10,500 people). Population shall be based on the decennial census conducted by the United States Census Bureau. In all other years, population shall be based on annual estimates by the Colorado State Demography Office.

COMPARISON - MARIJUANA REGULATIONS					
Jurisdiction	Population (2019 est)	Marijuana Uses Allowed	Distance / Separation / Buffering Requirements		Other Restrictions
Frazier	1219	Retail Medical	1000	educational institutions, public or private	Not allowed in any building containing a residential dwelling or lodging use
			500	any existing medical marijuana business	
			200	licensed child care facility	
Crested Butte	1487		500	educational institutions or schools, public and private	not allowed adjacent to a residential dwelling separated by a vertical wall
			500	licensed child care	
			175	public park or playground	
New Castle	4518	NO MARIJUANA USES ALLOWED			
Gunnison	5054	Retail Medica Manufacturing Testing Cultivation	1000	schools	not allowed adjacent to residential zoning district
			1000	day care	
			1000	mental health facility	
Carbondale	6427	Retail Medical Manufacturing	500	school or day care	no more than 5 retail or medical no more than 5 product manufacturing no more than 5 testing no more than 3 cultivation
			500	alcohol or drug treatment	
			400	retail and/or medical marijuana	
Eagle	6454	Retail Medical Manufacturing Cultivation			1 of each type of facility allowed / 5000 people restricted to one location/area - east of Nogal Gulch

COMPARISON - MARIJUANA REGULATIONS					
Jurisdiction	Population (2019 est)	Marijuana Uses Allowed	Distance / Separation / Buffering Requirements		Other Restrictions
Avon	6511	NO MARIJUANA USES ALLOWED			
Glenwood Springs	9437	Retail Medical Manufacturing Cultivation	500 900	k-12 grade schools retail and/or medical marijuana facility	
Rifle	9706	Retail	1000	k-12th grade schools	3 licensed stores allowed, total
		Medical	1000	alcohol or drug treatment facilities	7 licensed cultivation facilities allowed, total locations restricted to south of I-70, light industrial zones, industrial PUDs if listed as an allowed use
		Cultivation	1000	child care facilities	
			0 allowed in central business district		
Steamboat Springs	11,810	Retail Medical Manufacturing Testing Cultivation	1,000 1,000	schools, colleges, seminaries, child care (cultivation uses allow for a 500 foot buffer to child care facilities) parks	
Gunnison County	14,474	Manufacturing			Retail & medical stores prohibited
		Testing Cultivation			Allowed in only 3 designated zoning districts

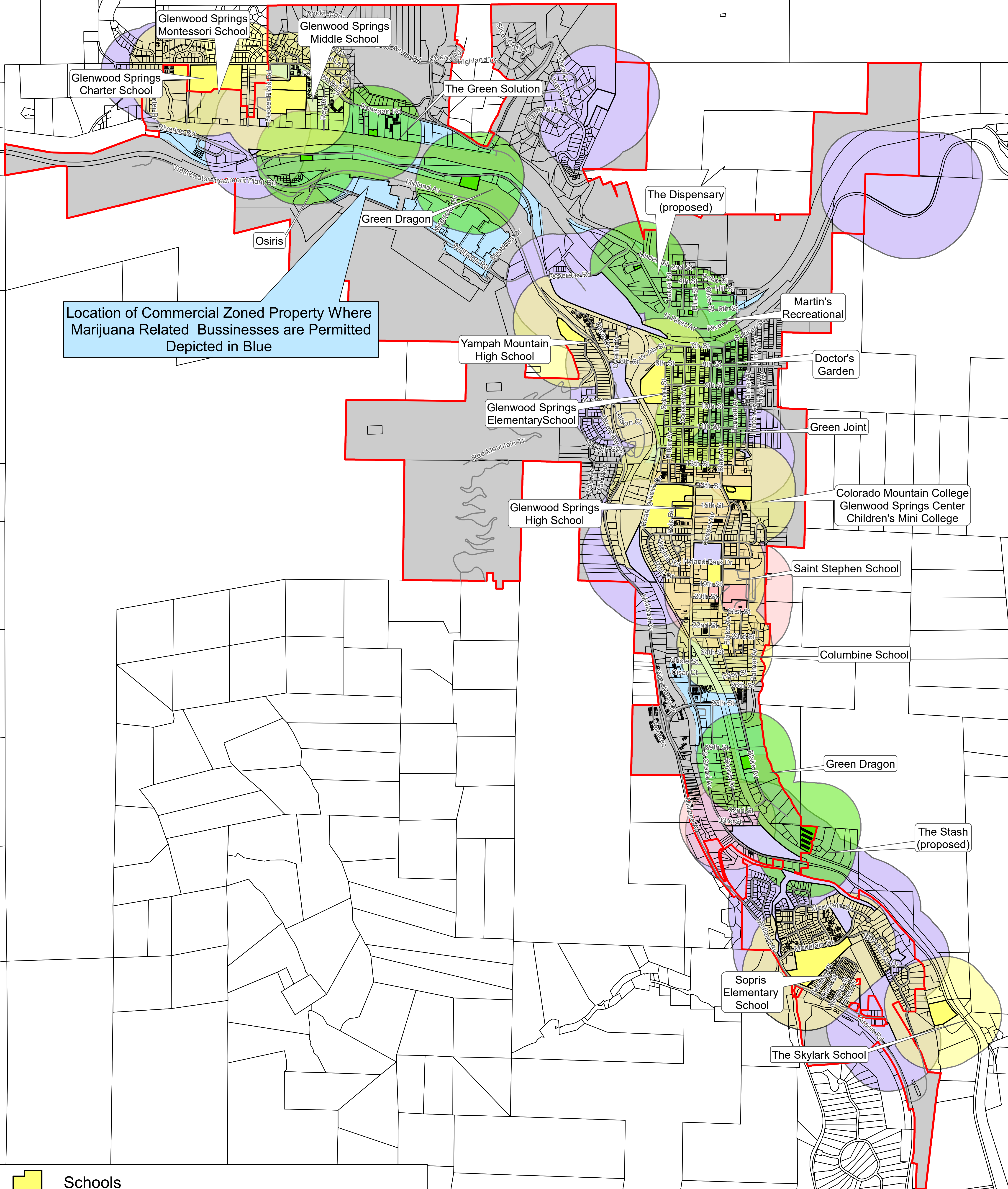
COMPARISON - MARIJUANA REGULATIONS

Jurisdiction	Population (2019 est)	Marijuana Uses Allowed	Distance / Separation / Buffering Requirements		Other Restrictions
Summit County	31,011	Retail Medical Manufacturing Testing Cultivation	50 1,000 1,000 500 500	residentially used property, or residential zone district, or residential use in a PUD licensed child care facility any school, public or private including colleges halfway house or correctional facility any marijuana business	
Longmont	97,261	Retail Medical	250 1,000	residential zoning districts K - 12th grade schools	No more than 4 licenses allowed in the city 1 license/application allowed per person or company Testing, manufacturing and cultivation are not allowed
Pueblo	102,550	Retail Medical Testing Cultivation	1,000 500 300	Schools hospital or substance abuse treatment centers residential use or residential zone district	
Thornton	137,000	Retail Medical Testing	1,000 500 500 1,500	schools, public and private licensed child care facilities alcohol or drug treatment facility any other retail marijuana store	Cultivation is not allowed Only 1 retail marijuana store allowed in each of the city's 4 quadrants

COMPARISON - MARIJUANA REGULATIONS					
Jurisdiction	Population (2019 est)	Marijuana Uses Allowed	Distance / Separation / Buffering Requirements		Other Restrictions
Aurora	322,522	Retail	1,000	preK - 12th grade, excluding day care	Allow up to 4 retail marijuana licenses in each Council ward, or 24 in total Cultivation, manufacturing/testing not restricted as to total number
		Medical	500	hospitals, substance abuse treatment centers	
		Cultivation	300	residential uses (applies to Cultivation-only)	
		Manufacturing & testing	300	parks and open space (applies to Cultivation-only) residential uses in adjacent jurisdictions (applies to	
			300	Cultivation-only)	
Boulder County	326,196	Retail	500	any other marijuana establishment in the same zoning district	
		Medical	1000	alcohol or drug treatment facility	
		Manufacturing	1000	licensed child care	
		Testing	1000	educational facilities below college level	
		Cultivation			

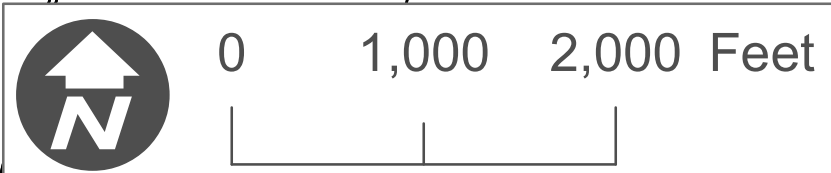


Marijuana Related Businesses and Potential 1000 Foot Buffers



Location of Commercial Zoned Property Where Marijuana Related Bussinesses are Permitted Depicted in Blue

- Schools
- Marijuana Related Business (9 approved, 7 open)
- Parks
- Mental Health and Drug Treatment Facilities
- City Boundary



This map was produced by the Community Development Department. Use of this map should be for general purposes only. The City of Glenwood Springs does not warrant the accuracy of the data contained herein. Map is based on best available data as of September 2020. V:\Comm Dev\GIS\Marijuana\Schools_Marijuana_Locations_Discussion_092220_1000_Feet.mxd Phone - 970.384.6427 <http://www.cogs.us/> Layout: THyatt